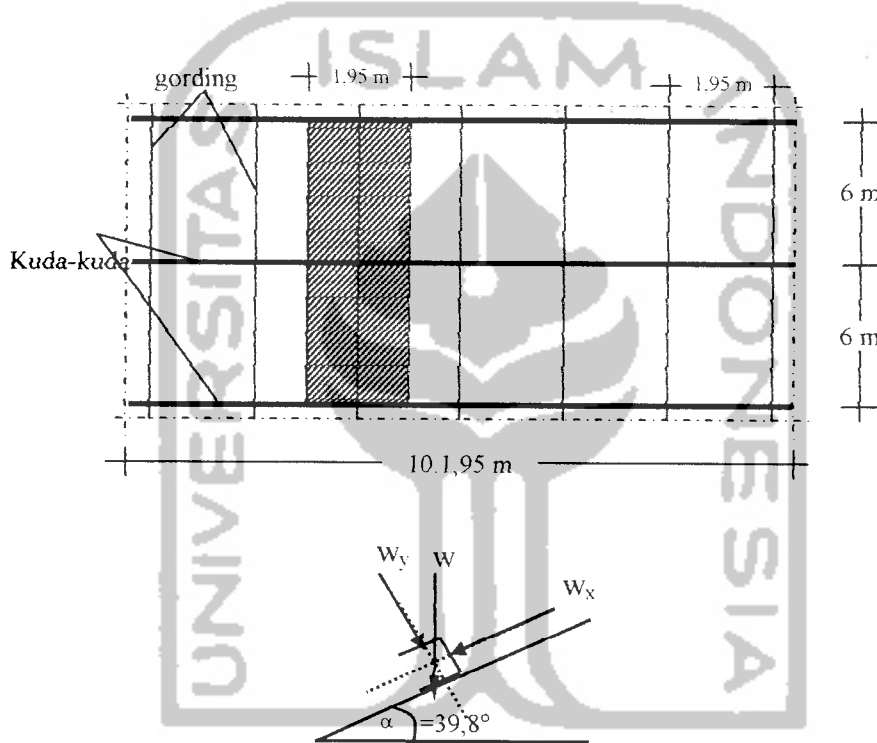


BAB IV
PERHITUNGAN STRUKTUR

4.1. Perencanaan Gording (untuk kuda-kuda KA₁, KA₂, KA₃, KA₄)



Gambar 4.1 Arah Pembebanan Gording

4.1.1 Pembebanan Gording

a. Beban Mati, (Genting beton = $0,5 \text{ kN/m}^2$), (dari 3.1)

– Genteng beton, reng, usuk : $0,5 \times 1,95 = 0,975 \text{ kN/m}$

– Gording taksiran : $= 0,15 \text{ kN/m}$

$$W_D = 1,125 \text{ kN/m}$$

b. Beban Hidup, ($L = 0,2 \text{ kN/m}^2$), (dari 3.2)

– Beban pekerja atap : $W_L = 0,2 \times 1,95 = 0,39 \text{ kN/m}$

c. Beban Air Hujan, (dari 3.3)

– Beban air hujan : $W_R = (40 - 0,8\alpha) \cdot 0,0195 = 0,159 \text{ kN/m}$

d. Beban Angin ($W = 0,4 \text{ kN/m}^2$), (dari 3.4)

– Tiup angin:

$$c_1 = (0,02 \alpha) - 0,4 = (0,02 \times 39,8) - 0,4 = 0,396 \quad (\text{dari 3.4a})$$

$$w_t = 0,396 \times 0,4 \times 1,95 = 0,309 \text{ kN/m}$$

– Hisap angin

$$c_2 = -0,4 \quad (\text{dari 3.4b})$$

$$w_h = -0,4 \cdot 0,4 \cdot 1,95 = -0,312 \text{ kN/m}$$

e. Momen arah sumbu x dan sumbu y, (dari 3.5)

– Sumbu x (satu sagrod)



– Sumbu y (dua sagrod)



Tabel 4.1 Pembebanan dan momen sumbu x dan sumbu y

No	Jenis pembebanan	Wx	Wy	Mx	My
		$w \cdot \sin 39,8^\circ$	$w \cdot \cos 39,8^\circ$	$M_x = \frac{1}{11} \cdot w_y \cdot L^2$	$M_y = \frac{1}{11} \cdot w_x \cdot L^2$
1	Beban mati (D)	0,720	0,864	0,262	2,828
2	Beban hidup (L)	0,250	0,300	0,091	0,981
3	Beban hujan (R)	0,102	0,122	0,037	0,399
4	Beban angin (W)				
	- Tiup angin	0	0,309	0	1,011
	- Hisap angin	0	-0,312	0	-1,021

Tabel 4.2 Kombinasi pembebanan LRFD

No	Kombinasi	Mu,x (kNm)	Mu,y (kNm)
1	1,4 MD	0,367	3,959
2	1,2 MD + 0,5 ML	0,460	3,672
3	1,2 MD + 0,5MR	0,333	3,381
4	1,2 MD + 1,6 ML + 0,8 MW _{tiup}	0,460	5,772
5	1,2 MD + 1,6 ML + 0,8 MW _{hisap}	0,460	3,934
6	1,2 MD + 1,3 MW _{tiup} + 0,5 ML	0,339	4,986
7	1,2 MD + 1,3 MW _{hisap} + 0,5 ML	0,339	2,345

4.1.2 Pendimensian Gording

a. Arah sumbu x

$$Mu,x = 0,440 \text{ kNm}$$

$$F_y = 250 \text{ MPa,}$$

Asumsi penampang kompak, kuat desain:

$$S_y \geq \frac{0,460 \times 10^6}{0,9 \times 250} = 1955,56 \text{ mm}^3 = 1,96 \text{ cm}^3 \quad (\text{dari 3.6})$$

Dipakai Profil I 250x50x20x4,0

$$S_x = 83,7 \text{ cm}^3 \quad ; \quad S_y = 5,66 \text{ cm}^3$$

$$I_x = 1050 \text{ cm}^4 \quad ; \quad I_y = 23,3 \text{ cm}^4$$

b. Arah sumbu y

$$Mu,y = 5,559 \text{ kNm}$$

$$S_x = \frac{5,559 \times 10^6}{0,9 \times 250} = 24706,667 \text{ mm}^3$$

$$= 24,707 \text{ cm}^3 < 83,7 \text{ cm}^3$$

profil aman digunakan.

c. Kontrol Lendutan

- Lendutan sejajar gording

$$E = 210.000 \text{ MPa}$$

$$L = 2000 \text{ mm} \quad (\text{dipasang dua sagrod})$$

$$W_x = 1,2 \cdot 0,720 + 1,6 \cdot 0,250 + 0,8 \cdot 0 = 1,264 \text{ kN/m}$$

dari persamaan 3.7a, didapat:

$$\delta_x = \frac{5}{384} \times 1,264 \times \frac{2000^4}{2 \cdot 10^5 \times 23,3 \cdot 10^4} = 5,4 \text{ mm} < \frac{2000}{360} = 5,6 \text{ mm}$$

- Lendutan tegak lurus gording

$$E = 210.000 \text{ MPa}$$

$$L = 6000 \text{ mm}$$

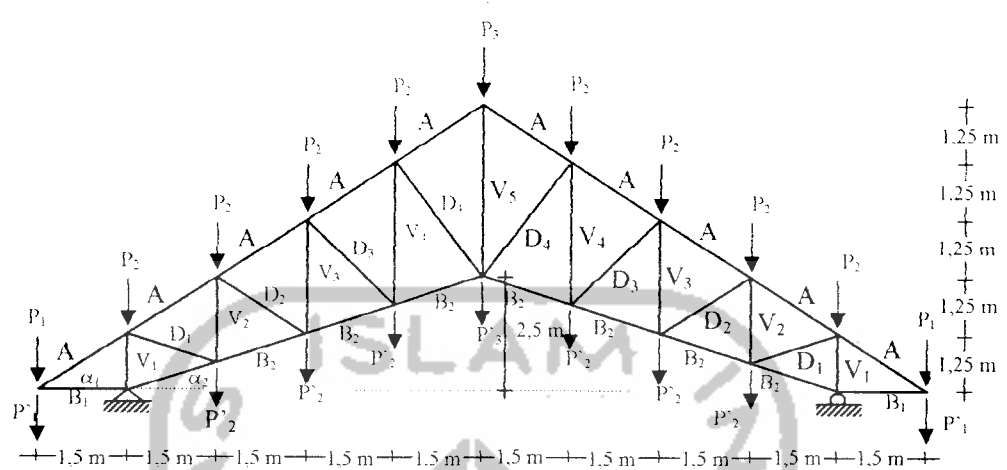
$$W_x = 1,2 \cdot 0,864 + 1,6 \cdot 0,300 + 0,8 \cdot 0,309 = 1,764 \text{ kN/m}$$

dari persamaan 3.7b, didapat:

$$\delta_y = \frac{5}{384} \times 1,764 \times \frac{6000^4}{2 \cdot 10^5 \times 1050 \cdot 10^4} = 14,175 \text{ mm} < \frac{6000}{360} = 16,67$$

4.2 Perencanaan Rangka Kuda-Kuda

4.2.1 Perencanaan Pembebanan Kuda-kuda KA₁



Gambar 4.2 Kuda-kuda baja KA₁

4.2.1.1 Beban Mati (dari 3.8)

$$P_1 = \text{berat gording} : 0,11 \times 6 = 0,66 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times 6 \times 0,5 \times 1,95 = 2,93 \text{ kN}$$

$$\text{Berat total } P_1 = 3,59 \text{ kN}$$

$$P_2 = \text{berat gording} : 0,11 \times 6 = 0,66 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times 6 \times 1,95 = 5,86 \text{ kN}$$

$$\text{Berat total } P_2 = 6,52 \text{ kN}$$

$$P_3 = \text{berat gording} : 0,11 \times 6 \times 2 = 1,23 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times 6 \times 1,95 = 5,86 \text{ kN}$$

$$\text{Berat total } P_3 = 7,09 \text{ kN}$$

$$P'_1 = \text{berat eternit + penggantung} : 0,18 \times 6 \times 0,5 \times 1,5 = 0,81 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 0,5 \times 1,5 = 0,38 \text{ kN}$$

$$\text{Berat total } P'_1 = 1,19 \text{ kN}$$

$$P'_2 = \text{berat eternit} + \text{penggantung} = 0,18 \times 6 \times 1,625 = 1,76 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} = 0,5 \times 1,625 = 0,81 \text{ kN}$$

$$\text{Berat total } P'_2 = 2,57 \text{ kN}$$

4.2.1.2 Beban Hidup, (dari 3.9)

$$P_1 = \text{beban hidup} = 0,2 \times 6 \times 0,5 \times 1,95 = 1,17 \text{ kN}$$

$$P_2 = P_3 = \text{beban hidup} = 0,2 \times 6 \times 1,95 = 2,34 \text{ kN}$$

4.2.1.3 Beban Angin, (dari 3.9)

$$\text{muatan angin} = 0,4 \text{ kN/m}^2$$

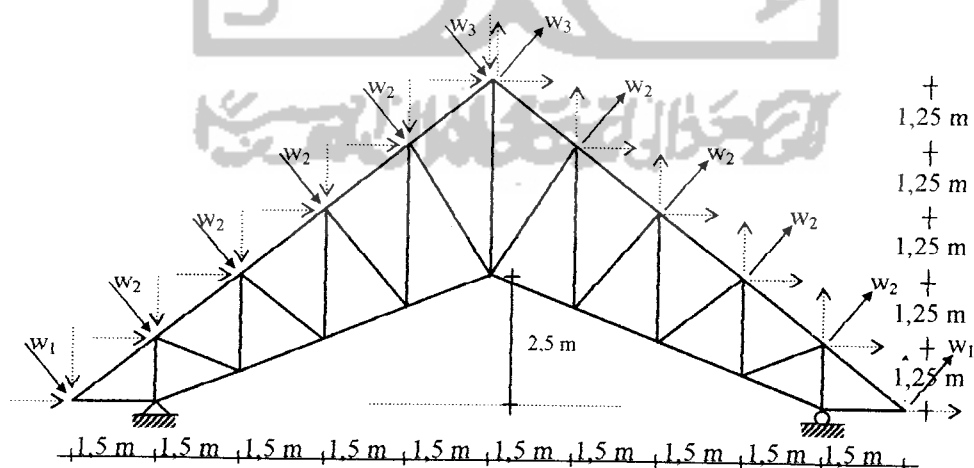
$$\text{koefisien angin tekan, } c_1 = (0,02 \times \alpha) - 0,4 = 0,02 (39,8) - 0,4 = 0,396$$

$$\text{koefisien angin hisap, } c_2 = -0,4$$

$$w_t = 0,396 \times 0,4 = 0,16 \text{ kN/m}^2$$

$$w_h = -0,4 \times 0,4 = -0,16 \text{ kN/m}^2$$

a. Angin Kiri



Gambar 4.3 Beban angin kiri pada kuda-kuda baja KA₁

– sisi kiri = angin tekan

$$wt_1 = 0,16 \times 0,5 \times 1,95 \times 6 = 0,93 \text{ kN}$$

$$wt_{x1} = 0,93 \times \sin 39,8 = 0,59 \text{ kN}$$

$$wt_{y1} = 0,93 \times \cos 39,8 = 0,71 \text{ kN}$$

$$wt_2 = 0,16 \times 1,95 \times 6 = 1,85 \text{ kN}$$

$$wt_{x2} = 1,85 \times \sin 39,8 = 1,19 \text{ kN}$$

$$wt_{y2} = 1,85 \times \cos 39,8 = 1,42 \text{ kN}$$

– sisi kanan = angin hisap

$$wh_1 = -0,16 \times 0,5 \times 1,95 \times 6 = -0,93 \text{ kN}$$

$$wh_{x1} = 0,93 \times \sin 39,8 = 0,59 \text{ kN}$$

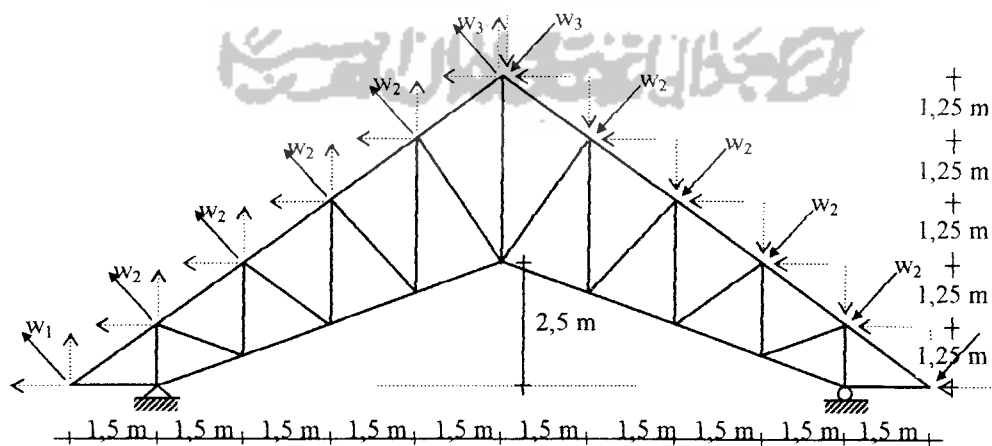
$$wh_{y1} = 0,93 \times \cos 39,8 = 0,72 \text{ kN}$$

$$wh_2 = -0,16 \times 1,95 \times 6 = -1,87 \text{ kN}$$

$$wh_{x2} = 1,87 \times \sin 39,8 = 1,19 \text{ kN}$$

$$wh_{y2} = 1,87 \times \cos 39,8 = 1,44 \text{ kN}$$

b. Angin Kanan



Gambar 4.4 Beban angin kanan pada kuda-kuda baja KA₁

– sisi kiri = hisap

$$wh_1 = -0,16 \times 0,5 \times 1,95 \times 6 = -0,94 \text{ kN}$$

$$wh_{x1} = 0,94 \times \sin 39,8 = 0,59 \text{ kN}$$

$$wh_{y1} = 0,94 \times \cos 39,8 = 0,72 \text{ kN}$$

$$wh_2 = -0,16 \times 1,95 \times 6 = -1,87 \text{ kN}$$

$$wh_{x2} = 1,87 \times \sin 39,8 = 1,19 \text{ kN}$$

$$wh_{y2} = 1,87 \times \cos 39,8 = 1,44 \text{ kN}$$

– sisi kanan = tekan

$$wt_1 = 0,16 \times 0,5 \times 1,95 \times 6 = 0,93 \text{ kN}$$

$$wt_{x1} = 0,93 \times \sin 39,8 = 0,59 \text{ kN}$$

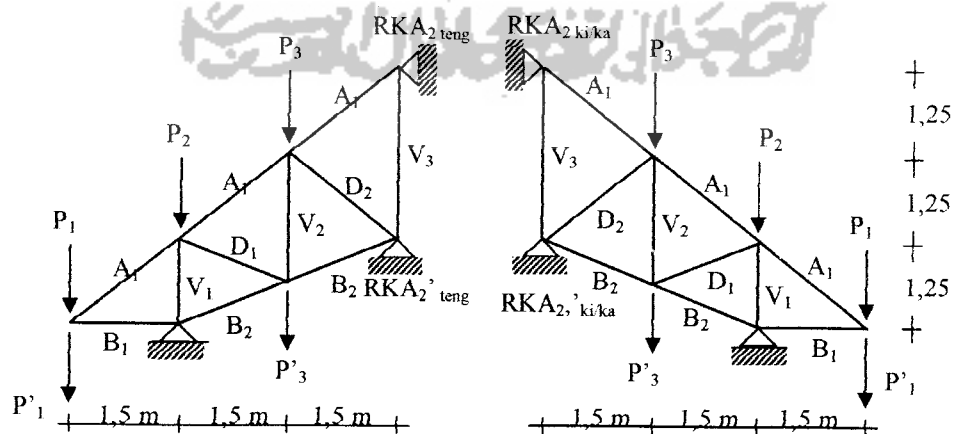
$$wt_{y1} = 0,93 \times \cos 39,8 = 0,71 \text{ kN}$$

$$wt_2 = 0,16 \times 1,95 \times 6 = 1,85 \text{ kN}$$

$$wt_{x2} = 1,85 \times \sin 39,8 = 1,19 \text{ kN}$$

$$wt_{y2} = 1,85 \times \cos 39,8 = 1,42 \text{ kN}$$

4.2.2 Perencanaan Pembebanan Kuda-Kuda KA₂



Gambar 4.5 Kuda-Kuda KA₂

4.2.2.1 Beban Mati

$$P_1 = \text{berat gording} : 0,11 \times 6 = 0,66 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 2,75 \text{ kN}$$

$$\text{Berat total } P_1 = 3,41 \text{ kN}$$

$$P_2 = \text{berat gording} : 0,11 \times 4,5 = 0,49 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 4,39 \text{ kN}$$

$$\text{Berat total } P_2 = 4,88 \text{ kN}$$

$$P_3 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 2,93 \text{ kN}$$

$$\text{Berat total } P_3 = 3,26 \text{ kN}$$

$$P_4 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] = 3,21 \text{ kN}$$

$$\text{Berat total } P_4 = 3,54 \text{ kN}$$

$$P'_1 = \text{berat eternit + penggantung} : 0,18 \times \left[\frac{(6 + 5,25) \times 0,75}{2} \right] = 0,76 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 0,5 \times 1,5 = 0,38 \text{ kN}$$

$$\text{Berat total } P'_1 = 1,14 \text{ kN}$$

$$P'_3 = \text{berat eternit + penggantung} = 0,18 \times \left[\frac{(3,75 + 2,25) \times 1,63}{2} \right] = 0,88 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} = 0,5 \times 1,5 = 0,75 \text{ kN}$$

$$\text{Berat total } P'_3 = 1,63 \text{ kN}$$

P'_4 = berat eternit + penggantung :

$$0,18 \times \left[\left(\frac{(2,25 + 0,75) \times 1,63}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] = 0,98 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 2,25 = 1,13 \text{ kN}$$

Berat total $P'_4 = 2,11 \text{ kN}$

4.2.2.2 Beban Hidup

$$P_1 = 0,2 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 1,09 \text{ kN}$$

$$P_2 = 0,2 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,76 \text{ kN}$$

$$P_3 = 0,2 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 1,17 \text{ kN}$$

$$P_4 = 0,2 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] = 1,28 \text{ kN}$$

4.2.2.3 Beban Angin

$$\text{muatan angin} = 0,4 \text{ kN/m}^2$$

$$\text{koefisien angin tekan, } c_1 = (0,02 \times \alpha) - 0,4 = 0,02 (39,8) - 0,4 = 0,396$$

$$\text{koefisien angin hisap, } c_2 = -0,4$$

$$w_t = 0,396 \times 0,4 = 0,16 \text{ kN/m}^2$$

$$w_h = -0,4 \times 0,4 = -0,16 \text{ kN/m}^2$$

a. Angin Kiri

– sisi kiri = tekan

$$w_{t1} = 0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,87 \text{ kN}$$

$$wt_2 = 0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,39 \text{ kN}$$

$$wt_3 = 0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,93 \text{ kN}$$

$$wt_4 = 0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,508 \text{ kN}$$

– sisi kanan = hisap

$$wh_1 = -0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,88 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,41 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,94 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,52 \text{ kN}$$

b. Angin Kanan

– sisi kiri = hisap

$$wh_1 = -0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,88 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,41 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,94 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,52 \text{ kN}$$

– sisi kanan = tekan

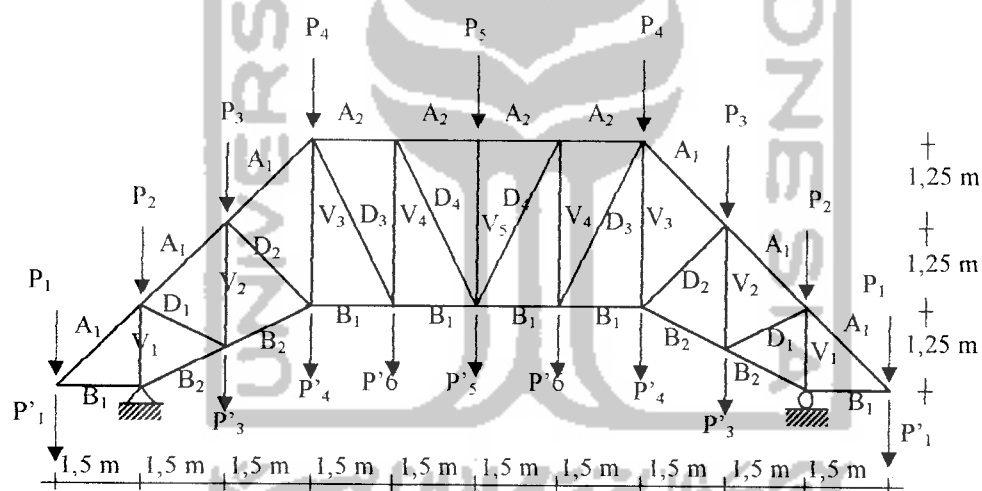
$$wt_1 = 0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,88 \text{ kN}$$

$$wt_2 = 0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,39 \text{ kN}$$

$$wt_3 = 0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,93 \text{ kN}$$

$$wt_4 = 0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,51 \text{ kN}$$

4.2.3 Perencanaan Pembebanan Kuda-kuda KA₃



Gambar 4.6 Kuda-Kuda KA₃

4.2.3.1 Beban Mati

$$P_1 = \text{berat gording} : 0,11 \times 6 = 0,66 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 2,75 \text{ kN}$$

$$\text{Berat total } P_1 = 3,41 \text{ kN}$$

$$P_2 = \text{berat gording} : 0,11 \times 4,5 = 0,49 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 4,39 \text{ kN}$$

$$\text{Berat total } P_2 = 4,88 \text{ kN}$$

$$P_3 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 2,93 \text{ kN}$$

$$\text{Berat total } P_3 = 3,26 \text{ kN}$$

$$P_4 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] = 3,21 \text{ kN}$$

$$\text{RKA}_{2 \text{ ki/ka}} : = 2,17 \text{ kN}$$

$$\text{Berat total } P_4 = 5,71 \text{ kN}$$

$$P_5 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times 3 \times 2,25 = 3,38 \text{ kN}$$

$$\text{RKA}_{2 \text{ teng}} : = 2,61 \text{ kN}$$

$$\text{Berat total } P_5 = 6,32 \text{ kN}$$

$$P'_1 = \text{berat eternit + penggantung} : 0,18 \times \left[\frac{(6 + 5,25) \times 0,75}{2} \right] = 0,76 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 0,5 \times 1,5 = 0,38 \text{ kN}$$

$$\text{Berat total } P'_1 = 1,14 \text{ kN}$$

$$P'_3 = \text{eternit + penggantung} = 0,18 \times \left[\frac{(3,75 + 2,25) \times 1,625}{2} \right] = 0,88 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} = 0,5 \times 1,5 = 0,75 \text{ kN}$$

$$\text{Berat total } P'_3 = 1,63 \text{ kN}$$

$P'_4 =$ berat eternit + penggantung :

$$0,18 \times \left[\left(\frac{(2,25 + 0,75) \times 1,63}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] = 0,98 \text{ kN}$$

berat taksiran kuda-kuda : $0,5 \times 2,25 = 1,13 \text{ kN}$

$RKA_2'_{ki/ka} = 0,26 \text{ kN}$

Berat total $P'_4 = 4,07 \text{ kN}$

$P'_5 =$ berat eternit + penggantung = $0,18 \times 3 \times 2,44 = 1,32 \text{ kN}$

berat taksiran kuda-kuda = $0,5 \times 3 = 1,50 \text{ kN}$

$RKA_2'_{teng} = 1,69 \text{ kN}$

Berat total $P'_5 = 4,51 \text{ kN}$

$P'_6 =$ berat eternit + penggantung = $0,18 \times 3 \times 2,44 = 1,32 \text{ kN}$

berat taksiran kuda-kuda = $0,5 \times 3 = 1,50 \text{ kN}$

Berat total $P'_6 = 2,82 \text{ kN}$

4.2.3.2 Beban Hidup

$$P_1 = 0,2 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 1,09 \text{ kN}$$

$$P_2 = 0,2 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,76 \text{ kN}$$

$$P_3 = 0,2 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 1,17 \text{ kN}$$

$$P_4 = 0,2 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) \times 2 + \left(\frac{0,75}{2} \times 0,75 \times 2 \right) \right] + 0,66 = 1,94 \text{ kN}$$

$$P_4' = RKA_2'_{ki/ka} = 0,04 \text{ kN}$$

$$P_5 = 0,2 \times 3 \times 2,25 + 0,78 = 2,13 \text{ kN}$$

$$P_5' = RKA_2'_{teng} = 0,44 \text{ kN}$$

4.2.3.3 Beban Angin

$$\text{muatan angin} = 0,4 \text{ kN/m}^2$$

$$\text{koefisien angin tekan, } c_1 = (0,02 \times \alpha) - 0,4 = 0,02 (39,8) - 0,4 = 0,4$$

$$\text{koefisien angin hisap, } c_2 = -0,4$$

$$w_t = 0,4 \times 0,4 = 0,16 \text{ kN/m}^2$$

$$w_h = -0,4 \times 0,4 = -0,16 \text{ kN/m}^2$$

a. Angin Kiri

– sisi kiri = tekan

$$w_{t1} = 0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,87 \text{ kN}$$

$$w_{t_{x1}} = 0,87 \times \sin 39,8 = 0,56 \text{ kN}$$

$$w_{t_{y1}} = 0,87 \times \cos 39,8 = 0,67 \text{ kN}$$

$$w_{t2} = 0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,39 \text{ kN}$$

$$w_{t_{x2}} = 1,39 \times \sin 39,8 = 0,89 \text{ kN}$$

$$w_{t_{y2}} = 1,39 \times \cos 39,8 = 1,07 \text{ kN}$$

$$w_{t3} = 0,1584 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,93 \text{ kN}$$

$$w_{t_{x3}} = 0,93 \times \sin 39,8 = 0,59 \text{ kN}$$

$$w_{t_{y3}} = 0,93 \times \cos 39,8 = 0,71 \text{ kN}$$

$$w_{t4} = 0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,51 \text{ kN}$$

$$w_{t_{x4}} = 0,51 \times \sin 39,8 = 0,33 \text{ kN}$$

$$w_{t_{y4}} = 0,51 \times \cos 39,8 = 0,39 \text{ kN}$$

$$P_4 = RKA_{2\text{ki/ka}} = -0,02 \text{ kN}$$

$$P_4' = RKA_{2'\text{ki/ka}} = 0,04 \text{ kN}$$

$$P_5 = RKA_{2\text{teng}} = 0,16 \text{ kN}$$

$$P_5' = RKA_{2'\text{teng}} = 0,62 \text{ kN}$$

– sisi kanan = hisap

$$wh_1 = -0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,88 \text{ kN}$$

$$wh_{x1} = 0,88 \times \sin 39,8 = 0,56 \text{ kN}$$

$$wh_{y1} = 0,88 \times \cos 39,8 = 0,68 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,41 \text{ kN}$$

$$wh_{x2} = 1,41 \times \sin 39,8 = 0,90 \text{ kN}$$

$$wh_{y2} = 1,41 \times \cos 39,8 = 1,08 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,94 \text{ kN}$$

$$wh_{x3} = 0,94 \times \sin 39,8 = 0,60 \text{ kN}$$

$$wh_{y3} = 0,94 \times \cos 39,8 = 0,72 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,51 \text{ kN}$$

$$wh_{x4} = 0,51 \times \sin 39,8 = 0,33 \text{ kN}$$

$$wh_{y4} = 0,51 \times \cos 39,8 = 0,39 \text{ kN}$$

b. Angin Kanan

– sisi kiri = hisap

$$wh_1 = -0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,88 \text{ kN}$$

$$wh_{x1} = 0,88 \times \sin 39,8 = 0,56 \text{ kN}$$

$$wh_{y1} = 0,88 \times \cos 39,8 = 0,66 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,41 \text{ kN}$$

$$wh_{x2} = 1,41 \times \sin 39,8 = 0,90 \text{ kN}$$

$$wh_{y2} = 1,41 \times \cos 39,8 = 1,08 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,94 \text{ kN}$$

$$wh_{x3} = 0,94 \times \sin 39,8 = 0,60 \text{ kN}$$

$$wh_{y3} = 0,94 \times \cos 39,8 = 0,72 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,51 \text{ kN}$$

$$wh_{x4} = 0,51 \times \sin 39,8 = 0,33 \text{ kN}$$

$$wh_{y4} = 0,51 \times \cos 39,8 = 0,39 \text{ kN}$$

$$P_4 = RKA_{2 \text{ ki'ka}} = -0,02 \text{ kN}$$

$$P_4' = RKA_{2' \text{ ki'ka}} = -0,04 \text{ kN}$$

$$P_5 = RKA_{2 \text{ teng}} = -0,63 \text{ kN}$$

$$P_5' = RKA_{2' \text{ teng}} = -0,63 \text{ kN}$$

_ sisi kanan = tekan

$$wt_1 = 0,16 \times \left[\frac{(6 + 5,25) \times 0,98}{2} \right] = 0,87 \text{ kN}$$

$$wt_{x1} = 0,87 \times \sin 39,8 = 0,56 \text{ kN}$$

$$wt_{y1} = 0,87 \times \cos 39,8 = 0,67 \text{ kN}$$

$$wt_2 = 0,16 \times \left[\frac{(5,25 + 3,75) \times 1,95}{2} \right] = 1,39 \text{ kN}$$

$$wt_{x2} = 1,39 \times \sin 39,8 = 0,89 \text{ kN}$$

$$wt_{y2} = 1,39 \times \cos 39,8 = 1,07 \text{ kN}$$

$$wt_3 = 0,16 \times \left[\frac{(3,75 + 2,25) \times 1,95}{2} \right] = 0,93 \text{ kN}$$

$$wt_{x3} = 0,93 \times \sin 39,8 = 0,59 \text{ kN}$$

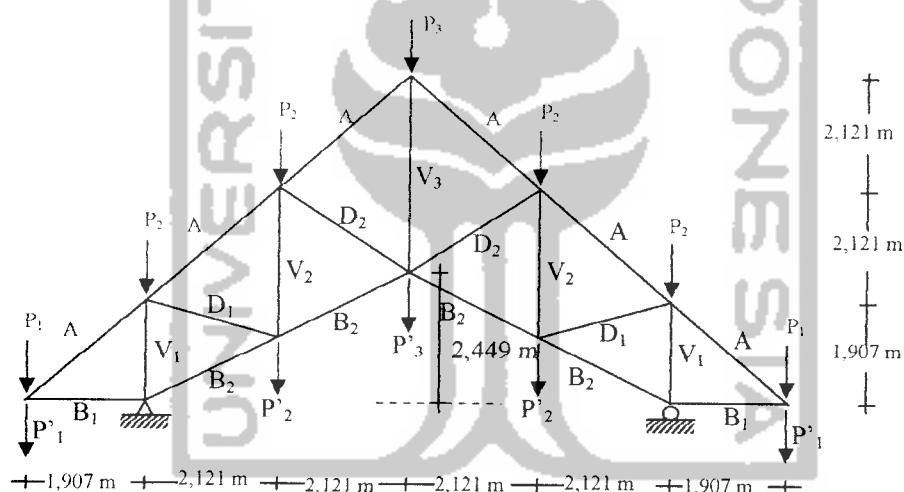
$$wt_{y3} = 0,93 \times \cos 39,8 = 0,71 \text{ kN}$$

$$wt_4 = 0,16 \times \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) + \left(\frac{0,75}{2} \times 0,75 \right) \right] = 0,51 \text{ kN}$$

$$wt_{x4} = 0,51 \times \sin 39,8 = 0,33 \text{ kN}$$

$$wt_{y4} = 0,51 \times \cos 39,8 = 0,39 \text{ kN}$$

4.2.4 Perencanaan Kuda-Kuda KA₄



Gambar 4.7 Kuda-Kuda KA₄

4.2.4.1 Beban Mati

$$P_1 = \text{berat gording} : 0,11 \times 8,7 = 0,96 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\left(\frac{(4,35 + 3,68) \times 0,95}{2} \right) \times 2 \right] = 3,82 \text{ kN}$$

$$\text{Berat total } P_1 = 4,78 \text{ kN}$$

$$P_2 = \text{berat gording} : 0,11 \times 6 = 0,66 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\left(\frac{(3,68 + 2,25) \times 2,01}{2} \right) \times 2 \right] = 5,97 \text{ kN}$$

$$\text{Berat total } P_2 = 6,63 \text{ kN}$$

$$P_3 = \text{berat gording} : 0,11 \times 3 = 0,33 \text{ kN}$$

$$\text{berat genteng} : 0,5 \times \left[\left(\frac{(2,25 + 0,75) \times 2,12}{2} \right) \times 2 \right] = 3,18 \text{ kN}$$

$$\text{Berat total } P_3 = 3,41 \text{ kN}$$

$$P_4 = \text{berat genteng} : 0,5 \cdot 4 \cdot \left[\left(\frac{(2,25 + 0,75) \times 1,95}{2} \right) \cdot 2 + \left(\frac{0,75}{2} \times 0,75 \right) \cdot 2 \right] = 1,13 \text{ kN}$$

$$P'_1 = \text{berat eternit + penggantung} : 0,18 \left[\left(\frac{(4,35 + 3,68) \cdot 0,675}{2} \right) \cdot 2 \right] = 0,98 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 0,5 \times 1,91 = 0,48 \text{ kN}$$

$$\text{Berat total } P'_1 = 1,46 \text{ kN}$$

$$P'_3 = \text{berat eternit + penggantung} = 0,18 \cdot \left[\left(\frac{(2,25 + 0,75) \cdot 2,12}{2} \right) \cdot 2 \right] = 1,14 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} = 0,5 \times 2,12 = 1,07 \text{ kN}$$

$$\text{Berat total } P'_3 = 2,21 \text{ kN}$$

$$P'_4 = \text{berat eternit + penggantung} : 0,18 \times \left[\frac{0,75}{2} \times 0,75 \times 2 \right] = 0,10 \text{ kN}$$

$$\text{berat taksiran kuda-kuda} : 0,5 \times 1,06 = 0,53 \text{ kN}$$

$$\text{Berat total } P'_4 = 0,63 \text{ kN}$$

$$P'_4 = 0,63 \times 4 = 2,52 \text{ kN}$$

4.2.4.2 Beban Hidup

$$P_1 = 0,2 \times \left[\left(\frac{(6 + 5,25) \times 0,98}{2} \right) \times 2 \right] = 1,53 \text{ kN}$$

$$P_2 = 0,2 \times \left[\left(\frac{(5,25 + 3,75) \times 1,95}{2} \right) \times 2 \right] = 2,39 \text{ kN}$$

$$P_3 = 0,2 \times \left[\left(\frac{(3,75 + 2,25) \times 1,95}{2} \right) \times 2 \right] = 1,27 \text{ kN}$$

$$P_4 = 0,2 \times 4 \times \left[\frac{0,75}{2} \times 0,75 \times 2 \right] = 0,45 \text{ kN}$$

4.2.4.3 Beban Angin

$$\text{muatan angin} = 0,4 \text{ kN/m}^2$$

$$\text{sudut kemiringan } \alpha = 45^\circ$$

$$\text{koefisien angin tekan, } c_1 = (0,02 \times \alpha) - 0,4 = 0,02 (45) - 0,4 = 0,5$$

$$\text{koefisien angin hisap, } c_2 = -0,4$$

$$w_t = 0,5 \times 0,4 = 0,20 \text{ kN/m}^2$$

$$w_h = -0,4 \times 0,4 = -0,16 \text{ kN/m}^2$$

a. Angin Kiri

_ sisi kiri = tekan

$$w_{t1} = 0,2 \times \left[\frac{(4,35 + 3,68) \times 0,95}{2} \right] = 0,77 \text{ kN}$$

$$w_{tx1} = w_{ty1} = 0,77 \times \sin 45 = 0,54 \text{ kN}$$

$$w_{t2} = 0,2 \times \left[\frac{(3,68 + 2,25) \times 2,01}{2} \right] = 1,19 \text{ kN}$$

$$w_{tx2} = w_{ty2} = 1,19 \times \sin 45 = 0,84 \text{ kN}$$

$$wt_3 = 0,2 \times \left[\frac{(2,25 + 0,75) \times 2,12}{2} \right] = 0,64 \text{ kN}$$

$$wt_{x3} = wt_{y3} = 0,64 \times \sin 45 = 0,45 \text{ kN}$$

$$wt_4 = 0,2 \times \left[\frac{0,75}{2} \times 0,75 \right] = 0,06 \text{ kN}$$

$$wt_{x4} = wt_{y4} = 2 \times (0,06 \times \sin 45) = 2 \times 0,04 = 0,08 \text{ kN}$$

– sisi kanan = hisap

$$wh_1 = -0,16 \times \left[\frac{(4,35 + 3,68) \times 0,95}{2} \right] = 0,61 \text{ kN}$$

$$wh_{x1} = wh_{y1} = 0,61 \times \sin 45 = 0,43 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(3,68 + 2,25) \times 2,01}{2} \right] = 0,96 \text{ kN}$$

$$wh_{x2} = wh_{y2} = 0,96 \times \sin 45 = 0,68 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(2,25 + 0,75) \times 2,12}{2} \right] = 0,51 \text{ kN}$$

$$wh_{x3} = wh_{y3} = 0,51 \times \sin 45 = 0,36 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\frac{0,75}{2} \times 0,75 \right] = 0,05 \text{ kN}$$

$$wh_{x4} = wh_{y4} = 2 \times (0,05 \times \sin 45) = 0,06 \text{ kN}$$

b. Angin Kanan

– sisi kiri = hisap

$$wh_1 = -0,16 \times \left[\frac{(4,35 + 3,68) \times 0,95}{2} \right] = 0,61 \text{ kN}$$

$$wh_{x1} = wh_{y1} = 0,61 \times \sin 45 = 0,43 \text{ kN}$$

$$wh_2 = -0,16 \times \left[\frac{(3,68 + 2,25) \times 2,01}{2} \right] = 0,96 \text{ kN}$$

$$wh_{x2} = wh_{y2} = 0,96 \times \sin 45 = 0,68 \text{ kN}$$

$$wh_3 = -0,16 \times \left[\frac{(2,25 + 0,75) \times 2,12}{2} \right] = 0,51 \text{ kN}$$

$$wh_{x3} = wh_{y3} = 0,51 \times \sin 45 = 0,36 \text{ kN}$$

$$wh_4 = -0,16 \times \left[\frac{0,75}{2} \times 0,75 \right] = 0,05 \text{ kN}$$

$$wh_{x4} = wh_{y4} = 2 \times (0,05 \times \sin 45) = 0,06 \text{ kN}$$

– sisi kanan = tekan

$$wt_1 = 0,2 \times \left[\frac{(4,35 + 3,68) \times 0,95}{2} \right] = 0,77 \text{ kN}$$

$$wt_{x1} = wt_{y1} = 0,77 \times \sin 45 = 0,54 \text{ kN}$$

$$wt_2 = 0,2 \times \left[\frac{(3,68 + 2,25) \times 2,01}{2} \right] = 1,19 \text{ kN}$$

$$wt_{x2} = wt_{y2} = 1,19 \times \sin 45 = 0,84 \text{ kN}$$

$$wt_3 = 0,2 \times \left[\frac{(2,25 + 0,75) \times 2,12}{2} \right] = 0,64 \text{ kN}$$

$$wt_{x3} = wt_{y3} = 0,64 \times \sin 45 = 0,45 \text{ kN}$$

$$wt_4 = 0,2 \times \left[\frac{0,75}{2} \times 0,75 \right] = 0,06 \text{ kN}$$

$$wt_{x4} = wt_{y4} = 2 \times (0,06 \times \sin 45) = 0,08 \text{ kN}$$

$$wt_1 = 0,2 \times \left[\frac{(4,35 + 3,68) \times 0,95}{2} \right] = 0,77 \text{ kN}$$

$$wt_{x1} = wt_{y1} = 0,77 \times \sin 45 = 0,54 \text{ kN}$$

$$wt_2 = 0,2 \times \left[\frac{(3,68 + 2,25) \times 2,01}{2} \right] = 1,19 \text{ kN}$$

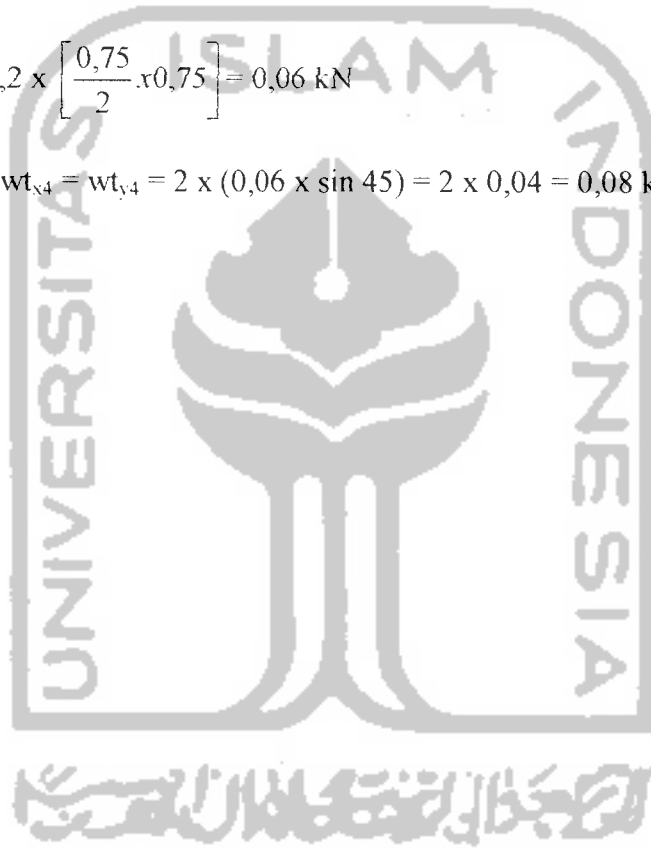
$$wt_{x2} = wt_{y2} = 1,19 \times \sin 45 = 0,84 \text{ kN}$$

$$wt_3 = 0,2 \times \left[\frac{(2,25 + 0,75) \times 2,12}{2} \right] = 0,64 \text{ kN}$$

$$wt_{x3} = wt_{y3} = 0,64 \times \sin 45 = 0,45 \text{ kN}$$

$$wt_4 = 0,2 \times \left[\frac{0,75}{2} \times 0,75 \right] = 0,06 \text{ kN}$$

$$wt_{x4} = wt_{y4} = 2 \times (0,06 \times \sin 45) = 2 \times 0,04 = 0,08 \text{ kN}$$



4.3 Pendimensionian Rangka Kuda-Kuda

4.3.1 Dimensi Rangka Kuda-Kuda KA₁

a. Perhitungan Batang Desak Vertikal

$$N_u = -64,588 \text{ kN}$$

$$L = 1,250 \text{ m}$$

$$F_y = 250 \text{ MPa} = 250 \cdot 10^3 \text{ kN/m}^2$$

Menentukan N_n perlu

$$N_n = \frac{-64,588}{0,85} = -75,99 \text{ kN} \quad (\text{dari 3.10})$$

Estimasi jarak kopel minimum

$$L_1 = \frac{1250}{3} = 416,667 \text{ mm} \quad (\text{dari 3.11})$$

Menentukan i minimum perlu

$$i_{x \min} = i_{y \min} ; \quad i_{\min} = \frac{416,667}{200} = 2,083 \text{ mm} \quad (\text{dari 3.12a})$$

Dicoba profil 2L 50.50.5

$$A_g = 480 \text{ mm}^2 \quad i_{\eta} = 9,8 \text{ mm}$$

$$I_x = I_y = 11 \cdot 10^4 \text{ mm}^4$$

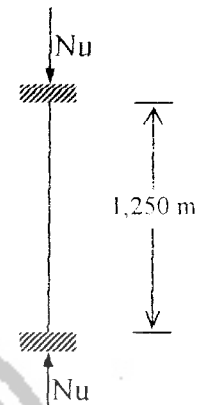
Kontrol kestabilan elemen batang

$$\lambda_1 = \frac{1 \times 416,67}{9,8} = 42,52 < 50 \quad (\text{dari 3.13})$$

Jari-jari batang ganda

$$I_x = 2 \cdot 11 \cdot 10^4 = 22 \cdot 10^4 \text{ mm}^4 \quad (\text{dari 3.14a})$$

$$i_x = \sqrt{\frac{22 \cdot 10^4}{2 \cdot 480}} = 15,1 \text{ mm} \quad (\text{dari 3.14c})$$



$$I_y = 2 \left[10000 + 480(14 + 0,5x5)^2 \right] = 481360 \text{ mm}^4 \quad (\text{dari 3.14d})$$

$$i_y = \sqrt{\frac{481360}{960}} = 22,392 \quad (\text{dari 3.14e})$$

Kontrol batas kelangsingan dan batas kekompakan profil

$$\lambda_r = \frac{50}{5} = 10 < \lambda_r = \frac{200}{\sqrt{250}} = 12,65 \quad (\text{dari 3.15})$$

Kelangsingan arah sumbu bahan (sumbu x-x)

$$\lambda_x = \frac{1 \times 1250}{30,2} = 82,781 \quad (\text{dari 3.16})$$

Kontrol kestabilan batang

$$\frac{\lambda_x}{\lambda_1} = \frac{82,781}{42,517} = 1,947 > 1,2 \quad (\text{dari 3.17})$$

Kelangsingan arah sumbu bebas bahan (sumbu y-y)

$$\lambda_y = \frac{1 \times 1250}{22,392} = 55,823 \quad (\text{dari 3.18})$$

Kelangsingan ideal, (dari 3.19)

$$\lambda_{iy} = \sqrt{55,823^2 + \frac{2}{2} \times 42,517^2} = 70,170 > 1,2\lambda_1 = 1,2 \times 42,517 = 51,02$$

Tekuk terjadi pada sumbu bahan

$$\lambda_{cx} = \frac{82,781}{\pi} \sqrt{\frac{250}{2 \times 10^5}} = 0,932 ; (0,25 < \lambda_c < 1,2) \quad (\text{dari 3.20})$$

$$\omega_x = \frac{1,43}{(1,6 - 0,67 \times 0,932)} = 1,466 \quad (\text{dari 3.20b})$$

Tekuk terjadi pada sumbu bebas bahan

$$\lambda_{ey} = \frac{70,170}{\pi} \sqrt{\frac{250}{2 \times 10^5}} = 0,789 ; (0,25 < \lambda_c < 1,2) \quad (\text{dari 3.21})$$

$$\omega_y = 1,3347 \quad (\text{dari 3.20b})$$

Kapasitas gaya tekan, (dari 3.22a dan 22b)

$$\phi N_{nx} = 0,85 \times 960 \times \frac{250}{1,466} \times 10^{-3} = 139,154 \text{ kN} > N_u = 64,5876 \text{ kN}$$

$$\phi N_{ny} = 0,85 \times 960 \times \frac{250}{1,3347} \times 10^{-3} = 152,843 \text{ kN} > N_u = 64,5876 \text{ kN}$$

Kontrol tekuk lentur-torsi ($\nu = 0,3$)

$$G = \frac{2 \cdot 10^5}{2(1+0,3)} = 76923,077 \text{ MPa} \quad (\text{dari 3.23a})$$

$$J = \sum 2 \left[\frac{1}{3} \times 50 \times 5^3 + \frac{1}{3} \left(50 - \frac{5}{2} \right) \times 5^3 \right] = 8125 \text{ mm}^4 \quad (\text{dari 3.23b})$$

$$y_o = 14 - \frac{5}{2} = 11,5 \text{ mm} \quad (\text{dari 3.23c})$$

$$x_o = 0 \quad (\text{dari 3.23d})$$

$$r_o^2 = \frac{110000 + 110000}{960} + 0 + 11,5^2 = 361,417 \text{ mm}^2 \quad (\text{dari 3.23e})$$

$$f_{crz} = \frac{76923,077 \times 8125}{960 \times 361,417} = 1801,359 \text{ MPa} \quad (\text{dari 3.23f})$$

$$H = 1 - \left(\frac{0 + 11,5^2}{361,417} \right) = 0,634 \quad (\text{dari 3.23g})$$

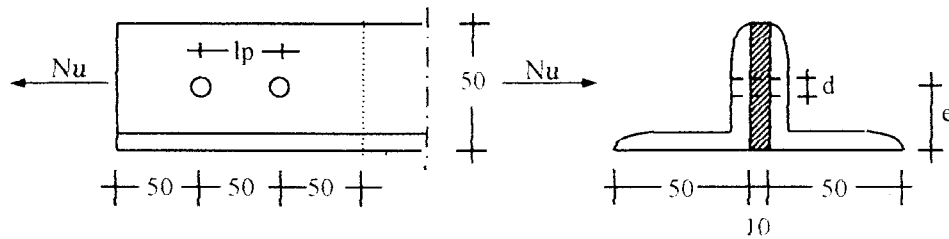
$$f_{cry} = \frac{250}{1,466} = 170,532 \text{ MPa} \quad (\text{dari 3.23h})$$

$$f_{ct} = \left(\frac{170,532 + 1801,359}{2 \times 0,634} \right) \left[1 - \sqrt{1 - \frac{4 \times 170,532 \times 1801,359 \times 0,634}{(170,532 + 1801,359)^2}} \right]$$

$$= 164,483 \text{ MPa} \quad (\text{dari 3.23i})$$

$$\phi N_{nt} = 134,218 \text{ kN} > N_u = 64,588 \text{ kN} \quad (\text{dari 3.23j})$$

b. Perencanaan Batang Tarik Bawah



Gambar 4.8 Gambar Batang Tarik Ganda

$$Nu = 60,023 \text{ kN}$$

$$L = 1,625 \text{ m}$$

Rumus yang digunakan dalam perhitungan batang tarik sama dengan perhitungan batang desak sampai pada perhitungan kestabilan batang ganda

Estimasi jarak kopel minimum

$$L_1 = \frac{1625}{3} = 541,67 \text{ mm}$$

Menentukan i minimum perlu

$$i_{x \min} = i_{y \min} ; i_{\min} = \frac{541,67}{240} = 2,257 \text{ mm}$$

Menentukan luas (A_g) perlu

$$A_g = \frac{60,023 \cdot 1000}{0,9 \cdot 250} = 266,77 \text{ mm}^2$$

Menentukan luas efektif (A_e) perlu

$$A_g = \frac{60,023 \cdot 1000}{0,75 \cdot 310} = 258,164 \text{ mm}^2$$

Dicoba profil 2 L 50.50.5

$$A_g = 480 \text{ mm}^2$$

$$i_{\eta} = 9,8 \text{ mm} > i_{\min} = 2,257 \text{ mm}$$

$$I_x = I_y = 11.10^4 \text{ mm}^4$$

Kestabilan batang ganda

$$I_x = 2.11.10^4 = 22.10^4 \text{ mm}^4$$

$$i_x = \sqrt{\frac{22.10^4}{2.480}} = 15,1 \text{ mm}$$

$$I_y = 2 \left[110000 + 480(14 + 0,5 \cdot 5)^2 \right] = 481360 \text{ mm}^4$$

$$i_y = \sqrt{\frac{481360}{960}} = 22,392$$

$$\frac{L}{i_{\min}} = \frac{1625}{15,1} = 107,6 < 240$$

Luas penampang netto

$$A_n = 960 - 1 \times 14 \times 5 = 890 \text{ mm}^2 \quad (\text{dari 3.24})$$

Luas penampang efektif (tarik murni)

$$A_e = \left(1 - \frac{14}{50} \right) 890 = 640,8 \text{ mm}^2 \quad (\text{dari 3.25})$$

Luas bidang geser (geser murni)

$$A_{ns} = 2(2 \times 50) \times 5 = 1000 \text{ mm}^2 \quad (\text{dari 3.26})$$

Luas bidang kombinasi geser + tarik pada blok ujung

$$A_{nt} = \left[(50 - 14) - \frac{1}{2} \times 14 \right] \times 5 = 145 \text{ mm}^2 \quad (\text{dari 3.27a})$$

$$A_{gt} = (50 - 14) \times 5 = 180 \text{ mm}^2 \quad (\text{dari 3.27b})$$

$$A_{nv} = (100 - 14 \times 1,5) \times 5 = 395 \text{ mm}^2 \quad (\text{dari 3.27c})$$

$$A_{gv} = 100 \times 5 = 500 \text{ mm}^2 \quad (\text{dari 3.27d})$$

Kontrol kapasitas penampang

a. Tarik murni (kriteria leleh), (dari 3.28a dan 3.28b)

- Leleh ; $\phi_t N_n = 0,9 \times 250 \times (2 \times 480) \times 10^{-3} = 216 \text{ kN}$
- Fraktur; $\phi_t N_n = 0,75 \times 310 \times 640,8 \times 10^{-3} = 190,65 \text{ kN}$

b. Geser murni (kriteria fraktur), (dari 3.28c)

$$\phi_t N_n = 0,75 (0,6 \times 310) 500 \times 10^{-3} = 69,75 \text{ kN}$$

c. Kombinasi geser + tarik (pada blok ujung), (dari 3.28d dan 3.28e)

- Geser fraktur; $N_n = 0,6 \times 310 \times 395 \times 10^{-3} = 73,47 \text{ kN}$
- Tarik fraktur; $N_n = 310 \times 145 \times 10^{-3} = 44,95 \text{ kN}$

Kapasitas satu penampang adalah:

- Kriteria pelepasan geser – retakan tarik

$$\begin{aligned} \phi_t N &= 0,75(0,6 \times 250 \times 500 + 310 \times 145) \times 10^{-3} \\ &= 90,017 \text{ kN} > \frac{1}{2} N_u = 30,0116 \text{ kN} \end{aligned} \quad (\text{dari 3.29a})$$

- Kriteria retakan geser – pelepasan tarik

$$\begin{aligned} \phi N &= 0,75(250 \times 180 + 0,6 \times 310 \times 395) \times 10^{-3} \\ &= 88,852 \text{ kN} > \frac{1}{2} N_u = 30,0116 \text{ kN} \end{aligned} \quad (\text{dari 3.29b})$$

Untuk perhitungan rangka batang kuda-kuda yang lainnya ditabelkan dibawah ini

Lanjutan Tabel 4.3.a Perencanaan Batang Tarik Kuda-Kuda KA₁

Batang	Beban Rencana		L (mm)	Profil	Data Profil			Penamapng Perlu				Nn Fraktur (kN)	Cek Blok Geser			Aman/tdk aman Nn frak = 190,65 kN Nn PGRT = 89,9 kN
	Tarik (kN)	Tekan (kN)			Luas Ag mm ²	i min (mm)	Ae=U ₁ An (mm ²)	Ag mm ²	i min (mm)	Ae (mm ²)	Nn Leleh (kN)		Nn PGRT	Nn PGRT	Nn RGPT	
V ₁	-	-64,5876	1250	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
V ₂	-	-32,5632	1875	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
V ₃	-	-13,0000	2500	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
V ₄	4,5174	-	3125	2L 50.50.5	960	9,8	1353,6	20,09	6,510	19,43	423	405,01	198,84	157,01	-	Aman
V ₅	59,9688	-	3750	2L 50.50.5	960	9,8	2676,6	266,53	7,813	257,9	697,5	622,31	279,5	242,16	-	Aman
V ₆	7,0992	-	3125	2L 50.50.5	960	9,8	1353,6	31,56	6,510	30,53	423	405,01	198,84	157,01	-	Aman
V ₇	-	-13,5485	2500	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
V ₈	-	-32,5632	1875	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
V ₉	-	-64,5876	1250	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
D ₁	46,3380	-	1625	2L 50.50.5	960	9,8	640,8	205,96	6,771	199,3	216	190,65	146,94	89,9	-	Aman
D ₂	16,7468	-	1953	2L 50.50.5	960	9,8	640,8	74,45	8,138	72,03	216	190,65	146,94	89,9	-	Aman
D ₃	3,7960	-	2401	2L 50.50.5	960	9,8	1166,2	16,89	5,002	16,33	283,95	271,11	191,39	114,04	-	Aman
D ₄	-	-12,5953	2915	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
D ₅	-	-14,6025	2915	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-
D ₆	6,2764	-	2401	2L 50.50.5	960	9,8	1166,2	27,89	5,002	26,94	283,95	271,11	191,39	114,04	-	Aman
D ₇	17,3171	-	1953	2L 50.50.5	960	9,8	640,8	76,98	8,138	74,36	216	190,65	146,94	89,9	-	Aman
D ₈	46,3380	-	1625	2L 50.50.5	960	9,8	640,8	205,96	6,771	198,9	216	190,65	146,94	89,9	-	Aman

Tabel 4.3.b Perencanaan Batang Tekan Kuda-Kuda KA1

Btg	Beban Rencana		L (mm)	Profil	Data Profil			Penampang perlu			λR	λ_{cx}	ω_x	Nnx kN	λ_{cy}	ω_y	Nny kN	Aman/ tdk aman Nnx&Nny > Nn perlu
	Tarik (kN)	Tekan (kN)			Ag mm ²	i min (mm)	L ₁	L ₁ /i min <50	Nn perlu (kN)	i min (mm)								
A ₁	11,19	-	1953	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
A ₂	-	-44,49	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₂	-	-61,24	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₄	-	-62,41	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₅	-	-55,80	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₆	-	-55,80	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₇	-	-62,41	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₈	-	-61,24	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₉	-	-44,49	1953	2L 50.50.5	960	9,8	448,25	45,73	2,24	10	12,64	0,52	1,139	210,64	1,09	1,647	123,88	Aman
A ₁₀	11,19	-	1953	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₁	-	-8,93	1500	2L 50.50.5	960	9,8	375	38,27	1,87	10	12,64	0,43	1,090	220,042	0,89	1,428	142,87	Aman
B ₂	-	-22,37	1625	2L 50.50.5	960	9,8	406,25	41,45	2,03	10	12,64	0,47	1,087	220,709	0,95	1,481	137,78	Aman
B ₃	48,78	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₄	60,02	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₅	59,09	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₆	52,90	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₇	50,97	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₈	37,03	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-
B ₉	-	-9,68	1625	2L 50.50.5	960	9,8	406,25	41,45	2,03	10	12,64	0,47	1,087	220,71	0,95	1,481	137,78	Aman
B ₁₀	-	-8,93	1500	2L 50.50.5	960	9,8	375	38,27	1,87	10	12,64	0,43	1,090	220,04	0,89	1,428	142,87	Aman

Tabel 4.4.a Perencanaan Batang Tarik Kuda-Kuda KA₂

Batang	Beban Rencana		L (mm)	Profil	Data Profil			Penampang Perlu			Nn Leleh (kN)	Nn Fraktur (kN)	Cek Blok Geser		Aman/tdk aman Nn frak = 190,65 kN Nn PGRT = 89,9 kN
	Tarik (kN)	Tekan (kN)			I min (mm)	Ae=U ₁ An (mm ²)	Ag mm ²	i min (mm)	Ae (mm ²)	Nn PGRT			Nn RGPT		
A ₁	10,5764	-	1952	2L 50.50.5	9,8	640,8	49,179	8,133	47,51	216	190,65	146,94	89,9	Aman	
A ₂	4,1708	-	1952	2L 50.50.5	9,8	640,8	18,537	8,133	17,91	216	190,65	146,94	89,9	Aman	
A ₂	6,8436	-	1952	2L 50.50.5	9,8	640,8	30,416	8,133	29,38	216	190,65	146,94	89,9	Aman	
B ₁₁	-	-8,4437	1500	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
B ₁₂	-	-2,6666	1625	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
B ₁₃	2,6666	-	1625	2L 50.50.5	9,8	640,8	11,852	6,771	11,45	216	190,65	146,94	89,9	Aman	
V ₂₁	-	-14,504	1250	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
V ₂₂	-	-2,148	1874	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
V ₂₃	0	-	2499	2L 50.50.5	9,8	640,8	0	5,206	0	216	190,65	146,94	89,9	Aman	
D ₃₀	5,3308	-	1625	2L 50.50.5	9,8	640,8	23,692	6,771	22,89	216	190,65	146,94	89,9	Aman	
D ₃₁	-	-3,4919	1952	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
A ₈	5,7124	-	1952	2L 50.50.5	9,8	640,8	25,388	8,133	24,53	216	190,65	146,94	89,9	Aman	
A ₉	3,4168	-	1952	2L 50.50.5	9,8	640,8	15,186	8,133	14,67	216	190,65	146,94	89,9	Aman	
A ₁₀	10,5764	-	1952	2L 50.50.5	9,8	640,8	47,006	8,133	45,41	216	190,65	146,94	89,9	Aman	
B ₁₈	-	-4,0999	1625	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
B ₁₉	-	-9,1472	1625	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
B ₂₀	-	-8,4437	1500	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
V ₂₇	0	-	2499	2L 50.50.5	9,8	640,8	0	5,206	0	216	190,65	146,94	89,9	Aman	
V ₂₈	-	-2,632	1874	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
V ₂₉	-	-15,2284	1250	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
D ₃₆	-	-3,0634	1952	2L 50.50.5	-	-	-	-	-	-	-	-	-	-	
D ₃₇	5,9588	-	1625	2L 50.50.5	9,8	640,8	26,484	6,771	25,58	216	190,65	146,94	89,9	Aman	

Lanjutan Tabel 4.5.a Perencanaan Batang Tarik Kuda-Kuda KA₃

Batang	Beban Rencana		L (mm)	Profil	Data Profil			Penampang Perlu			Nn Leleh (kN)	Nn Fraktur (kN)	Cek Blok Geser		Aman/tdk aman Nn frak = 190,65 kN Nn PGRT = 89,9 kN
	Tarik (kN)	Tekan (kN)			Luas Ag (mm ²)	i min (mm)	Ae=U ₁ A _n (mm ²)	Ag (mm ²)	i min (mm)	Ae (mm ²)			Nn PGRT	Nn RGPT	
V ₂₁	-	-32,7524	1250	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₂	-	-14,3144	1874	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₃	5,4873	-	2499	2L 50.50.5	960	9,8	1211,5	31,56	5,206	30,53	283,95	281,67	194,98	113,62	Aman
V ₂₄	-	-4,8840	2499	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₅	-	-6,3888	2499	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₆	-	-5,2426	2499	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₇	3,3166	-	2499	2L 50.50.5	960	9,8	1764,2	14,74	5,206	14,23	283,95	281,67	194,98	113,62	Aman
V ₂₈	-	-14,3144	1874	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₂₉	-	-32,7524	1250	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
D ₃₀	21,1516	-	1625	2L 50.50.5	960	9,8	640,8	205,96	6,771	590,4	216	190,65	146,94	89,9	Aman
D ₃₁	6,8324	-	1952	2L 50.50.5	960	9,8	640,8	74,45	8,133	590,4	216	190,65	146,94	89,9	Aman
D ₃₂	6,1143	-	2915	2L 50.50.5	960	9,8	1166,2	16,89	6,073	590,4	283,95	271,11	191,39	114,04	Aman
D ₃₃	6,1143	-	2915	2L 50.50.5	960	9,8	1166,2	16,89	6,073	590,4	283,95	271,11	191,39	114,04	Aman
D ₃₄	6,1143	-	2915	2L 50.50.5	960	9,8	1166,2	16,89	6,073	590,4	283,95	271,11	191,39	114,04	Aman
D ₃₅	6,1143	-	2915	2L 50.50.5	960	9,8	1166,2	16,89	6,073	590,4	283,95	271,11	191,39	114,04	Aman
D ₃₆	7,2263	-	1952	2L 50.50.5	960	9,8	640,8	76,98	8,133	590,4	216	190,65	146,94	89,9	Aman
D ₃₇	21,1516	-	1625	2L 50.50.5	960	9,8	640,8	205,96	6,771	590,4	216	190,65	146,94	89,9	Aman

Tabel 4.5.b Perencanaan Batang Tekan Kuda-Kuda KA₃

Btg	Beban Rencana		L (mm)	Profil	Data Profil		L ₁	L ₁ /i min <50	Penampang perlu		λR	λcx	ωx	Nhx kN	λcy	ωy	Nhy kN	Aman/ tak aman Nhx&Nhy > Nn perlu	
	Tarik (kN)	Tekan (kN)			Ag Mm ²	i min (mm)			Nn perlu (kN)	i min (mm)									$\lambda = \frac{b}{t}$
A ₁	10,58	-	1952	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-	-	-	-	
A ₂	-	-14,84	1952	2L 50.50.5	960	9,8	448	49,79	-17,46	2,24	10	12,64	0,51	1,14	210,63	1,09	1,646	123,93	Aman
A ₃	-	-21,67	1952	2L 50.50.5	960	9,8	448	49,79	-25,49	2,24	10	12,64	0,51	1,14	210,63	1,09	1,646	123,93	Aman
A ₄	-	-19,58	1500	2L 50.50.5	960	9,8	375	38,26	-23,04	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman
A ₅	-	-22,51	1500	2L 50.50.5	960	9,8	375	38,26	-26,48	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman
A ₆	-	-22,51	1500	2L 50.50.5	960	9,8	375	38,26	-26,48	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman
A ₇	-	-19,58	1500	2L 50.50.5	960	9,8	375	38,26	-23,04	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman
A ₈	-	-21,67	1952	2L 50.50.5	960	9,8	448	49,79	-25,49	2,24	10	12,64	0,51	1,14	210,63	1,09	1,646	123,93	Aman
A ₉	-	-14,84	1952	2L 50.50.5	960	9,8	448	49,79	-17,46	2,24	10	12,64	0,51	1,14	210,63	1,09	1,646	123,93	Aman
A ₁₀	10,58	-	1952	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₁	-	-8,44	1500	2L 50.50.5	960	9,8	375	38,26	-10,50	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman
B ₁₂	-	-12,00	1625	2L 50.50.5	960	9,8	406	41,45	-26,31	2,03	10	12,64	0,46	1,08	220,71	0,94	1,481	137,78	Aman
B ₁₃	17,00	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₄	19,98	-	1500	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₅	21,41	-	1500	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₆	10,48	-	1500	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₇	17,34	-	1500	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₈	12,77	-	1625	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₉	-	-9,15	1625	2L 50.50.5	960	9,8	406	41,45	-11,38	2,03	10	12,64	0,46	1,08	220,71	0,94	1,481	137,78	Aman
B ₂₀	-	-8,45	1500	2L 50.50.5	960	9,8	375	38,26	-10,50	1,87	10	12,64	0,43	1,09	220,04	0,89	1,427	142,90	Aman

Tabel 4.6.a Perencanaan Batang Tarik Kuda-Kuda KA₁

Batang	Beban Rencana		L (mm)	Profil	Data Profil				Penampang Perlu				Cek Blok Geser		Aman/tdk aman Nn frak = 190,65 kN Nn PGRT = 89,9 kN
	Tarik (kN)	Tekan (kN)			Luas Ag mm ²	i min (mm)	Ae=U ₁ An (mm ²)	Ag mm ²	i min (mm)	Ae (mm ²)	Nn Fraktur (kN)	Nn Leleh (kN)	Nn PGRT	Nn RGPT	
A ₁	13,4108	-	2697	2L 50.50.5	960	9,8	1373,5	187,5	5,619	181,1	310,95	319,3	199,17	121,94	Aman
A ₂	-	-1,9206	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
A ₃	-	-2,7701	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
A ₄	-	-2,5057	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
A ₅	-	-2,1013	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
A ₆	13,4108	-	2697	2L 50.50.5	960	9,8	1373,5	187,5	5,619	181,1	310,95	319,3	199,17	121,94	Aman
B ₇	-	-9,4832	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
B ₈	-	-14,1809	2449	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
B ₉	4,7395	-	2449	2L 50.50.5	960	9,8	1764,2	31,56	5,102	30,53	283,95	281,67	194,98	113,62	Aman
B ₁₀	2,9806	-	2449	2L 50.50.5	960	9,8	1764,2	31,56	5,102	30,53	283,95	281,67	194,98	113,62	Aman
B ₁₁	-	-10,9508	2449	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
B ₁₂	-	-9,4832	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₁₃	-	-25,4940	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₁₄	-	-7,0516	2803	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₁₅	5,3046	-	3700	2L 50.50.5	960	9,8	2041	95,158	7,708	92,09	553,5	474,71	276,36	144,97	Aman
V ₁₆	-	-6,8948	2803	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
V ₁₇	-	-25,4940	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-
D ₁₈	11,3322	-	2228	2L 50.50.5	960	9,8	640,8	56,765	4,642	54,93	216	190,65	146,94	89,9	Aman
D ₁₉	1,6828	-	2644	2L 50.50.5	960	9,8	1373,5	187,5	5,508	181,1	310,95	319,3	199,17	121,94	Aman
D ₂₀	1,4228	-	2644	2L 50.50.5	960	9,8	1373,5	187,5	5,508	181,1	310,95	319,3	199,17	121,94	Aman

Lanjutan Tabel 4.6.a Perencanaan Batang Tarik Kuda-Kuda KA₁

Batang	Beban Rencana		L (mm)	Profil	Data Profil				Penampang Perlu				Nn Leleh (kN)	Nn Fraktur (kN)	Cek Blok Geser		Aman/tdk aman Nn frak = 190,65 kN Nn PGRT = 89,9 kN
	Tarik (kN)	Tekan (kN)			Luas Ag mm ²	i min (mm)	Ae=U,An (mm ²)	Ag mm ²	i min (mm)	Ae (mm ²)	Nn PGRT	Nn RGPT					
D ₂₁	11,1496	-	2228	2L 50.50.5	960	9,8	820	56,765	4,642	54,93	216	190,65	146,94	89,9	Aman		
A ₂₂	13,4108	-	2697	2L 50.50.5	960	9,8	1373,5	187,5	5,619	181,1	310,95	319,3	199,17	121,94	Aman		
A ₂₃	-	-1,9596	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
A ₂₄	-	-3,1861	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
A ₂₅	-	-3,1861	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
A ₂₆	-	-2,5355	3000	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
A ₂₇	13,4108	-	2697	2L 50.50.5	960	9,8	1373,5	187,5	5,619	181,1	310,95	319,3	199,17	121,94	Aman		
B ₂₈	-	-9,4832	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
B ₂₉	-	-13,9664	2449	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
B ₃₀	4,5497	-	2449	2L 50.50.5	960	9,8	1764,2	31,56	5,102	30,53	283,95	281,67	194,98	113,62	Aman		
B ₃₁	2,8636	-	2449	2L 50.50.5	960	9,8	1764,2	31,56	5,102	30,53	283,95	281,67	194,98	113,62	Aman		
B ₃₂	-	-10,9508	2449	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
B ₃₃	-	-9,4832	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
V ₃₄	-	-25,4940	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
V ₃₅	-	-7,2310	2803	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
V ₃₆	-	-6,8948	2803	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
V ₃₇	-	-25,4940	1907	2L 50.50.5	960	-	-	-	-	-	-	-	-	-	-		
D ₃₈	11,5428	-	2228	2L 50.50.5	960	9,8	640,8	56,765	4,642	54,93	216	190,65	146,94	89,9	Aman		
D ₃₉	1,8115	-	2644	2L 50.50.5	960	9,8	1373,5	187,5	5,508	181,1	310,95	319,3	199,17	121,94	Aman		
D ₄₀	1,3565	-	2644	2L 50.50.5	960	9,8	1373,5	187,5	5,508	181,1	310,95	319,3	199,17	121,94	Aman		
D ₄₁	11,1496	-	2228	2L 50.50.5	960	9,8	640,8	56,765	4,642	54,93	216	190,65	146,94	89,9	Aman		

Tabel 4.6.b Perencanaan Batang Tekan Kuda-Kuda KA₁

Btg	Beban Rencana		L (mm)	Profil	Data Profil		L ₁	L ₁ /l min <50	Nn perlu (kN)	i min (mm)	b x = t	λR	λcx	ωx	Nhx kN	λcy	ωy	Nny kN	Aman/tdk aman Nnx&Nny > Nn perlu		
	Tarik (kN)	Tekan (kN)			Ag mm ²	i min mm															
A ₁	13,41	-	2697	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
A ₂	-	-1,92	3000	2L 50.50.5	960	9,8	600	47,6	-2,2295	3	9,28	12,64	0,53	1,15	377,47	1,58	0,35	71,49	-	Aman	
A ₂	-	-2,77	3000	2L 50.50.5	960	9,8	600	47,6	-3,2589	3	9,28	12,64	0,53	1,15	377,47	1,58	0,35	71,49	-	Aman	
A ₄	-	-2,50	3000	2L 50.50.5	960	9,8	600	47,6	-2,9479	3	9,28	12,64	0,53	1,15	377,47	1,58	0,35	71,49	-	Aman	
A ₅	-	-2,10	3000	2L 50.50.5	960	9,8	600	47,6	-2,4721	3	9,28	12,64	0,53	1,15	377,47	1,58	0,35	71,49	-	Aman	
A ₆	13,41	-	2697	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	Aman
B ₇	-	-9,48	1907	2L 50.50.5	960	9,8	476,7	48,6	-11,1567	2,3	10	12,64	0,54	1,15	206,96	1,07	1,62	125,8	-	Aman	
B ₈	-	-14,18	2449	2L 50.50.5	960	9,8	489,8	45,7	-16,6834	2,4	9,16	12,64	0,51	1,13	276,84	1,34	2,25	90,34	-	Aman	
B ₉	4,73	-	2449	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₀	2,98	-	2449	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
B ₁₁	-	-10,95	2449	2L 50.50.5	960	9,8	489,8	45,7	-12,8833	2,4	9,16	12,64	0,51	1,13	276,84	1,34	2,25	90,34	-	Aman	
B ₁₂	-	-9,48	1907	2L 50.50.5	960	9,8	476,7	48,6	-10,8038	2,3	10	12,64	0,54	1,15	206,96	1,07	1,62	125,8	-	Aman	
V ₁₃	-	-25,49	1907	2L 50.50.5	960	9,8	476,7	48,6	-29,9929	2,3	10	12,64	0,54	1,15	206,96	1,07	1,62	125,8	-	Aman	
V ₁₄	-	-7,05	2803	2L 50.50.5	960	9,8	560,6	47,9	-8,2960	2,8	10	12,64	0,53	1,15	299,28	1,48	2,76	73,72	-	Aman	
V ₁₅	5,30	-	3700	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
V ₁₆	-	-6,89	2803	2L 50.50.5	960	9,8	560,6	47,9	-8,1115	2,8	10	12,64	0,53	1,15	299,28	1,48	2,76	73,72	-	Aman	
V ₁₇	-	-25,49	1907	2L 50.50.5	960	9,8	476,7	48,6	-29,9929	2,3	10	12,64	0,54	1,15	206,96	1,07	1,62	125,8	-	Aman	
D ₁₈	11,33	-	2228	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
D ₁₉	1,68	-	2644	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
D ₂₀	1,42	-	2644	2L 50.50.5	960	9,8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

4.4 Perencanaan Plat Lantai

4.4.1 Lantai ruang kuliah (plat 10)

a. Pembebanan plat per meter panjang

- Beban mati (dari 3.30a)

$$\text{slab beton} : 0,12 \times 24 \times 1 \times 1 = 2,88 \text{ kN/m}$$

$$\text{pasir} : 0,04 \times 18 \times 1 \times 1 = 0,72 \text{ kN/m}$$

$$\text{speci} : 0,02 \times 21 \times 1 \times 1 = 0,42 \text{ kN/m}$$

$$\text{tegel} : 0,01 \times 24 \times 1 \times 1 = 0,24 \text{ kN/m}$$

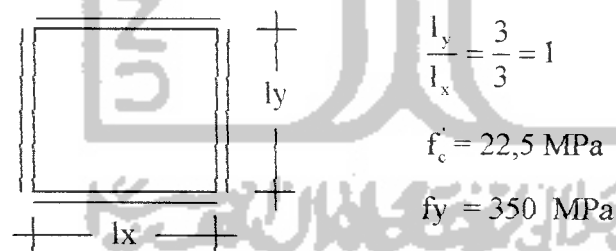
$$W_D = 4,26 \text{ kN/m}$$

- Beban hidup ruang kuliah

$$W_L = 2,5 \text{ kN/m} \quad (\text{dari 3.30b})$$

- Beban berfaktor yang bekerja adalah:

$$W_U = 1,2 \times 4,26 + 1,6 \times 2,5 = 9,112 \text{ kN/m} \quad (\text{dari 3.30c})$$



- Momen-momen yang bekerja

$$M_{l_x} = 0,025 \times 9,112 \times 3^2 = 1,953 \text{ kNm} \quad (\text{dari 3.31a})$$

$$M_{l_y} = 0,025 \times 9,112 \times 3^2 = 1,953 \text{ kNm} \quad (\text{dari 3.31b})$$

$$M_{l_x} = -0,051 \times 9,112 \times 3^2 = -3,9841 \text{ kNm} \quad (\text{dari 3.31c})$$

$$M_{l_y} = -0,051 \times 9,112 \times 3^2 = -3,9841 \text{ kNm} \quad (\text{dari 3.31d})$$

b. Penulangan Plat

- Tebal plat

$$h_{\min} = \frac{1}{24} \times 3000 \left(0,4 + \frac{350}{700} \right) = 112,5 \text{ mm} \quad (\text{dari 3.32})$$

dipakai $h = 120 \text{ mm}$

- Tinggi efektif balok



selimut beton (p_b) = 20 mm

tulangan pokok polos (ϕ_p) = 10 mm

tulangan bagi polos (ϕ_b) = 8 mm

$$d_x = 120 - 20 - \frac{1}{2} 10 = 95 \text{ mm} \quad (\text{dari 3.33a})$$

$$d_y = 120 - 20 - 10 - \frac{1}{2} 10 = 85 \text{ mm} \quad (\text{dari 3.33b})$$

- Rasio penulangan

$$\rho_{\min} = \frac{1,4}{350} = 0,004 \quad (\text{dari 3.34})$$

$$\rho_b = \frac{0,85 \times 22,5}{350} \times 0,85 \left(\frac{600}{600 + 350} \right) = 0,0293 \quad (\text{dari 3.35})$$

$$\rho_{\max} = 0,75 \times 0,2608 = 0,022 \quad (\text{dari 3.36})$$

- Luas tulangan

✧ Tulangan lapangan arah x

$$R_n = \frac{1,953 \times 10^6}{0,8 \times 1000 \times 95^2} = 0,2705 \quad (\text{dari 3.37})$$

$$m = \frac{350}{0,85 \times 22,5} = 18,30 \quad (\text{dari 3.38})$$

Dari persamaan 3.39)

$$\rho_{\text{perlu}} = \frac{1}{18,30} \left(1 - \sqrt{1 - \frac{2 \times 18,30 \times 0,2705}{350}} \right) = 0,00078 < \rho_{\text{min}} = 0,004$$

$$A_{s_{\text{perlu}}} = 0,004 \times 1000 \times 95 = 380 \text{ mm}^2 \quad (\text{dari 3.40})$$

dipakai tulangan : \emptyset_{10-200}

Seperti langkah - langkah perhitungan tulangan lapangan arah x, didapat:

- Tulangan lapangan arah y

$$R_n = \frac{1,953 \times 10^6}{0,8 \times 1000 \times 85^2} = 0,2705$$

$$m = 18,30$$

$$\rho_{\text{perlu}} = 0,00078 < \rho_{\text{min}} = 0,004$$

$$A_{s_{\text{perlu}}} = 0,004 \times 1000 \times 85 = 340 \text{ mm}^2$$

dipakai tulangan : \emptyset_{10-200}

- Tulangan tumpuan arah x

$$R_n = \frac{3,9841 \times 10^6}{0,8 \times 1000 \times 95^2} = 0,5518$$

$$m = 18,30$$

$$\rho_{\text{perlu}} = 0,0016 < \rho_{\text{min}} = 0,004$$

$$A_{s_{\text{perlu}}} = 0,004 \times 1000 \times 95 = 380 \text{ mm}^2$$

dipakai tulangan : \emptyset_{10-200}

- Tulangan tumpuan arah y

$$R_n = \frac{3,9841 \times 10^6}{0,8 \times 1000 \times 85^2} = 0,5518$$

$$m = 18,30$$

$$\rho_{\text{perlu}} = 0,0016 < \rho_{\text{min}} = 0,004$$

$$A_{s_{\text{perlu}}} = 0,004 \times 1000 \times 85 = 340 \text{ mm}^2$$

dipakai tulangan : \emptyset_{10-200}

- Tulangan bagi untuk tumpuan dan lapangan arah x

$$A_{s_{\text{bagi}}} = 0,0014 \times 1000 \times 95 = 168 \text{ mm}^2 \quad (\text{dari 3.41})$$

dipakai tulangan : \emptyset_{8-300}

- Tulangan bagi untuk tumpuan dan lapangan arah y

$$A_{s_{\text{bagi}}} = 0,0014 \times 1000 \times 85 = 119 \text{ mm}^2$$

dipakai tulangan : \emptyset_{8-300}



Tabel 4.7 Penulangan plat

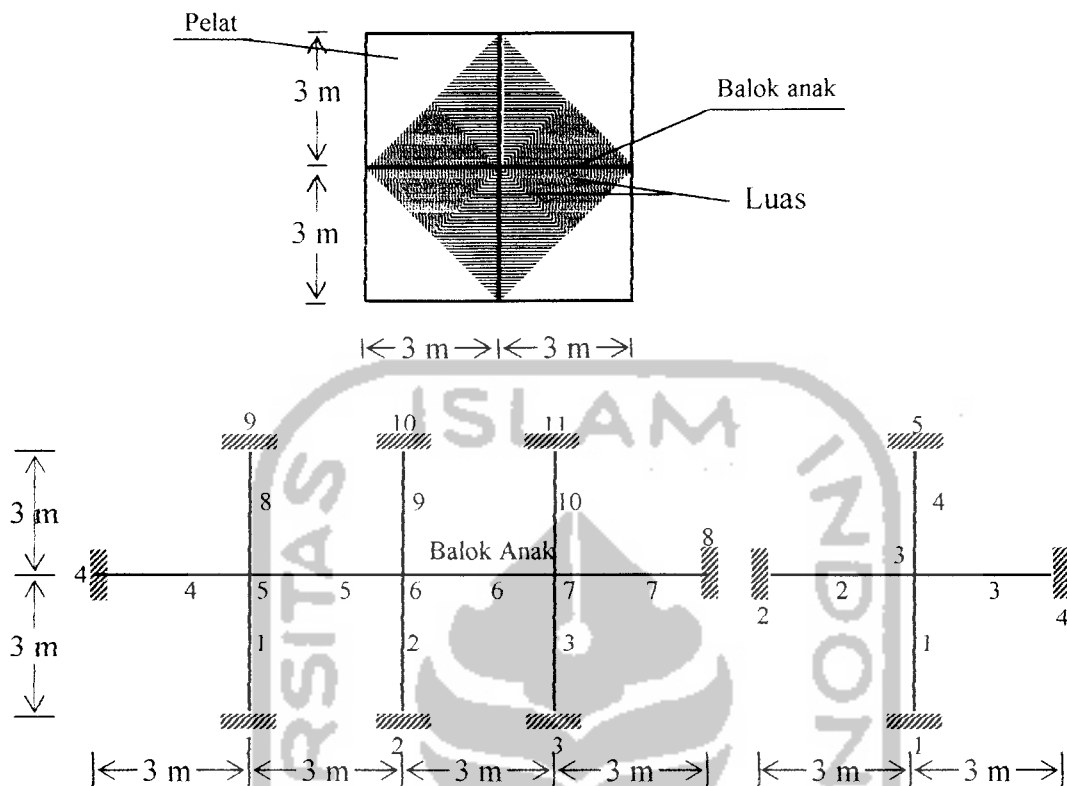
plat	tebal (mm)	tebal efektif (mm)	arah plat	M $\phi.b.d^2$	Rasio tulangan $\rho_{min} = 0,004$		tulangan pokok terpasang		tulangan bagi	
					ρ_{perlu}	ρ_{pasang}	As (mm ²)	Tulangan	As (mm ²)	tulangan pasang
1	120	95	M_{lx}	0,2705	0,0008	0,0066	628	$\phi 10-115$	251,3	$\phi 8-200$
		85	M_{ly}	0,2705	0,0008	-	-	-	251,3	$\phi 8-200$
		95	M_{tx}	0,5578	0,0016	0,0066	628	$\phi 10-115$	251,3	$\phi 8-200$
		85	M_{ty}	0,5578	0,0016	-	-	-	251,3	$\phi 8-200$
2	120	95	M_{lx}	1,0092	0,0029	0,0066	628	$\phi 10-115$	251,3	$\phi 8-200$
		85	M_{ly}	0,5971	0,0017	-	-	-	251,3	$\phi 8-200$
		95	M_{tx}	1,7926	0,0054	0,0066	628	$\phi 10-115$	251,3	$\phi 8-200$
		85	M_{ty}	1,8245	0,0055	-	-	-	251,3	$\phi 8-200$
3	120	95	M_{lx}	1,0092	0,0029	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,5971	0,0017	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	1,7926	0,0054	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,8245	0,0055	0,0055	471	$\phi 10-165$	-	-
4	120	95	M_{lx}	1,0092	0,0029	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,5971	0,0017	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	1,7926	0,0054	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,8245	0,0055	0,0055	471	$\phi 10-165$	-	-
5	120	95	M_{lx}	0,9029	0,0026	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,7298	0,0021	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	1,9121	0,0058	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,7914	0,0054	0,0055	471	$\phi 10-165$	-	-
6	120	95	M_{lx}	1,0092	0,0029	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,5971	0,0017	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	1,7926	0,0054	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,8245	0,0055	0,0055	471	$\phi 10-165$	-	-
7	120	95	M_{lx}	1,5403	0,0046	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,4976	0,0015	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	2,1776	0,0066	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,7250	0,0052	0,0055	471	$\phi 10-165$	-	-

Lanjutan Tabel 4.7 Penulangan plat

plat	tebal (mm)	Tebal Efektif (mm)	arah plat	$\frac{M}{\phi \cdot b \cdot d^2}$	Rasio tulangan $\rho_{min} = 0,004$		tulangan pokok terpasang		tulangan bagi	
					ρ_{perlu}	ρ_{pasang}	As (mm ²)	Tulangan	As (mm ²)	tulangan pasang
8	120	95	M_{lx}	1,0092	0,0029	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,5971	0,0017	0,0055	471	$\phi 10-165$	-	-
		95	M_{tx}	1,7926	0,0054	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	1,8245	0,0055	0,0055	471	$\phi 10-165$	-	-
9	120	95	M_{lx}	0,4112	0,0012	0,0066	628	$\phi 10-115$	-	-
		85	M_{ly}	0,2703	0,0008	0,0040	392,7	$\phi 10-200$	-	-
		95	M_{tx}	0,7358	0,0022	0,0066	628	$\phi 10-115$	-	-
		85	M_{ty}	0,7434	0,0022	0,0040	392,7	$\phi 10-200$	-	-
10	120	95	M_{lx}	0,2705	0,0008	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ly}	0,2705	0,0008	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		95	M_{tx}	0,5578	0,0016	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ty}	0,5578	0,0016	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
11 & 12	120	95	M_{lx}	0,4972	0,0014	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ly}	0,4969	0,0014	0,0040	392,7	$\phi 10-200$	-	-
		95	M_{tx}	0,9447	0,0028	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ty}	1,09722	0,0032	0,0040	392,7	$\phi 10-200$	-	-
13 & 14	120	95	M_{tx}	0,3529	0,0010	0,0040	392,7	$\phi 10-200$	-	-
		85	M_{ly}	0,2255	0,0007	0,0040	392,7	$\phi 10-200$	-	-
		95	M_{tx}	0,5170	0,0015	0,0040	392,7	$\phi 10-200$	-	-
		85	M_{ty}	0,5536	0,0016	0,0040	392,7	$\phi 10-200$	-	-
15	120	95	M_{tx}	0,5088	0,0015	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ly}	0,1440	0,0004	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		95	M_{tx}	0,6811	0,0019	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
		85	M_{ty}	0,5228	0,0015	0,0040	392,7	$\phi 10-200$	168	$\phi 8-300$
16	120	80	M_{lx}	2,367	0,0072	0,0096	769,3	$\phi 10-100$	168	$\phi 8-300$
		75	M_{ly}	0,666	0,0020	0,0068	509	$\phi 10-150$	168	$\phi 8-300$
		80	M_{tx}	3,069	0,0096	0,0096	769,3	$\phi 10-100$	168	$\phi 8-300$
		75	M_{ty}	2,23	0,0068	0,0068	509	$\phi 10-150$	168	$\phi 8-300$

Keterangan: plat 16 adalah plat atap (mangkok)

4.5 Perencanaan Balok Anak



4.5.1 Pembebanan Balok anak

Perhitungan pembebanan seperti perhitungan sebelumnya

- Beban plat

$$W_D = 4,26 \times \frac{4,5}{3} = 6,39 \text{ KN/m}$$

- Berat taksiran balok anak

$$W_{D_0} = 3,0 \text{ KN/m}$$

- Beban mati total

$$W_D = 6,39 + 3 = 9,39 \text{ KN/m}$$

- Beban hidup

$$W_L = 2,5 \times \frac{4,5}{3} = 3,75 \text{ KN/m}$$

Tabel 4.8 Reaksi Akibat Beban Rencana (SAP 90)

EI	L (m)	Akibat beban mati			Akibat beban hidup			1.4.D			1.2.D+1.2.L		
		Torsi (KNm)	Geser (KN)	Momen (KNm)	Torsi (KNm)	Geser (KN)	Momen (KNm)	Torsi (KNm)	Geser (KN)	Momen (KNm)	Torsi (KNm)	Geser (KN)	Momen (KNm)
1	0.0	1,857	35,361	-38,95	0,744	14,159	-15,599	2,598	49,505	-54,5384	3,418	65,0876	-71,705
	3.0	1,857	7,191	24,87	0,744	2,879	9,959	2,600	10,067	34,8194	3,418	13,2356	45,779
2	0.0	0,060	43,157	-50,56	0,023	17,281	-20,282	0,084	60,419	-70,7854	0,108	79,438	-93,124
	3.0	0,061	14,987	36,56	0,024	6,001	14,642	0,085	20,981	51,1924	0,111	27,586	67,306
3	0.0	0	42,861	-50,20	0	17,163	-20,104	0,000	60,005	-70,2884	0	78,894	-92,413
	3.0	0	14,691	36,12	0	5,883	14,464	0,000	20,567	50,5694	0	27,042	66,487
4	0.0	-0,060	43,157	-50,56	-0,023	17,281	-20,282	-0,084	60,419	-70,7854	-0,108	79,438	-93,124
	3.0	-0,060	14,987	36,56	-0,023	6,001	14,642	-0,084	20,981	51,1924	-0,108	27,586	67,306
5	0.0	-1,857	35,361	-38,95	-0,743	14,159	-15,599	-2,600	49,505	-54,5384	-3,417	65,0876	-71,705
	3.0	-1,857	7,191	24,87	-0,743	2,879	9,959	-2,600	10,067	34,8194	-3,417	13,2356	45,779
6	0.0	0	25,464	-29,05	0	10,169	-11,636	0,000	35,649	-40,6826	0	46,8272	-53,488
	3.0	0	-2,706	5,07	0	-1,084	2,033	0,000	-3,7884	7,1092	0	-4,9816	9,3464
7	0.0	0	11,675	1,36	0	4,675	0,547	0,000	16,345	1,9124	0	21,49	2,5144
	3.0	0	-16,495	-5,86	0	-6,605	-2,348	0,000	-23,093	-8,2082	0	-30,362	-10,792
8	0.0	0	13,479	-5,97	0	5,398	-2,394	0,000	18,870	-8,3706	0	24,8116	-11,005
	3.0	0	-14,691	-7,79	0	-5,883	-3,122	0,000	-1000,5	-10,9144	0	-17,9891	-14,350
9	0.0	0	14,691	-7,79	0	5,883	-3,122	0,000	20,567	-10,9144	0	27,042	-14,350
	3.0	0	-13,467	-5,97	0	-5,398	-2,394	0,000	-18,853	-8,3706	0	-24,7972	-11,005
10	0.0	0	16,495	-5,86	0	6,605	-2,348	0,000	23,093	-8,2082	0	30,362	-10,792
	3.0	0	-11,675	1,36	0	-4,675	0,547	0,000	-16,345	1,9124	0	-21,49	2,405
11	0.0	0	2,706	5,07	0	1,084	2,033	0,000	3,7884	7,1092	0	4,9816	9,3464
	3.0	0	-25,464	-29,05	0	-10,19	-11,636	0,000	-35,649	-40,6826	0	-46,8704	-53,488
12	0.0	-1,857	-7,191	24,87	-0,743	-2,879	9,959	-2,600	-10,067	34,8194	-3,417	-13,2356	45,779
	3.0	-1,857	-35,361	-38,95	-0,743	-14,15	-15,599	-2,600	-49,505	-54,5384	-3,417	-65,0876	-71,705
13	0.0	-0,060	-14,987	36,56	-0,023	-6,001	14,642	-0,084	-20,981	51,1924	-0,108	-27,586	67,306
	3.0	-0,060	-43,157	-50,56	-0,023	-17,28	-20,282	-0,084	-60,419	-70,7854	-0,108	-79,438	-93,124
14	0.0	0	-14,691	36,12	0	-5,883	14,464	0,000	-20,567	50,5694	0	-27,042	66,487
	3.0	0	-42,861	-50,20	0	-17,16	-20,104	0,000	-60,005	-70,2884	0	-78,894	-92,413
15	0.0	0,060	-14,987	36,56	0,023	-6,001	14,642	0,084	-20,981	51,1924	0,108	-27,586	67,306
	3.0	0,060	-43,157	-50,56	0,023	-17,28	-20,282	0,084	-60,419	-70,7854	0,108	-79,438	-93,124
16	0.0	1,856	-7,191	24,87	0,743	-2,879	9,959	2,598	-10,067	34,8194	3,416	-13,2356	45,779
	3.0	1,856	-35,361	-38,95	0,743	-14,15	-15,599	2,598	-49,505	-54,5384	3,416	-65,0876	-71,705

4.5.2 Dimensi balok anak

Langkah-langkah perencanaan penulangan balok anak (balok non struktural) sama seperti perencanaan penulangan pelat lantai

$$M_u = 93,1244 \text{ KNm}$$

$$f'_c = 22,5 \text{ MPa}$$

$$f_y = 350 \text{ MPa}$$

$$\rho_{\min} = 0,004$$

$$\rho_b = \frac{0,85 \times 22,5}{350} \times 0,85 \times \left(\frac{600}{600 + 350} \right) = 0,029335$$

$$\rho_{\max} = 0,75 \times 0,029335 = 0,022$$

$$\text{dicoba } \rho = 0,0165$$

$$m = 18,3$$

$$R_n = \rho \cdot f_y (1 - 0,5 \cdot \rho \cdot m) = 0,0165 \times 350 (1 - 0,5 \times 0,0165 \times 18,3) = 4,903$$

$$bd^2 = \frac{93,1244}{0,8 \times 4,903} \times 10^6 = 23741688,76 \text{ mm}^3$$

tabel dimensi balok berdasarkan lebar coba-coba

b (mm)	d (mm)
200	344,5
250	308,2

Diambil nilai $b = 250 \text{ mm}$; $h = 400 \text{ mm}$

- Kontrol berat sendiri balok

$$W = 0,25 \times 0,4 \times 24 = 2,4 \text{ KN/m} < 3 \text{ KN/m}$$

- Dipakai ukuran balok 250/400 mm

$$d' = 40 \text{ mm}^2$$

$$d = 400 - 40 = 360 \text{ mm}$$

$$R_n = \frac{93,1244}{0,8 \times 250 \times 360^2} \times 10^6 = 3,6$$

$$\rho' = \frac{3,6}{4,903} \times 0,0165 = 0,012$$

$$A_s = 0,012 \cdot 250 \cdot 360 = 1090,4 \text{ mm}^2$$

- Kapasitas balok

$$M_n = \frac{93,1244}{0,8} = 116,406 \text{ KNm}$$

$$a = \frac{1090,4 \times 350}{0,85 \times 22,5 \times 250} = 80 \text{ mm}$$

$$\begin{aligned} M_n &= 1090,4 \times 350 \left(360 - \frac{80}{2} \right) \times 10^{-6} \\ &= 122,16 \text{ KNm} > M_n = 116,406 \text{ KNm} \end{aligned}$$

dipakai tulangan sebelah $4D_{20} = 1256,6 \text{ mm}^2 > A_s = 1090,4 \text{ mm}^2$.

- kontrol regangan yang terjadi

$$\epsilon_y = \frac{350}{210.000} = 0,00167$$

$$c = \frac{80}{0,85} = 94 \text{ mm}$$

$$\epsilon_s = \frac{360 - 94}{94} (0,003) = 0,0085 > \epsilon_y$$

- kontrol tulangan torsi

$$T = 3,4188 \text{ KNm}$$

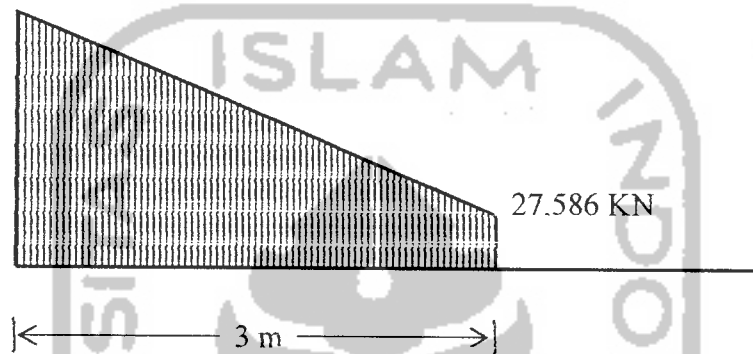
$$\Sigma x^2 y = 250^2 (600) = 37500000 \text{ mm}^3$$

$$\begin{aligned}\phi \frac{1}{24} f_c' \Sigma x^2 y &= 0,6 \frac{1}{24} \times 22,5 \times 37500000 \times 10^{-6} \\ &= 21,09 \text{ KNm} > 3,4188 \text{ KNm}\end{aligned}$$

efek torsi boleh diabaikan

- kontrol tulangan geser

79,438 KN



$$V_u = \frac{3000 - 360}{3000} \times 79,438 = 69,905 \text{ KN}$$

$$V_n = \frac{69,905}{0,60} = 116,509 \text{ KN}$$

$$V_c = \left(\frac{1}{6} \sqrt{22,5} \right) \times 250 \times 360 \times 10^{-3} = 71,151 \text{ KN} < 116,509 \text{ KN}$$

$$\frac{1}{2} V_c = 35,5755 \text{ KN}$$

- Kuat geser tulangan geser

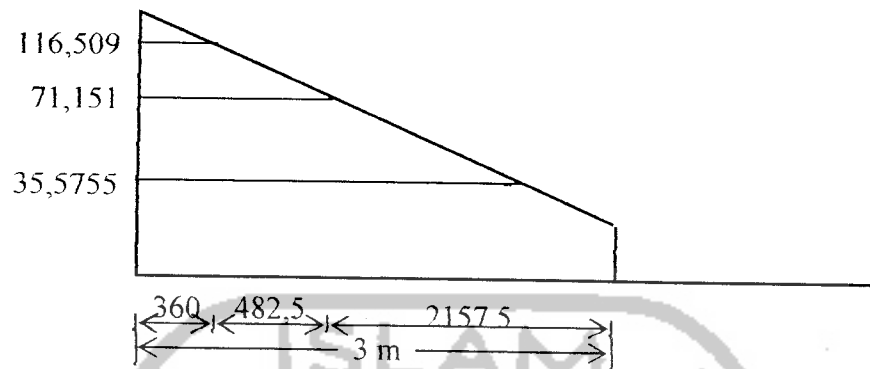
$$V_{s1} = \frac{1}{3} \sqrt{22,5} \times 250 \times 360 \times 10^{-3} = 142,302 \text{ KN}$$

$$V_c + V_{s1} = 71,151 + 142,302 = 213,453 \text{ KN}$$

$$V_c = 71,151 \text{ KN} < V_n = 116,509 \text{ KN} < V_c + V_{s1} = 213,453 \text{ KN}$$

Penampang memerlukan tulangan geser untuk menahan gaya geser yang terjadi.

Dipakai tulangan geser $2(\phi 10) = 157 \text{ mm}^2$



- Untuk semua daerah

Jarak sengkang

$$V_s = 116,509 - 71,151 = 45,358 \text{ KN}$$

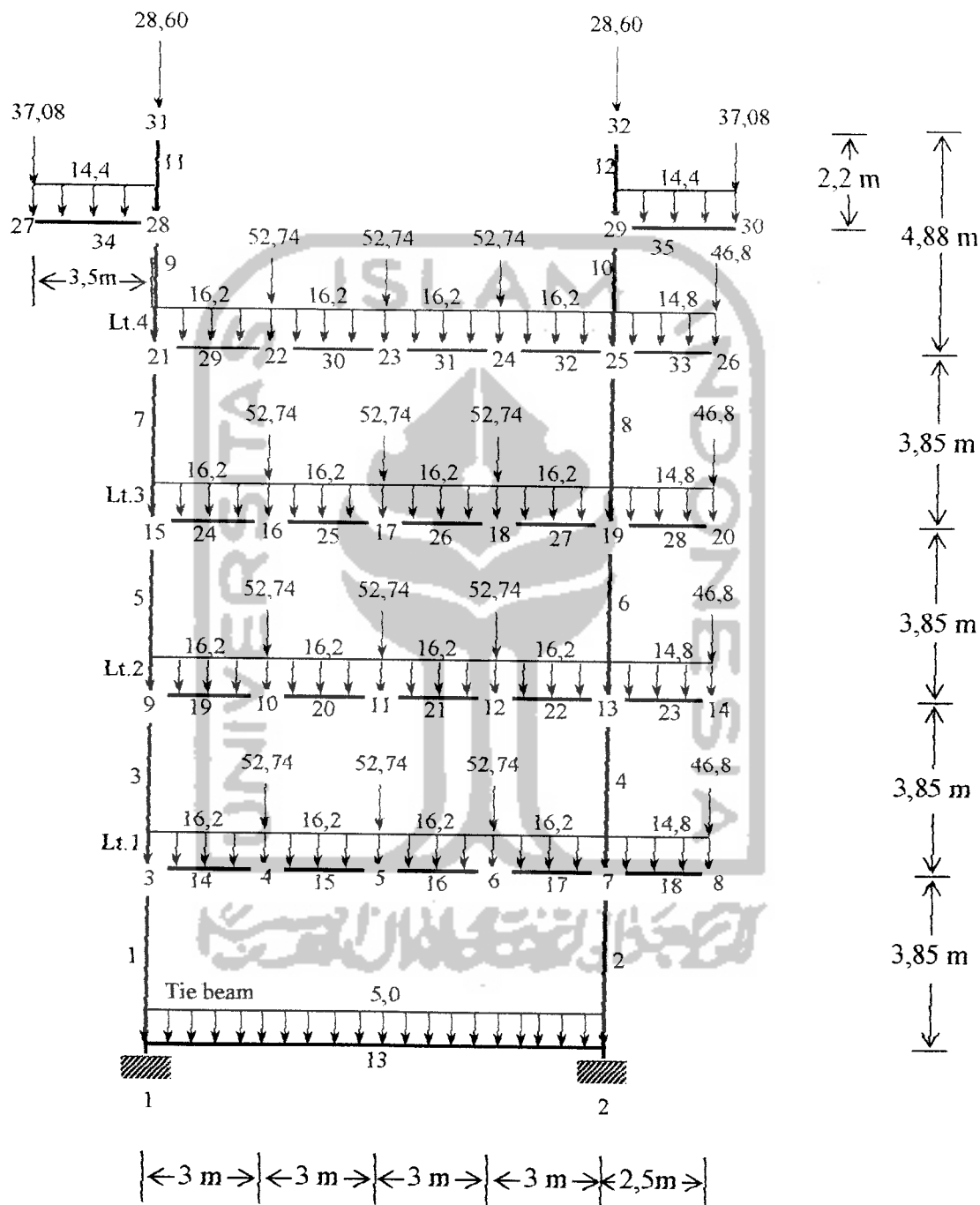
$$s = \frac{157 \times 350 \times 360 \times 10^{-3}}{45,358} = 436 \text{ mm}$$

$$\frac{d}{2} = 180 \text{ mm}$$

dipakai sengkang P10 - 180 mm

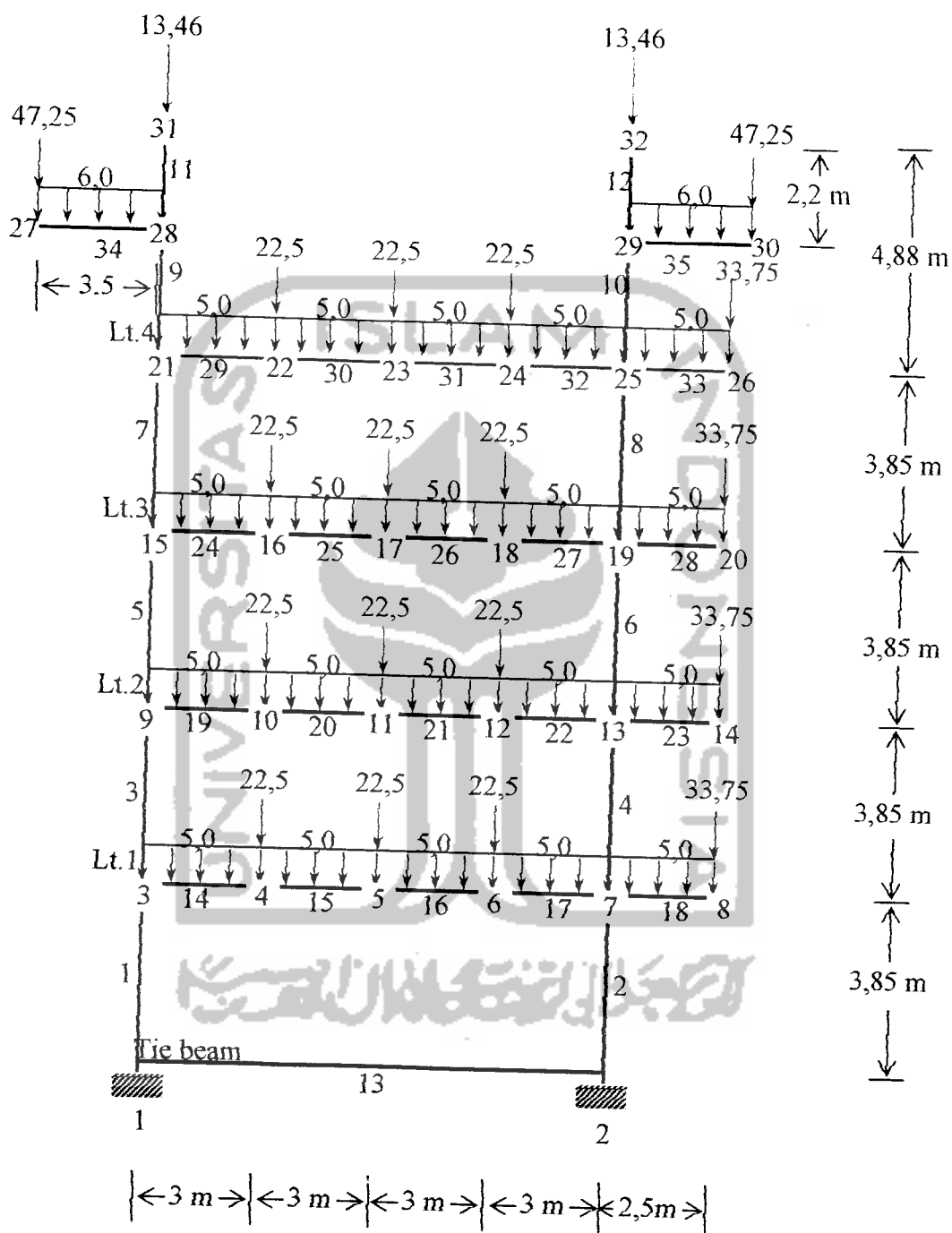
Pembebanan Portal As. 9 dan As 10

a. Beban mati



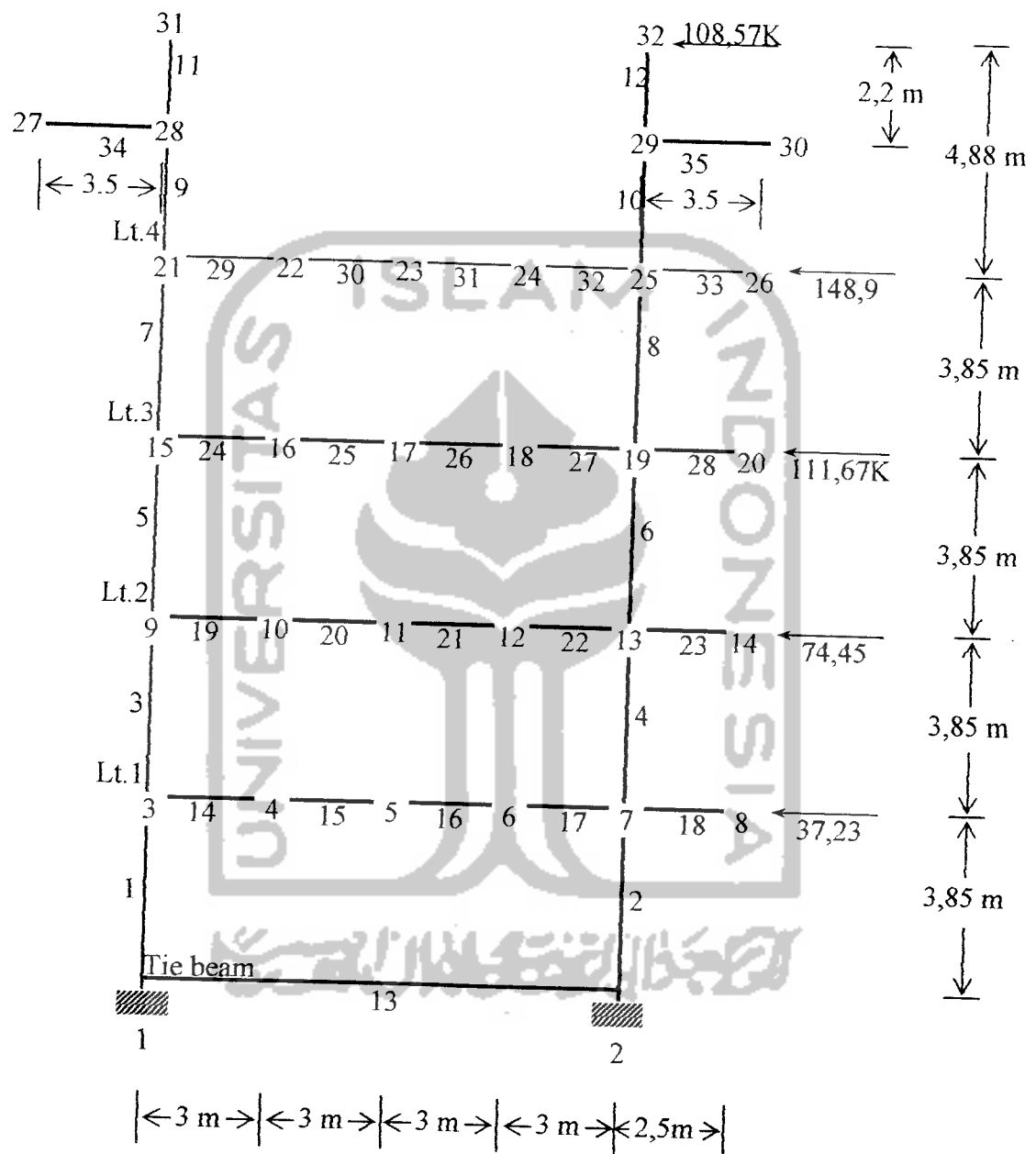
Gambar 4.9.a Pembebanan mati Portal As. 9 dan As 10

b. Beban hidup



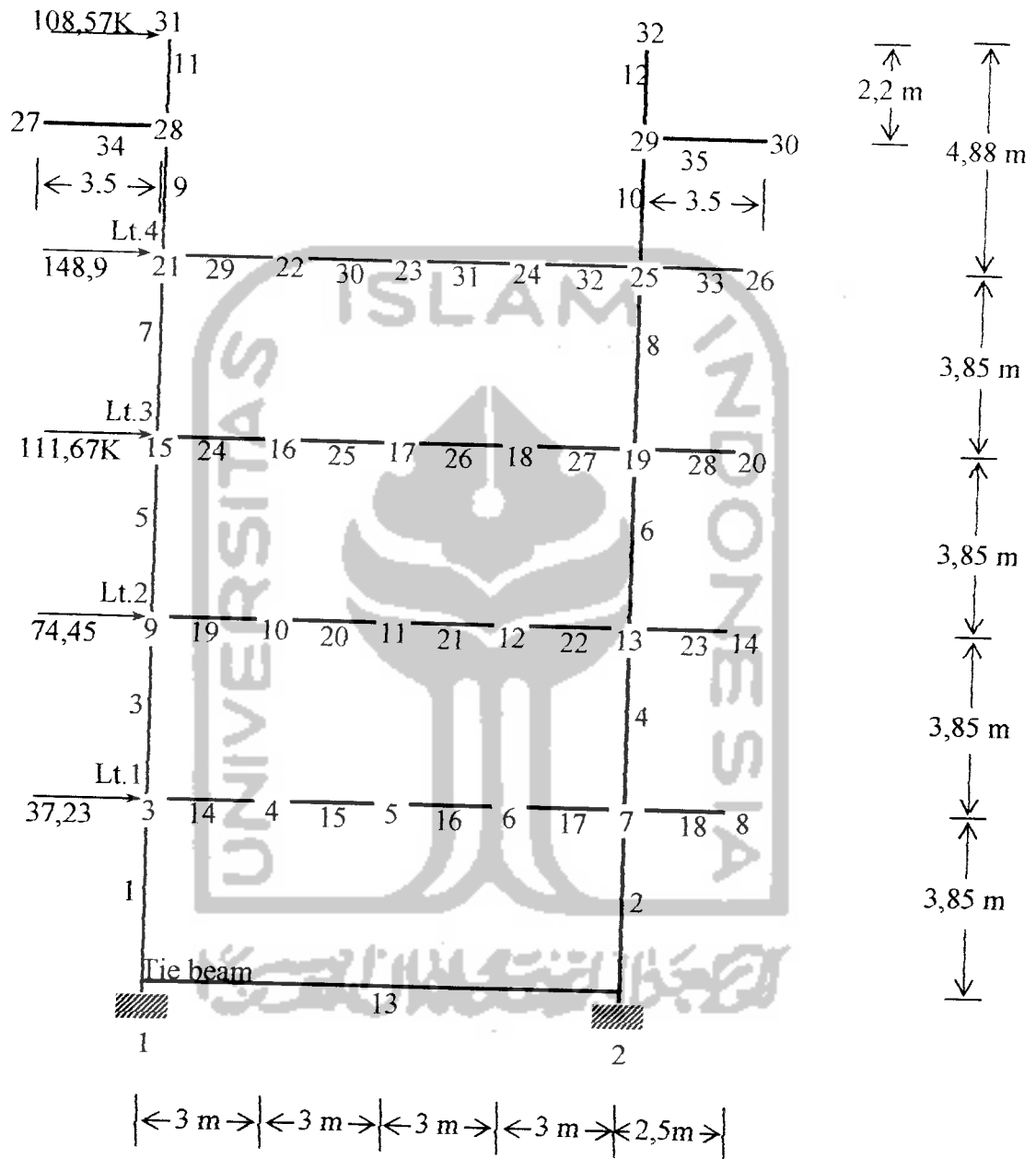
Gambar 4.9.b Pembebanan hidup Portal As. 9 dan As 10

c. Beban gempa kanan



Gambar 4.9.c Pembebanan gempa kanan Portal As. 9 dan As 10

d. Beban gempa kiri



Gambar 4.9.d Pembebanan gempa kiri Portal As. 9 dan As 10

a. Beban mati

- Beban terdistribusi merata elemen 14 s/d 17, 19 s/d 22, 24 s/d 27, 29 s/d 32

$$\text{Lantai : } 2.2/3.1,5.4,26 = 8,52 \text{ KN/m}$$

$$\text{Balok : } 0,4.0,8.24 = \underline{7,68 \text{ KN/m}}$$

$$W_{D1} = 16,2 \text{ KN/m}$$

- Beban terdistribusi merata elemen 18, 23, 28, 33

$$\text{Lantai : } 2.2/3.1,25.4,26 = 7,10 \text{ KN/m}$$

$$\text{Balok : } 0,4.0,8.24 = \underline{7,68 \text{ KN/m}}$$

$$W_{D2} = 14,8 \text{ KN/m}$$

- Beban terdistribusi merata elemen 34 & 35

$$\text{Lantai : } 2.2/3.1,75.0,12.24 = 6,72 \text{ KN/m}$$

$$\text{Balok : } 0,4.0,8.24 = \underline{7,68 \text{ KN/m}}$$

$$W_{D3} = 14,40 \text{ KN/m}$$

- Beban terpusat

- Nodal 4 s/d 6, 10 s/d 12, 16 s/d 18, 22 s/d 24

$$P_{D1} \text{ balok anak : } 2.(2.1/2.3.1,5).4,26 + 0,25.0,4.24.6 = 52,74 \text{ KN}$$

- Nodal 31 & 32

$$P_{D2} \text{ R perletakan atap} = 28,60 \text{ KN}$$

- Nodal 27 & 30

$$P_{D3} \text{ penutup Lt 4 : } 1,75.(1,5 + 3).0,12.24 + 0,25.0,4.24.6 = 37,08 \text{ KN}$$

- Nodal 8, 14, 20, 26

$$P_{D4} \text{ Lt selasar : } 2.1,25.(1,5 + 3).0,12.24 + 0,25.0,4.24.6 = 46,8 \text{ KN}$$

b. Beban hidup

- Beban terdistribusi merata elemen 14 s/d 17, 19 s/d 22, 24 s/d 27, 29 s/d 32

$$W_{L1} = 2.2/3.1,5.2,5 = 5,0 \text{ KN/m}$$

- Beban terdistribusi merata elemen 18, 23, 28, 33

$$W_{L2} = 2.2/3.1,25.3 = 5,0 \text{ KN/m}$$

- Beban terdistribusi merata elemen 34 & 35

$$W_{L3} = 2.2/3.1,5.3 = 6,0 \text{ KN/m}$$

- Beban terpusat

- Nodal 4 s/d 6, 10 s/d 12, 16 s/d 18, 22 s/d 24

$$P_{L1 \text{ R balok anak}} : 2.(2.1/2.3.1,5).2,5 = 22,50 \text{ KN}$$

- Nodal 31 & 32

$$P_{L2 \text{ R perletakan atap}} : 7,166 + 6,304 = 13,46 \text{ KN}$$

- Nodal 27 & 30

$$P_{L3 \text{ penutup Lt 4}} : 2.1,75.(1,5+3).3 = 47,25 \text{ KN}$$

- Nodal 8, 14, 20, 26

$$P_{L4 \text{ Lt selasar}} : 2.1,25.(1,5+3).3 = 33,75 \text{ KN}$$

c. Beban gempa

Reaksi atap : $2 \cdot 28,6 = 57,20 \text{ KN}$

Plat penutup Lt.4 : $2 \cdot 3,5 \cdot 6 \cdot 0,12 \cdot 24 = 120,96 \text{ KN}$

Kolom : $2 \cdot 0,45 \cdot 0,7 \cdot 24 \cdot 4,8 = 72,58 \text{ KN}$

Balok : $2 \cdot 0,4 \cdot 0,8 \cdot 24 \cdot 12 = 184,38 \text{ KN}$

Balok : $2 \cdot 0,25 \cdot 0,4 \cdot 24 \cdot 6 = 46,08 \text{ KN}$

Beban hidup tereduksi: $0,6 \cdot (2 \cdot 3,5 \cdot 6 \cdot 3) = 75,60 \text{ KN}$

Dibulatkan $W_{\text{atap}} = 556,80 \text{ KN}$

- Berat lantai 4 (tipikal dengan lt. 3, 2, 1)

Lantai : $6 \cdot 15 \cdot 4,26 = 383,40 \text{ KN}$

Kolom : $2 \cdot 0,45 \cdot 0,7 \cdot 3,85 \cdot 24 = 58,21 \text{ KN}$

Balok : $2 \cdot 0,4 \cdot 0,8 \cdot 24 \cdot 12 = 184,32 \text{ KN}$

Balok : $4 \cdot 0,25 \cdot 0,4 \cdot 24 \cdot 6 + 2 \cdot 0,25 \cdot 0,4 \cdot 24 \cdot 2,5 = 111,36 \text{ KN}$

Balok : $2 \cdot 0,25 \cdot 0,4 \cdot 24 \cdot 12 = 92,16 \text{ KN}$

Tembok : $2,5 \cdot 3,85 \cdot 12 = 115,50 \text{ KN}$

B.hidup tereduksi : $0,6(6 \cdot 12 \cdot 2,5 + 6 \cdot 2,5 \cdot 3) = 60,60 \text{ KN}$

Dibulatkan $W_4 = 1005,6 \text{ KN}$

$W_{\text{Total}} : 556,8 + 4 \cdot 1005,6 = 4579,2 \text{ KN}$

Gaya geser untuk tingkat daktilitas penuh ($K=1$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.1,0.4579,2 = 480,82 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap	556,8	20,28	11291,91	108,57
4	1005,6	15,40	15486,24	148,90
3	1005,6	11,55	11614,68	111,67
2	1005,6	7,70	7743,12	74,45
1	1005,6	3,85	3871,56	37,23
			50007,51	480,82

Gaya geser untuk tingkat daktilitas penuh ($K=2$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.2,0.4579,2 = 961,64 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap	556,8	20,28	11291,91	217,14
4	1005,6	15,40	15486,24	297,80
3	1005,6	11,55	11614,68	223,34
2	1005,6	7,70	7743,12	148,90
1	1005,6	3,85	3871,56	74,46
			50007,51	961,64

4.6 Perencanaan Penulangan Portal As 9 dan As 10 Daktilitas Penuh (K=1)

4.6.1 Penulangan lentur dan geser balok

tabel momen rencana balok didapat:

$$M_{tumpuan}^- = 1196,13 \text{ KNm}$$

$$M_{tumpuan}^+ = 397,08 \text{ KNm}$$

$$M_{lapangan} = 506,15 \text{ KNm}$$

a. Pemeriksaan rasio tulangan apakah boleh dilakukan redistribusi momen

$$\rho_b = \frac{0,85 \cdot 22,5}{350} \cdot 0,85 \left(\frac{600}{600 + 350} \right) = 0,0029 \quad (\text{dari 3.3})$$

$$R_n = \frac{1196,13 \cdot 10^6}{0,8 \cdot 400 \cdot 750^2} = 5,316 \quad (\text{dari 3.37})$$

$$\rho = \frac{1}{18,3} \left(1 - \sqrt{1 - \frac{2 \cdot 5,316 \cdot 18,3}{350}} \right) = 0,018 \quad (\text{dari 3.39})$$

$$R_n = \frac{397,08 \cdot 10^6}{0,8 \cdot 400 \cdot 750^2} = 1,76$$

$$\rho' = \frac{1}{18,3} \left(1 - \sqrt{1 - \frac{2 \cdot 1,76 \cdot 18,3}{350}} \right) = 0,0053$$

$$0,018 - 0,0053 = 0,0127 < 0,5 \cdot 0,029 = 0,0145$$

maka redistribusi momen negatif boleh dilakukan

- faktor redistribusi maksimum

$$30 \cdot \left(1 - \frac{4 \cdot 0,018 - 0,0053}{0,029} \right) \% = 12 \% \quad (\text{dari 3.3})$$

- Redistribusi momen negatif pada pertemuan kolom tepi

$$\partial M = 1196,13 \cdot 12\% = 143,06 \text{ KNm}$$

- Momen balok rencana terredistribusi

$$M_{tump}^- = 1196,13 - 143,06 = 1049,07 \text{ KNm}$$

$$M_{tump}^+ = 397,08 + 143,06 = 540,14 \text{ KNm}$$

$$M_{lap} = 506,15 + 143,06 = 649,21 \text{ KNm}$$

- Perencanaan tulangan tumpuan negatif

$$f'_c = 22,5 \text{ MPa}$$

$$f_y = 350 \text{ MPa}$$

$$d' = 50 \text{ mm}$$

$$d = 800 - 50 = 750 \text{ mm}$$

$$m = 18,3$$

$$\rho_{min} = 0,004$$

$$\rho_{max} = 0,022$$

$$R_n = \frac{1049,07 \cdot 10^6}{0,8 \cdot 400 \cdot 750^2} = 4,66$$

$$\rho_{perlu} = \frac{1}{18,3} \left(1 - \sqrt{1 - \frac{2 \cdot 4,66 \cdot 18,3}{350}} \right) = 0,016$$

$$A_s = 0,016 \cdot 400 \cdot 750 = 4721,90 \text{ mm}^2 \quad (\text{dari 3.40a})$$

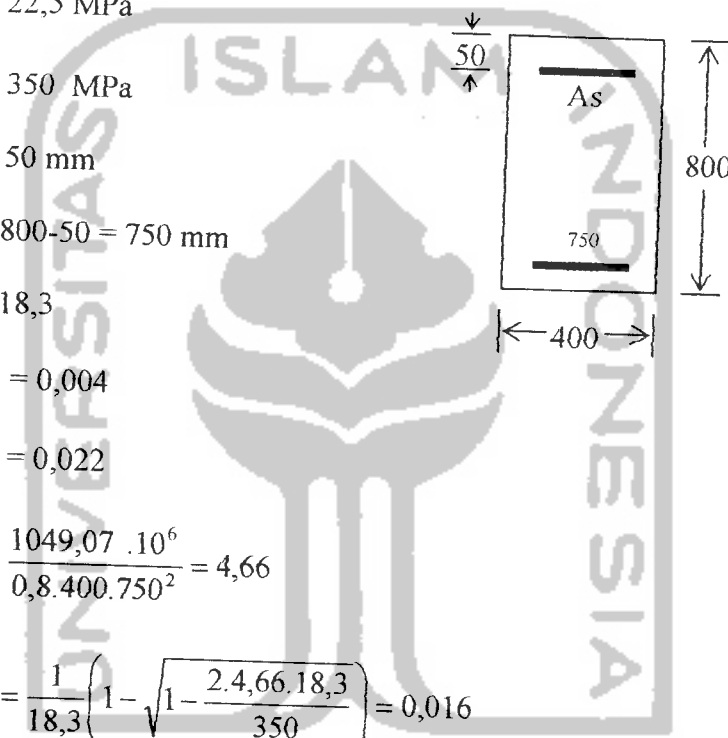
$$A_s' = 0,5 \cdot A_s = 0,5 \cdot 4721,90 = 2360,95 \text{ mm}^2 \quad (\text{dari 3.40b})$$

- Perencanaan tulangan tumpuan positif

$$R_n = 2,40$$

$$\rho_{perlu} = \frac{1}{18,3} \left(1 - \sqrt{1 - \frac{2 \cdot 2,40 \cdot 18,3}{350}} \right) = 0,0074$$

$$A_s' = 0,0074 \cdot 400 \cdot 750 = 2206,12 \text{ mm}^2$$



Tulangan tumpuan terpasang berdasarkan luasan maksimum

$$A_s = 0,016.400.750 = 4721,90 \text{ mm}^2$$

$$\text{Dipakai tulangan } 10D25 = 4909,0 \text{ mm}^2$$

$$A_s' = 0,5.A_s = 0,5. 4721,90 = 2360,95 \text{ mm}^2$$

$$\text{Dipakai tulangan } 5D25 = 2454,5 \text{ mm}^2$$

- Perencanaan tulangan lapangan

$$R_n = 2,88$$

$$\rho_{\text{perlu}} = 0,0089$$

$$A_s = 0,008.400.750 = 2694,65 \text{ mm}^2$$

$$\text{Dipakai tulangan } 6D25 = 2945,4 \text{ mm}^2$$

b. Momen kapasitas balok berdasarkan tulangan tarik terpasang (akibat gempa kiri)

$$C_c = 0,85.22,5.400.a = 7650.a \text{ N} \quad (\text{dari 3.59c})$$

$$C_s = 2454,5.(350 - 0,85.22,5) = 812132,69 \text{ N} \quad (\text{dari 3. 59d})$$

$$T = 4909,0.350 = 1718150 \text{ N} \quad (\text{dari 3. 59e})$$

$$a = \frac{1718150 - 812132,69}{7650} = 118 \text{ mm} \quad (\text{dari 3. 59g})$$

$$C_c = 7650.118 = 906017,31 \text{ N}$$

dari persamaan 3. 59h, didapat nilai:

$$\begin{aligned} M_{n,ak,b} &= \left[906017,31 \left(750 - \frac{142}{2} \right) + 812132,69.(750 - 50) \right] \cdot 10^{-6} \\ &= 626,06 + 568,49 = 1194,55 \text{ KNm} \end{aligned}$$

$$M_{\text{kap},b} = 1,25.1194,55 = 1492,96 \text{ KNm} \quad (\text{dari 3. 59a})$$

- c. Momen kapasitas balok berdasarkan tulangan tekan terpasang (gempa kanan)

Perencanaan menggunakan anggapan bahwa tulangan tarik dan tulangan tekan berjumlah sama yaitu berdasarkan tulangan tekan terpasang momen positif akibat gempa kanan (gambar 4.6.1.c), sehingga didapat:

$$C_c = 0,85 \cdot 22,5 \cdot 400 \cdot a = 7650 \cdot a \quad \text{N}$$

$$C_s = 2454,5 \cdot (350 - 0,85 \cdot 22,5) = 812132,69 \quad \text{N}$$

$$T = 2454,5 \cdot 350 = 859075 \quad \text{N}$$

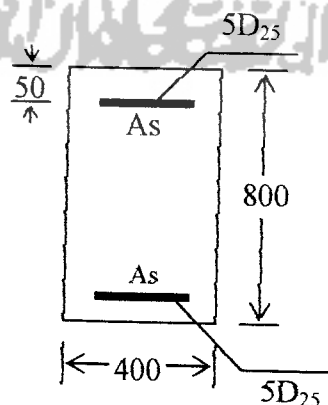
$$a = \frac{859075 - 812132,69}{7650} = 6,2 \quad \text{mm}$$

$$C_c = 7650 \cdot 6,2 = 46942,31 \quad \text{N}$$

dari persamaan 3. 59h, didapat nilai:

$$\begin{aligned} M_{n,ak,b} &= \left[46942,31 \cdot \left(750 - \frac{6,2}{2} \right) + 812132,69 \cdot (750 - 50) \right] \cdot 10^{-6} \\ &= 35,06 + 568,49 = 603,55 \quad \text{KNm} \end{aligned}$$

$$M'_{kap,b} = 1,25 \cdot 603,55 = 754,44 \quad \text{KNm}$$



Gambar 4.10 Anggapan penampang balok tulangan tekan terpasang

d. Tulangan geser balok

$$V_D = 188,58 \text{ KN}$$

$$V_L = 71,30 \text{ KN}$$

$$V_E = 112,54 \text{ KN}$$

$$V_g = 188,58 + 71,30 = 259,88 \text{ KN}$$

$$l_n = 12 - 0,70 = 11,3 \text{ m}$$

Gaya geser rencana:

$$V_{u,b} = 0,7 \left(\frac{192,96 + 754,44}{11,3} \right) + 1,05 \cdot 229,92 = 425,74 \text{ KN} \quad (\text{dari 3.60a})$$

dari persamaan (dari 3.60) didapat gaya geser terpakai:

$$\begin{aligned} V_{u,b} &= \left[229,92 - 0,7 \left(\frac{1492,96 + 754,44}{11,3} \right) \right] \\ &\quad + \frac{11,3 - 0,7}{11,3} \left[425,74 - \left(229,92 - 0,7 \left(\frac{1492,96 + 754,44}{11,3} \right) \right) \right] \\ &= 407,64 \text{ KN} \end{aligned}$$

tidak perlu lebih dari

$$V_{u,b} = 1,05 \left(189,73 + 72,00 + \frac{4,0}{1} \cdot 91,71 \right) = 626,59 \text{ KN} \quad (\text{dari 3.60b})$$

- Pada daerah sendi plastis ($V_c = 0$), ($x = 2.750 = 1500 \text{ mm}$)

$$V_s = 407,64 / 0,6 = 679,4 \text{ KN} \quad (\text{dari 3.60m})$$

Dipakai sengkang 3D10 = 235,5 mm²

$$s = \frac{235,5 \cdot 350 \cdot 750 \cdot 10^{-3}}{679,4} = 90 \text{ mm} \quad (\text{dari 3.60j})$$

$$s \leq \frac{750}{4} = 187,5 \text{ mm} \quad (\text{dari 3.60n})$$

$$s \leq 8.25 = 200 \text{ mm}$$

(dari 3.60n)

$$s \leq 24.10 = 240 \text{ mm}$$

$$s \leq \frac{1600.350.157}{490,9.350} = 500 \text{ mm}$$

- Diluar daerah sendi plastis

$$V_u = \frac{6-1,5}{6} (396,79 - 184,20) + 184,20 = 343,64 \text{ KN}$$

$$V_c = .1/6 \cdot \sqrt{22,5 \cdot 400 \cdot 750 \cdot 10^{-3}} = 237,17 \text{ KN}$$

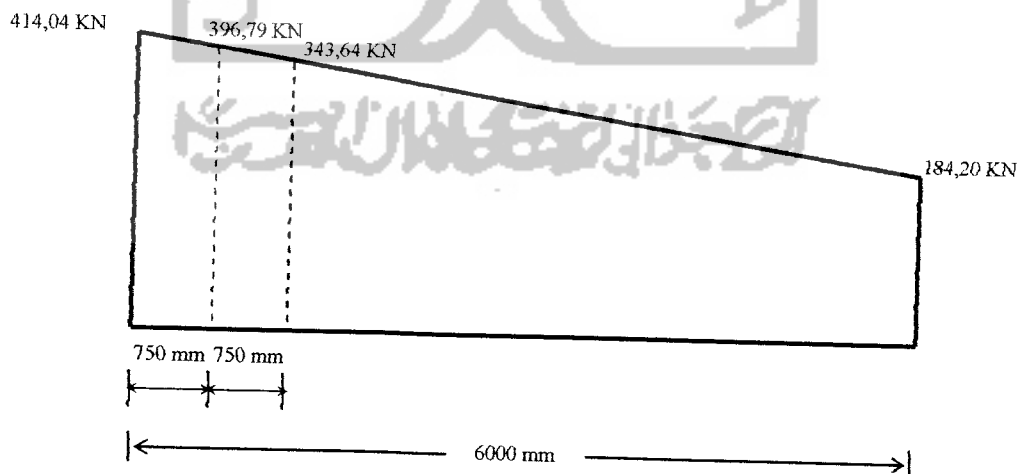
(dari 3.60e)

$$V_s = 343,64 / 0,6 - 237,17 = 335,57 \text{ KN}$$

$$s \leq \frac{235,5 \cdot 350 \cdot 750 \cdot 10^{-3}}{335,57} = 150 \text{ mm}$$

$$s \leq \frac{750}{2} = 187,5 \text{ mm}$$

$$s \leq 300 \text{ mm}$$



4.6.2 Penulangan kolom akibat beban lentur dan aksial

$$I_b = 1/12 \cdot 0,40 \cdot 0,8^3 = 0,0171 \text{ m}^4$$

$$I_k = 1/12 \cdot 0,45 \cdot 0,7^3 = 0,0141 \text{ m}^4$$

$$K_{k,b} = \frac{E \cdot I_k}{h_k} = \frac{E \cdot 0,0141}{3,85} = 0,003662 \cdot E \quad (\text{dari 3.61c})$$

$$K_{k,a} = \frac{E \cdot I_k}{h_k} = \frac{E \cdot 0,0141}{3,85} = 0,003662 \cdot E$$

$$\Sigma K = K_{k,a} + K_{k,b} = 0,003662 \cdot E + 0,003662 \cdot E = 0,007324 \cdot E$$

$$\alpha_{ka} = \frac{K_k}{\Sigma K} = \frac{0,003662 \cdot E}{0,007324 \cdot E} = 0,5 \quad (\text{dari 3.61d})$$

a. Momen rencana kolom (dari 3.61b)

$$M_{u,k} = \frac{3,85}{3,05} \cdot 0,7 \cdot 1,3 \cdot 0,5 \cdot \left[\left(\frac{12}{11,3} \cdot 1492,96 + 0 \right) + 0,3 \left(\frac{6}{5,6} \cdot 310,82 + \frac{6}{5,6} \cdot 310,82 \right) \right]$$

$$= 910,59 \text{ KNm}$$

tidak perlu lebih dari (pers. 3.62)

$$M_{u,k} = 1,05 \cdot (165,76 + 78,08 + 4/1 \cdot (346,96 + 0,3 \cdot 185,78)) = 1713,25 \text{ KNm}$$

b. Gaya aksial rencana kolom

$$n = 5 \text{ lantai}$$

$$R_v = 1,1 - 0,025 \cdot 5 = 0,975$$

$$N_D = 662,88 \text{ KN}$$

$$N_L = 318,05 \text{ KN}$$

$$N_g = 193,11 \text{ KN}$$

$$N_g = 662,88 + 318,05 = 980,93 \text{ KN}$$

dari persamaan 3.64a

$$N_{u,k} = 0,7 \cdot 0,975 \left[\left(\frac{910,59}{12} - 0 \right) + 0,3 \left(\frac{6}{5,6} \cdot 310,82 - \frac{6}{5,6} \cdot 310,82 \right) \right] + 1,05 \cdot 980,93$$

$$= 1081,76 \text{ KN}$$

tidak perlu lebih dari 3.64b

$$N_{u,k} = 1,05 \cdot (980,93 + 4,0/1 \cdot (193,11 + 0,3 \cdot 73,38)) = 1841,04 \text{ KN}$$

c. Perencanaan tulangan desak dan lentur kolom

Kontrol kelangsingan kolom

$$r = \sqrt{\frac{1,41 \cdot 10^{10}}{3,15 \cdot 10^3}} = 216,5 \text{ mm} = 0,22 \text{ m} \quad (\text{dari 3.65a})$$

$$\frac{kl}{r} = \frac{1,0 \cdot (3,85 - 0,8)}{0,22} = 14,1 < 22 \rightarrow \text{kolom pendek} \quad (\text{dari 3.65c})$$

Tinjauan terhadap kondisi seimbang untuk luas tulangan 1% A_g

$$A_{st} = 0,01 \cdot 450 \cdot 700 = 3150 \text{ mm}^2 \quad (\text{dari 3.65e})$$

$$A_s = A_s' = 0,5 \cdot 3150 = 1575 \text{ mm}^2$$

$$d = 700 - 50 = 650 \text{ mm}^2 \quad (\text{dari 3.65f})$$

$$c_b = \frac{600 \cdot 650}{600 + 350} = 410 \text{ mm} \quad (\text{dari 3.65g})$$

$$a_b = 0,85 \cdot 410 = 348 \text{ mm} \quad (\text{dari 3.65h})$$

$$f_s' = 200000 \cdot \left[0,003 \cdot \left(\frac{410 - 50}{410} \right) \right] = 526,8 \text{ MPa} > f_y \quad (\text{dari 3.65i})$$

maka dianggap $f_s' = f_y$

$$f_s = 200000 \cdot \left[0,003 \cdot \left(\frac{650 - 410}{410} \right) \right] = 351,2 \text{ MPa} > f_y \quad (\text{dari 3.65j})$$

maka dianggap $f_s = f_y$

$$C_{cb} = 0,85 \cdot 22,5 \cdot 348 \cdot 450 \cdot 10^{-3} = 2994,975 \text{ KN} \quad (\text{dari 3.65k})$$

$$C_{sb} = 1575 \cdot (350 - 0,85 \cdot 22,5) \cdot 10^{-3} = 521,128 \text{ KN} \quad (\text{dari 3.65l})$$

$$T_{sb} = 1575 \cdot 350 \cdot 10^{-3} = 551,25 \text{ KN} \quad (\text{dari 3.65m})$$

$$P_{nb} = (2994,975 + 521,128 - 551,25) = 2964,85 \text{ KN}$$

$$\phi P_{nb} = 0,65 \cdot 2964,85 = 1927,15 \text{ KN} \quad (\text{dari 3.65n})$$

$$M_{nb} = \left[2994,975 \left(\frac{700 - 348}{2} \right) + 521,128(350 - 50) + 551,25(650 - 350) \right] 10^{-3}$$

$$= 848,83 \text{ KNm}$$

$$\phi M_{nb} = 0,65 \cdot 848,83 = 551,74 \text{ KNm} \quad (\text{dari 3.65o})$$

$$e_b = \frac{551,74}{1927,15} \cdot 10^3 = 286 \text{ mm} \quad (\text{dari 3.65p})$$

Dengan cara yang sama ditinjau kondisi patah desak

$$\text{Diambil } c > c_b \rightarrow c = 500 \text{ mm}$$

$$a = 0,85 \cdot 500 = 425 \text{ mm}$$

$$f'_s = 200000 \cdot \left[0,003 \cdot \left(\frac{500 - 50}{500} \right) \right] = 529 \text{ MPa} > f_y$$

maka dianggap $f'_s = f_y$

$$f'_s = 200000 \cdot \left[0,003 \cdot \left(\frac{650 - 500}{500} \right) \right] = 240 \text{ MPa} < f_y$$

maka digunakan f_s

$$C_c = 0,85 \cdot 22,5 \cdot 425 \cdot 450 \cdot 10^{-3} = 3657,66 \text{ KN}$$

$$C_s = 1575.(350 - 0,85.22,5)10^{-3} = 521,128 \text{ KN}$$

$$T_s = 1575.240.10^{-3} = 378 \text{ KN}$$

$$P_n = 3657,66 + 521,128 - 378 = 4556,79 \text{ KN}$$

$$\phi P_n = 0,65.4556,79 = 2961,91 \text{ KN}$$

$$M_n = \left[3657,66 \cdot \left(\frac{700 - 425}{2} \right) + 521,128.(350 - 50) + 378.(650 - 350) \right] 10^{-3}$$

$$= 772,67 \text{ KNm}$$

$$\phi M_n = 0,65.772,67 = 502,24 \text{ KNm}$$

$$e = \frac{502,24}{2961,91} \cdot 10^3 = 170 \text{ mm}$$

Dengan cara yang sama ditinjau kondisi patah tarik

Diambil $c < c_b \rightarrow c = 300 \text{ mm}$

$$a = 0,85.300 = 255 \text{ mm}$$

$$f'_s = 200000 \cdot \left[0,003 \cdot \left(\frac{300 - 50}{300} \right) \right] = 500 \text{ MPa} > f_y$$

maka dianggap $f'_s = f_y$

$$f'_s = 200000 \cdot \left[0,003 \cdot \left(\frac{650 - 300}{300} \right) \right] = 800 \text{ MPa} > f_y$$

maka dianggap $f'_s = f_y$

$$C_c = 0,85.22,5.255.450.10^{-3} = 2194,59 \text{ KN}$$

$$C_s = 1575.(350 - 0,85.22,5)10^{-3} = 521,128 \text{ KN}$$

$$T_s = 1575.350.10^{-3} = 551,25 \text{ KN}$$

$$P_n = 2194,59 + 521,128 - 551,25 = 2164,47 \text{ KN}$$

$$\phi P_n = 0,65 \cdot 2164,47 = 1406,91 \text{ KN}$$

$$M_n = \left[2194,59 \left(\frac{700 - 255}{2} \right) + 521,128 \cdot (350 - 50) + 551,25 \cdot (650 - 350) \right] \cdot 10^{-3}$$

$$= 810,01 \text{ KNm}$$

$$\phi M_n = 0,65 \cdot 810,01 = 526,51 \text{ KNm}$$

$$e = \frac{526,51}{1406,91} \cdot 10^3 = 374 \text{ mm}$$

Keruntuhan akibat beban sentris (aksial murni)

$$P_n = \{0,85 \cdot 22,5 \cdot (315000 - 3150) + 3150 \cdot 350\} \cdot 10^{-3}$$

$$= 7066,63 \text{ KN}$$

(dari 3.65q)

$$\phi P_n = 0,65 \cdot 7066,63 = 4593,31 \text{ KN}$$

(dari 3.65r)

Keruntuhan tanpa beban aksial (lentur murni)

Anggap tulangan baja desak belum luluh, keseimbangan gaya menjadi:

$$0,85 \cdot 22,5 \cdot 0,85_1 \cdot c^2 \cdot 400 + (600 \cdot 1575 - 1575 \cdot 350) \cdot c - 600 \cdot 1575 \cdot 50 = 0$$

$$6502,5c^2 + 393750c - 47250000 = 0$$

(dari 3.65s)

$$\text{didapat: } c = 120 \text{ mm, } a = 0,85 \cdot 120 = 96 \text{ mm}$$

$$M_n = [0,85 \cdot 22,5 \cdot 96 \cdot 400 \cdot (650 - 96/2) + 1575 \cdot 350 \cdot (650 - 50)] \cdot 10^{-6}$$

$$= 830,93 \text{ KNm}$$

$$\phi M_n = 0,8 \cdot 830,93 = 664,75 \text{ KN}$$

(dari 3.65u)

Keruntuhan akibat beban sentris (aksial murni)

$$P_n = \{0,85 \cdot 22,5 \cdot (315000 - 3150) + 3150 \cdot 350\} \cdot 10^{-3}$$

$$= 7066,63 \text{ KN} \quad (\text{dari 3.65q})$$

$$\phi P_n = 0,65 \cdot 7066,63 = 4593,31 \text{ KN} \quad (\text{dari 3.65r})$$

Keruntuhan tanpa beban aksial (lentur murni)

Anggap tulangan baja desak belum luluh, keseimbangan gaya menjadi:

$$0,85 \cdot 22,5 \cdot 0,85 \cdot c^2 \cdot 400 + (600 \cdot 1575 - 1575 \cdot 350)c - 600 \cdot 1575 \cdot 50 = 0$$

$$6502,5c^2 + 393750c - 47250000 = 0 \quad (\text{dari 3.65s})$$

didapat: $c = 120 \text{ mm}$, $a = 0,85 \cdot 120 = 96 \text{ mm}$

$$M_n = [0,85 \cdot 22,5 \cdot 96 \cdot 400 \cdot (650 - 96/2) + 1575 \cdot 350 \cdot (650 - 50)] \cdot 10^{-6}$$

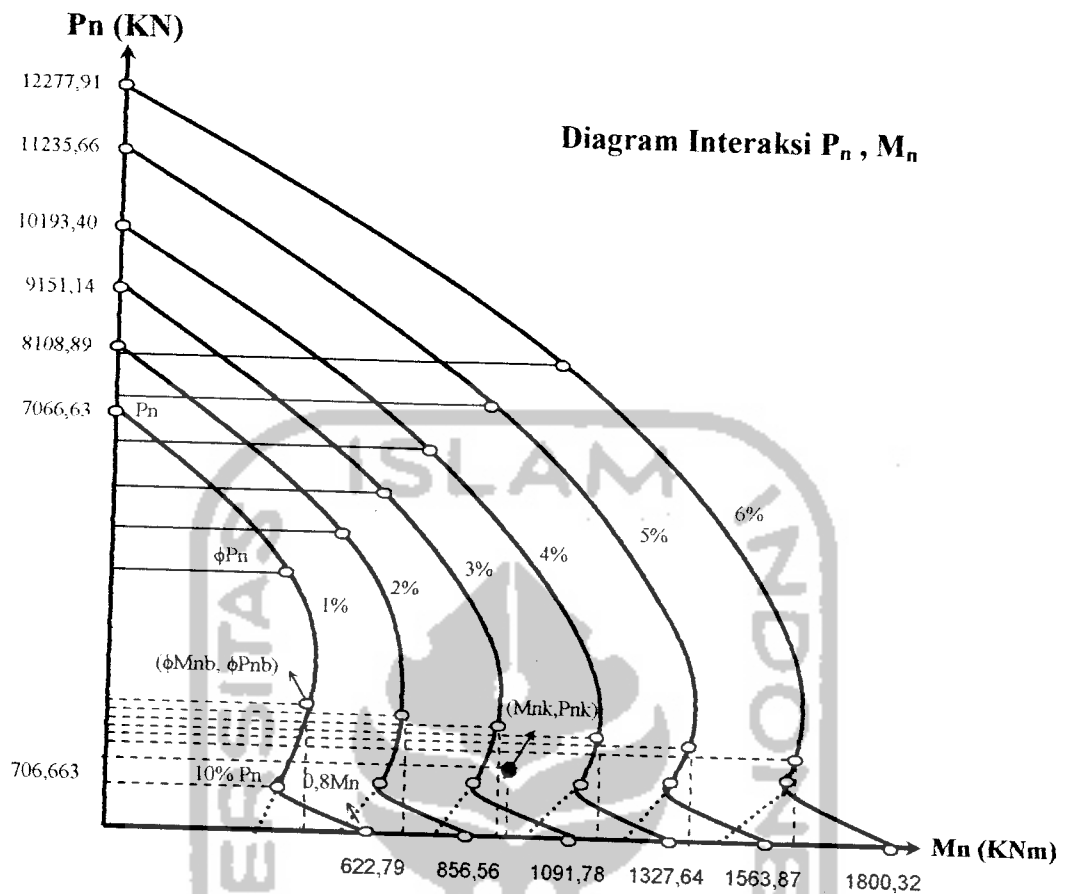
$$= 830,93 \text{ KNm}$$

$$\phi M_n = 0,8 \cdot 830,93 = 664,75 \text{ KN} \quad (\text{dari 3.65u})$$

Dengan cara yang sama didapat diagram interaksi kolom 450/700 untuk luas tulangan 2%, 3%, 4%, 5%, 6%

Tabel 4.9 Interaksi $P_n - M_n$ (kolom 450/700)

Luas tulangan	1%	2%	3%	4%	5%	6%
Ast	3150	6300	9450	12600	15750	18900
As = As'	1575	3150	4725	6300	7875	9450
0.65.P _{nb}	1927.15	1907.58	1888.00	1868.42	1848.84	1829.26
0.65.M _{nb}	551.74	760.85	969.97	1179.08	1388.19	1597.31
P _n	7066.63	8108.89	9151.14	10193.40	11235.66	12277.91
10%P _n	706.66	810.89	915.11	1019.34	1123.57	1227.79
0.65P _n	4593.31	5270.78	5948.24	6625.71	7303.18	7980.64
0.65M _n	506.02	695.96	887.07	1078.71	1270.64	1462.76
0.8M _n	622.79	856.56	1091.78	1327.64	1563.87	1800.32



Dari analisis momen dan aksial rencana kolom, didapat:

$$M_{u,k} = 910,59 \text{ KN}$$

$$N_{u,k} = 1081,76 \text{ KN}$$

$$e = \frac{910,59}{1081,76} \cdot 10^{-3} = 842 \text{ mm} > e_b = 286 \text{ mm} \text{ (patah tarik)}$$

Rasio tulangan menurut diagram interaksi kolom, dilakukan interpolasi:

$$\rho = \frac{(0,04 - 0,03) \cdot (910,59 - 887,07)}{(1078,71 - 887,07)} + 0,03 = 0,031$$

$$A_{st} = 3,1\% \cdot A_g = 3,1\% \cdot 450 \cdot 700 = 9836 \text{ mm}^2$$

$$\text{Dipakai tulangan } 22D25 = 10799,8 \text{ mm}^2$$

$$A_s = 11.409,9 = 5399,9 \text{ mm}^2$$

$$A_s' = 11.409,9 = 5399,9 \text{ mm}^2$$

Kontrol kapasitas kolom terhadap keruntuhan tarik

$$m = 18,3$$

$$\left(1 - \frac{e'}{d}\right) = \frac{700 - 2.842}{2.650} = -0,757 \quad (\text{dari 3.65w})$$

$$\left(1 - \frac{d'}{d}\right) = \left(1 - \frac{50}{650}\right) = 0,923$$

dari persamaan (dari 3.65x), didapat:

$$P_n = 0,85 \cdot 22,5 \cdot 450 \cdot 650 \cdot \left[-0,757 + \sqrt{(-0,757)^2 + 2 \cdot 18,3 \cdot 0,031 \cdot (0,923)}\right] \cdot 10^{-3}$$

$$= 2885,99 \text{ KN}$$

$$\phi \cdot P_n = 0,65 \cdot 2885,99 = 1875,89 \text{ KN} > P_{n,k} = 1081,76 \text{ KN}$$

d. Perencanaan tulangan geser kolom

$$M_{u,ka} = M_{u,kb} = 910,59 \text{ KN}$$

$$V_{D,k} = 80,50 \text{ KN}$$

$$V_{L,k} = 33,06 \text{ KN}$$

$$V_{E,k} = 201,14 \text{ KN}$$

Gaya geser rencana kolom

$$V_{u,k} = \frac{910,59 + 910,59}{(3,85 - 0,8)} = 597,11 \text{ KN} \quad (\text{dari 3.66a})$$

tidak perlu lebih dari

$$V_{u,k} = 1,05 \cdot (80,50 + 33,06 + 4/1 \cdot 201,14) = 963,55 \text{ KN} \quad (\text{dari 3.66b})$$

Tulangan geser didaerah ujung kolom ($V_c = 0$)

$$V_s = 597,11/0,6 - 0 = 995,18 \text{ KN}$$

Dipakai sengkang tertutup 4P10, $A_v = 314 \text{ mm}^2$

$$s \leq \frac{314.350.650.10^{-3}}{995,18} = 70 \text{ mm} \quad (\text{dari 3.60k})$$

$$s \leq \frac{1}{4}.450 = 112,5 \text{ mm} \quad (\text{dari 3.66d})$$

$$s \leq 8.25 = 200 \text{ mm} \quad (\text{dari 3.66e})$$

$$s \leq 100 \text{ mm} \quad (\text{dari 3.66f})$$

Tulangan geser didaerah tengah bentang kolom

$$V_c = (1 + 1081,76 / 14.315000) \left(\frac{1}{6} \cdot \sqrt{22,5} \right) 450.650.10^{-3} \quad (\text{dari 3.66c})$$

$$= 232,24 \text{ KN}$$

$$V_s = 995,18 - 232,24 = 762,94 \text{ KN}$$

$$s = \frac{314.350.650.10^{-3}}{762,94} = 90 \text{ mm}$$

$$s \leq 100 \text{ mm}$$

Daerah-daerah yang berpotensi mengalami sendi plastis terletak sepanjang l_0

dari muka yang ditinjau, dengan nilai l_0 tidak boleh kurang dari:

$$h = 450 \text{ mm} \text{ untuk } N_{u,k} < 0,3.A_g.f'_c$$

$$1,5.h \text{ untuk } N_{u,k} > 0,3.A_g.f'_c$$

$$1/6.l = 1/6.3050 = 508 \text{ mm}$$

$$450 \text{ mm}$$

4.6.3 Pertemuan balok kolom

$$V_{kol} = \frac{0,7(12/11,3 \cdot 1492,96)}{1/2(3,85 + 3,85)} = 371,53 \text{ KN} \quad (\text{dari 3.67a})$$

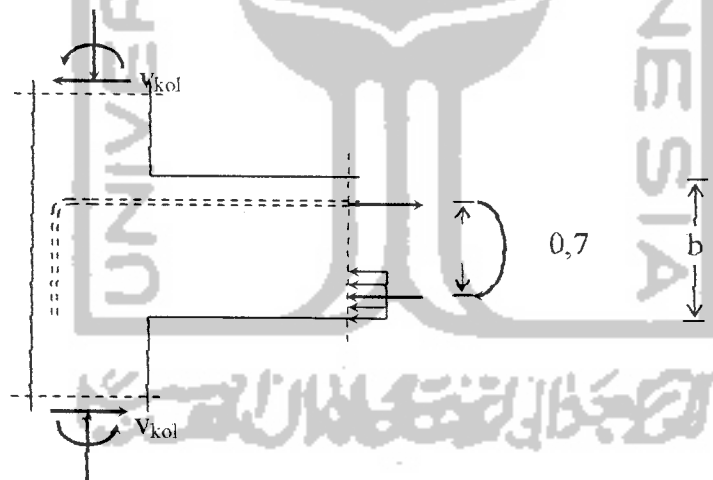
$$T = \frac{0,7 \cdot 1492,96 \cdot 10^3}{(650 - 348/2)} = 2195 \text{ KN} \quad (\text{dari 3.67d})$$

$$V_{j,h} = 2195 - 371,53 = 1423,47 \text{ KN} \quad (\text{dari 3.67b})$$

kontrol tegangan geser horizontal minimal

$$V_{j,h} = \frac{V_{j,h}}{b_j \cdot h_c} = \frac{1423,47 \cdot 10^3}{800 \cdot 700} = 4,07 \text{ MPa} \quad (\text{dari 3.67e})$$

$$1,5 \cdot \sqrt{f'_c} = 1,5 \cdot \sqrt{22,5} = 7,12 \text{ MPa} > V_{j,h} = 4,07 \text{ MPa} \quad (\text{dari 3.67f})$$



Gambar 4.11 Join balok kolom

Penulangan geser horizontal (dari 3.67g)

$$N_{u,k} = 1081,76 \text{ KN}$$

$$\frac{N_{u,k}}{A_g} = \frac{1081,76 \cdot 10^3}{450 \cdot 700} = 3,59 \text{ MPa} > 0,1 \cdot f'_c = 0,1 \cdot 22,5 = 2,25 \text{ MPa}$$

maka sumbangan gaya geser beton horizontal adalah:

$$V_{c,h} = 2/3 \cdot \sqrt{\left\{ \left(1130,95 \cdot 10^3 / 450 \cdot 700 \right) - 0,122,5 \right\} \cdot 800 \cdot 700 \cdot 10^{-3}}$$

$$= 563,25 \text{ KN}$$

sehingga:

$$V_{s,h} = 1423,47 - 563,25 = 860,22 \text{ KN} \quad (\text{dari 3.67j})$$

Luas tulangan geser horizontal

$$A_{j,h} = \frac{V_{s,h}}{f_y} = \frac{860,22 \cdot 10^3}{350} = 2457,77 \text{ mm}^2 \quad (\text{dari 3.67l})$$

$$\text{luasan tersedia 2 sengkang tertutup P10 } (2A_v) = 2 \cdot 157 = 314 \text{ mm}^2$$

$$\text{jumlah lapis sengkang} = 2457,77 / 314 = 7,8 \approx 8 \text{ lapis}$$

Luas tulangan geser vertikal

$$A'_s = A_s$$

$$V_{c,v} = 6148,5 \cdot 1423,47 / 6558,4 \cdot (0,6 + 1425,75 / 315 \cdot 22,5)$$

$$= 1069,16 \text{ KN} \quad (\text{dari 3.67o})$$

$$V_{s,v} = 1423,47 - 1069,16 = 354,31 \text{ KN} \quad (\text{dari 3.67n})$$

$$A_{j,v} = \frac{354,31 \cdot 10^3}{350} = 1012,33 \text{ mm}^2 \quad (\text{dari 3.67m})$$

$$\text{luasan tersedia 2 sengkang tertutup P10 } (2A_v) = 2 \cdot 157 = 314 \text{ mm}^2$$

$$\text{jumlah lapis sengkang} = 1012,33 / 314 = 3,2 \approx 4 \text{ lapis}$$

Keperluan tulangan geser diatas sebenarnya sudah dapat ditahan oleh tulangan lentur balok maupun tulangan kolom terpasang.

Tabel 4.10.a Momen Rencana Balok Portal As 9-10 (K=1)

Elm	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
14	0	-319.34	-104.77	577.40	-577.94	-550.83	232.25	-614.43	204.95	-782.85	blk. lt.1
	1.5	-87.16	-23.34	432.98	-433.52	-141.93	311.24	-411.17	348.41	-494.40	
	3	108.57	46.85	288.57	-289.10	205.24	357.42	-218.03	446.51	-224.85	
15	0	108.57	46.85	288.57	-289.10	205.24	357.42	-218.03	446.51	-224.85	blk. lt.1
	1.5	188.74	72.00	144.15	-144.69	341.68	299.60	-65.42	394.89	-30.96	
	3	232.46	85.90	-0.27	-0.27	416.38	208.97	77.07	297.92	144.03	
16	0	232.46	85.90	-0.27	-0.27	416.38	208.97	77.07	297.92	144.03	blk. lt.1
	1.5	160.61	54.77	-144.69	144.15	280.36	14.33	179.03	51.23	243.37	
	3	52.32	12.39	-289.10	288.57	82.6	-213.10	270.86	-240.81	323.81	
17	0	52.32	12.39	-289.10	288.57	82.6	-213.10	270.86	-240.81	323.81	blk. lt.1
	1.5	-171.53	-75.03	-433.52	432.98	-325.88	-544.54	322.16	-682.57	328.58	
	3	-431.83	-173.69	-577.94	577.40	-796.09	-908.79	363.33	-1169.68	314.47	
18	0	-163.25	-100.00	0	0	-355.9	-146.93	-90.00	-234.41	-168.00	blk. lt.1
	1.25	-70.06	-46.09	0	0	-157.83	-63.05	-41.48	-102.60	-77.43	
	2.5	0	0	0	0	0	0	0	0	0	
19	0	-306.40	-96.54	677.01	-673.43	-522.14	333.54	-692.97	328.32	-869.28	blk. lt.2
	1.5	-78.57	-17.77	508.20	-504.62	-122.72	386.67	-470.15	439.92	-559.70	
	3	112.81	49.74	339.40	-335.82	214.96	406.98	-257.47	506.15	-269.04	
20	0	112.81	49.74	339.40	-335.82	214.96	406.98	-257.47	506.15	-269.04	blk. lt.2
	1.5	188.63	72.23	170.60	-167.02	341.92	323.30	-85.31	422.69	-54.02	
	3	228.00	83.46	1.79	1.79	407.14	206.81	76.73	293.86	142.09	
21	0	228.00	83.46	1.79	1.79	407.14	206.81	76.73	293.86	142.09	blk. lt.2
	1.5	151.81	49.67	-167.02	170.60	261.64	-13.68	198.24	15.33	262.57	
	3	39.17	4.63	-335.82	339.40	54.4	-266.98	309.62	-308.56	364.14	

Lanjutan Tabel 4.10.a Momen Rencana Balok Portal As 9-10 (K=1)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)		0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN	KIRI	KANAN	KIRI	KANAN	KIRI	KANAN	
22	0	39.17	4.63	-335.82	339.40	54.4		-266.98	309.62	-308.56	364.14	blk. lt.2
	1.5	-189.03	-85.45	-504.62	508.20	-363.55		-624.29	380.48	-782.17	390.05	
	3	-453.68	-186.77	-673.43	677.01	-843.26		-1014.39	333.54	-1196.13	397.08	
23	0	-163.25	-100.00	0	0	-355.9		-146.93	-90.00	-234.41	-168.00	blk. lt.2
	1.25	-70.06	-46.09	0	0	-157.83		-63.05	-41.48	-102.60	-77.43	
	2.5	0	0	0	0	0		0	0	0	0.00	
24	0	-297.45	-87.02	600.28	-616.57	-496.17		272.55	-633.23	263.15	-793.59	blk. lt.3
	1.5	-70.95	-9.07	448.18	-464.46	-99.66		339.50	-426.18	390.37	-502.92	
	3	119.09	57.63	296.07	-312.36	235.11		373.64	-229.25	472.22	-231.15	
25	0	119.09	57.63	296.07	-312.36	235.11		373.64	-229.25	472.22	-231.15	blk. lt.3
	1.5	193.58	79.29	143.97	-160.25	359.16		303.79	-72.86	404.37	-35.06	
	3	231.61	89.71	-8.14	-8.14	421.47		201.12	73.41	291.16	142.17	
26	0	231.61	89.71	-8.14	-8.14	421.47		201.12	73.41	291.16	142.17	blk. lt.3
	1.5	154.09	55.10	-160.25	143.97	273.06		-5.54	179.16	28.25	243.73	
	3	40.11	9.24	-312.36	296.07	62.92		-245.02	274.78	-280.04	326.40	
27	0	40.11	9.24	-312.36	296.07	62.92		-245.02	274.78	-280.04	326.40	blk. lt.3
	1.5	-189.42	-81.65	-464.46	448.18	-357.96		-588.49	329.87	-738.01	333.41	
	3	-455.41	-183.80	-616.57	600.28	-840.57		-964.78	324.83	-1141.37	321.51	
28	0	-163.25	-100.00	0	0	-355.9		-146.93	-90.00	-234.41	-168.00	blk. lt.3
	1.25	-70.06	-46.09	0	0	-157.83		-63.05	-41.48	-102.60	-77.43	
	2.5	0	0	0	0	0		0	0	0	0.00	
29	0	-307.41	-111.97	585.88	-514.62	-548.04		250.62	-563.93	221.85	-728.46	blk. lt.4
	1.5	-81.29	-34.25	448.32	-377.06	-152.35		330.32	-370.18	363.80	-453.45	
	3	108.38	32.21	310.76	-239.50	181.6		377.22	-186.56	460.38	-197.36	
30	0	108.38	32.21	310.76	-239.50	181.6		377.22	-186.56	460.38	-197.36	blk. lt.4
	1.5	182.49	53.65	173.19	-101.94	304.83		320.11	-43.46	407.26	-16.90	
	3	220.15	63.84	35.63	35.63	366.31		230.20	89.52	308.79	144.66	

Lanjutan Tabel 4.10.a Momen Rencana Balok Portal As 9-10 (K=I)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD+1.6ML (KNm)		0.9(MD+ME) (KNm)		1.05(MD+0.6ML+ME) (KNm)		KET
				KIRI	KANAN	KIRI	KANAN	KIRI	KANAN	KIRI	KANAN	
31	0	220.15	63.84	-35.63	35.63	366.31		166.07	89.52	233.97	144.66	
	1.5	142.24	29.00	-101.94	173.19	217.08		36.27	181.97	60.59	230.57	blk. lt.4
	3	27.89	-17.10	-239.50	310.76	6.11		-190.44	264.29	-232.96	297.56	
32	0	27.89	-17.10	-239.50	310.76	6.11		-190.44	264.29	-232.96	297.56	
	1.5	-202.02	-108.22	-377.06	448.32	-415.58		-521.17	306.09	-676.21	288.92	blk. lt.4
	3	-468.39	-210.59	-514.62	585.88	-899.01		-884.71	337.76	-1164.83	261.38	
33	0	-163.25	-100.00	0	0	-355.9		-146.93	-90.00	-234.41	-168.00	
	1.25	-70.06	-46.09	0	0	-157.83		-63.05	-41.48	-102.60	-77.43	blk. lt.4
	2.5	0	0	0	0	0		0	0	0	0	
34	0	0	0	0	0	0		0	0	0	0	
	1.75	-86.94	-91.88	0	0	-251.33		-78.25	-82.69	-149.17	-154.36	blk. atap
	3.5	-217.98	-202.13	0	0	-584.98		-196.18	-181.92	-356.22	-339.58	
35	0	-217.98	-202.13	0	0	-584.98		-196.18	-181.92	-356.22	-339.58	
	1.75	-86.94	-91.88	0	0	-251.33		-78.25	-82.69	-149.17	-154.36	blk. atap
	3.5	0	0	0	0	0		0	0	0	0	

INDONESIA

Tabel 4.10.b Gaya Geser Rencana Balok Portal As.9 dan As.10 (K=1)

Elm	Jarak (m)	V _{GB} (KN)	V _{LB} (KN)	V _{EB} (KN)	V _{GB} (KN)	M _{KAP.B}	M' _{KAP.B}	V _{UB} = 0,7(M _{KAP.B} + M' _{KAP.B})/Ln+1,05V _{GB} (KN)	V _{up} terpakai (KN)	V _{US} =1,05(V _{GB} + V _{LB} +4, V _{EB}) (KN)	KETERANGAN
14	0	156.94	58.04	96.28	224.97	1492.96	754.44	387.26	369.28	640.59	
	1.5	142.64	50.54	96.28	193.17	1492.96	754.44	352.20	334.32	607.20	BALOK LT.1
	3	118.34	43.04	96.28	161.37	1492.96	754.44	317.14	299.37	573.81	
15	0	65.60	20.52	96.28	86.112	1492.96	754.44	234.17	216.64	494.79	
	1.5	41.30	13.02	96.28	54.312	1492.96	754.44	199.11	181.68	461.40	BALOK LT.1
	3	17.00	5.52	96.28	22.512	1492.96	754.44	164.05	146.73	428.01	
16	0	35.75	17.00	96.28	52.748	1492.96	754.44	197.38	179.96	459.75	
	1.5	60.05	24.50	96.28	84.548	1492.96	754.44	232.44	214.91	493.14	BALOK LT.1
	3	84.35	32.00	96.28	116.35	1492.96	754.44	267.50	249.87	526.53	
17	0	137.09	54.52	96.28	191.61	1492.96	754.44	350.47	332.60	605.36	
	1.5	161.39	62.02	96.28	223.41	1492.96	754.44	385.53	367.55	638.95	BALOK LT.1
	3	185.69	69.52	96.28	255.21	1492.96	754.44	420.59	402.51	672.34	
18	0	83.80	46.25	0	130.05	1492.96	754.44	282.60	264.93	136.55	
	1.25	65.30	40.00	0	105.3	1492.96	754.44	255.31	237.72	110.57	BALOK LT.1
	2.5	46.80	33.75	0	80.55	1492.96	754.44	228.03	210.52	84.58	
19	0	164.04	58.26	112.54	220.3	1492.96	754.44	382.10	364.14	703.96	
	1.5	139.74	48.76	112.54	212.8	1492.96	754.44	347.04	329.18	696.08	BALOK LT.2
	3	115.44	41.26	112.54	181	1492.96	754.44	311.98	294.22	662.69	
20	0	62.70	18.74	112.54	134.18	1492.96	754.44	228.01	211.49	613.53	
	1.5	38.40	11.24	112.54	73.936	1492.96	754.44	193.95	176.54	550.28	BALOK LT.2
	3	14.10	3.74	112.54	42.136	1492.96	754.44	158.89	141.58	516.89	
21	0	38.64	18.78	112.54	32.876	1492.96	754.44	202.53	185.09	507.17	
	1.5	62.94	26.28	112.54	64.924	1492.96	754.44	237.58	220.05	540.82	BALOK LT.2
	3	87.24	33.78	112.54	96.724	1492.96	754.44	272.64	255.00	574.21	
22	0	139.98	56.30	112.54	143.54	1492.96	754.44	355.62	337.73	623.37	
	1.5	164.28	63.80	112.54	203.78	1492.96	754.44	390.68	372.69	686.62	BALOK LT.2
	3	188.58	71.30	112.54	235.58	1492.96	754.44	425.74	407.64	720.01	
23	0	83.80	46.25	0	234.83	1492.96	754.44	282.60	264.93	246.58	
	1.25	65.30	40.00	0	123.8	1492.96	754.44	255.31	237.72	129.99	BALOK LT.2
	2.5	46.80	33.75	0	99.05	1492.96	754.44	228.03	210.52	104.00	
24	0	163.15	55.72	101.40	102.52	1492.96	754.44	380.52	362.56	533.54	
	1.5	138.85	48.22	101.40	211.36	1492.96	754.44	345.46	327.61	647.83	BALOK LT.3
	3	114.55	40.72	101.40	179.56	1492.96	754.44	310.40	292.65	614.44	
25	0	51.81	18.20	101.40	132.74	1492.96	754.44	387.26	369.28	565.27	
	1.5	37.51	10.70	101.40	72.501	1492.96	754.44	352.20	334.32	502.02	BALOK LT.3
	3	13.21	3.20	101.40	40.701	1492.96	754.44	317.14	299.37	468.63	
26	0	39.53	19.33	101.40	32.531	1492.96	754.44	234.17	216.64	460.05	
	1.5	53.83	26.83	101.40	66.359	1492.96	754.44	199.11	181.68	495.57	BALOK LT.3
	3	88.13	34.33	101.40	98.159	1492.96	754.44	164.05	146.73	528.96	

Lanjutan Tabel 4.10.b Gaya Geser Rencana Balok Portal As.9 dan As.10 (K=1)

Elm	Jarak (m)	V _{DB} (KN)	V _{LR} (KN)	V _{EB} (KN)	V _{GB} (KN)	M _{KAP.B}	M' _{KAP.B}	V _{UB} = 0,7(M _{KAP.E} + M' _{KAP.B})/Ln+1,05.V _{3.B} (KN)	V _{ubterpakai} (KN)	V _{UB} =1,05(V _{DB} + V _{LR} +4. V _{E.R}) (KN)	KETERANGAN
27	0	140.87	56.85	101.40	144.98	1492.96	754.44	227.43	209.92	578.12	BALOK LT.3
	1.5	165.17	64.35	101.40	205.22	1492.96	754.44	192.37	174.97	641.38	
	3	189.47	71.85	101.40	237.02	1492.96	754.44	157.31	140.01	674.77	
28	0	83.80	46.25	0	235.72	1492.96	754.44	204.11	186.67	247.51	BALOK LT.3
	1.25	65.30	40.00	0	123.8	1492.96	754.44	239.17	221.63	129.99	
	2.5	46.80	33.75	0	99.05	1492.96	754.44	274.23	256.58	104.00	
29	0	162.90	55.56	91.71	102.36	1492.96	754.44	357.21	339.31	492.65	BALOK LT.4
	1.5	138.60	48.06	91.71	210.96	1492.96	754.44	392.27	374.27	606.68	
	3	114.30	40.56	91.71	179.16	1492.96	754.44	427.32	409.23	573.29	
30	0	61.56	18.04	91.71	132.34	1492.96	754.44	282.60	264.93	524.13	BALOK LT.4
	1.5	37.26	10.54	91.71	72.096	1492.96	754.44	255.31	237.72	460.87	
	3	12.96	3.04	91.71	40.296	1492.96	754.44	228.03	210.52	427.48	
31	0	39.79	19.48	91.71	66.764	1492.96	754.44	380.07	362.11	419.23	BALOK LT.4
	1.5	64.09	26.98	91.71	32.434	1492.96	754.44	345.01	327.16	455.28	
	3	88.39	34.48	91.71	98.564	1492.96	754.44	309.95	292.20	488.67	
32	0	141.13	57.00	91.71	145.38	1492.96	754.44	226.98	209.47	537.83	BALOK LT.4
	1.5	165.43	64.50	91.71	198.12	1492.96	754.44	191.92	174.52	593.20	
	3	189.73	72.00	91.71	229.92	1492.96	754.44	156.86	139.56	626.59	
33	0	83.80	46.25	0	261.72	1492.96	754.44	204.56	187.12	274.81	BALOK LT.4
	1.25	65.30	40.00	0	130.05	1492.96	754.44	239.62	222.08	136.55	
	2.5	46.80	33.75	0	105.3	1492.96	754.44	274.68	257.04	110.57	
34	0	37.08	47.25	0	0	0	0	357.66	339.77	84.58	BALOK ATAP
	1.75	62.28	57.75	0	0	0	0	392.72	374.72	88.55	
	3.5	87.48	68.25	0	0	0	0	427.78	409.68	126.03	
35	0	87.48	68.25	0	0	0	0	282.60	284.93	163.52	BALOK ATAP
	1.75	62.28	57.75	0	0	0	0	255.31	237.72	163.52	
	3.5	37.08	47.25	0	0	0	0	228.03	210.52	126.03	

Keterangan:

$$V_{u,b \text{ terpakai}} = \left[1,05 \cdot V_G - 0,7 \left(\frac{M_{\text{kap,b}} + M'_{\text{Kap,b}}}{I_n} \right) \right] + \frac{\ln-d}{\ln} \left[V_{u,b} - \left[1,05 \cdot V_G - 0,7 \left(\frac{M_{\text{kap,b}} + M'_{\text{Kap,b}}}{I_n} \right) \right] \right]$$

Tabel 4.10.c Momen Rencana Kolom Portal As.9 dan As.10 (K=1)

Elm	Jarak (m)	M _{D,k} (KNm)	M _{L,k} (KNm)	M _{ESi} (KNm)	M _{ESk} (KNm)	M _{KAP, kiri} (KNm)	M _{KAP, kanan} (KNm)	$M_{0,y} = \frac{h}{h_n} - 0,7, 0$ (KNm)	$M_{U,y} = \frac{1}{d} \cdot 0,5 \cdot \left(\frac{M_{kap,b}}{I_n} \right)$ (KNm)	$1,05(M_{D,k} + M_{L,k} \pm 4/1 M_{E,k})$ (Kiri) (KNm)	$M_{U,k} = 1,05(M_{D,k} + M_{L,k} \pm 4/1 M_{E,k})$ (kanan) (KNm)	KETERANGAN
1	0	24.65	-3.37	-840.60	837.56	0	1492.96	910.59		-3508.18	3540.10	KOL BASEMEN
	1.925	-69.35	-32.87	-375.77	376.81	0	1492.96	910.59		-1685.57	1475.27	
	3.85	-163.35	-62.37	89.06	-83.94	0	1492.96	910.59		137.05	-589.55	
2	0	-98.92	-42.08	-837.56	840.60	1492.96	0	910.59		-3665.80	3382.47	KOL BASEMEN
	1.925	-4.92	-12.58	-376.81	375.77	1492.96	0	910.59		-1600.98	1559.86	
	3.85	89.07	16.92	83.94	-89.06	1492.96	0	910.59		463.84	-262.76	
3	0	155.99	42.40	-488.34	493.99	0	1492.96	910.59		-1842.72	2283.07	KOLOM LT.1
	1.925	-3.11	-2.46	-68.20	60.22	0	1492.96	910.59		-292.29	247.08	
	3.85	-162.21	-47.33	351.93	-373.56	0	1492.96	910.59		1258.09	-1788.97	
4	0	-179.51	-56.77	-493.99	488.34	1492.96	0	910.59		-2322.85	1802.93	KOLOM LT.1
	1.925	-20.41	-11.90	-60.22	68.20	1492.96	0	910.59		-286.85	252.51	
	3.85	138.69	32.96	373.56	-351.93	1492.96	0	910.59		1749.18	-1297.87	
5	0	144.18	49.21	-325.07	299.86	0	1492.96	910.59		-1162.23	1462.47	KOLOM LT.2
	1.925	-10.79	-14.43	62.11	-23.55	0	1492.96	910.59		234.38	-125.39	
	3.85	-165.76	-78.08	449.30	-346.96	0	1492.96	910.59		1631.03	-1713.26	
6	0	-151.74	-53.82	-299.86	325.07	1492.96	0	910.59		-1475.25	1149.46	KOLOM LT.2
	1.925	3.23	9.83	23.55	-62.11	1492.96	0	910.59		112.62	-247.15	
	3.85	158.20	73.48	346.96	-449.30	1492.96	0	910.59		1700.50	-1643.80	
7	0	131.69	8.94	-150.98	269.61	0	1492.96	910.59		-486.45	1280.02	KOLOM LT.3
	1.925	21.13	49.55	-47.46	-122.51	0	1492.96	910.59		125.12	-440.33	
	3.85	-89.43	90.16	56.06	-514.62	0	1492.96	910.59		236.22	-2160.64	
8	0	-133.96	-10.32	-269.61	150.98	1492.96	0	910.59		-1283.86	482.62	KOLOM LT.3
	1.925	-23.40	-50.92	122.51	47.46	1492.96	0	910.59		436.51	-121.30	
	3.85	87.16	-91.53	514.62	-56.06	1492.96	0	910.59		2156.82	-240.04	
9	0	217.98	202.13	-529.82	0.00	0	1492.96	910.59		-1784.13	441.12	KOLOM LT.4
	1.34	217.98	202.13	-384.34	0.00	0	1492.96	910.59		-173.11	441.12	
	2.68	217.98	202.13	-238.85	0.00	0	1492.96	910.59		-562.05	441.12	
10	0	-217.98	-202.13	0	529.82	1492.96	0	910.59		-441.11	1996.37	KOLOM LT.4
	1.34	-217.98	-202.13	0	384.34	1492.96	0	910.59		-441.12	1784.13	
	2.68	-217.98	-202.13	0	238.85	1492.96	0	910.59		-441.12	1173.11	
11	0	0	0	-238.85	0	0	0	0		-1003.17	0	KOLOM LT.4
	1.1	0	0	-119.43	0	0	0	0		-501.61	0	
	2.2	0	0	0	0	0	0	0		0	0	
12	0	0	0	0	238.85	0	0	0		0	1003.17	KOLOM LT.4
	1.1	0	0	0	119.43	0	0	0		0	501.61	
	2.2	0	0	0	0	0	0	0		0	0	

Tabel 4.10.d Gaya Geser Rencana Kolom As.9 dan As.10 (K=1)

Elm	Jarak (m)	V _{D,k} (KN)	V _{L,k} (KN)	V _{E,k} (KN)	M _{U,k} atas (KN)	M _{U,k} bawah (KN)	V _{U,k} = (M _{U,k} a + M _{U,k} b)/h _i (KN)	V _{U,max} = 1,05(V _{D,k} + V _{L,k} + 4. V _{E,k}) (KN)	KETERANGAN
1	0	48.83	15.32	241.47	910.59	910.59	597.11	946.81	KOL BASEMEN
	1.925	48.83	15.32	241.47	910.59	910.59	597.11	946.81	
	3.85	48.83	15.32	241.47	910.59	910.59	597.11	946.81	
2	0	48.83	15.32	239.35	910.59	910.59	597.11	1072.63	KOL BASEMEN
	1.925	48.83	15.32	239.35	910.59	910.59	597.11	1072.63	
	3.85	48.83	15.32	239.35	910.59	910.59	597.11	1072.63	
3	0	82.65	23.31	218.25	910.59	910.59	597.11	805.40	KOLOM LT.1
	1.925	82.65	23.31	218.25	910.59	910.59	597.11	805.40	
	3.85	82.65	23.31	218.25	910.59	910.59	597.11	805.40	
4	0	82.65	23.31	225.34	910.59	910.59	597.11	1057.68	KOLOM LT.1
	1.925	82.65	23.31	225.34	910.59	910.59	597.11	1057.68	
	3.85	82.65	23.31	225.34	910.59	910.59	597.11	1057.68	
5	0	80.50	33.06	201.14	910.59	910.59	597.11	725.53	KOLOM LT.2
	1.925	80.50	33.06	201.14	910.59	910.59	597.11	725.53	
	3.85	80.50	33.06	201.14	910.59	910.59	597.11	725.53	
6	0	80.50	33.06	168.00	910.59	910.59	597.11	824.86	KOLOM LT.2
	1.925	80.50	33.06	168.00	910.59	910.59	597.11	824.86	
	3.85	80.50	33.06	168.00	910.59	910.59	597.11	824.86	
7	0	57.43	21.10	53.78	910.59	910.59	597.11	308.33	KOLOM LT.3
	1.925	57.43	21.10	53.78	910.59	910.59	597.11	308.33	
	3.85	57.43	21.10	53.78	910.59	910.59	597.11	308.33	
8	0	57.43	21.10	203.70	910.59	910.59	597.11	893.67	KOLOM LT.3
	1.925	57.43	21.10	203.70	910.59	910.59	597.11	893.67	
	3.85	57.43	21.10	203.70	910.59	910.59	597.11	893.67	
9	0	0	0	108.57	910.59	910.59	597.11	455.99	KOLOM LT.4
	1.34	0	0	108.57	910.59	910.59	597.11	455.99	
	2.68	0	0	108.57	910.59	910.59	597.11	455.99	
10	0	0	0	108.57	910.59	910.59	597.11	455.99	KOLOM LT.4
	1.34	0	0	108.57	910.59	910.59	597.11	455.99	
	2.68	0	0	108.57	910.59	910.59	597.11	455.99	
11	0	0	0	108.57	910.59	910.59	597.11	455.99	KOLOM LT.4
	1.1	0	0	108.57	910.59	910.59	597.11	455.99	
	2.2	0	0	108.57	910.59	910.59	597.11	455.99	
12	0	0	0	108.57	910.59	910.59	597.11	455.99	KOLOM LT.4
	1.1	0	0	108.57	910.59	910.59	597.11	455.99	
	2.2	0	0	108.57	910.59	910.59	597.11	455.99	

4.7 Perencanaan Penulangan Portal As.9-10 Daktilitas Terbatas (K=2)

4.7.1 Penulangan Lentur dan Geser Balok

Dari tabel momen rencana balok didapat:

$$M_{tumpuan}^- = 1808,22 \text{ KNm}$$

$$M_{tumpuan}^+ = 939,17 \text{ KNm}$$

$$M_{lapangan} = 862,52 \text{ KNm}$$

a. Pemeriksaan rasio tulangan apakah boleh dilakukan redistribusi momen

$$\rho_b = 0,029$$

$$R_n = 8,0$$

$$\rho = 0,032, \text{ dipakai } \rho_{maks} = 0,022$$

$$R_n = 4,17$$

$$\rho' = 0,013$$

$$0,022 - 0,013 = 0,009 < 0,5 \cdot 0,029 = 0,0145$$

maka redistribusi momen negatif tidak boleh dilakukan

- faktor redistribusi maksimum

$$30 \cdot \left(1 - \frac{4 \cdot 0,009}{3 \cdot 0,029} \right) \% = 12 \% \quad (\text{dari 3.3})$$

- Redistribusi momen negatif pada pertemuan kolom tepi, ($\partial = 15\%$)

$$\partial M = 1808,22 \cdot 12\% = 180,82 \text{ KNm}$$

- Momen balok rencana terredistribusi

$$M_{tump}^- = 1808,22 - 180,82 = 1627,40 \text{ KNm}$$

$$M_{tump}^+ = 939,17 + 180,82 = 1119,39 \text{ KNm}$$

$$M_{lap} = 862,52 + 180,82 = 1042,74 \text{ KNm}$$

- Perencanaan tulangan tumpuan negatif

$$f'_c = 22,5 \text{ MPa}$$

$$f_y = 350 \text{ MPa}$$

$$d = 800 - 50 = 750 \text{ mm}$$

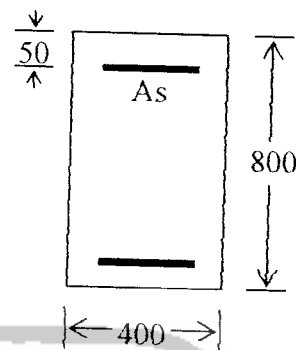
$$m = 18,3$$

$$\rho_{\min} = 0,004$$

$$\rho_{\text{perlu}} = 0,022$$

$$A_s = 0,020 \cdot 400 \cdot 750 = 6000 \text{ mm}^2$$

$$A_s' = 0,5 \cdot 6000 = 3000 \text{ mm}^2$$



- Perencanaan tulangan tumpuan positif

$$R_n = 4,91119,39$$

$$\rho_{\text{perlu}} = 0,016$$

$$A_s' = 0,016 \cdot 400 \cdot 750 = 4946,17 \text{ mm}^2$$

Tulangan tumpuan terpasang berdasarkan luasan maksimum

$$A_s = 6000 \text{ mm}^2$$

$$\text{Dipakai tulangan } 13D25 = 6381,7 \text{ mm}^2$$

$$A_s' = 4946,17 \text{ mm}^2$$

$$\text{Dipakai tulangan } 11D25 = 5399,9 \text{ mm}^2$$

- Perencanaan tulangan lapangan

$$R_n = 4,63$$

$$\rho_{\text{perlu}} = 0,015$$

$$A_s = 0,015 \cdot 400 \cdot 750 = 4624,6 \text{ mm}^2$$

$$\text{Dipakai tulangan } 10D25 = 4909 \text{ mm}^2$$

b. Tulangan geser balok

$$V_D = 165,17 \text{ KN}$$

$$V_L = 64,35 \text{ KN}$$

$$V_E = 112,54 \text{ KN}$$

$$V_g = 188,58 + 71,30 = 259,88 \text{ KN}$$

$$l_n = 12 - 0,70 = 11,3 \text{ m}$$

$$V_{u,b} = 1,05 \cdot \left(165,17 + 72 \cdot 0,0 + \frac{4,0}{2} \cdot 91,71 \right) = 487,33 \text{ KN}$$

$$V_{u,b \text{ terpakai}} = \frac{6 - 0,7}{6} \cdot 487,33 = 401,9 \text{ KN}$$

- Pada daerah sepanjang $d = 700 \text{ mm}$

$$V_c = 1/6 \cdot \sqrt{22,5 \cdot 400 \cdot 750 \cdot 10^{-3}} = 237,17 \text{ KN}$$

$$\frac{1}{2} \cdot V_c = 118,5 \text{ KN}$$

$$V_s = 401,9 / 0,6 - 118,5 = 551,3 \text{ KN}$$

$$\text{Dipakai sengkang } 3D10 = 235,5 \text{ mm}^2$$

$$s = \frac{235,5 \cdot 350 \cdot 750 \cdot 10^{-3}}{551,3} = 110 \text{ mm}$$

$$s \leq \frac{750}{4} = 187,5 \text{ mm}$$

- Diluar daerah d

$$V_s = 401,9 / 0,6 - 237,17 = 432,66 \text{ KN}$$

$$s \leq \frac{235,5 \cdot 350 \cdot 750 \cdot 10^{-3}}{432,66} = 140 \text{ mm}$$

$$s \leq \frac{750}{2} = 187,5 \text{ mm}$$

4.7.2 Penulangan kolom akibat beban lentur dan aksial

a. Momen rencana kolom

$$M_{u,k} = 1,05 \cdot (163,35 + 0,3 \cdot 9,73 + 62,37 + 0,3 \cdot 4,01 + 1,3 \cdot (83,94 + 0,3 \cdot 357))$$

$$= 1291,32 \text{ KNm}$$

b. Gaya aksial rencana kolom

$$N_{u,k} = 1,05 \cdot (662,88 + 0,3 \cdot 314,25 + 318,05 + 0,3 \cdot 149,66 + 1,3 \cdot (1239,75 + 0,3 \cdot 619,87))$$

$$= 2592,6 \text{ KN}$$

$$e = \frac{1291,32}{2592,6} \cdot 10^{-3} = 498 \text{ mm} > e_b = 286 \text{ mm} \text{ (patah tarik)}$$

Rasio tulangan menurut diagram interaksi kolom, dilakukan interpolasi:

$$\rho = 0,04$$

$$A_{st} = 4\% \cdot 450 \cdot 700 = 12600 \text{ mm}^2$$

$$\text{Dipakai tulangan } 26D25 = 12763,4 \text{ mm}^2$$

$$A_s = 13.409,9 = 6381,7 \text{ mm}^2$$

$$A_s' = 13.409,9 = 6381,7 \text{ mm}^2$$

Kontrol kapasitas kolom terhadap keruntuhan tarik

$$P_n = 0,85 \cdot 22,5 \cdot 450 \cdot 650 \left[-0,227 + \sqrt{(-0,227)^2 + 2 \cdot 18,3 \cdot 0,04 \cdot (0,923)} \right] \cdot 10^{-3}$$

$$= 5355,74 \text{ KN}$$

$$\phi \cdot P_n = 0,65 \cdot 5355,74 = 3481,2 \text{ KN} > P_{n,k} = 2592,6 \text{ KN}$$

c. Perencanaan tulangan geser kolom

$$V_{u,k} = 1,05 \cdot (82,65 + 23,31 + 1,3 \cdot 225,34) = 418,85 \text{ KN} \quad (\text{dari 3.66b})$$

$$V_c = (1 + 2592,6 / 14.315000) \left(\frac{1}{6} \cdot \sqrt{22,5} \right) 450.650 \cdot 10^{-3} \quad (\text{dari 3.66c})$$

$$= 367,18 \text{ KN}$$

$$1/2 \cdot V_c = 183,59 \text{ KN}$$

Tulangan geser didaerah ujung kolom

$$V_s = 418,85 / 0,6 - 183,59 = 514,49 \text{ KN}$$

Dipakai sengkang tertutup 4P10, $A_v = 314 \text{ mm}^2$

$$s \leq \frac{314.350.650.10^{-3}}{514,49} = 120 \text{ mm} \quad (\text{dari 3.60k})$$

$$s \leq \frac{1}{4} \cdot 450 = 112,5 \text{ mm} \quad (\text{dari 3.66d})$$

$$s \leq 8.25 = 200 \text{ mm} \quad (\text{dari 3.66e})$$

$$s \leq 100 \text{ mm (menentukan)} \quad (\text{dari 3.66f})$$

Tulangan geser didaerah tengah bentang kolom

$$V_s = 418,85 / 0,6 - 367,18 = 330,9 \text{ KN}$$

$$s = \frac{314.350.650.10^{-3}}{330,9} = 210 \text{ mm}$$

$$s \leq 200 \text{ mm (menentukan)}$$

Tabel 4.11.a Momen Rencana Balok Portal As 9-10 (K=2)

ELEM	Jarak (m)	MD (kNm)	ML (kNm)	M.GEMPA (kNm)		1.2MD + 1.6ML (kNm)	0.9(MD ± ME) (kNm)		1.05(MD+0.6ML ± ME) (kNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
14	0	-319.34	-104.77	1154.79	-1155.87	-550.83	751.91	-1327.68	811.23	-1614.96	blk. It. 1
	1.5	-87.16	-23.34	865.96	-867.03	-141.93	700.92	-858.77	803.04	-1016.60	
	3	108.57	46.85	577.13	-578.20	205.24	617.13	-422.67	749.50	-463.60	
15	0	108.57	46.85	577.13	-578.20	205.24	617.13	-422.67	749.50	-463.60	blk. It. 1
	1.5	188.74	72.00	288.30	-289.37	341.68	429.33	-90.57	546.24	-60.31	
	3	232.46	85.90	-0.54	-0.54	416.38	208.73	208.73	297.63	297.63	
16	0	232.46	85.90	-0.54	-0.54	416.38	208.73	208.73	297.63	297.63	blk. It. 1
	1.5	160.61	54.77	-289.37	288.30	280.36	-115.88	404.02	-100.69	505.86	
	3	52.32	12.39	-578.20	577.13	82.60	-473.29	566.51	-544.37	668.73	
17	0	52.32	12.39	-578.20	577.13	82.60	-473.29	566.51	-544.37	668.73	blk. It. 1
	1.5	-171.53	-75.03	-867.03	865.96	-325.88	-934.71	624.99	-1137.76	681.89	
	3	-431.83	-173.69	-1155.87	1154.79	-796.09	-1428.93	650.67	-1676.50	649.69	
18	0	-163.25	-100.00	0	0	-355.90	-146.93	-146.93	-234.41	-234.41	blk. It. 1
	1.25	-70.06	-46.09	0	0	-157.83	-63.06	-63.06	-102.61	-102.61	
	2.5	0	0	0	0	0	0	0	0	0	
19	0	-306.40	-96.54	1354.01	-1346.85	-522.14	942.85	-1487.92	939.17	-1796.72	blk. It. 2
	1.5	-78.57	-17.77	1016.40	-1009.24	-122.72	844.05	-979.03	973.53	-1153.39	
	3	112.81	49.74	678.79	-671.63	214.96	712.44	-502.94	862.52	-555.42	
20	0	112.81	49.74	678.79	-671.63	214.96	712.44	-502.94	862.52	-555.42	blk. It. 2
	1.5	188.63	72.23	341.19	-334.03	341.92	476.84	-130.86	601.81	-107.16	
	3	228.00	83.46	3.58	3.58	407.14	208.42	208.42	295.74	295.74	
21	0	228.00	83.46	3.58	3.58	407.14	208.42	208.42	295.74	295.74	blk. It. 2
	1.5	151.81	49.67	-334.03	341.19	261.64	-164.00	443.70	-160.04	548.94	
	3	39.17	4.63	-671.63	678.79	54.40	-569.22	646.17	-661.17	756.77	
22	0	39.17	4.63	-671.63	678.79	54.40	-569.22	646.17	-661.17	756.77	blk. It. 2
	1.5	-189.03	-85.45	-1009.24	1016.40	-363.55	-1078.44	744.63	-1312.02	814.90	
	3	-453.68	-186.77	-1346.85	1354.01	-843.26	-1620.48	810.29	-1808.22	827.67	
23	0	-163.25	-100.00	0	0	-355.90	-146.93	-146.93	-234.41	-234.41	blk. It. 2
	1.25	-70.06	-46.09	0	0	-157.83	-63.06	-63.06	-102.61	-102.61	
	2.5	0	0	0	0	0	0	0	0	0	
24	0	-297.45	-87.02	1200.56	-1233.13	-496.17	812.81	-1377.52	893.45	-1661.93	blk. It. 3
	1.5	-70.95	-9.07	896.35	-928.92	-99.66	742.86	-899.88	860.95	-1055.58	
	3	119.09	57.63	592.14	-624.71	235.11	640.11	-455.05	783.10	-494.59	
25	0	119.09	57.63	592.14	-624.71	235.11	640.11	-455.05	783.10	-494.59	blk. It. 3
	1.5	193.58	79.29	287.93	-320.50	359.16	433.36	-114.23	555.54	-83.31	
	3	231.61	89.71	-16.28	-16.28	421.47	193.80	193.80	282.61	282.61	
26	0	231.61	89.71	-16.28	-16.28	421.47	193.80	193.80	282.61	282.61	blk. It. 3
	1.5	154.09	55.10	-320.50	287.93	273.06	-149.77	397.81	-140.02	498.83	
	3	40.11	9.24	-624.71	592.14	62.92	-526.14	569.03	-608.00	669.68	
27	0	40.11	9.24	-624.71	592.14	62.92	-526.14	569.03	-608.00	669.68	blk. It. 3
	1.5	-189.42	-81.65	-928.92	896.35	-357.96	-1006.51	636.23	-1225.70	690.83	
	3	-455.41	-183.80	-1233.13	1200.56	-840.57	-1519.69	670.64	-1788.76	666.62	
28	0	-163.25	-100.00	0	0	-355.90	-146.93	-146.93	-234.41	-234.41	blk. It. 3
	1.25	-70.06	-46.09	0	0	-157.83	-63.06	-63.06	-102.61	-102.61	
	2.5	0	0	0	0	0	0	0	0	0	
29	0	-307.41	-111.97	1171.76	-1029.24	-548.04	777.92	-1202.98	837.03	-1474.02	blk. It. 4
	1.5	-81.29	-34.25	896.63	-754.12	-152.35	733.81	-751.86	834.53	-898.75	
	3	108.38	32.21	621.51	-478.99	181.60	656.90	-333.55	786.68	-368.85	

Lanjutan Tabel 4.11.a Momen Rencana Balok Portal As 9-10 (K=2)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD+1.6ML (KNm)	0.9(MD+ME) (KNm)		1.05(MD+0.6ML+ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
30	0	108.38	32.21	621.51	-478.99	181.60	656.90	-333.55	786.68	-368.85	blk. lt. 4
	1.5	182.49	53.65	346.38	-203.87	304.83	475.98	-19.24	589.11	11.35	
	3	220.15	63.84	71.26	71.26	366.31	262.26	262.26	346.19	346.19	
31	0	220.15	63.84	-71.26	71.26	366.31	134.00	262.26	196.55	346.19	blk. lt. 4
	1.5	142.24	29.00	-203.87	346.38	217.08	-55.46	439.76	-46.44	531.32	
	3	27.89	-17.10	-478.99	621.51	6.11	-405.99	584.46	-484.43	671.10	
32	0	27.89	-17.10	-478.99	621.51	6.11	-405.99	584.46	-484.43	671.10	blk. lt. 4
	1.5	-202.02	-108.22	-754.12	896.63	-415.58	-860.52	625.15	-1072.12	661.16	
	3	-468.39	-210.59	-1029.24	1171.76	-899.01	-1347.86	633.03	-1605.18	605.87	
33	0	-163.25	-100.00	0	0	-355.90	-146.93	-146.93	-234.41	-234.41	blk. lt. 4
	1.25	-70.06	-46.09	0	0	-157.83	-63.06	-63.06	-102.61	-102.61	
	2.5	0	0	0	0	0	0	0	0	0	
34	0	0	0	0	0	0	0	0	0	0	blk. atap
	1.75	-86.94	-91.88	0	0	-251.33	-78.25	-78.25	-149.17	-149.17	
	3.5	-217.98	-202.13	0	0	-584.98	-196.18	-196.18	-356.22	-356.22	
35	0	-217.98	-202.13	0	0	-584.98	-196.18	-196.18	-356.22	-356.22	blk. atap
	1.75	-86.94	-91.88	0	0	-251.33	-78.25	-78.25	-149.17	-149.17	
	3.5	0	0	0	0	0	0	0	0	0	



Tabel 4.11.b Gaya Geser Rencana Balok Portal As.9-10 (K=2)

Elm	Jarak (m)	V_{DB} (KN)	V_{LB} (KN)	V_{EB} (KN)	$V_{UB}=1,05(V_{DB} + V_{LB} + 4/2 \cdot V_{EB})$ (KN)	KET
14	0	166.94	58.04	96.28	405.03	Bik LT.1
	1.5	142.64	50.54	96.28	371.64	
	3	118.34	43.04	96.28	292.61	
15	0	65.60	20.52	96.28	259.22	Bik LT.1
	1.5	41.30	13.02	96.28	225.83	
	3	17.00	5.52	96.28	257.58	
16	0	35.75	17.00	96.28	290.97	Bik LT.1
	1.5	60.05	24.50	96.28	324.36	
	3	84.35	32.00	96.28	403.38	
17	0	137.09	54.52	96.28	436.77	Bik LT.1
	1.5	161.39	62.02	96.28	470.16	
	3	185.69	69.52	96.28	136.55	
18	0	83.80	46.25	0	110.57	Bik LT.1
	1.25	65.30	40.00	0	84.58	
	2.5	46.80	33.75	0	467.65	
19	0	164.04	56.26	112.54	434.26	Bik LT.2
	1.5	139.74	48.76	112.54	400.87	
	3	115.44	41.26	112.54	321.85	
20	0	62.70	18.74	112.54	288.46	Bik LT.2
	1.5	38.40	11.24	112.54	255.07	
	3	14.10	3.74	112.54	296.63	
21	0	38.64	18.78	112.54	330.02	Bik LT.2
	1.5	62.94	26.28	112.54	363.41	
	3	87.24	33.78	112.54	442.43	
22	0	139.98	56.30	112.54	509.21	Bik LT.2
	1.5	164.28	63.80	112.54	475.82	
	3	188.58	71.30	112.54	136.55	
23	0	83.80	46.25	0	110.57	Bik LT.2
	1.25	65.30	40.00	0	84.58	
	2.5	46.80	33.75	0	442.75	
24	0	163.15	55.72	101.40	409.36	Bik LT.3
	1.5	138.85	48.22	101.40	375.97	
	3	114.55	40.72	101.40	296.95	
25	0	61.81	18.20	101.40	263.56	Bik LT.3
	1.5	37.51	10.70	101.40	230.17	
	3	13.21	3.20	101.40	274.74	
26	0	39.53	19.33	101.40	308.13	Bik LT.3
	1.5	63.83	26.83	101.40	341.52	
	3	88.13	34.33	101.40	420.55	
27	0	140.87	56.85	101.40	453.94	Bik LT.3
	1.5	165.17	64.35	101.40	487.33	
	3	189.47	71.85	101.40	136.55	
28	0	83.80	46.25	0	110.57	Bik LT.3
	1.25	65.30	40.00	0	84.58	
	2.5	46.80	33.75	0	405.03	
29	0	162.90	55.56	91.71	421.97	Bik LT.4
	1.5	138.60	48.06	91.71	388.58	
	3	114.30	40.56	91.71	355.19	
30	0	61.56	18.04	91.71	276.17	Bik LT.4
	1.5	37.26	10.54	91.71	242.78	
	3	12.96	3.04	91.71	209.39	
31	0	39.79	19.48	91.71	254.82	Bik LT.4
	1.5	64.09	26.98	91.71	288.21	
	3	88.39	34.48	91.71	321.60	
32	0	141.13	57.00	91.71	400.63	Bik LT.4
	1.5	165.43	64.50	91.71	434.02	
	3	189.73	72.00	91.71	467.41	
33	0	83.80	46.25	0	136.55	Bik LT.4
	1.25	65.30	40.00	0	110.57	
	2.5	46.80	33.75	0	84.58	

Tabel 4.11.c Momen Rencana Kolom Portal As.9-10 (K=2)

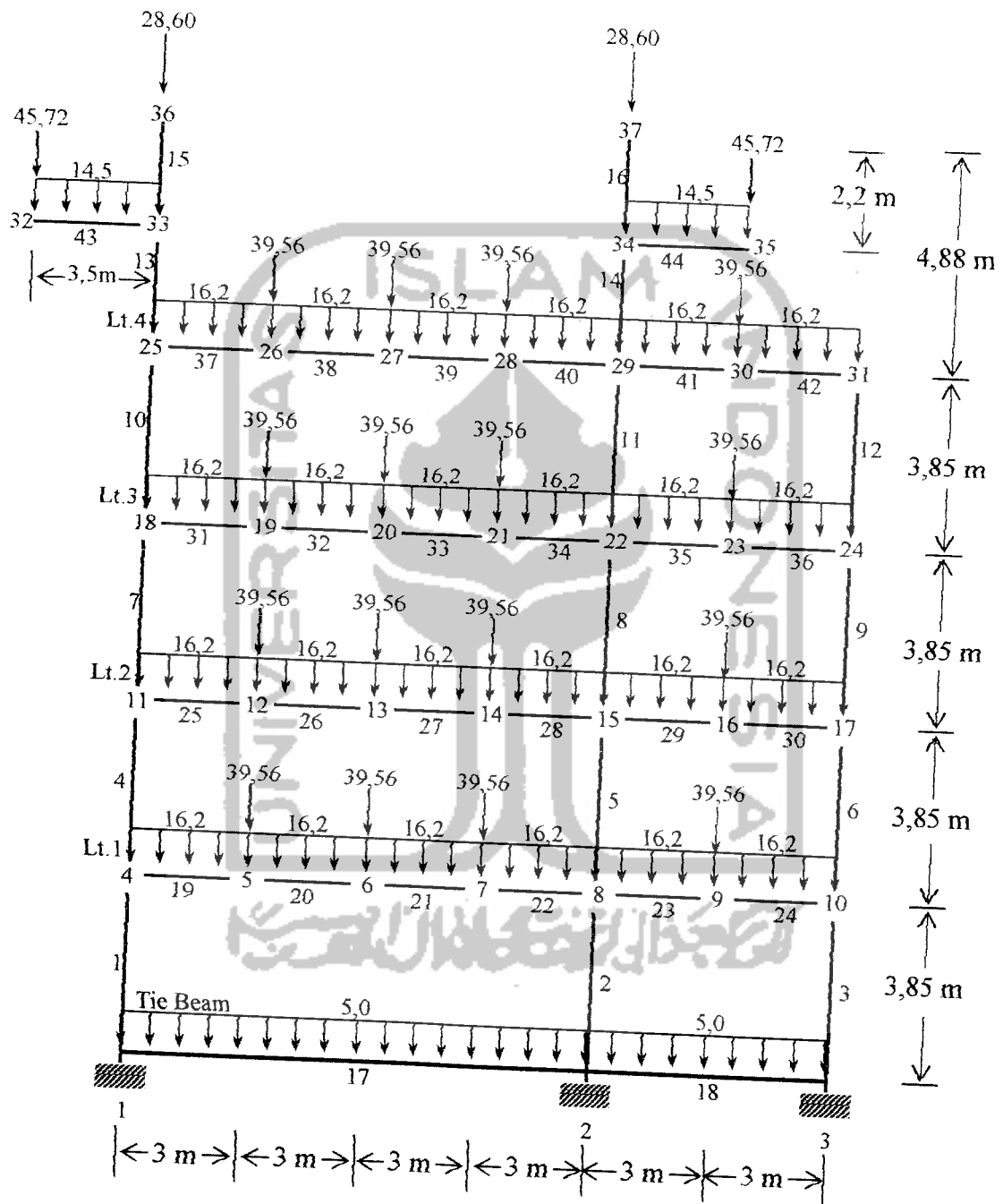
Elm	Jarak (m)	$M_{D,k}$ (KNm)	$M_{L,k}$ (KNm)	M_{Ek} (KNm)	M_{Eka} (KNm)	$M_{U,k} =$ $1.05(M_{D,k} + M_{L,k} \pm \omega_d M_{E,k})$ (KNm)	KETERANGAN
1	0	24.65	-3.37	-840.60	837.56	1165.61	KOL BASEMEN
	1.925	-69.35	-32.87	-375.77	376.81	407.01	
	3.85	-163.35	-62.37	89.06	-83.94	-1291.32	
2	0	-98.92	-42.08	-837.56	840.60	999.37	KOL BASEMEN
	1.925	-4.92	-12.58	-376.81	375.77	494.55	
	3.85	89.07	16.92	83.94	-89.06	-458.27	
3	0	155.99	42.40	-488.34	493.99	882.61	KOLOM LT.1
	1.925	-3.11	-2.46	-68.20	60.22	260.37	
	3.85	-162.21	-47.33	351.93	-373.56	-922.39	
4	0	-179.51	-56.77	-493.99	488.34	418.49	KOLOM LT.1
	1.925	-20.41	-11.90	-60.22	68.20	690.14	
	3.85	138.69	32.96	373.56	-351.93	-300.15	
5	0	144.18	49.21	-325.07	299.86	612.37	KOLOM LT.2
	1.925	-10.79	-14.43	62.11	-23.55	357.26	
	3.85	-165.76	-78.08	449.30	-346.96	-729.63	
6	0	-151.74	-53.82	-299.86	325.07	227.88	KOLOM LT.2
	1.925	3.23	9.83	23.55	-62.11	716.86	
	3.85	158.20	73.48	346.96	-449.30	-370.03	
7	0	131.69	8.94	-150.98	269.61	515.68	KOLOM LT.3
	1.925	21.13	49.55	-47.46	-122.51	-93.01	
	3.85	-89.43	90.16	56.06	-514.62	-701.69	
8	0	-133.96	-10.32	-269.61	150.98	89.19	KOLOM LT.3
	1.925	-23.40	-50.92	122.51	47.46	697.87	
	3.85	87.16	-91.53	514.62	-56.06	-282.09	
9	0	217.98	202.13	-529.82	0	441.12	KOLOM LT.4
	1.34	217.98	202.13	-384.34	0	441.12	
	2.68	217.98	202.13	-238.85	0	441.12	
10	0	-217.98	-202.13	0	529.82	282.09	KOLOM LT.4
	1.34	-217.98	-202.13	0	384.34	-441.12	
	2.68	-217.98	-202.13	0	238.85	-326.03	
11	0	0	0	-238.85	0	-163.02	KOLOM LT.4
	1.1	0	0	-119.43	0	0	
	2.2	0	0	0	0	0	
12	0	0	0	0	238.85	326.03	KOLOM LT.4
	1.1	0	0	0	119.43	163.02	
	2.2	0	0	0	0	0	

Tabel 4.11.d Gaya Geser Rencana Kolom As.9-10 (K=2)

Elm	Jarak (m)	$V_{D,k}$ (KN)	$V_{L,k}$ (KN)	$V_{E,k}$ (KN)	$V_{U,k} =$ $1,05(V_{D,k} + V_{L,k} + \omega_d \cdot V_{E,k})$ (KN)	KET
1	0	-48.83	-15.32	241.47	262.25	KOL BASEMEN
	1.925	-48.83	-15.32	241.47	262.25	
	3.85	-48.83	-15.32	241.47	262.25	
2	0	48.83	15.32	239.35	394.07	KOL BASEMEN
	1.925	48.83	15.32	239.35	394.07	
	3.85	48.83	15.32	239.35	394.07	
3	0	-82.65	-23.31	218.25	186.65	KOLOM LT.1
	1.925	-82.65	-23.31	218.25	186.65	
	3.85	-82.65	-23.31	218.25	186.65	
4	0	82.65	23.31	225.34	418.85	KOLOM LT.1
	1.925	82.65	23.31	225.34	418.85	
	3.85	82.65	23.31	225.34	418.85	
5	0	-80.50	-33.06	201.14	155.32	KOLOM LT.2
	1.925	-80.50	-33.06	201.14	155.32	
	3.85	-80.50	-33.06	201.14	155.32	
6	0	80.50	33.06	168.00	348.56	KOLOM LT.2
	1.925	80.50	33.06	168.00	348.56	
	3.85	80.50	33.06	168.00	348.56	
7	0	-57.43	21.10	53.78	35.26	KOLOM LT.3
	1.925	-57.43	21.10	53.78	35.26	
	3.85	-57.43	21.10	53.78	35.26	
8	0	57.43	-21.10	203.70	316.20	KOLOM LT.3
	1.925	57.43	-21.10	203.70	316.20	
	3.85	57.43	-21.10	203.70	316.20	
9	0	0	0	108.57	148.20	KOLOM LT.4
	1.34	0	0	108.57	148.20	
	2.68	0	0	108.57	148.20	
10	0	0	0	0	0	KOLOM LT.4
	1.34	0	0	0	0	
	2.68	0	0	0	0	
11	0	0	0	0	0	KOLOM LT.4
	1.1	0	0	0	0	
	2.2	0	0	0	0	
12	0	0	0	108.57	148.20	KOLOM LT.4
	1.1	0	0	108.57	148.20	
	2.2	0	0	108.57	148.20	

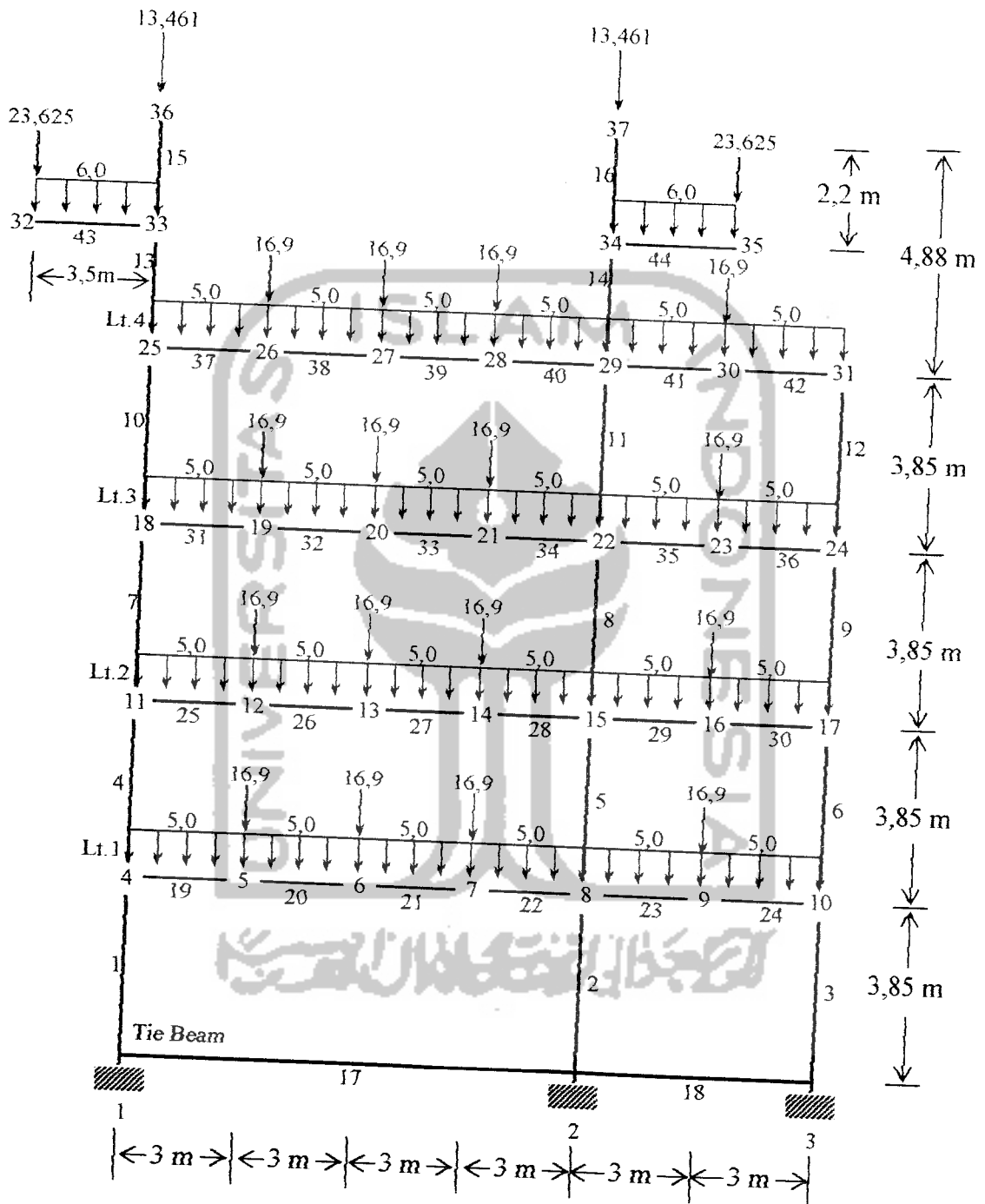
Pembebanan Portal As 6 dan As 7

a. Beban mati



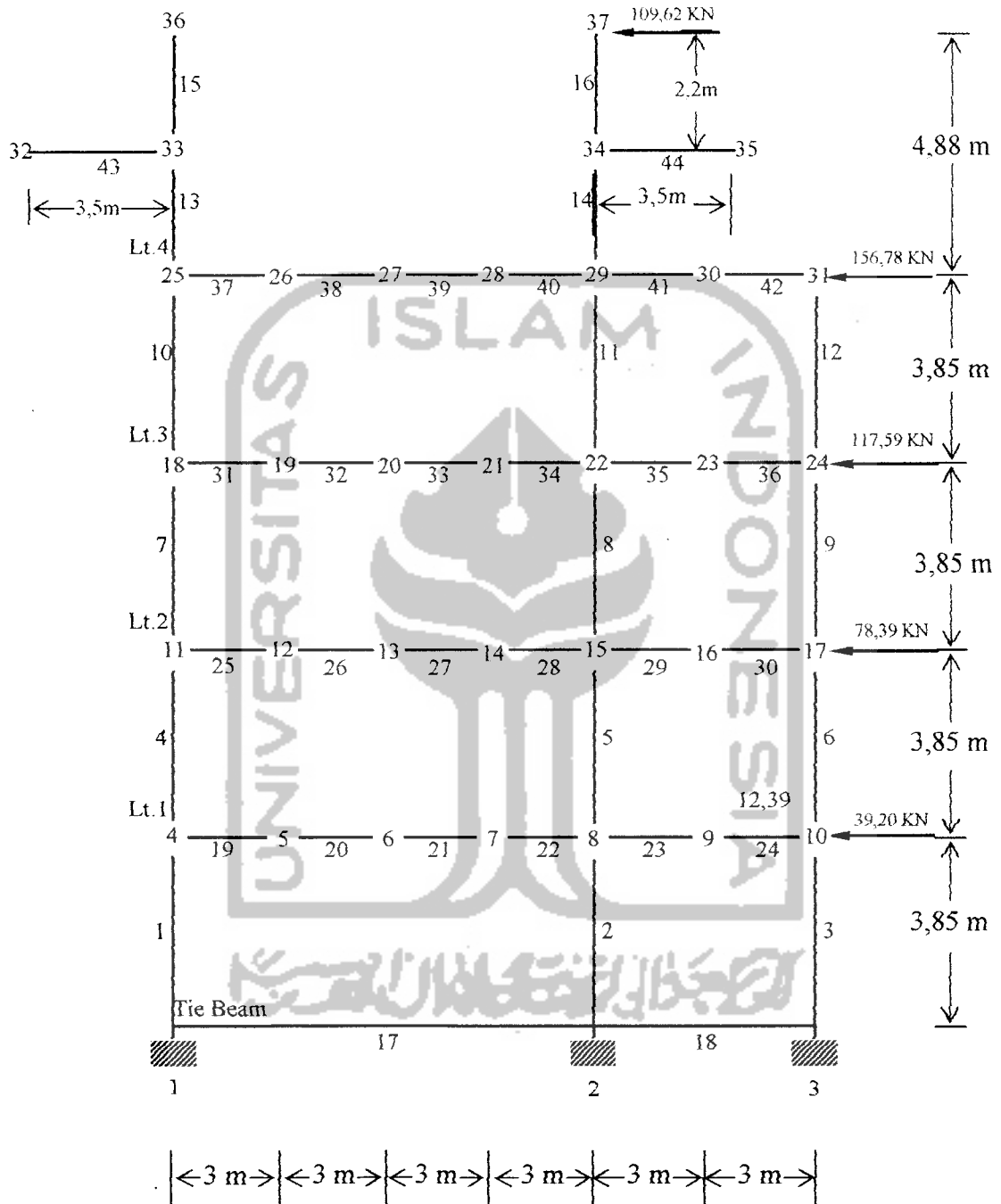
Gambar 4.12.a Pembebanan mati Portal As 6 dan As 7

b. Beban hidup



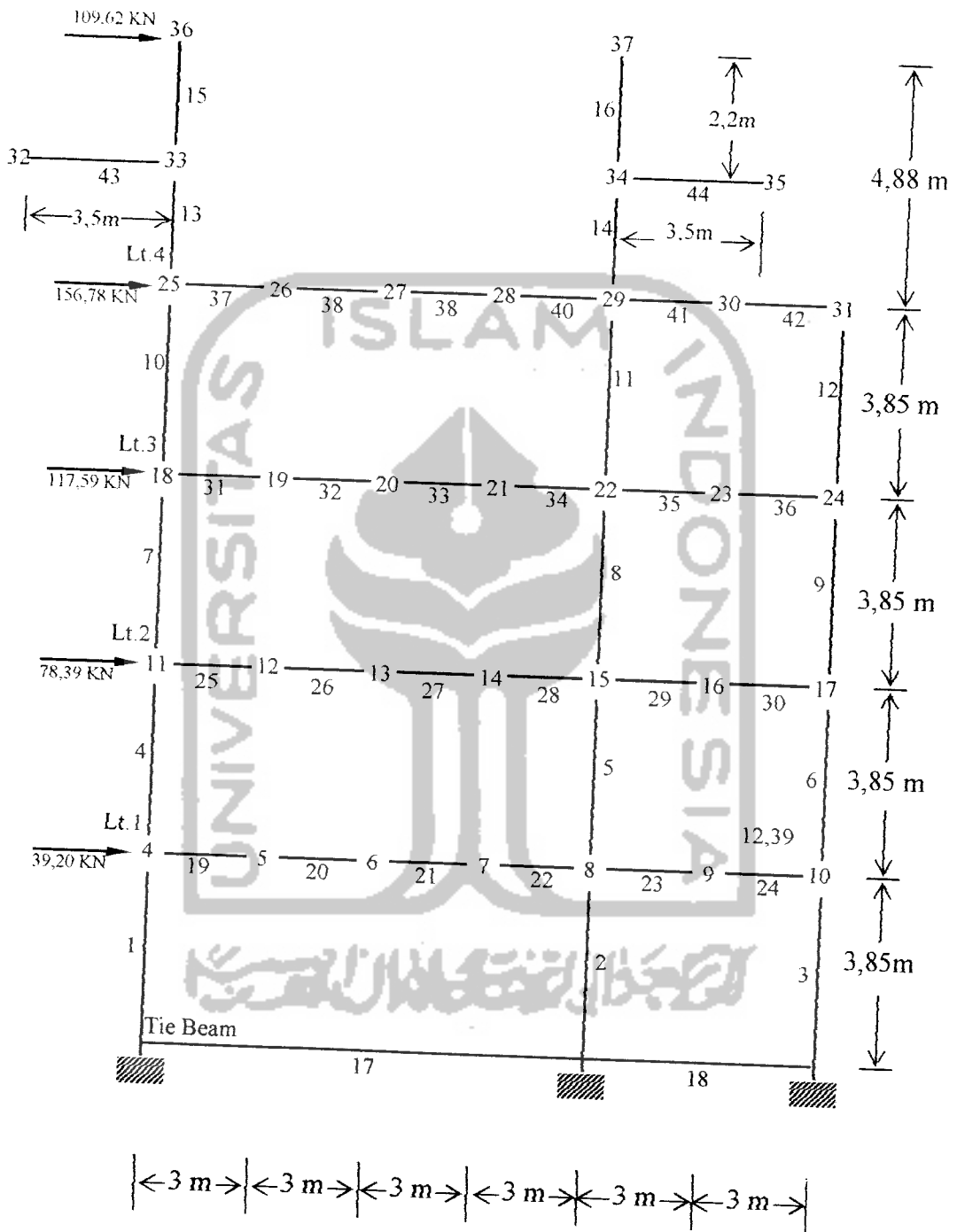
Gambar 4.12.b Pembebanan hidup Portal As 6 dan As 7

c. Beban gempa kanan



Gambar 4.12.c Pembebanan gempa kanan Portal As 6 dan As 7

d. Beban gempa kiri



Gambar 4.12.d Pembebanan gempa kiri Portal As 6 dan As 7

a Beban mati

- Beban terdistribusi merata elemen 19 s/d 42

$$\text{Lantai} : 2.2/3.1,5.4,26 = 8,52 \text{ KN/m}$$

$$\text{Balok} : \quad 0,4.0,8.24 = 7,68 \text{ KN/m}$$

$$W_{D1} = 16,2 \text{ KN/m}$$

- Beban terdistribusi merata elemen 43 & 44

$$\text{Lantai} : 2.2/3.1,75.0,12.24 = 5,86 \text{ KN/m}$$

$$\text{Balok} : \quad 0,4.0,8.24 = 7,68 \text{ KN/m}$$

$$W_{D2} = 14,50 \text{ KN/m}$$

- Beban terpusat nodal 5 s/d 30

$$P_{D1} \text{ balok anak} : 1,5.(2.1/2.3.1,5).4,26 + 0,25.0,40.24.4.5 = 39,56 \text{ KN}$$

- Beban terpusat nodal 32 & 35

$$P_{D2} \text{ R perletakan atap} = 28,60 \text{ KN}$$

- Beban terpusat nodal 36 & 37

$$P_{D3} \text{ penutup L1 4} : 1,75.(1,5+3).0,12.24 + 0,4.0,4.24.6 = 45,72 \text{ KN}$$

b Beban hidup

- Beban terdistribusi merata elemen 19 s/d 42

$$W_{L1} = 2.2/3.1,5.2,5 = 5,0 \text{ KN/m}$$

- Beban terdistribusi merata elemen 43 & 44

$$W_{L2} = 2.2/3.1,5.3 = 6,0 \text{ KN/m}$$

- Beban terpusat 5 s/d 30

$$P_{L1} \text{ R balok anak} : \quad 1,5.(2.1/2.3.1,5).2,5 = 10,622 \text{ KN}$$

- Beban terpusat nodal 32 & 35

$$P_{1.2 \text{ R perletakan atap}} : 7,166 + 6,304 = 13,461 \text{ KN}$$

- Beban terpusat nodal 36 & 37

$$P_{1.3 \text{ penutup Lt.4}} : 1,75 \cdot (1,5+3) \cdot 3 = 23,625 \text{ KN}$$

c Beban gempa

$$\text{Reaksi atap} : 2 \cdot 28,6 = 57,20 \text{ KN}$$

$$\text{Plat penutup Lt.4} : 2,3,5,6,0,12,24 = 120,96 \text{ KN}$$

$$\text{Kolom} : 2,0,45,0,7,4,88,24 = 72,58 \text{ KN}$$

$$\text{Balok} : 2,0,25,0,4,24,6 = 46,08 \text{ KN}$$

$$\text{Balok} : 4,0,4,0,8,24,6 = 184,32 \text{ KN}$$

$$\text{Beban hidup tereduksi} : 0,6 \cdot (2,3,5,6,3) = 75,60 \text{ KN}$$

$$\text{Dibulatkan } W_{\text{atap}} = 559,74 \text{ KN}$$

- Berat lantai 4 (tipikal dengan lt. 3, 2,1)

$$\text{Lantai} : 6,18,0,12,24 = 311,04 \text{ KN}$$

$$\text{Kolom} : (2,0,45,0,7 + 0,45,0,45) \cdot 3,85,24 = 76,92 \text{ KN}$$

$$\text{Balok} : 11,0,25,0,4,24,6 = 158,40 \text{ KN}$$

$$\text{Balok} : 0,4,0,8,24,30 = 230,40 \text{ KN}$$

$$\text{Tembok} : 2,5,3,85,12 = 115,50 \text{ KN}$$

$$\text{B.hidup tereduksi} : 0,6(6,18,2,5) = 162,00 \text{ KN}$$

$$\text{Dibulatkan } W_4 = 1054,3 \text{ KN}$$

$$W_{\text{Total}} : 559,74 + 4 \cdot 1054,3 = 4776,94 \text{ KN}$$

Gaya geser gempa untuk tingkat daktilitas penuh ($K=1$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.1,0.4776,94 = 501,58 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap	559,74	20,28	11351,53	109,62
4	1054,3	15,40	16236,22	156,78
3	1054,3	11,55	12177,17	117,59
2	1054,3	7,70	8118,11	78,39
1	1054,3	3,85	4059,06	39,20
			51942,08	501,58

Gaya geser gempa untuk tingkat daktilitas terbatas ($K = 2$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.2,0.4887,4 = 1003,16 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap	559,74	20,28	11351,53	219,14
4	1054,3	15,40	16236,22	313,56
3	1054,3	11,55	12177,17	235,18
2	1054,3	7,70	8118,11	156,78
1	1054,3	3,85	4059,06	78,40
			54128,59	1003,16

Tabel 4.12.a Momen Rencana Balok Portal As 6,7-8 (K=1)

Elm	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD+1.6ML (KNm)	0.9.(MD + ME) (KNm)		1.05.(MD+0.6ML + ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
19	0	-297.1	-98.40	202.10	-200.29	-513.97	-85.51	-447.66	-161.75	-625.59	blk. tt.1
	1.5	-105.5	-35.30	151.90	-150.36	-183.16	41.71	-230.33	26.42	-305.78	
	3	49.54	16.55	101.69	-100.42	85.92	136.11	-45.79	169.22	-36.05	
	0	49.54	16.55	101.69	-100.42	85.92	136.11	-45.79	169.22	-36.05	
20	0	164.12	54.62	1.27	-0.54	284.34	148.85	147.22	211.33	121.58	blk. tt.1
	1.5	123.59	40.85	-48.94	49.40	213.67	67.19	155.69	104.13	224.53	
	3	46.61	15.83	-99.14	99.33	81.27	-47.28	131.35	-45.18	169.87	
	0	46.61	15.83	-99.14	99.33	81.27	-47.28	131.35	-45.18	169.87	
21	0	-109.9	-36.37	-149.35	149.27	-190.14	-233.38	35.38	-295.19	3.09	blk. tt.1
	1.5	-75.99	-24.79	271.02	-272.29	-130.85	175.53	-313.45	189.16	-391.72	
	3	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
	0	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
22	0	57.11	19.14	48.81	-48.78	99.16	95.32	7.49	123.27	28.84	blk. tt.1
	1.5	25.84	8.30	-62.30	62.98	44.29	95.32	7.49	123.27	28.84	
	3	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
	0	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
23	0	-297.6	-98.82	273.97	-272.13	-515.26	-21.29	-512.78	-87.10	-702.00	blk. tt.2
	1.5	-105.9	-35.60	205.81	-204.49	-184.11	89.87	-279.40	82.42	-363.35	
	3	49.28	16.36	137.64	-136.84	85.30	168.23	-78.81	206.57	-74.77	
	0	49.28	16.36	137.64	-136.84	85.30	168.23	-78.81	206.57	-74.77	
24	0	124.92	41.13	69.48	-69.20	215.72	174.96	50.15	230.03	101.70	blk. tt.2
	1.5	123.71	54.66	1.31	-1.55	284.39	146.88	146.30	208.13	228.08	
	3	46.86	16.10	-135.02	133.74	81.99	-79.35	162.54	-82.43	206.53	
	0	46.86	16.10	-135.02	133.74	81.99	-79.35	162.54	-82.43	206.53	
25	0	-109.5	-35.99	-209.19	201.39	-189.09	-291.49	82.63	-351.09	58.61	blk. #.2
	1.5	-75.99	-24.79	271.02	-272.29	-130.85	175.53	-313.45	189.16	-391.72	
	3	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
	0	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
26	0	57.11	19.14	48.81	-48.78	99.16	95.32	7.49	123.27	28.84	blk. tt.2
	1.5	25.84	8.30	-62.30	62.98	44.29	95.32	7.49	123.27	28.84	
	3	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
	0	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
27	0	-296.9	-99.01	284.81	-281.19	-514.71	-10.90	-520.30	-75.09	-710.97	blk. tt.3
	1.5	-105.2	-35.64	213.86	-211.47	-183.32	97.75	-285.05	91.59	-369.98	
	3	49.97	16.48	142.90	-141.74	86.33	173.58	-82.59	212.90	-79.06	
	0	49.97	16.48	142.90	-141.74	86.33	173.58	-82.59	212.90	-79.06	
28	0	125.60	41.41	71.95	-72.01	216.98	177.79	48.23	233.51	99.75	blk. tt.3
	1.5	124.38	41.60	-69.96	67.44	215.81	48.98	172.64	83.34	245.08	
	3	47.52	16.85	-140.91	137.17	83.97	-84.05	166.22	-87.45	211.61	
	0	47.52	16.85	-140.91	137.17	83.97	-84.05	166.22	-87.45	211.61	
29	0	-108.9	-35.09	-211.87	206.89	-186.86	-288.72	88.16	-358.95	66.02	blk. tt.3
	1.5	-75.99	-24.79	271.02	-272.29	-130.85	175.53	-313.45	189.16	-391.72	
	3	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
	0	8.78	2.80	159.91	-160.53	15.02	151.82	-136.57	178.89	-156.39	
30	0	57.11	19.14	48.81	-48.78	99.16	95.32	7.49	123.27	28.84	blk. tt.3
	1.5	25.84	8.30	-62.30	62.98	44.29	95.32	7.49	123.27	28.84	
	3	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
	0	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
31	0	-300.2	-101.7	303.66	-274.68	-523.12	3.22	-517.46	-60.34	-710.54	blk. tt.4
	1.5	-108.6	-38.34	231.48	-204.75	-191.69	110.57	-292.03	104.85	-369.30	
	3	46.58	13.81	159.09	-134.83	78.00	185.11	-79.42	224.66	-78.16	
	0	46.58	13.81	159.09	-134.83	78.00	185.11	-79.42	224.66	-78.16	
32	0	122.20	38.78	86.70	-64.91	208.68	188.01	51.56	243.78	100.87	blk. tt.4
	1.5	120.95	39.03	-58.07	74.93	207.59	56.59	176.30	90.61	246.66	
	3	44.08	14.32	-130.46	144.86	75.80	-77.74	170.04	-81.68	213.42	
	0	44.08	14.32	-130.46	144.86	75.80	-77.74	170.04	-81.68	213.42	
33	0	-112.3	-37.58	-202.85	214.78	-194.99	-283.71	92.16	-354.67	68.06	blk. tt.4
	1.5	-77.77	-11.95	286.79	-337.93	-76.44	215.12	-347.13	243.44	-417.53	
	3	30.02	12.42	179.58	-215.36	55.89	188.64	-166.81	227.90	-181.57	
	0	30.02	12.42	179.58	-215.36	55.89	188.64	-166.81	227.90	-181.57	
34	0	57.11	19.14	48.81	-48.78	99.16	95.32	7.49	123.27	28.84	blk. tt.4
	1.5	25.84	8.30	-62.30	62.98	44.29	95.32	7.49	123.27	28.84	
	3	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
	0	-41.88	-13.79	-173.41	174.73	-72.32	-193.76	119.57	-234.74	125.02	
35	0	0	0	0	0	0	0	0	0	0	blk. atap
	1.75	-102.2	-50.53	0	0	-203.50	-91.99	-91.99	-139.16	-160.38	
	3.5	-248.8	-119.4	0	0	-489.70	-223.95	-223.95	-336.52	-386.68	
	0	-248.8	-119.4	0	0	-489.70	-223.95	-223.95	-336.52	-386.68	
36	0	0	0	0	0	0	0	0	0	0	blk. atap
	1.75	-102.2	-50.53	0	0	-203.50	-91.99	-91.99	-139.16	-160.38	
	3.5	-248.8	-119.4	0	0	-489.70	-223.95	-223.95	-336.52	-386.68	
	0	-248.8	-119.4	0	0	-489.70	-223.95	-223.95	-336.52	-386.68	

Tabel 4.12.b Gaya Geser Rencana Balok Portal As.6,7-8 (K=1)

Elm	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	MKAP,B	M'KAP,B	$V_{UB} = 0,7(MKAP,B + M'KAP,B) / Ln + 1,05.VG,B$ (KN)	Vub terpakai	VUB maks (KN)	KET
19	0	139.85	45.82	33.29	185.67	911.86	426.37				blk. tt. 1
	1.5	115.55	38.32	33.29	153.87	911.86	426.37	277.85	261.98	334.77	
	3	91.25	30.82	33.29	122.07	911.86	426.37	244.46	228.59	301.38	
20	0	62.49	20.19	33.29	82.68	911.86	426.37				blk. tt. 1
	1.5	38.19	12.69	33.29	50.88	911.86	426.37	169.72	153.85	226.64	
	3	13.89	5.19	33.29	19.08	911.86	426.37	136.33	120.46	193.25	
21	0	14.87	5.43	33.29	20.30	911.86	426.37				blk. tt. 1
	1.5	39.17	12.93	33.29	52.10	911.86	426.37	104.21	88.34	161.14	
	3	63.47	20.43	33.29	83.90	911.86	426.37	137.60	121.73	194.53	
22	0	92.23	31.05	33.29	123.28	911.86	426.37				blk. tt. 1
	1.5	116.53	38.55	33.29	155.08	911.86	426.37	212.34	196.47	269.27	
	3	140.83	46.05	33.29	186.88	911.86	426.37	245.73	229.86	302.66	
23	0	68.67	22.15	74.50	90.81	911.86	426.37				blk. tt. 1
	1.5	44.37	14.65	74.50	59.01	911.86	426.37	178.25	162.38	406.27	
	3	20.07	7.14	74.50	27.21	911.86	426.37	144.86	128.99	374.88	
24	0	8.69	3.48	74.50	12.17	911.86	426.37				blk. tt. 1
	1.5	33.00	10.98	74.50	43.97	911.86	426.37	111.47	95.60	341.49	
	3	57.30	18.48	74.50	75.77	911.86	426.37	95.68	79.81	325.70	
25	0	139.94	45.89	45.10	185.83	911.86	426.37				blk. tt. 2
	1.5	115.64	38.39	45.10	154.03	911.86	426.37	162.46	146.59	392.48	
	3	91.34	30.89	45.10	122.23	911.86	426.37	278.02	262.15	384.53	
26	0	62.58	20.27	45.10	82.84	911.86	426.37				blk. tt. 2
	1.5	38.28	12.77	45.10	51.04	911.86	426.37	211.24	195.37	317.74	
	3	13.98	5.27	45.10	19.24	911.86	426.37	169.89	154.02	276.39	
27	0	14.78	5.35	45.10	20.14	911.86	426.37				blk. tt. 2
	1.5	39.08	12.85	45.10	51.94	911.86	426.37	136.50	120.63	243.00	
	3	63.38	20.35	45.10	83.74	911.86	426.37	103.11	87.24	209.61	
28	0	92.14	30.98	45.10	123.12	911.86	426.37				blk. tt. 2
	1.5	116.44	38.48	45.10	154.92	911.86	426.37	104.04	88.17	210.55	
	3	140.74	45.98	45.10	186.72	911.86	426.37	137.43	121.56	243.94	
29	0	63.99	20.83	99.90	84.82	911.86	426.37				blk. tt. 2
	1.5	39.69	13.33	99.90	53.02	911.86	426.37	170.82	154.95	277.33	
	3	15.39	5.83	99.90	21.22	911.86	426.37	212.17	196.30	318.68	
30	0	13.37	4.80	99.90	18.16	911.86	426.37				blk. tt. 2
	1.5	37.67	12.30	99.90	49.96	911.86	426.37	245.56	229.69	352.07	
	3	61.97	19.80	99.90	81.76	911.86	426.37	278.95	263.06	365.46	
31	0	139.93	45.99	46.48	185.92	1061.95	568.49				blk. tt. 3
	1.5	115.63	38.49	46.48	154.12	1061.95	568.49	171.96	156.09	508.62	
	3	91.33	30.99	46.48	122.33	1061.95	568.49	138.57	122.70	475.23	
32	0	62.57	20.37	46.48	82.94	1061.95	568.49				blk. tt. 3
	1.5	38.27	12.87	46.48	51.14	1061.95	568.49	105.18	89.31	441.84	
	3	13.97	5.37	46.48	19.34	1061.95	568.49	101.97	86.10	438.63	
33	0	14.79	5.25	46.48	20.04	1061.95	568.49				blk. tt. 3
	1.5	39.09	12.75	46.48	51.84	1061.95	568.49	135.36	119.49	472.02	
	3	63.39	20.25	46.48	83.64	1061.95	568.49	168.75	152.66	505.41	
34	0	92.15	30.87	46.48	123.02	1061.95	568.49				blk. tt. 3
	1.5	116.45	38.37	46.48	154.82	1061.95	568.49	296.22	276.88	390.45	
	3	140.75	45.87	46.48	186.62	1061.95	568.49	262.83	243.49	357.06	
								229.44	210.10	323.67	
								188.09	168.75	282.32	
								154.70	135.36	248.93	
								121.31	101.97	215.54	
								122.04	102.70	216.27	
								155.43	136.09	249.66	
								188.82	169.48	283.05	
								230.17	210.83	324.40	
								263.56	244.22	357.80	
								200.62	181.28	294.85	

Lanjutan Tabel 4.12.b Gaya Geser Rencana Balok Portal As.6,7-8 (K=1)

Elm	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	MKAP,B	M'KAP,B	VUB= 0,7(MKAP,B + M'KAP,B) /Ln+ 1,05 VG,B (KN)	Vub terpakai	Vub maks (KN)	KET
35	0	79.04	19.97	104.20	99.01	1061.95	568.49				blk. lt. 3
	1.5	54.74	12.47	104.20	67.21	1061.95	568.49	204.96	185.62	541.60	
	3	30.44	4.97	104.20	35.41	1061.95	568.49	171.57	152.23	508.21	
36	0	1.68	-5.65	104.20	-3.97	1061.95	568.49	138.18	118.84	474.82	blk. lt. 3
	1.5	51.22	13.15	104.20	64.37	1061.95	568.49	96.83	77.49	433.47	
	3	75.52	20.65	104.20	96.17	1061.95	568.49	168.59	149.25	505.23	
37	0	168.52	46.02	46.62	214.54	1061.95	568.49	201.98	182.64	538.62	blk. lt. 4
	1.5	115.62	38.52	46.62	154.14	1061.95	568.49	326.26	306.92	421.05	
	3	91.32	31.02	46.62	122.34	1061.95	568.49	262.84	243.50	357.63	
38	0	62.56	20.40	46.62	82.96	1061.95	568.49	229.46	210.12	324.24	blk. lt. 4
	1.5	38.26	12.90	46.62	51.16	1061.95	568.49	188.11	168.77	282.89	
	3	13.96	5.40	46.62	19.36	1061.95	568.49	154.72	135.38	249.50	
39	0	14.80	5.23	46.62	9.57	1061.95	568.49	121.33	101.99	216.11	blk. lt. 4
	1.5	39.10	12.73	46.62	51.83	1061.95	568.49	111.05	91.71	205.83	
	3	63.40	20.23	46.62	83.63	1061.95	568.49	155.42	136.08	250.20	
40	0	92.16	30.85	46.62	123.01	1061.95	568.49	188.81	169.47	283.59	blk. lt. 4
	1.5	116.46	38.35	46.62	154.81	1061.95	568.49	230.16	210.82	324.94	
	3	140.76	45.85	46.62	186.61	1061.95	568.49	263.55	244.21	359.33	
41	0	64.01	19.99	81.71	84.00	1061.95	568.49	296.94	277.60	391.72	blk. lt. 4
	1.5	39.71	12.49	81.71	52.20	1061.95	568.49	189.20	169.86	431.40	
	3	15.41	4.99	81.71	20.40	1061.95	568.49	155.81	136.47	398.01	
42	0	13.35	5.63	81.71	7.72	1061.95	568.49	122.43	103.09	364.62	blk. lt. 4
	1.5	37.65	13.13	81.71	50.78	1061.95	568.49	109.11	89.77	351.30	
	3	61.95	20.63	81.71	82.58	1061.95	568.49	154.32	134.98	396.51	
43	0	45.72	23.63	0	69.35	0	0	187.71	168.37	429.90	blk. atap
	1.75	71.10	34.13	0	105.22	0	0	72.81	72.81	72.81	
	3.5	96.47	44.63	0	141.10	0	0	110.48	110.48	110.48	
44	0	96.47	44.63	0	141.10	0	0	148.15	148.15	148.15	blk. atap
	1.75	71.10	34.13	0	105.22	0	0	148.15	148.15	148.15	
	3.5	45.72	23.63	0	69.35	0	0	110.48	110.48	110.48	
								72.81	72.81	72.81	

Keterangan:

$$V_{u,b \text{ terpakai}} = \left[1,05 \cdot V_G - 0,7 \left(\frac{M_{kap,b} + M'_{Kap,b}}{l_n} \right) \right] + \frac{ln - d}{ln} \left[V_{u,b} - \left[1,05 \cdot V_G - 0,7 \left(\frac{M_{kap,b} + M'_{Kap,b}}{l_n} \right) \right] \right]$$

Tabel 4.12.c Momen Rencana Kolom Portal As.6,7-8 (K=1)

Elm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	MEki (KNm)	MEka (KNm)	MKAP,b kiri (KNm)	MKAP,b kanan (KNm)	M _{U,k}	M _{U,k} = 1.05 (MD,k+ML,k + 4/K.ME,k) (KNm)	KET
1	0	60.22	21.04	-1202.40	1177.00	0	911.86	556.16	5028.72	Kol.Basmen
	1.925	-45.85	-14.07	-799.38	786.14	0	911.86	556.16	3238.88	
	3.85	-151.91	-49.18	-396.40	395.29	0	911.86	556.16	1449.08	
2	0	-87.04	-27.92	-1272.30	1287.10	911.86	911.86	1112.33	-5464.37	Kol.Basmen
	1.925	5.89	2.87	-769.55	773.43	911.86	911.86	1112.33	3257.60	
	3.85	98.81	33.67	-266.75	259.77	911.86	911.86	1112.33	1230.14	
3	0	-9.48	-3.09	-65.02	66.40	911.86	0	556.16	265.66	Kol.Basmen
	1.925	3.66	1.22	-5.25	5.37	911.86	0	556.16	27.66	
	3.85	16.80	5.54	54.51	-55.67	911.86	0	556.16	252.39	
4	0	145.20	49.22	-598.50	595.58	0	911.86	556.16	2705.58	Kol.Lt.1
	1.925	-4.05	-0.75	-281.17	288.22	0	911.86	556.16	1205.48	
	3.85	-153.30	-50.72	36.16	-19.13	0	911.86	556.16	-294.57	
5	0	-128.17	-41.36	-737.33	731.27	1061.95	1061.95	1295.42	-3274.60	Kol.Lt.1
	1.925	-4.26	0.19	-280.33	284.59	1061.95	1061.95	1295.42	-1181.67	
	3.85	119.64	41.74	176.67	-202.09	1061.95	1061.95	1295.42	911.46	
6	0	-25.08	-8.25	-118.90	119.07	1061.95	0	647.71	-534.38	Kol.Lt.1
	1.925	0.26	0.17	-3.15	3.02	1061.95	0	647.71	13.14	
	3.85	25.61	8.59	112.61	-113.03	1061.95	0	647.71	508.87	
7	0	144.33	48.09	-237.82	253.00	0	1061.95	647.71	1264.65	Kol.Lt.2
	1.925	-12.00	-3.98	-40.34	41.15	0	1061.95	647.71	-186.21	
	3.85	-168.34	-56.06	157.14	-170.71	0	1061.95	647.71	-952.61	
8	0	-122.07	-37.28	-461.68	428.29	1061.95	1061.95	1295.42	1631.50	Kol.Lt.2
	1.925	3.85	6.15	-47.73	23.88	1061.95	1061.95	1295.42	-189.96	
	3.85	129.77	49.57	366.23	-380.53	1061.95	1061.95	1295.42	1726.48	
9	0	-29.07	-8.63	-129.13	125.00	1061.95	0	647.71	-581.93	Kol.Lt.2
	1.925	1.34	0.02	-1.38	2.09	1061.95	0	647.71	10.21	
	3.85	31.76	8.68	126.37	-120.82	1061.95	0	647.71	573.21	
10	0	128.58	42.94	-127.67	110.48	0	1061.95	647.71	644.11	Kol.Lt.3
	1.925	38.57	30.32	-179.38	-82.10	0	1061.95	647.71	-681.07	
	3.85	-51.45	17.70	-231.08	-274.67	0	1061.95	647.71	-1189.06	
11	0	-91.60	-30.51	-296.26	265.68	1061.95	1061.95	1295.42	-1372.50	Kol.Lt.3
	1.925	-41.45	-30.58	132.88	89.00	1061.95	1061.95	1295.42	482.46	
	3.85	8.69	-30.66	562.02	-87.68	1061.95	1061.95	1295.42	2337.42	
12	0	-38.15	-11.55	-128.71	134.77	1061.95	0	647.71	-592.76	Kol.Lt.3
	1.925	1.72	1.15	6.88	-8.79	1061.95	0	647.71	-33.90	
	3.85	41.59	13.85	142.06	-152.35	1061.95	0	647.71	654.87	
13	0	248.83	119.44	-534.95	0	0	1061.95	647.71	-1860.11	Kol.Lt.4
	1.14	248.83	119.44	-409.98	0	0	1061.95	647.71	-1335.23	
	2.28	248.83	119.44	-285.01	0	0	1061.95	647.71	-810.36	
14	0	-248.83	-119.44	0	534.94	1061.95	1061.95	1295.42	1860.06	Kol.Lt.4
	1.14	-248.83	-119.44	0	409.98	1061.95	1061.95	1295.42	1335.23	
	2.28	-248.83	-119.44	0	285.02	1061.95	1061.95	1295.42	810.40	
15	0	0	0	-285.01	0	0	1061.95	647.71	-1197.04	Kol.Lt.4
	1.3	0	0	-142.51	0	0	1061.95	647.71	-598.54	
	2.6	0	0	0	0	0	1061.95	647.71	0	
16	0	0	0	0	285.02	1061.95	1061.95	1295.42	1197.08	Kol.Lt.4
	1.3	0	0	0	142.51	1061.95	1061.95	1295.42	598.54	
	2.6	0	0	0	0	1061.95	1061.95	1295.42	0	

Keterangan: $M_{U,k} = \frac{h}{h_n} \cdot 0,7 \cdot \omega_d \cdot 0,5 (l_{ki} / l_{n,ki} M_{kap,b,ki} + l_{ka} / l_{n,ka} M_{kap,b,ka})$ KNm

Tabel 4.12.d Gaya Geser Rencana Kolom As.6,7-8 (K=1)

Elm	Jarak (m)	VD,k (KN)	VL,k (KN)	VE,k (KN)	MU, k bawah (KN)	MU, k atas (KN)	$VU, k = (MU, k a + MU, k b) / h_k$ (KN)	$VU, k = 1,05(VD, k + VL, k + 4/1 VE, k)$ (KN)	KET
1	0	55.10	18.24	209.34	911.86	911.89	597.95	956.23	Kol Basmen
	1.925	55.10	18.24	209.34	911.86	911.89	597.95	956.23	
	3.85	55.10	18.24	209.34	911.86	911.89	597.95	956.23	
2	0	48.27	16.00	261.19	911.86	911.89	597.95	1164.48	Kol Basmen
	1.925	48.27	16.00	261.19	911.86	911.89	597.95	1164.48	
	3.85	48.27	16.00	261.19	911.86	911.89	597.95	1164.48	
3	0	6.83	2.24	31.05	911.86	911.89	597.95	139.91	Kol Basmen
	1.925	6.83	2.24	31.05	911.86	911.89	597.95	139.91	
	3.85	6.83	2.24	31.05	911.86	911.89	597.95	139.91	
4	0	77.53	25.96	164.85	911.86	911.89	597.95	801.04	KOL LT.1
	1.925	77.53	25.96	164.85	911.86	911.89	597.95	801.04	
	3.85	77.53	25.96	164.85	911.86	911.89	597.95	801.04	
5	0	64.37	21.58	237.40	911.86	911.89	597.95	1087.33	KOL LT.1
	1.925	64.37	21.58	237.40	911.86	911.89	597.95	1087.33	
	3.85	64.37	21.58	237.40	911.86	911.89	597.95	1087.33	
6	0	13.17	4.38	60.13	911.86	911.89	597.95	270.98	KOL LT.1
	1.925	13.17	4.38	60.13	911.86	911.89	597.95	270.98	
	3.85	13.17	4.38	60.13	911.86	911.89	597.95	270.98	
7	0	81.21	27.05	102.59	911.86	1061.95	647.15	544.56	KOL LT.2
	1.925	81.21	27.05	102.59	911.86	1061.95	647.15	544.56	
	3.85	81.21	27.05	102.59	911.86	1061.95	647.15	544.56	
8	0	65.41	22.56	215.04	911.86	1061.95	647.15	995.54	KOL LT.2
	1.925	65.41	22.56	215.04	911.86	1061.95	647.15	995.54	
	3.85	65.41	22.56	215.04	911.86	1061.95	647.15	995.54	
9	0	15.80	4.49	66.36	911.86	1061.95	647.15	300.04	KOL LT.2
	1.925	15.80	4.49	66.36	911.86	1061.95	647.15	300.04	
	3.85	15.80	4.49	66.36	911.86	1061.95	647.15	300.04	
10	0	46.76	6.56	26.86	1061.95	1061.95	696.36	168.80	KOL LT.3
	1.925	46.76	6.56	26.86	1061.95	1061.95	696.36	168.80	
	3.85	46.76	6.56	26.86	1061.95	1061.95	696.36	168.80	
11	0	26.05	0.04	222.93	1061.95	1061.95	696.36	963.70	KOL LT.3
	1.925	26.05	0.04	222.93	1061.95	1061.95	696.36	963.70	
	3.85	26.05	0.04	222.93	1061.95	1061.95	696.36	963.70	
12	0	20.71	6.60	70.33	1061.95	1061.95	696.36	324.07	KOL LT.3
	1.925	20.71	6.60	70.33	1061.95	1061.95	696.36	324.07	
	3.85	20.71	6.60	70.33	1061.95	1061.95	696.36	324.07	
13	0	0	0	109.62	1061.95	1061.95	696.36	460.40	KOL LT.4
	1.925	0	0	109.62	1061.95	1061.95	696.36	460.40	
	3.85	0	0	109.62	1061.95	1061.95	696.36	460.40	
14	0	0	0	0	1061.95	1061.95	696.36	0	KOL LT.4
	1.925	0	0	0	1061.95	1061.95	696.36	0	
	3.85	0	0	0	1061.95	1061.95	696.36	0	
15	0	0	0	109.62	1061.95	1061.95	696.36	460.40	KOL LT.4
	1.925	0	0	109.62	1061.95	1061.95	696.36	460.40	
	3.85	0	0	109.62	1061.95	1061.95	696.36	460.40	
16	0	0	0	0	1061.95	1061.95	696.36	0	KOL LT.4
	1.925	0	0	0	1061.95	1061.95	696.36	0	
	3.85	0	0	0	1061.95	1061.95	696.36	0	

Tabel 4.13.a Momen Rencana Balok Portal As 6,7-8 (K=2)

Elm	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD+1.6ML (KNm)	0.9.(MD ± ME) (KNm)		1.05.(MD+0.6ML±ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
				19	0		-297.11	-98.40	404.16	-400.54	
1.5	-105.56	-35.30	303.76	-300.68	-183.16	178.38	-365.62	185.87	-463.62		
3	49.54	16.55	203.35	-200.81	85.92	227.60	-136.14	275.96	-141.46		
20	0	49.54	16.55	203.35	-200.81	85.92	227.60	-136.14	275.96	-141.46	blk. tt.1
1.5	125.06	41.21	102.95	-100.95	216.00	205.21	21.70	265.37	68.58		
3	164.12	54.62	2.54	-1.08	284.34	150.00	146.73	209.41	228.54		
21	0	164.12	54.62	2.54	-1.08	284.34	150.00	146.73	209.41	228.54	blk. tt.1
1.5	123.59	40.85	-97.86	98.78	213.67	23.16	200.13	52.75	276.38		
3	46.61	15.83	-198.26	198.64	81.27	-136.48	220.73	-149.26	274.14		
22	0	46.61	15.83	-198.26	198.64	81.27	-136.48	220.73	-149.26	274.14	blk. tt.1
1.5	-109.96	-36.37	-298.67	298.51	-190.14	-367.77	169.70	-451.97	159.79		
3	-302.98	-99.82	-399.07	398.37	-523.29	-631.85	85.85	-800.04	-4.65		
23	0	-75.99	-24.79	541.98	-544.52	-130.85	419.39	-558.46	473.67	-677.57	blk. tt.1
1.5	8.78	2.80	319.79	-321.03	15.02	295.71	-281.02	346.77	-324.92		
2.5	57.11	19.14	97.60	-97.55	99.16	139.23	-36.40	174.50	-22.36		
24	0	57.11	19.14	97.60	-97.55	99.16	139.23	-36.40	174.50	-22.36	blk. tt.1
1.5	25.84	8.30	-124.59	125.94	44.29	-88.88	136.60	-98.46	168.08		
3	-41.88	-13.79	-346.78	349.43	-72.32	-349.79	276.80	-416.78	308.45		
25	0	-297.63	-98.82	547.87	-544.19	-515.26	225.22	-757.64	200.50	-987.67	blk. tt.2
1.5	-105.95	-35.60	411.56	-408.92	-184.11	275.05	-463.38	298.46	-578.00		
3	49.28	16.36	275.24	-273.65	85.30	292.07	-201.93	351.05	-218.42		
26	0	49.28	16.36	275.24	-273.65	85.30	292.07	-201.93	351.05	-218.42	blk. tt.2
1.5	124.92	41.13	138.93	-138.37	215.72	237.47	-12.11	302.96	29.07		
3	164.11	54.66	2.62	-3.10	284.39	150.05	144.91	209.50	226.46		
27	0	164.11	54.66	2.62	-3.10	284.39	150.05	144.91	209.50	226.46	blk. tt.2
1.5	123.71	41.01	-133.70	132.17	214.06	-8.99	230.29	15.34	311.73		
3	46.86	16.10	-270.01	267.45	81.99	-200.84	282.88	-224.17	346.93		
28	0	46.86	16.10	-270.01	267.45	81.99	-200.84	282.88	-224.17	346.93	blk. tt.2
1.5	-109.58	-35.99	-406.32	402.72	-189.08	-464.31	263.83	-564.37	270.01		
3	-302.47	-99.33	-542.64	537.99	-521.88	-760.60	211.97	-949.94	143.00		
29	0	-60.76	-20.31	733.88	-722.60	-105.41	605.80	-705.03	693.98	-843.86	blk. tt.2
1.5	17.00	5.31	429.56	-422.95	28.89	401.91	-365.35	472.23	-420.67		
3	58.32	19.67	125.24	-123.31	101.45	165.20	-58.49	205.13	-47.59		
30	0	58.32	19.67	125.24	-123.31	101.45	165.20	-58.49	205.13	-47.59	blk. tt.2
1.5	20.05	6.85	-179.08	176.34	35.02	-143.13	176.75	-162.67	213.40		
3	-54.68	-17.22	-483.40	475.99	-93.16	-484.27	379.18	-575.83	424.30		
31	0	-296.92	-99.01	569.52	-562.28	-514.71	245.34	-773.28	223.86	-1006.12	blk. tt.3
1.5	-105.25	-35.64	427.63	-422.86	-183.32	290.14	-475.30	316.05	-591.94		
2.5	49.97	16.48	285.75	-283.43	86.33	302.15	-210.11	362.89	-227.83		
32	0	49.97	16.48	285.75	-283.43	86.33	302.15	-210.11	362.89	-227.83	blk. tt.3
1.5	125.60	41.41	143.87	-144.00	216.98	242.52	-16.56	309.03	24.16		
3	164.78	55.09	1.99	-4.57	285.89	150.09	144.19	209.81	226.07		

Tabel 4.13.b Gaya Geser Rencana Balok Portal As.6,7-8 (K=2)

E(m)	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VUB=1,05(VD,B + VL,B +1,3 VE,B) (KN)	KET
19	0	139.85	45.82	66.58	334.76	BIK LT.1
	1.5	115.55	38.32	66.58	301.37	
	3	91.25	30.82	66.58	267.98	
20	0	62.49	20.19	66.58	226.63	BIK LT.1
	1.5	38.19	12.69	66.58	193.24	
	3	13.89	5.19	66.58	159.85	
21	0	14.87	5.43	66.58	161.12	BIK LT.1
	1.5	39.17	12.93	66.58	194.51	
	3	63.47	20.43	66.58	227.90	
22	0	92.23	31.05	66.58	269.25	BIK LT.1
	1.5	116.53	38.55	66.58	302.64	
	3	140.83	46.05	66.58	336.03	
23	0	68.67	22.15	148.99	408.23	BIK LT.1
	1.25	44.37	14.65	148.99	374.84	
	2.5	20.07	7.14	148.99	341.45	
	3	8.69	3.48	148.99	325.66	
24	0	33.00	10.96	148.99	359.05	BIK LT.1
	1.5	57.30	18.46	148.99	392.44	
	3	81.60	25.96	148.99	425.83	
25	0	139.94	45.89	90.18	384.50	BIK LT.2
	1.5	115.64	38.39	90.18	351.11	
	3	91.34	30.89	90.18	317.72	
26	0	62.58	20.27	90.18	276.37	BIK LT.2
	1.5	38.28	12.77	90.18	242.98	
	3	13.98	5.27	90.18	209.59	
27	0	14.78	5.35	90.18	210.53	BIK LT.2
	1.5	39.08	12.85	90.18	243.92	
	3	63.38	20.35	90.18	277.31	
28	0	92.14	30.98	90.18	318.66	BIK LT.2
	1.25	116.44	38.48	90.18	352.04	
	2.5	140.74	45.98	90.18	385.43	
29	0	63.99	20.83	199.76	508.56	BIK LT.2
	1.5	39.69	13.33	199.76	475.17	
	3	15.39	5.83	199.76	441.78	
30	0	13.37	4.60	199.76	438.57	BIK LT.2
	1.5	37.67	12.30	199.76	471.96	
	3	61.97	19.80	199.76	505.35	
31	0	139.93	45.99	92.95	390.42	BIK LT.3
	1.5	115.63	38.49	92.95	357.03	
	3	91.33	30.99	92.95	323.64	
32	0	62.57	20.37	92.95	262.29	BIK LT.3
	1.5	38.27	12.87	92.95	248.90	
	3	13.97	5.37	92.95	215.51	
33	0	14.79	5.25	92.95	216.24	BIK LT.3
	1.5	39.09	12.75	92.95	249.63	
	3	63.39	20.25	92.95	283.02	
34	0	92.15	30.87	92.95	324.37	BIK LT.3
	1.5	116.45	38.37	92.95	357.76	
	3	140.75	45.87	92.95	391.15	
35	0	79.04	19.97	208.36	541.52	BIK LT.3
	1.5	54.74	12.47	208.36	508.13	
	3	30.44	4.97	208.36	474.74	
36	0	1.68	-5.65	208.36	433.39	BIK LT.3
	1.5	51.22	13.15	208.36	505.15	
	3	75.52	20.65	208.36	538.54	

Lanjutan Tabel 4.13 Gaya Geser Rencana Balok Portal As.6,7-8

(K=2)

Elem	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VUB=1,05(VD,B + VL,B +1,3. VE,B) (KN)	KET
37	0	168.52	46.02	93.21	421.00	Bik LT.4
	1.25	115.62	38.52	93.21	357.58	
	2.5	91.32	31.02	93.21	324.19	
38	0	62.56	20.40	93.21	282.84	Bik LT.4
	1.5	38.26	12.90	93.21	249.45	
	3	13.96	5.40	93.21	216.06	
39	0	14.80	-5.23	93.21	205.79	Bik LT.4
	1.5	39.10	12.73	93.21	250.16	
	3	63.40	20.23	93.21	283.55	
40	0	92.16	30.85	93.21	324.90	Bik LT.4
	1.5	116.46	38.35	93.21	359.29	
	3	140.76	45.85	93.21	391.68	
41	0	64.01	19.99	163.39	431.32	Bik LT.4
	1.5	39.71	12.49	163.39	397.93	
	3	15.41	4.99	163.39	364.54	
42	0	13.35	-5.63	163.39	351.23	Bik LT.4
	1.25	37.65	13.13	163.39	396.44	
	2.5	61.95	20.63	163.39	429.83	
43	0	45.72	23.63	0	72.81	Bik atap
	1.75	71.10	34.13	0	110.48	
	3.5	96.47	44.63	0	148.15	
44	0	96.47	44.63	0	148.15	Bik atap
	1.75	71.10	34.13	0	110.48	
	3.5	45.72	23.63	0	72.81	

Tabel 4.13.c Momen Rencana Kolom Portal As.6,7-8 (K=2)

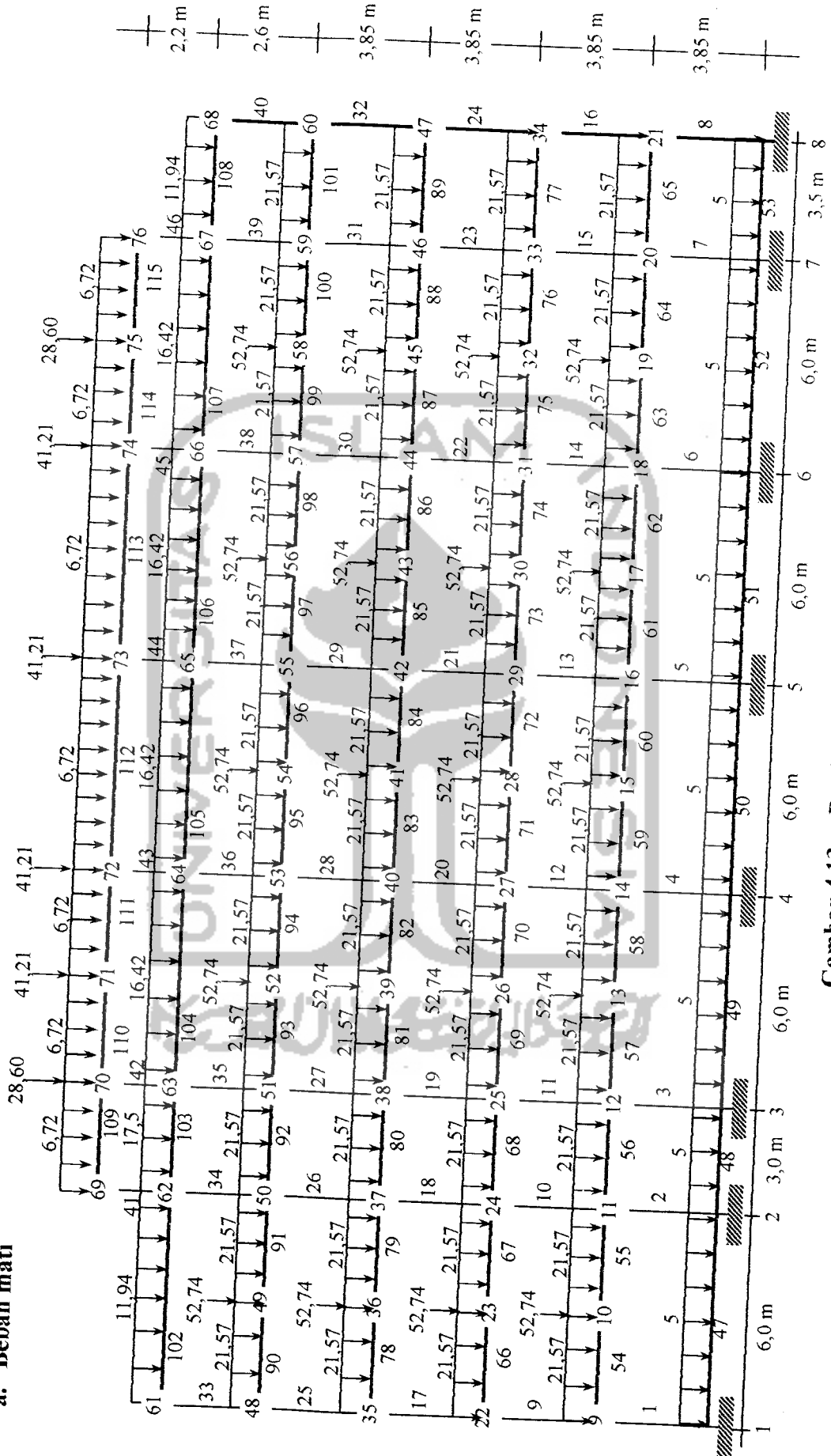
EIm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	MEki (KNm)	MEka (KNm)	MU,k= 1.05(MD,k+ML,k+1/2ME,k)	KETERANGAN
1	0	60.22	21.04	-961.562	956.403	1390.81	KOL BASEMEN
	1.92	-45.85	-14.07	-392.519	392.901	473.39	
	3.85	-151.91	-49.18	176.525	-170.601	-444.01	
2	0	-87.04	-27.92	-1132.06	1135.821	1429.69	KOL BASEMEN
	1.92	5.89	2.87	-302.961	301.335	420.52	
	3.85	98.81	33.67	526.141	-533.15	-588.65	
3	0	-9.48	-3.09	-727.672	728.626	981.38	KOL BASEMEN
	1.92	3.66	1.22	-194.928	195.723	272.29	
	3.85	16.80	5.54	337.817	-337.18	-436.79	
4	0	145.20	49.22	-382.788	388.417	734.33	KOLOM LT.1
	1.92	-4.05	-0.75	-23.622	11.903	11.21	
	3.85	-153.30	-50.72	335.545	-364.612	-711.92	
5	0	-128.17	-41.36	-890.199	878.91	1021.71	KOLOM LT.1
	1.92	-4.26	0.19	-19.981	29.487	35.98	
	3.85	119.64	41.74	850.238	-819.936	-949.76	
6	0	-25.08	-8.25	-565.603	566.237	737.92	KOLOM LT.1
	1.92	0.26	0.17	-15.017	12.204	17.11	
	3.85	25.61	8.59	535.569	-541.829	-703.69	
7	0	144.33	48.09	-258.813	222.428	505.66	KOLOM LT.2
	1.93	-12.00	-3.98	107.764	-52.349	-88.24	
	3.85	-168.34	-56.06	474.341	-327.125	-682.15	
8	0	-122.07	-37.28	-648.878	692.883	778.47	KOLOM LT.2
	1.93	3.85	6.15	20.347	-78.657	-96.87	
	3.85	129.77	49.57	689.572	-850.197	-972.21	
9	0	-29.07	-8.63	-420.94	414.449	526.14	KOLOM LT.2
	1.93	1.34	0.02	21.427	-17.403	-22.33	
	3.85	31.76	8.68	463.793	-449.256	-570.77	
10	0	128.58	42.94	0.867	166.231	407.00	KOLOM LT.3
	1.92	38.57	30.32	-220.013	-123.094	-95.69	
	3.85	-51.45	17.70	-440.893	-412.418	-598.39	
11	0	-91.60	-30.51	-650.02	358.288	360.85	KOLOM LT.3
	1.92	-41.45	-30.58	138.544	88.532	45.21	
	3.85	8.69	-30.66	927.107	-181.224	-270.44	
12	0	-38.15	-11.55	-395.678	363.061	443.39	KOLOM LT.3
	1.92	1.72	1.15	62.086	-103.305	-138.00	
	3.85	41.59	13.85	519.85	-569.672	-719.39	
13	0	248.83	119.44	-1069.4	1069.403	1846.42	KOLOM LT.4
	1.14	248.83	119.44	-775.756	775.756	1445.59	
	2.28	248.83	119.44	-482.108	482.108	1044.76	
14	0	-248.83	-119.44	0	0	-386.68	KOLOM LT.4
	1.14	-248.83	-119.44	0	0	-386.68	
	2.28	-248.83	-119.44	0	0	-386.68	
15	0	0	0	-482.108	482.108	658.08	KOLOM LT.4
	1.3	0	0	-241.054	241.054	329.04	
	2.6	0	0	0	0	0.00	
16	0	0	0	0	569.77	777.74	KOLOM LT.4
	1.3	0	0	0	284.88	388.86	
	2.6	0	0	0	0	0.00	

Tabel 4.13.d Gaya Geser Rencana Kolom Portal As.6,7-8 (K=2)

Elm	Jarak (m)	VD,k (KN)	VL,k (KN)	VE,k (KN)	VU,k = 1,05(VD,k+ VL,k+ 0,5 VE,k) (KN)	KETERANGAN
1	0	55.10	18.24	418.64	648.45	KOL BASEMEN
	1.92	55.10	18.24	418.64	648.45	
	3.85	55.10	18.24	418.64	648.45	
2	0	48.27	16.00	522.34	780.48	KOL BASEMEN
	1.92	48.27	16.00	522.34	780.48	
	3.85	48.27	16.00	522.34	780.48	
3	0	6.83	2.24	62.08	94.27	KOL BASEMEN
	1.92	6.83	2.24	62.08	94.27	
	3.85	6.83	2.24	62.08	94.27	
4	0	77.53	25.96	329.66	558.65	KOLOM LT 1
	1.92	77.53	25.96	329.66	558.65	
	3.85	77.53	25.96	329.66	558.65	
5	0	64.37	21.58	474.76	738.30	KOLOM LT 1
	1.92	64.37	21.58	474.76	738.30	
	3.85	64.37	21.58	474.76	738.30	
6	0	13.17	4.38	120.25	182.56	KOLOM LT 1
	1.92	13.17	4.38	120.25	182.56	
	3.85	13.17	4.38	120.25	182.56	
7	0	81.21	27.05	205.15	393.71	KOLOM LT 2
	1.93	81.21	27.05	205.15	393.71	
	3.85	81.21	27.05	205.15	393.71	
8	0	65.41	22.56	430.02	679.35	KOLOM LT 2
	1.93	65.41	22.56	430.02	679.35	
	3.85	65.41	22.56	430.02	679.35	
9	0	15.80	4.49	132.71	202.46	KOLOM LT 2
	1.93	15.80	4.49	132.71	202.46	
	3.85	15.80	4.49	132.71	202.46	
10	0	46.76	6.56	200.01	329.00	KOLOM LT 3
	1.92	46.76	6.56	200.01	329.00	
	3.85	46.76	6.56	200.01	329.00	
11	0	26.05	0.04	445.74	635.83	KOLOM LT 3
	1.92	26.05	0.04	445.74	635.83	
	3.85	26.05	0.04	445.74	635.83	
12	0	20.71	6.60	140.63	220.63	KOLOM LT 3
	1.92	20.71	6.60	140.63	220.63	
	3.85	20.71	6.60	140.63	220.63	
13	0	0	0	219.14	299.13	KOLOM LT 4
	1.14	0	0	219.14	299.13	
	2.28	0	0	219.14	299.13	
14	0	0	0	0	0	KOLOM LT 4
	1.14	0	0	0	0	
	2.28	0	0	0	0	
15	0	0	0	219.14	299.13	KOLOM LT 4
	1.3	0	0	219.14	299.13	
	2.6	0	0	219.14	299.13	
16	0	0	0	0	0	KOLOM LT 4
	1.3	0	0	0	0	
	2.6	0	0	0	0	

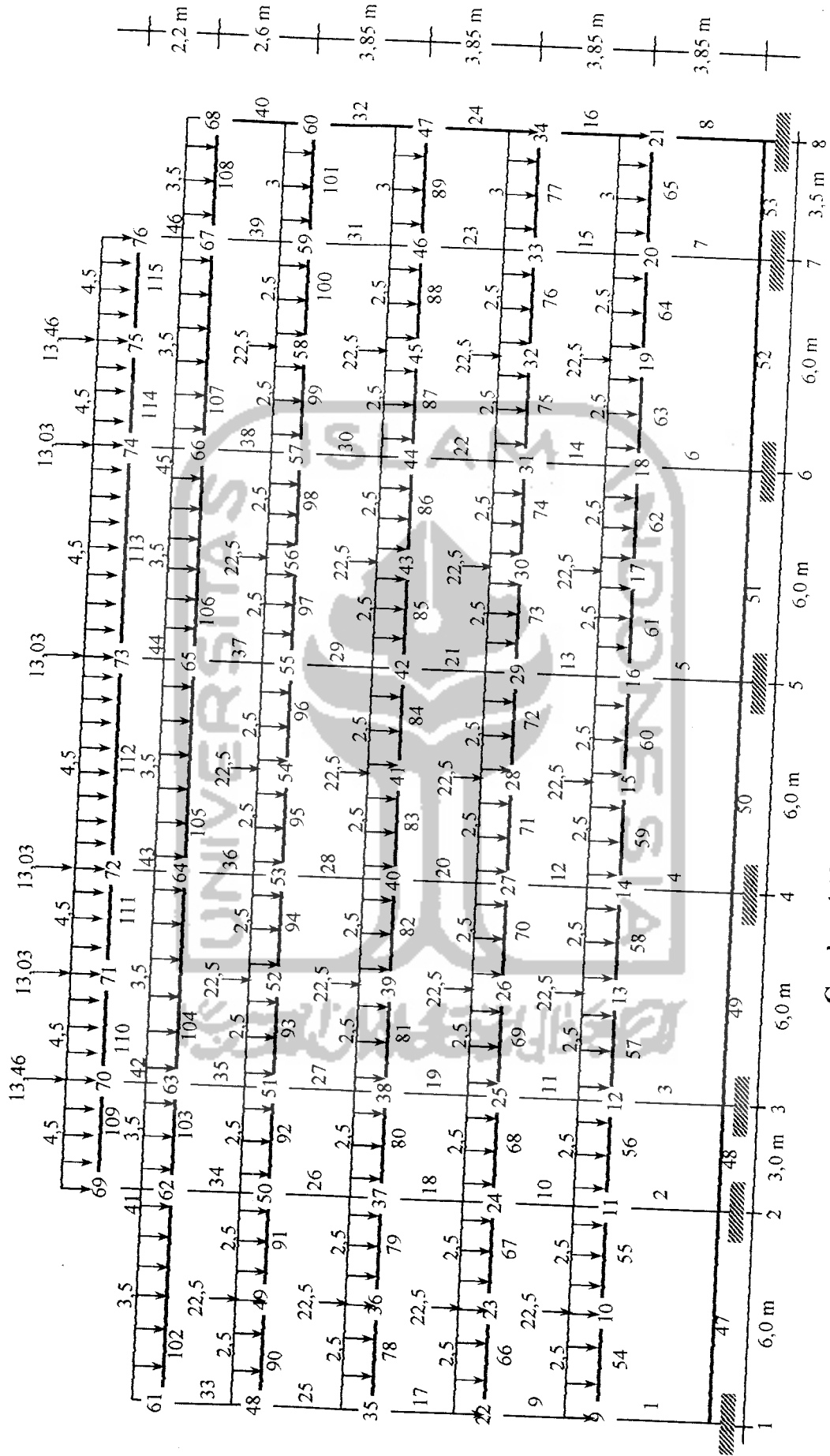
Pembebanan Poratal As A-D

a. Beban mati



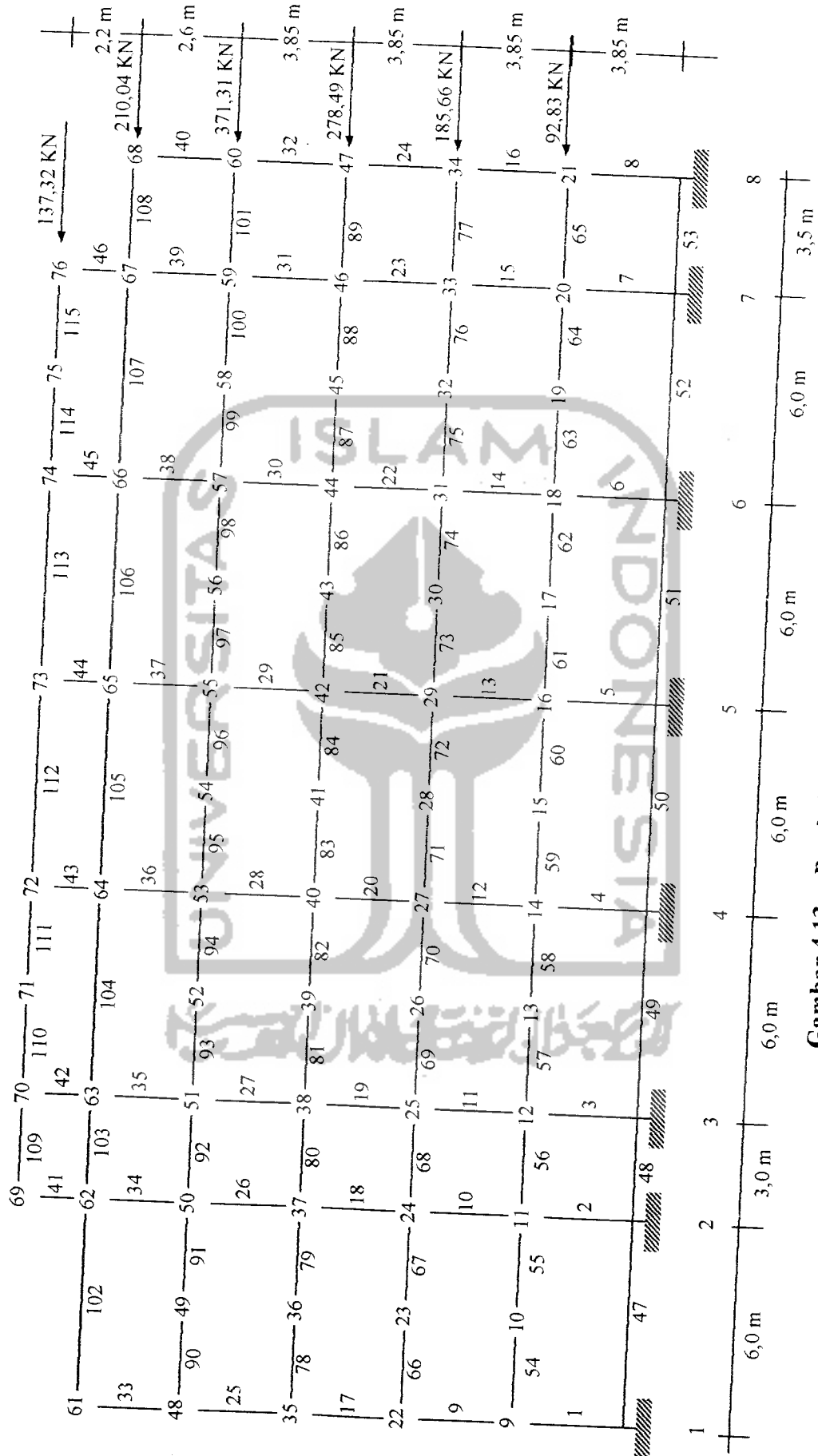
Gambar 4.13.a Pembebanan mati Poratal As A

b. Beban hidup



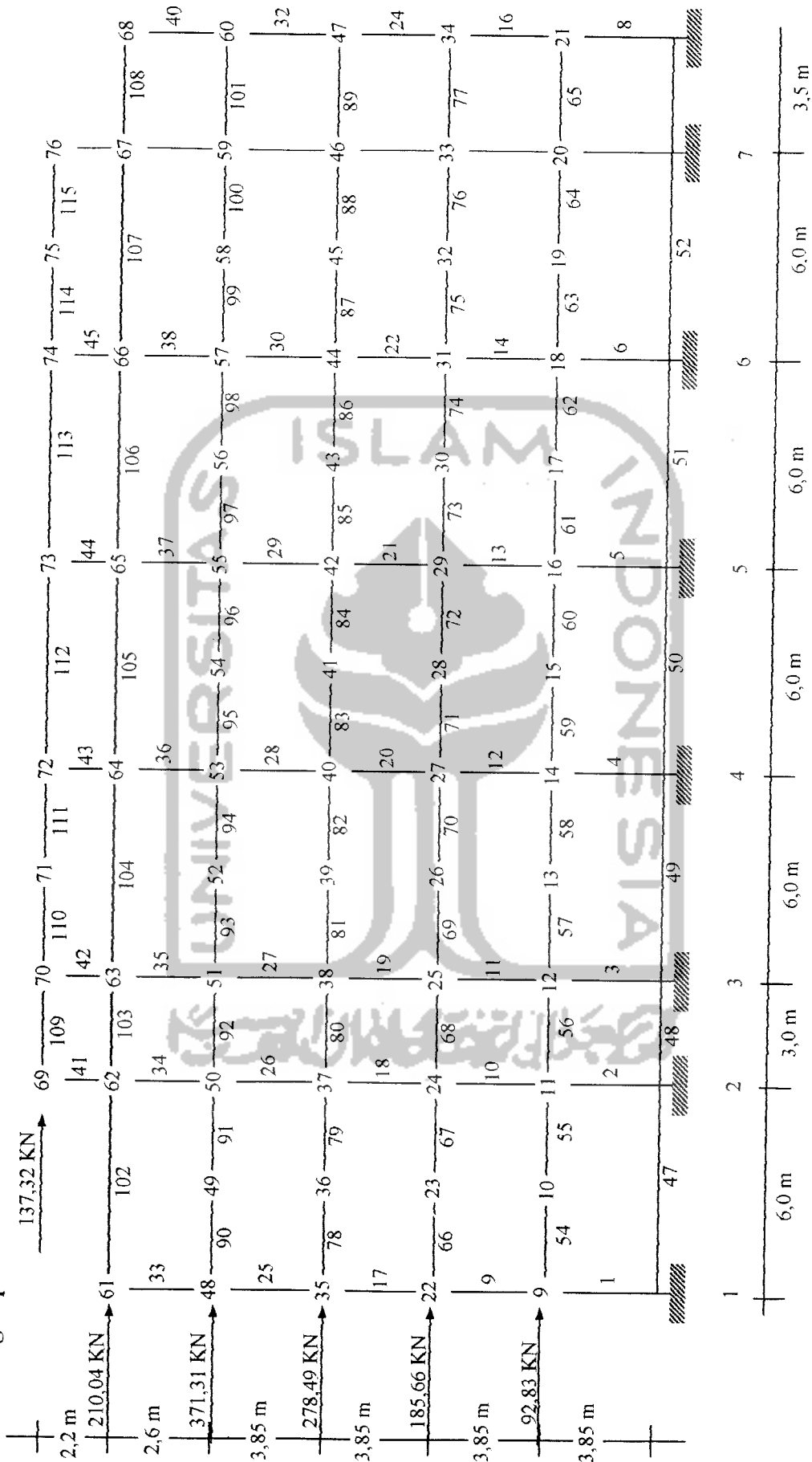
Gambar 4.13.b Pembebanan hidup Portal As A

c. Beban gempa kanan



Gambar 4.13.c Pembebanan gempa kanan Portal As A

d. Beban gempa kiri



Gambar 4.13.d Pembebanan gempa kiri Portal As A

a. Beban mati

- Beban terdistribusi merata elemen 54 s/d 101

$$\text{Lantai} : 2/3 \cdot 1,5 \cdot 4,26 = 4,26 \text{ KN/m}$$

$$\text{Tembok} : 2,5 \cdot 3,85 = 9,63 \text{ KN/m}$$

$$\text{Balok} : 0,4 \cdot 0,8 \cdot 24 = 7,68 \text{ KN/m}$$

$$W_{D1} = 21,57 \text{ KN/m}$$

- Beban terdistribusi merata elemen 102 & 108

$$\text{Lantai} : 2/3 \cdot 1,5 \cdot 4,26 = 4,26 \text{ KN/m}$$

$$\text{Balok} : 0,4 \cdot 0,8 \cdot 24 = 7,68 \text{ KN/m}$$

$$W_{D2} = 11,94 \text{ KN/m}$$

- Beban terdistribusi merata elemen 103 s/d 107

$$\text{Lantai} : 2/3 \cdot 1,5 \cdot 4,26 = 4,26 \text{ KN/m}$$

$$\text{Balok} : 0,4 \cdot 0,8 \cdot 24 = 7,68 \text{ KN/m}$$

$$\text{Tembok} : 2,2 \cdot 2,5 = 5,50 \text{ KN/m}$$

$$W_{D4} = 17,44 \text{ KN/m}$$

- Beban terbagi merata elemen 109 s/d 115

$$\text{Lantai} : 1,5 \cdot 0,12 \cdot 24 = 4,32 \text{ KN/m}$$

$$\text{Balok} : 0,25 \cdot 0,4 \cdot 24 = 2,40 \text{ KN/m}$$

$$W_{D5} = 6,72 \text{ KN/m}$$

- Beban terpusat nodal 70, 75

$$P_{D1 \text{ RKA3}} = 28,5995 \text{ KN}$$

- Beban terpusat nodal 71 s/d 74

$$P_{D2 \text{ RKA1}} = 41,2076 \text{ KN}$$

- Beban terpusat nodal 10 s/d 19, 23 s/d 32, 36 s/d 45, 49 s/d 56

$$P_{D3 \text{ R balok anak}} = 2 \cdot (2 \cdot 1/2 \cdot 3 \cdot 1,5 \cdot 4,26) + 0,25 \cdot 0,4 \cdot 24 \cdot 6 = 52,74 \text{ KN}$$

b. Beban hidup

- Beban terbagi merata elemen 54 s/d 64, 66 s/d 76, 78 s/d 88, 90 s/d 100

$$W_{L1} = 2/3 \cdot 1,5 \cdot 2,5 = 2,5 \text{ KN/m}$$

- Beban terbagi merata elemen 65, 77, 89, 101,

$$W_{L2} = 2/3 \cdot 1,5 \cdot 3 = 3,0 \text{ KN/m}$$

- Beban terbagi merata elemen 102 s/d 108

$$W_{L3} = 2/3 \cdot 1,75 \cdot 3 = 3,5 \text{ KN/m}$$

- Beban terbagi merata elemen 109 s/d 115

$$W_{L4} = 1,5 \cdot 3 = 4,5 \text{ KN/m}$$

- Beban terpusat nodal 70, 75

$$P_{L1 \text{ R KA3}} = 13,4605 \text{ KN}$$

- Beban terpusat nodal 71 s/d 74

$$P_{L2 \text{ R KA1}} = 13,0327 \text{ KN}$$

- Beban terpusat nodal 10 s/d 19, 23 s/d 32, 36 s/d 45, 49 s/d 56

$$P_{L3 \text{ R balok anak}} = 2 \cdot (2 \cdot 1/2 \cdot 3 \cdot 1,5 \cdot 2,5) = 22,50 \text{ KN}$$

c. Beban gempa

Reaksi atap	: $4.41,2076 + 2.28,5995 = 222,03 \text{ KN}$
Plat atap	: $1,5.27.0,12.24 = 116,64 \text{ KN}$
Balok ring	: $0,4.0,8.9.3.27 = 207,36 \text{ KN}$
Kolom	: $6.0,45.0,7.2,6.24 = 99,79 \text{ KN}$
Beban hidup tereduksi:	$0,6.(1,5.27.3) = 72,90 \text{ KN}$

Dibulatkan $W_{\text{atap 1}} = 718,72 \text{ KN}$

Plat atap (mangkok)	: $3,5.36,5.0,12.24 = 367,92 \text{ KN}$
Kolom	: $8.0,45.0,7.2,2.24 = 157,25 \text{ KN}$
Tembok	: $2,5.2,6.30,5 = 198,25 \text{ KN}$
Balok ring	: $0,4.0,8.36,5.24 = 280,32 \text{ KN}$
Beban hidup tereduksi:	$0,6.(3,5.36,5.3) = 229,95 \text{ KN}$

Dibulatkan $W_{\text{atap 2}} = 1233,70 \text{ KN}$

- Berat lantai 4 (tipikal dengan lt. 3, 2,1)

Kolom	: $8.0,45.0,7.3,85.24 = 232,85 \text{ KN}$
Lantai	: $6.36,5.4,26 = 932,94 \text{ KN}$
Balok	: $0,4.0,8.24.78,5 = 602,88 \text{ KN}$
Balok	: $0,25.0,4.24.66 = 158,40 \text{ KN}$
Tembok	: $2,5.3,85.30,5 = 293,56 \text{ KN}$
B.hidup tereduksi:	$0,6(6.36,5.2,5) = 328,50 \text{ KN}$

Dibulatkan $W_4 = 2549,2 \text{ KN}$

$$W_{\text{Total}} : 718,72 + 1233,70 + 4.2549,20 = 12147,94 \text{ KN}$$

Gaya geser untuk tingkat daktilitas penuh ($K=1$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.1,0.12147,94 = 1275,64 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap 1	718,72	20,20	14518,15	137,32
Atap 2	1233,70	18,00	22206,60	210,04
4	2549,20	15,40	39257,68	371,31
3	2549,20	11,55	29443,26	278,49
2	2549,20	7,70	19628,84	185,66
1	2549,20	3,85	9814,42	92,83
			134868,95	1275,64

Gaya geser untuk tingkat daktilitas terbatas ($K=2$)

$$V = C.I.K.W_T$$

$$= 0,07.1,5.2,0.12147,94 = 2551,28 \text{ KN}$$

distribusi gaya-gaya lateral akibat gempa

Tingkat	Wi (KN)	hi (m)	Wi hi (KNm)	Fi (KN)
Atap 1	718,72	20,20	14518,15	274,44
Atap 2	1233,70	18,00	22206,60	420,08
4	2549,20	15,40	39257,68	742,62
3	2549,20	11,55	29443,26	556,98
2	2549,20	7,70	19628,84	371,32
1	2549,20	3,85	9814,42	185,66
			134868,95	2551,28

Tabel 4.14.a Momen Rencana Balok Portal As A - D (K=1)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
54	0	-45.96	-13.51	204.45	-202.00	-76.77	142.64	-223.16	157.90	-268.87	blk. tt.1
	1.5	10.47	2.95	108.92	-107.55	17.27	107.45	-87.37	127.21	-100.08	
	3	35.24	10.97	13.39	-13.09	59.83	43.77	19.93	57.97	30.16	
55	0	35.24	10.97	13.39	-13.09	59.83	43.77	19.93	57.97	30.16	blk. tt.1
	1.5	8.17	2.41	-82.14	81.36	13.65	-66.57	80.58	-76.15	95.53	
	3	-50.56	-14.59	-177.67	175.82	-84.01	-205.40	112.73	-248.83	122.33	
56	0	-24.47	-5.38	265.01	-262.92	-37.98	216.48	-258.65	249.17	-305.16	blk. tt.1
	1.5	0.91	-0.64	-1.06	1.14	0.06	-0.13	1.84	-0.56	1.74	
	3	-27.02	-4.34	-267.12	265.19	-39.36	-264.73	214.36	-311.88	247.35	
57	0	-72.96	-13.62	165.15	-164.31	-109.34	82.97	-213.55	88.22	-257.72	blk. tt.1
	1.5	14.33	2.53	80.41	-79.94	21.25	85.26	-59.05	101.07	-67.29	
	3	48.31	10.24	-4.34	4.44	74.36	39.58	47.48	52.62	61.84	
58	0	48.31	10.24	-4.34	4.44	74.36	39.58	47.48	52.62	61.84	blk. tt.1
	1.5	8.79	1.37	-89.08	88.82	12.75	-72.26	87.85	-83.44	103.36	
	2.5	-84.04	-15.93	-173.83	173.19	-126.33	-232.08	80.24	-280.79	83.58	
59	0	-80.93	-15.23	178.42	-178.26	-121.48	87.74	-233.27	92.77	-281.74	blk. tt.1
	1.5	8.94	1.44	89.36	-89.22	13.03	88.48	-72.25	104.13	-83.38	
	3	45.50	9.67	0.31	-0.18	70.07	41.23	40.79	54.19	53.68	
60	0	45.50	9.67	0.31	-0.18	70.07	41.23	40.79	54.19	53.68	blk. tt.1
	1.5	8.56	1.32	-88.75	88.86	12.38	-72.17	87.68	-83.36	103.12	
	3	-81.69	-15.47	-177.80	177.90	-122.78	-233.54	86.59	-282.21	91.27	
61	0	-80.54	-15.20	177.51	-178.12	-120.97	87.28	-232.79	92.25	-281.17	blk. tt.1
	1.5	9.15	1.46	88.68	-88.91	13.31	88.05	-71.78	103.64	-82.83	
	3	45.53	9.68	-0.16	0.29	70.12	40.83	41.24	53.73	54.21	
62	0	45.53	9.68	-0.16	0.29	70.12	40.83	41.24	53.73	54.21	blk. tt.1
	1.5	8.41	1.32	-89.00	89.50	12.21	-72.53	88.12	-83.79	103.64	
	3	-82.02	-15.48	-177.84	178.71	-123.18	-233.87	87.02	-282.60	91.77	
63	0	-82.28	-15.45	173.65	-174.96	-123.46	82.23	-231.52	86.20	-279.83	blk. tt.1
	1.25	9.52	1.60	88.52	-89.13	13.99	88.24	-71.64	103.95	-82.57	
	2.5	48.01	10.22	3.38	-3.29	73.96	46.26	40.25	60.40	53.39	
64	0	48.01	10.22	3.38	-3.29	73.96	46.26	40.25	60.40	53.39	blk. tt.1
	1.5	13.01	2.25	-81.75	82.54	19.21	-61.87	85.99	-70.76	101.74	
	3	-75.31	-14.15	-166.89	168.37	-113.02	-217.98	83.76	-263.22	88.80	
65	0	-31.58	-4.51	243.64	-246.49	-45.10	190.86	-250.26	219.83	-294.81	blk. tt.1
	1.5	8.81	0.73	-2.18	2.34	11.73	5.97	10.03	7.42	12.16	
	3	-15.22	-2.08	-248.00	251.16	-21.59	-236.89	212.35	-277.68	246.43	
66	0	-48.02	-14.11	206.41	-204.28	-80.19	142.55	-227.06	157.42	-273.80	blk. tt.2
	1.75	8.96	2.51	108.72	-107.54	14.76	105.91	-88.73	125.14	-101.94	
	3.5	34.27	10.69	11.03	-10.81	58.22	40.77	21.12	54.30	31.37	
67	0	34.27	10.69	11.03	-10.81	58.22	40.77	21.12	54.30	31.37	blk. tt.2
	1.5	7.74	2.29	-86.66	85.93	12.95	-71.03	84.31	-81.42	99.80	
	3	-50.44	-14.55	-184.35	182.67	-83.81	-211.31	119.00	-255.70	129.67	
68	0	-26.11	-5.40	266.49	-264.56	-39.98	216.34	-261.60	248.99	-308.60	blk. tt.2
	1.25	2.30	-0.28	-0.53	0.62	2.31	1.59	2.63	1.68	2.89	
	2.5	-22.61	-3.59	-267.55	265.80	-32.88	-261.15	218.87	-306.93	253.09	
69	0	-75.11	-14.10	174.55	-173.73	-112.69	89.49	-223.96	95.53	-270.17	blk. tt.2
	1.5	13.11	2.26	85.58	-85.12	19.34	88.82	-64.81	105.04	-74.19	
	3	48.02	10.17	-3.40	3.50	73.90	40.16	46.36	53.26	60.50	

Lanjutan Tabel 4.14.a Momen Rencana Balok Portal As A-D (K=1)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
70	0	48.02	10.17	-3.40	3.50	73.90	40.16	46.36	53.26	60.50	blk. It.2
	1.5	9.43	1.51	-92.37	92.11	13.73	-74.65	91.38	-86.14	107.57	
	3	-82.48	-15.59	-181.35	180.72	-123.92	-237.44	88.42	-286.83	93.34	
71	0	-80.18	-15.07	182.30	-182.13	-120.34	91.91	-236.08	97.73	-284.93	blk. It.2
	1.5	9.38	1.53	91.20	-91.05	13.70	90.52	-73.51	106.57	-84.80	
	3	45.63	9.69	0.09	0.03	70.26	41.14	41.09	54.11	54.04	
72	0	45.63	9.69	0.09	0.03	70.26	41.14	41.09	54.11	54.04	blk. It.2
	1.5	8.38	1.28	-91.02	91.10	12.09	-74.38	89.53	-85.97	105.26	
	3	-82.19	-15.58	-182.12	182.18	-123.55	-237.88	90.00	-287.34	95.18	
73	0	-80.81	-15.31	181.89	-182.40	-121.47	90.97	-236.89	96.48	-286.01	blk. It.2
	1.5	9.08	1.41	90.95	-91.15	13.15	90.03	-73.87	105.92	-85.29	
	3	45.64	9.70	0.02	0.10	70.30	41.10	41.17	54.06	54.14	
74	0	45.64	9.70	0.02	0.10	70.30	41.10	41.17	54.06	54.14	blk. It.2
	1.5	8.72	1.41	-90.91	91.34	12.72	-73.97	90.05	-85.41	105.95	
	3	-81.53	-15.31	-181.84	182.59	-122.33	-237.03	90.96	-286.18	96.47	
75	0	-80.53	-14.99	180.37	-181.54	-120.61	89.86	-235.86	95.39	-284.61	blk. It.2
	1.5	10.32	1.82	91.44	-91.97	15.30	91.58	-73.49	107.99	-84.59	
	3	47.86	10.18	2.50	-2.41	73.73	45.33	40.90	59.30	54.14	
76	0	47.86	10.18	2.50	-2.41	73.73	45.33	40.90	59.30	54.14	blk. It.2
	1.5	11.90	1.97	-86.43	87.15	17.43	-67.08	89.15	-77.01	105.25	
	3	-77.38	-14.68	-175.36	176.72	-116.34	-227.46	89.41	-274.62	95.06	
77	0	-25.74	-3.26	241.89	-244.37	-36.11	194.54	-243.10	224.91	-285.67	blk. It.2
	1.75	9.25	0.85	-2.53	2.67	12.46	6.05	10.73	7.59	13.05	
	3.5	-20.17	-3.08	-246.95	249.70	-29.12	-240.41	206.58	-282.42	239.07	
78	0	-48.63	-14.24	177.71	-175.40	-81.14	116.17	-201.63	126.56	-244.21	blk. It.3
	1.5	8.70	2.46	93.56	-92.27	14.37	92.03	-75.21	108.92	-86.20	
	3	34.36	10.73	9.41	-9.13	58.40	39.39	22.71	52.72	33.25	
79	0	34.36	10.73	9.41	-9.13	58.40	39.39	22.71	52.72	33.25	blk. It.3
	1.5	8.19	2.41	-74.74	74.01	13.69	-59.90	73.98	-68.36	87.83	
	3	-49.64	-14.34	-158.90	157.15	-82.51	-187.68	96.76	-228.00	103.85	
80	0	-29.64	-6.26	212.81	-211.03	-45.57	164.66	-216.60	188.39	-256.64	blk. It.3
	1.5	1.93	-0.36	-0.55	0.63	1.73	1.24	2.30	1.22	2.45	
	3	-19.82	-2.91	-213.91	212.28	-28.44	-210.36	173.21	-247.25	200.25	
81	0	-75.73	-14.23	151.31	-150.51	-113.64	68.02	-203.62	70.40	-246.52	blk. It.3
	1.5	12.90	2.21	74.43	-73.98	19.02	78.59	-54.97	93.09	-62.74	
	3	48.21	10.21	-2.46	2.56	74.20	41.18	45.69	54.48	59.74	
82	0	48.21	10.21	-2.46	2.56	74.20	41.18	45.69	54.48	59.74	blk. It.3
	1.5	10.03	1.64	-79.34	79.09	14.66	-62.38	80.21	-71.74	94.61	
	3	-81.46	-15.38	-156.23	155.62	-122.36	-213.92	66.74	-259.26	68.18	
83	0	-79.99	-15.03	154.34	-154.19	-120.03	66.91	-210.76	68.60	-255.35	blk. It.3
	1.5	9.47	1.55	77.17	-77.04	13.84	77.98	-60.81	91.95	-69.97	
	3	45.62	9.69	0.00	0.11	70.24	41.06	41.16	54.00	54.12	
84	0	45.62	9.69	0.00	0.11	70.24	41.06	41.16	54.00	54.12	blk. It.3
	1.5	8.27	1.25	-77.16	77.26	11.91	-62.01	76.97	-71.56	90.59	
	3	-82.40	-15.63	-154.33	154.41	-123.89	-213.06	64.81	-258.41	65.76	
85	0	-80.88	-15.39	154.17	-154.72	-121.68	65.96	-212.04	67.26	-257.08	blk. It.3
	1.5	9.04	1.36	77.12	-77.33	13.06	77.54	-61.46	91.33	-70.83	
	3	45.65	9.71	0.06	0.06	70.32	41.14	41.14	54.11	54.12	

Lanjutan Tabel 4.14.a Momen Rencana Balok Portal As A-D (K=1)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
86	0	45.65	9.71	0.06	0.06	70.32	41.14	41.14	54.11	54.12	blk. tt.3
	1.5	8.76	1.46	-77.00	77.45	12.86	-61.41	77.59	-70.72	91.45	
	3	-81.44	-15.22	-154.06	154.84	-122.08	-211.94	66.06	-256.86	67.48	
87	0	-79.12	-14.62	154.63	-155.84	-118.34	67.95	-211.46	70.07	-255.92	blk. tt.3
	1.5	11.11	2.02	78.19	-78.75	16.55	80.36	-60.88	95.03	-69.75	
	3	48.02	10.22	1.75	-1.66	73.97	44.79	41.73	58.69	55.12	
88	0	48.02	10.22	1.75	-1.66	73.97	44.79	41.73	58.69	55.12	blk. tt.3
	1.5	11.44	1.84	-74.69	75.43	16.67	-56.93	78.19	-65.26	92.37	
	3	-78.45	-14.98	-151.13	152.53	-118.11	-206.63	66.66	-250.50	68.34	
89	0	-22.68	-2.55	193.47	-196.01	-31.30	153.71	-196.83	177.72	-231.24	blk. tt.3
	1.75	9.27	0.86	-2.82	2.97	12.50	5.81	11.01	7.32	13.39	
	3.5	-23.19	-3.77	-199.10	201.95	-33.85	-200.06	160.88	-235.78	185.32	
90	0	-50.01	-14.77	113.89	-113.86	-83.65	57.49	-147.49	57.76	-181.37	blk. tt.4
	1.5	7.71	2.10	59.05	-59.04	12.61	60.08	-46.20	71.42	-52.57	
	3	33.77	10.54	4.22	-4.22	57.39	34.19	26.59	46.53	37.67	
91	0	33.77	10.54	4.22	-4.22	57.39	34.19	26.59	46.53	37.67	blk. tt.4
	1.5	7.99	2.40	-50.62	50.60	13.43	-38.37	52.73	-43.25	63.03	
	3	-49.45	-14.17	-105.45	105.43	-82.02	-139.41	50.38	-171.58	49.84	
92	0	-31.07	-6.72	124.13	-124.48	-48.03	83.76	-139.99	93.48	-167.55	blk. tt.4
	1.5	2.51	-0.27	-0.02	0.01	2.58	2.24	2.26	2.44	2.47	
	3	-17.23	-2.26	-124.18	124.50	-24.29	-127.27	96.54	-149.90	111.20	
93	0	-76.76	-14.38	103.67	-103.61	-115.12	24.22	-162.33	19.20	-198.44	blk. tt.4
	1.5	12.39	2.16	51.41	-51.37	18.32	57.42	-35.08	68.35	-39.57	
	3	48.23	10.25	-0.86	0.87	74.28	42.63	44.19	56.20	58.01	
94	0	48.23	10.25	-0.86	0.87	74.28	42.63	44.19	56.20	58.01	blk. tt.4
	1.5	10.56	1.78	-53.13	53.11	15.51	-38.31	57.31	-43.57	67.97	
	3	-80.42	-15.14	-105.39	105.35	-120.73	-167.22	22.44	-204.64	16.64	
95	0	-79.87	-15.06	100.40	-100.41	-119.95	18.47	-162.26	12.06	-198.79	blk. tt.4
	1.5	9.52	1.52	50.09	-50.10	13.86	53.66	-36.52	63.56	-41.65	
	3	45.61	9.67	-0.21	0.21	70.20	40.86	41.23	53.76	54.20	
96	0	45.61	9.67	-0.21	0.21	70.20	40.86	41.23	53.76	54.20	blk. tt.4
	1.5	8.19	1.24	-50.51	50.52	11.82	-38.09	52.84	-43.65	62.43	
	3	-82.54	-15.63	-100.81	100.83	-124.05	-165.02	16.46	-202.36	9.36	
97	0	-81.04	-15.43	100.91	-100.92	-121.93	17.88	-163.76	11.15	-200.77	blk. tt.4
	1.5	8.97	1.35	50.52	-50.53	12.92	53.54	-37.40	63.31	-42.79	
	3	45.66	9.69	0.14	-0.14	70.28	41.21	40.97	54.18	53.89	
98	0	45.66	9.69	0.14	-0.14	70.28	41.21	40.97	54.18	53.89	blk. tt.4
	1.5	8.85	1.44	-50.25	50.25	12.93	-37.26	53.19	-42.56	62.96	
	3	-81.27	-15.24	-100.63	100.64	-121.91	-163.72	17.43	-200.60	10.73	
99	0	-78.00	-14.24	103.88	-103.87	-116.38	23.29	-163.68	18.20	-199.93	blk. tt.4
	1.5	11.71	2.24	52.12	-52.12	17.64	57.45	-36.37	68.44	-41.02	
	3	48.11	10.29	0.37	-0.37	74.19	43.62	42.96	57.37	56.60	
100	0	48.11	10.29	0.37	-0.37	74.19	43.62	42.96	57.37	56.60	blk. tt.4
	1.5	11.00	1.75	-51.39	51.38	16.01	-36.35	56.14	-41.30	66.61	
	3	-79.41	-15.22	-103.15	103.13	-119.64	-164.30	21.34	-201.27	15.32	
101	0	-19.76	-1.85	112.29	-112.05	-26.67	83.28	-118.62	95.99	-139.56	blk. tt.4
	1.75	8.96	0.69	-2.15	2.19	11.85	6.13	10.03	7.59	12.14	
	3.5	-26.74	-4.81	-116.58	116.43	-39.78	-128.99	80.72	-153.51	91.14	

Lanjutan Tabel 4.14.a Momen Rencana Balok Portal As A-D (K=1)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
102	0	-28.52	-8.75	59.72	-61.01	-48.22	28.08	-80.57	27.25	-99.51	blk. nok
	3	18.76	5.79	0.59	-0.95	31.77	17.41	16.03	23.96	22.35	
	6	-32.25	-10.06	-58.55	59.11	-54.78	-81.71	24.18	-101.67	21.87	
103	0	-25.69	-7.79	47.82	-48.05	-43.30	19.92	-66.37	18.32	-82.34	blk. nok
	1.5	1.96	1.34	-0.58	0.60	4.51	1.25	2.31	2.30	3.54	
	3	2.61	0.36	-48.97	49.26	3.71	-41.72	46.69	-48.45	54.69	
104	0	-49.92	-10.69	63.26	-63.20	-77.00	12.01	-101.80	7.27	-125.50	blk. nok
	3	25.33	5.06	0.30	-0.29	38.49	23.06	22.54	30.09	29.48	
	6	-47.20	-9.57	-62.67	62.62	-71.95	-98.88	13.68	-121.39	10.16	
105	0	-46.18	-9.40	55.23	-55.23	-70.45	8.14	-91.27	3.58	-112.40	blk. nok
	3	25.19	5.22	-0.12	0.11	38.88	22.56	22.77	29.61	29.86	
	6	-51.22	-10.53	-55.46	55.46	-78.32	-96.02	3.81	-118.66	-2.19	
106	0	-50.78	-11.01	55.55	-55.54	-78.56	4.30	-95.69	-1.92	-118.57	blk. nok
	3	25.12	5.28	0.04	-0.05	38.58	22.64	22.56	29.74	29.65	
	6	-46.77	-8.81	-55.47	55.45	-70.22	-92.01	7.81	-112.90	3.57	
107	0	-45.04	-9.01	60.06	-60.06	-68.46	13.52	-94.58	10.10	-116.03	blk. nok
	3	25.16	4.92	-0.28	0.29	38.06	22.39	22.91	29.22	29.82	
	6	-52.43	-11.53	-60.62	60.64	-81.36	-101.74	7.39	-125.96	1.36	
108	0	-0.32	0.50	42.08	-41.21	0.42	37.59	-37.38	44.17	-43.29	blk. nok
	1.75	6.84	2.11	-1.68	1.30	11.58	4.65	7.33	6.75	9.88	
	3.5	-17.23	-4.32	-45.44	43.81	-27.60	-56.41	23.92	-68.53	25.19	
109	0	-9.56	-2.58	24.17	-23.04	-15.60	13.15	-29.34	13.71	-35.86	blk. atap
	1.5	0.03	0.19	5.23	-5.07	0.34	4.73	-4.53	5.64	-5.17	
	3	-19.00	-8.85	-13.71	12.91	-36.96	-29.44	-5.48	-39.92	-11.97	
110	0	-64.85	-23.37	37.74	-37.32	-115.20	-24.40	-91.94	-43.18	-121.99	blk. atap
	1.5	9.52	3.94	19.45	-19.22	17.74	26.08	-8.72	32.91	-7.69	
	3	55.27	19.44	1.17	-1.12	97.43	50.80	48.74	71.51	69.11	
111	0	55.27	19.44	1.17	-1.12	97.43	50.80	48.74	71.51	69.11	blk. atap
	1.5	10.59	3.58	-17.12	16.98	16.44	-5.88	24.81	-4.60	31.20	
	3	-62.72	-24.09	-35.40	35.08	-113.81	-88.30	-24.87	-118.20	-44.20	
112	0	-43.75	-17.96	26.77	-26.70	-81.23	-15.28	-63.40	-29.14	-85.28	blk. atap
	3	17.42	7.33	-0.37	0.42	32.63	15.35	16.06	22.52	23.35	
	6	-35.89	-14.64	-27.50	27.54	-66.49	-57.06	-7.51	-75.79	-17.99	
113	0	-36.74	-15.26	27.39	-27.65	-68.50	-8.41	-57.95	-19.43	-77.22	blk. atap
	3	18.02	7.28	0.40	-0.34	33.26	16.58	15.91	23.92	23.14	
	6	-41.70	-17.44	-26.59	26.96	-77.95	-61.47	-13.27	-82.70	-26.47	
114	0	-55.55	-25.29	33.69	-34.45	-107.14	-19.68	-81.00	-38.89	-110.44	blk. atap
	1.5	9.54	3.54	15.87	-16.17	17.11	22.87	-5.97	28.91	-4.73	
	3	46.02	20.55	-1.96	2.11	88.11	39.66	43.31	59.22	63.48	
115	0	46.02	20.55	-1.96	2.11	88.11	39.66	43.31	59.22	63.48	blk. atap
	1.5	10.98	5.57	-19.78	20.39	22.08	-7.92	28.23	-5.73	36.44	
	3	-52.69	-21.23	-37.60	38.67	-97.20	-81.26	-12.62	-108.18	-28.10	

Tabel 4.14.b Gaya Geser Rencana Balok Portal As.A-D (K=1)

Elm	Jarak (m)	V _{D.B} (KN)	V _{L.B} (KN)	V _{E.B} (KN)	V _{G.B} (KN)	M _{KAP.B}	M' _{KAP.B}	V _{UB} = 0,7(M _{KAP.B} + M' _{KAP.B}) / Ln + 1,05 V _{G.B} (KN)	V _{U.b} terpakai	V _{UB} maks (KN)	KET
54	0	48.17	13.78	62.97	61.96	459.96	284.25	111.16	102.33	329.52	blk. tt.1
	1.5	27.07	8.16	62.97	35.23	459.96	284.25	83.09	74.26	301.46	
	3	5.96	2.53	62.97	8.50	459.96	284.25	55.02	46.19	273.39	
55	0	7.49	2.89	62.97	10.39	459.96	284.25	57.01	48.18	275.38	blk. tt.1
	1.5	28.60	8.52	62.97	37.12	459.96	284.25	85.08	76.25	303.44	
	3	49.70	14.14	62.97	63.85	459.96	284.25	113.14	104.31	331.51	
56	0	34.69	5.97	176.04	40.67	459.96	284.25	88.80	79.97	782.06	blk. tt.1
	1.5	0.85	0.35	176.04	1.20	459.96	284.25	47.36	38.53	740.62	
	3	36.39	5.28	176.04	41.67	459.96	284.25	89.85	81.02	783.11	
57	0	75.97	13.58	56.25	89.55	459.96	284.25	140.12	131.29	330.28	blk. tt.1
	1.5	40.43	7.95	56.25	48.38	459.96	284.25	96.90	88.07	287.05	
	3	4.88	2.33	56.25	7.21	459.96	284.25	53.67	44.84	243.82	
58	0	8.57	3.10	56.25	11.67	459.96	284.25	58.36	49.53	248.51	blk. tt.1
	1.5	44.12	8.73	56.25	52.84	459.96	284.25	101.58	92.75	291.74	
	3	79.66	14.35	56.25	94.01	459.96	284.25	144.81	135.98	334.96	
59	0	77.69	13.92	59.36	91.61	459.96	284.25	142.29	133.46	345.50	blk. tt.1
	1.5	42.14	8.30	59.36	50.44	459.96	284.25	99.07	90.24	302.27	
	3	6.60	2.67	59.36	9.28	459.96	284.25	55.84	47.01	259.05	
60	0	6.86	2.75	59.36	9.61	459.96	284.25	56.19	47.36	259.40	blk. tt.1
	1.5	42.40	8.38	59.36	50.78	459.96	284.25	99.42	90.59	302.62	
	3	77.94	14.00	59.36	91.94	459.96	284.25	142.64	133.81	345.85	
61	0	77.57	13.92	59.47	91.48	459.96	284.25	142.16	133.33	345.84	blk. tt.1
	1.5	42.02	8.29	59.47	50.32	459.96	284.25	98.93	90.10	302.61	
	3	6.48	2.67	59.47	9.15	459.96	284.25	55.71	46.88	259.38	
62	0	6.97	2.76	59.47	9.73	459.96	284.25	56.32	47.49	260.00	blk. tt.1
	1.5	42.52	8.39	59.47	50.90	459.96	284.25	99.55	90.72	303.23	
	3	78.06	14.01	59.47	92.07	459.96	284.25	142.77	133.94	346.45	
63	0	78.97	14.18	57.22	93.15	459.96	284.25	143.91	135.08	338.14	blk. tt.1
	1.5	43.43	8.56	57.22	51.99	459.96	284.25	100.69	91.86	294.92	
	3	7.89	2.93	57.22	10.82	459.96	284.25	57.46	48.63	251.69	
64	0	5.57	2.50	57.22	8.06	459.96	284.25	54.57	45.74	248.80	blk. tt.1
	1.5	41.11	8.12	57.22	49.23	459.96	284.25	97.79	88.96	292.02	
	3	76.65	13.75	57.22	90.40	459.96	284.25	141.02	132.19	335.25	
65	0	41.48	5.29	142.19	46.77	459.96	284.25	95.21	86.38	646.29	blk. tt.1
	1.5	4.67	0.69	142.19	5.37	459.96	284.25	51.74	42.91	602.82	
	3	32.13	3.90	142.19	38.03	459.96	284.25	83.94	75.11	635.02	
66	0	48.53	13.89	64.49	62.42	610.58	426.37	129.78	117.48	336.41	blk. tt.2
	1.5	27.43	8.27	64.49	35.69	610.58	426.37	101.71	89.41	308.34	
	3	6.32	2.64	64.49	8.96	610.58	426.37	73.65	61.35	280.27	
67	0	7.13	2.79	64.49	9.92	610.58	426.37	74.65	62.35	281.28	blk. tt.2
	1.5	28.24	8.41	64.49	36.65	610.58	426.37	102.72	90.42	309.34	
	3	49.34	14.04	64.49	63.38	610.58	426.37	130.78	118.48	337.41	
68	0	36.71	6.23	176.79	42.94	610.58	426.37	109.32	97.02	787.58	blk. tt.2
	1.5	1.17	0.60	176.79	1.77	610.58	426.37	66.10	53.80	744.36	
	3	34.38	5.02	176.79	39.40	610.58	426.37	105.60	93.30	783.86	
69	0	76.59	13.72	59.08	90.30	610.58	426.37	159.05	146.75	342.93	blk. tt.2
	1.5	41.04	8.09	59.08	49.13	610.58	426.37	115.83	103.53	299.71	
	3	5.50	2.47	59.08	7.97	610.58	426.37	72.60	60.30	256.48	

Lanjutan Tabel 4.14.b Gaya Geser Rencana Balok Portal As.A-D (K=1)

Elm	Jarak (m)	Vd.B (KN)	Vi.B (KN)	VE.B (KN)	Vg.B (KN)	M _{KAP.B}	M' _{KAP.B}	VUB= 0,7(M _{KAP.B} + M' _{KAP.B}) /Ln+1,05.Vg.B (KN)	VU.b terpakai	VUB maks (KN)	KET
70	0	7.96	2.96	59.08	10.92	610.58	426.37	75.70	63.40	259.58	blk. tt.2
	1.5	43.50	8.59	59.08	52.09	610.58	426.37	118.93	106.63	302.81	
	3	79.04	14.21	59.08	93.25	610.58	426.37	162.15	149.85	346.03	
71	0	77.48	13.88	60.72	91.36	610.58	426.37	160.16	147.86	350.95	blk. tt.2
	1.5	41.94	8.26	60.72	50.19	610.58	426.37	116.94	104.64	307.72	
	3	6.39	2.63	60.72	9.02	610.58	426.37	73.71	61.41	264.50	
72	0	7.06	2.80	60.72	9.86	610.58	426.37	74.59	62.29	265.37	blk. tt.2
	1.5	42.60	8.42	60.72	51.03	610.58	426.37	117.81	105.51	308.60	
	3	78.15	14.05	60.72	92.20	610.58	426.37	161.04	148.74	351.82	
73	0	77.69	13.96	60.83	91.66	610.58	426.37	160.48	148.18	351.73	blk. tt.2
	1.5	42.15	8.34	60.83	50.49	610.58	426.37	117.25	104.95	308.51	
	3	6.61	2.71	60.83	9.32	610.58	426.37	74.02	61.72	265.28	
74	0	6.85	2.71	60.83	9.56	610.58	426.37	74.27	61.97	265.53	blk. tt.2
	1.5	42.39	8.34	60.83	50.73	610.58	426.37	117.50	105.20	308.76	
	3	77.93	13.96	60.83	91.90	610.58	426.37	160.73	148.43	351.99	
75	0	78.34	14.02	59.71	92.35	610.58	426.37	161.21	148.91	347.75	blk. tt.2
	1.5	42.80	8.39	59.71	51.19	610.58	426.37	117.98	105.68	304.52	
	3	7.25	2.77	59.71	10.02	610.58	426.37	74.75	62.45	261.30	
76	0	6.20	2.66	59.71	8.87	610.58	426.37	73.55	61.25	260.09	blk. tt.2
	1.5	41.75	8.29	59.71	50.03	610.58	426.37	116.77	104.47	303.31	
	3	77.29	13.91	59.71	91.20	610.58	426.37	160.00	147.70	346.54	
77	0	38.40	4.65	141.16	43.05	610.58	426.37	109.43	97.13	638.09	blk. tt.2
	1.75	1.59	0.05	141.16	1.65	610.58	426.37	65.96	53.66	594.62	
	3.5	35.22	4.54	141.16	39.76	610.58	426.37	105.98	93.68	634.63	
78	0	48.77	13.95	55.43	62.72	459.96	284.25	111.95	103.12	298.64	blk. tt.3
	1.5	27.67	8.32	55.43	35.99	459.96	284.25	83.89	75.06	270.57	
	3	6.56	2.70	55.43	9.26	459.96	284.25	55.82	46.99	242.50	
79	0	6.90	2.73	55.43	9.63	459.96	284.25	56.21	47.38	242.89	blk. tt.3
	1.5	28.00	8.36	55.43	36.36	459.96	284.25	84.27	75.44	270.96	
	3	49.11	13.93	55.43	63.09	459.96	284.25	112.34	103.51	299.02	
80	0	38.81	6.74	141.10	45.56	459.96	284.25	93.94	85.11	640.46	blk. tt.3
	1.5	3.27	1.12	141.10	4.39	459.96	284.25	50.71	41.88	597.24	
	3	32.27	4.51	141.10	36.78	459.96	284.25	84.72	75.89	631.25	
81	0	76.86	13.77	51.02	90.63	459.96	284.25	141.26	132.43	309.46	blk. tt.3
	1.5	41.32	8.15	51.02	49.46	459.96	284.25	98.04	89.21	266.23	
	3	5.77	2.52	51.02	8.29	459.96	284.25	54.81	45.98	223.01	
82	0	7.68	2.91	51.02	10.59	459.96	284.25	57.22	48.39	225.42	blk. tt.3
	1.5	43.23	8.53	51.02	51.76	459.96	284.25	100.45	91.62	268.64	
	3	78.77	14.16	51.02	92.92	459.96	284.25	143.67	134.84	311.87	
83	0	77.41	13.86	51.43	91.27	459.96	284.25	141.94	133.11	311.86	blk. tt.3
	1.5	41.87	8.24	51.43	50.11	459.96	284.25	98.71	89.88	268.63	
	3	6.33	2.61	51.43	8.94	459.96	284.25	55.49	46.66	225.41	
84	0	7.13	2.82	51.43	9.95	459.96	284.25	56.54	47.71	226.47	blk. tt.3
	1.5	42.67	8.44	51.43	51.11	459.96	284.25	99.77	90.94	269.69	
	3	78.22	14.07	51.43	92.28	459.96	284.25	143.00	134.17	312.92	
85	0	77.72	13.99	51.59	91.71	459.96	284.25	142.40	133.57	312.99	blk. tt.3
	1.5	42.18	8.37	51.59	50.55	459.96	284.25	99.17	90.34	269.76	
	3	6.64	2.74	51.59	9.38	459.96	284.25	55.95	47.12	226.54	

Lanjutan Tabel 4.14.b Gaya Geser Rencana Balok Portal As.A-D (K=1)

Elm	Jarak (m)	VD.B (KN)	VL.B (KN)	VE.B (KN)	VG.B (KN)	M _{KAP.B}	M' _{KAP.B}	VUB= 0,7(M _{KAP.B} + M' _{KAP.B}) /Ln+1,05.VG.B (KN)	VU.b terpakai	VUB maks (KN)	KET
86	0	6.82	2.69	51.59	9.51	459.96	284.25	56.08	47.25	226.67	blk. tt.3
	1.5	42.36	8.31	51.59	50.67	459.96	284.25	99.31	90.48	269.90	
	3	77.91	13.94	51.59	91.84	459.96	284.25	142.53	133.70	313.12	
87	0	77.93	13.90	51.39	91.83	459.96	284.25	142.52	133.69	312.28	blk. tt.3
	1.5	42.38	8.28	51.39	50.66	459.96	284.25	99.30	90.47	269.05	
	3	6.84	2.65	51.39	9.49	459.96	284.25	56.07	47.24	225.82	
88	0	6.62	2.77	51.39	9.39	459.96	284.25	55.96	47.13	225.71	blk. tt.3
	1.5	42.16	8.40	51.39	50.56	459.96	284.25	99.19	90.36	268.94	
	3	77.70	14.02	51.39	91.73	459.96	284.25	142.41	133.58	312.17	
89	0	36.66	4.25	113.70	40.91	459.96	284.25	89.05	80.22	520.51	blk. tt.3
	1.75	0.15	0.35	113.70	0.49	459.96	284.25	46.62	37.79	478.07	
	3.5	36.95	4.94	113.70	41.89	459.96	284.25	90.09	81.26	521.54	
90	0	49.03	14.06	36.55	63.10	310.82	284.25	103.11	96.05	219.75	blk. tt.4
	1.5	27.93	8.44	36.55	36.37	310.82	284.25	75.05	67.99	191.68	
	3	6.82	2.61	36.55	9.64	310.82	284.25	46.98	39.92	163.62	
91	0	6.63	2.61	36.55	9.25	310.82	284.25	46.57	39.51	163.21	blk. tt.4
	1.5	27.74	8.24	36.55	35.98	310.82	284.25	74.64	67.58	191.29	
	3	48.84	13.86	36.55	62.71	310.82	284.25	102.71	95.65	219.35	
92	0	40.15	7.11	82.99	47.27	310.82	284.25	86.49	79.43	398.19	blk. tt.4
	1.5	4.61	1.49	82.99	6.10	310.82	284.25	43.27	36.21	354.97	
	3	30.93	4.14	82.99	35.07	310.82	284.25	73.68	66.62	385.38	
93	0	77.20	13.84	34.83	91.04	310.82	284.25	132.45	125.39	241.87	blk. tt.4
	1.5	41.66	8.21	34.83	49.87	310.82	284.25	89.23	82.17	198.64	
	3	6.12	2.59	34.83	8.71	310.82	284.25	46.00	38.94	155.41	
94	0	7.34	2.84	34.83	10.18	310.82	284.25	47.55	40.49	156.96	blk. tt.4
	1.5	42.88	8.47	34.83	51.35	310.82	284.25	90.78	83.72	200.19	
	3	78.42	14.09	34.83	92.51	310.82	284.25	134.00	126.94	243.41	
95	0	77.37	13.87	33.54	91.24	310.82	284.25	132.66	125.60	236.67	blk. tt.4
	1.5	41.83	8.25	33.54	50.07	310.82	284.25	89.44	82.38	193.44	
	3	6.28	2.62	33.54	8.90	310.82	284.25	46.21	39.15	150.22	
96	0	7.17	2.81	33.54	9.98	310.82	284.25	47.34	40.28	151.35	blk. tt.4
	1.5	42.71	8.43	33.54	51.15	310.82	284.25	90.57	83.51	194.57	
	3	78.26	14.06	33.54	92.32	310.82	284.25	133.79	126.73	237.80	
97	0	77.77	14.00	33.59	91.77	310.82	284.25	133.22	126.16	237.45	blk. tt.4
	1.5	42.23	8.37	33.59	50.60	310.82	284.25	89.99	82.93	194.22	
	3	6.69	2.75	33.59	9.43	310.82	284.25	46.77	39.71	151.00	
98	0	6.77	2.68	33.59	9.45	310.82	284.25	46.79	39.73	151.01	blk. tt.4
	1.5	42.31	8.31	33.59	50.62	310.82	284.25	90.01	82.95	194.24	
	3	77.85	13.93	33.59	91.79	310.82	284.25	133.24	126.18	237.46	
99	0	77.58	13.80	34.50	91.36	310.82	284.25	132.81	125.75	240.84	blk. tt.4
	1.5	42.04	8.18	34.50	50.21	310.82	284.25	89.58	82.52	197.62	
	3	6.49	2.55	34.50	9.04	310.82	284.25	46.36	39.30	154.39	
100	0	6.96	2.88	34.50	9.84	310.82	284.25	47.19	40.13	155.23	blk. tt.4
	1.5	42.51	8.50	34.50	51.01	310.82	284.25	90.42	83.36	198.45	
	3	78.05	14.13	34.50	92.18	310.82	284.25	133.65	126.59	241.68	
101	0	34.81	3.75	65.28	38.56	310.82	284.25	77.35	70.29	314.65	blk. tt.4
	1.75	1.99	0.85	65.28	2.84	310.82	284.25	39.84	32.78	277.15	
	3.5	38.80	5.44	65.28	44.24	310.82	284.25	83.32	76.26	320.62	

Lanjutan Tabel 4.14.b Gaya Geser Rencana Balok Portal As.A-D (K=1)

Elm	Jarak (m)	V _{D.B} (KN)	V _{L.B} (KN)	V _{E.B} (KN)	V _{G.B} (KN)	M _{KAP.B}	M' _{KAP.B}	V _{UB} = 0,7(M _{KAP.B} + M' _{KAP.B}) / Ln + 1,05 V _{G.B} (KN)	V _{U.B} terpakai	V _{UB} maks (KN)	KET
102	0	32.14	9.91	20.02	42.05	310.82	284.25	81.01	73.95	128.23	blk. nok
	3	0.62	0.22	20.02	0.84	310.82	284.25	37.74	30.68	84.96	
	6	33.38	10.34	20.02	43.73	310.82	284.25	82.77	75.71	129.99	
103	0	27.43	9.47	32.44	36.90	310.82	284.25	75.61	68.55	174.99	blk. nok
	1.5	9.43	2.72	32.44	12.15	310.82	284.25	49.62	42.56	149.00	
	3	8.57	4.03	32.44	12.60	310.82	284.25	50.09	43.03	149.47	
104	0	49.71	10.31	20.97	60.02	310.82	284.25	99.89	92.83	151.10	blk. Nok
	3	0.45	0.19	20.97	0.64	310.82	284.25	37.53	30.47	88.74	
	6	48.81	9.94	20.97	58.75	310.82	284.25	98.55	91.49	149.76	
105	0	48.42	9.94	18.45	58.36	310.82	284.25	98.14	91.08	138.75	blk. Nok
	3	0.84	0.19	18.45	1.03	310.82	284.25	37.94	30.88	78.56	
	6	50.10	10.32	18.45	60.42	310.82	284.25	100.30	93.24	140.92	
106	0	49.93	10.49	18.50	60.42	310.82	284.25	100.30	93.24	141.14	blk. Nok
	3	0.67	0.37	18.50	1.04	310.82	284.25	37.95	30.89	78.78	
	6	48.59	9.76	18.50	58.35	310.82	284.25	98.13	91.07	138.96	
107	0	48.03	9.71	20.12	57.73	310.82	284.25	97.48	90.42	145.11	blk. Nok
	3	1.23	0.42	20.12	1.65	310.82	284.25	38.60	31.54	86.22	
	6	50.49	10.54	20.12	61.04	310.82	284.25	100.95	93.69	148.58	
108	0	13.02	3.22	24.29	16.23	310.82	284.25	53.91	46.85	119.08	blk. Nok
	1.75	4.83	1.38	24.29	6.21	310.82	284.25	43.39	36.32	108.55	
	3.5	22.68	5.97	24.29	28.66	310.82	284.25	66.95	59.89	132.12	
109	0	15.94	5.78	11.98	21.72	310.82	284.25	59.67	52.61	73.13	blk. atap
	1.5	3.15	2.09	11.98	5.24	310.82	284.25	42.36	35.30	55.82	
	3	22.23	9.97	11.98	32.19	310.82	284.25	70.66	63.60	84.12	
110	0	59.12	22.15	12.07	81.26	310.82	284.25	122.19	115.13	136.00	blk. Atap
	1.5	40.04	14.27	12.07	54.31	310.82	284.25	93.89	86.83	107.70	
	3	20.96	6.40	12.07	27.35	310.82	284.25	65.58	58.52	79.40	
111	0	20.25	6.64	12.07	26.89	310.82	284.25	65.09	58.03	78.91	blk. Atap
	1.5	39.33	14.51	12.07	53.84	310.82	284.25	93.40	86.34	107.21	
	3	58.41	22.39	12.07	80.80	310.82	284.25	121.70	114.64	135.51	
112	0	39.47	16.30	9.04	55.77	310.82	284.25	95.42	88.36	96.53	blk. Atap
	3	1.31	0.55	9.04	1.86	310.82	284.25	38.82	31.76	39.92	
	6	36.85	15.20	9.04	52.05	310.82	284.25	91.51	84.45	92.62	
113	0	37.33	15.39	9.10	52.72	310.82	284.25	92.22	85.16	93.58	blk. atap
	3	0.83	0.36	9.10	1.19	310.82	284.25	38.11	31.05	39.47	
	6	38.99	16.11	9.10	55.10	310.82	284.25	94.72	87.66	96.08	
114	0	52.94	23.16	12.19	76.10	310.82	284.25	116.76	109.70	131.09	blk. Atap
	1.5	33.86	15.28	12.19	49.14	310.82	284.25	88.46	81.40	102.78	
	3	14.78	7.41	12.19	22.19	310.82	284.25	60.16	53.10	74.48	
115	0	13.82	6.05	12.19	19.88	310.82	284.25	57.73	50.67	72.05	blk. atap
	1.5	32.90	13.93	12.19	46.83	310.82	284.25	86.03	78.97	100.35	
	3	51.98	21.80	12.19	73.79	310.82	284.25	114.34	107.28	128.66	

Tabel 4.14.c Momen Rencana Kolom Portal As.A-D (K=1)

Elm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	MEk (KNm)	Mek (KNm)	MKAP,b kiri (KNm)	MKAP,b kanan (KNm)	$M_{ik} = \frac{1}{2} \left(\frac{M_{ik} + M_{jk}}{2} + \frac{M_{ik} - M_{jk}}{2} \right)$	$M_{jk} = \frac{1}{2} \left(\frac{M_{ik} + M_{jk}}{2} - \frac{M_{ik} - M_{jk}}{2} \right)$	$M_{ik} + M_{jk} = 4/K(ME,k)$	KET
1	0	9.54	2.77	-242.34	238.62	0	459.96	280.54	1015.12	Kol Basmen	
	1.925	-5.23	-1.53	-75.28	74.29	0	459.96	280.54	304.90		
	3.85	-20.01	-5.63	91.77	-90.04	0	459.96	280.54	-405.31		
2	0	-5.40	-1.92	-290.32	287.37	459.96	459.96	561.08	1199.26	Kol Basmen	
	1.925	2.62	0.94	-49.28	48.87	459.96	459.96	561.08	208.98		
	3.85	10.63	3.79	191.76	-189.63	459.96	459.96	561.08	-781.31		
3	0	9.09	1.81	-287.91	285.74	459.96	459.96	561.08	1211.57	Kol Basmen	
	1.925	-5.13	-1.05	-50.31	49.97	459.96	459.96	561.08	203.39		
	3.85	-19.35	-3.91	187.30	-185.80	459.96	459.96	561.08	-804.79		
4	0	-0.99	-0.25	-271.04	270.35	459.96	459.96	561.08	1134.16	Kol Basmen	
	1.925	0.07	0.01	-58.73	58.59	459.96	459.96	561.08	246.17		
	3.85	1.13	0.27	153.58	-153.16	459.96	459.96	561.08	-641.83		
5	0	-0.81	-0.20	-271.40	272.08	459.96	459.96	561.08	1141.68	Kol Basmen	
	1.925	-0.19	-0.05	-58.13	58.25	459.96	459.96	561.08	244.38		
	3.85	0.43	0.11	155.14	-155.59	459.96	459.96	561.08	-652.92		
6	0	-0.75	-0.20	-270.34	272.39	459.96	459.96	561.08	1143.06	Kol Basmen	
	1.925	-0.39	-0.08	-58.45	58.82	459.96	459.96	561.08	246.53		
	3.85	-0.03	0.03	153.44	-154.76	459.96	459.96	561.08	-650.00		
7	0	-9.73	-2.18	-282.50	286.09	459.96	459.96	561.08	1189.11	Kol Basmen	
	1.925	4.24	0.93	-51.88	52.42	459.96	459.96	561.08	225.60		
	3.85	18.20	4.01	178.74	-181.25	459.96	459.96	561.08	-737.92		
8	0	-3.66	-0.55	-178.58	181.39	459.96	0	280.54	757.40	Kol Basmen	
	1.925	1.32	0.15	-34.94	35.40	459.96	0	280.54	150.23		
	3.85	6.29	0.65	108.71	-110.58	459.96	0	280.54	-456.95		
9	0	25.95	7.68	-112.88	111.96	0	610.58	372.41	505.53	KOL LT.1	
	1.925	0.84	0.27	1.14	-0.98	0	610.58	372.41	-2.95		
	3.85	-24.27	-7.14	114.96	-113.92	0	610.58	372.41	-511.43		
10	0	-15.45	-5.42	-250.92	249.11	610.58	610.58	744.81	1024.33	KOL LT.1	
	1.925	-1.04	-0.27	-3.45	3.46	610.58	610.58	744.81	13.15		
	3.85	13.38	4.89	244.01	-242.19	610.58	610.58	744.81	-998.03		
11	0	26.59	5.37	-244.98	243.71	610.58	610.58	744.81	1057.12	KOL LT.1	
	1.925	0.31	0.09	-2.95	2.96	610.58	610.58	744.81	12.84		
	3.85	-25.98	-5.19	239.09	-237.79	610.58	610.58	744.81	-1031.45		
12	0	-1.98	-0.44	-198.67	198.29	610.58	610.58	744.81	830.26	KOL LT.1	
	1.925	-0.26	-0.05	-1.12	1.13	610.58	610.58	744.81	4.40		
	3.85	1.46	0.34	196.43	-196.04	610.58	610.58	744.81	-821.46		
13	0	-0.72	-0.16	-200.18	200.42	610.58	610.58	744.81	840.86	KOL LT.1	
	1.925	-0.02	0.00	-1.54	1.52	610.58	610.58	744.81	6.37		
	3.85	0.67	0.16	197.10	-197.38	610.58	610.58	744.81	-828.12		
14	0	0.24	0.00	-198.05	198.90	610.58	610.58	744.81	835.64	KOL LT.1	
	1.925	0.19	0.05	-1.18	1.14	610.58	610.58	744.81	5.03		
	3.85	0.15	0.10	195.68	-196.63	610.58	610.58	744.81	-825.58		
15	0	-25.53	-5.64	-231.79	233.62	610.58	610.58	744.81	948.46	KOL LT.1	
	1.925	-0.10	-0.01	-2.89	2.85	610.58	610.58	744.81	11.84		
	3.85	25.32	5.62	226.00	-227.92	610.58	610.58	744.81	-924.78		
16	0	-8.93	-1.23	-139.29	140.58	610.58	0	372.41	579.77	KOL LT.1	
	1.925	0.26	0.08	-2.12	2.07	610.58	0	372.41	9.06		
	3.85	8.44	1.39	135.06	-136.44	610.58	0	372.41	-561.65		

Lanjutan Tabel 4.14.c Momen Rencana Kolom Portal As.A-D (K=1)

Elm	Jarak (m)	MO,k (KNm)	ML,k (KNm)	ME _{K1} (KNm)	ME _{K2} (KNm)	MKAP,b kiri (KNm)	MKAP,b kanan (KNm)	$M_{11,k} = \frac{h}{L} \left[\frac{1}{2} (1-\alpha) \left(\frac{1}{L} \right) \left(\frac{1}{L} \right) \right] M_{11,k}$	MU _K = 1.05 (MD,k+ML,k+ 4/K ME _K) (KNm)	KET
17	0	23.75	6.97	-91.45	90.36	0	459.96	280.54	411.77	KOL LT 2
	1.925	-0.21	-0.06	10.58	-10.68	0	459.96	280.54	-45.12	
	3.85	-24.16	-7.10	112.61	-111.71	0	459.96	280.54	-502.02	
18	0	-10.96	-4.26	-206.82	205.03	459.96	459.96	561.08	845.15	KOL LT 2
	1.925	-0.02	0.01	7.44	-7.42	459.96	459.96	561.08	-31.18	
	3.85	10.91	4.28	221.70	-219.87	459.96	459.96	561.08	-907.51	
19	0	26.53	5.31	-203.02	201.74	459.96	459.96	561.08	680.74	KOL LT 2
	1.925	-0.38	-0.07	7.42	-7.42	459.96	459.96	561.08	-31.62	
	3.85	-27.29	-5.46	217.86	-216.57	459.96	459.96	561.08	-943.99	
20	0	-0.84	-0.17	-167.22	166.82	459.96	459.96	561.08	699.59	KOL LT 2
	1.925	0.00	0.01	9.25	-9.24	459.96	459.96	561.08	-38.81	
	3.85	0.84	0.19	185.72	-185.31	459.96	459.96	561.08	-777.21	
21	0	-0.71	-0.11	-166.91	167.21	459.96	459.96	561.08	701.40	KOL LT 2
	1.925	-0.02	0.00	9.08	-9.07	459.96	459.96	561.08	-38.13	
	3.85	0.68	0.11	185.08	-185.35	459.96	459.96	561.08	-777.66	
22	0	-0.85	-0.23	-166.53	167.50	459.96	459.96	561.08	702.36	KOL LT 2
	1.925	-0.01	0.02	9.16	-9.14	459.96	459.96	561.08	-38.38	
	3.85	0.84	0.26	184.84	-185.78	459.96	459.96	561.08	-779.12	
23	0	-26.32	-5.80	-191.25	193.17	459.96	459.96	561.08	777.57	KOL LT 2
	1.925	0.31	0.08	7.69	-7.68	459.96	459.96	561.08	-31.83	
	3.85	26.93	5.95	206.63	-208.52	459.96	459.96	561.08	-841.24	
24	0	-10.73	-1.69	-111.89	113.27	459.96	0	280.54	462.69	KOL LT 2
	1.925	0.21	0.06	4.74	-4.72	459.96	0	280.54	-19.52	
	3.85	11.14	1.81	121.37	-122.70	459.96	0	280.54	-501.74	
25	0	24.47	7.14	-65.10	63.69	0	310.82	189.58	300.70	KOL LT 3
	1.925	0.60	0.23	24.32	-23.39	0	310.82	189.58	-97.36	
	3.85	-23.27	-6.69	113.74	-110.47	0	310.82	189.58	-495.41	
26	0	-9.09	-3.81	-150.01	148.30	310.82	310.82	379.15	609.33	KOL LT 3
	1.925	-0.56	-0.13	14.24	-13.85	310.82	310.82	379.15	-58.90	
	3.85	7.97	3.56	178.49	-176.01	310.82	310.82	379.15	-727.13	
27	0	28.62	5.66	-147.36	146.22	310.82	310.82	379.15	650.33	KOL LT 3
	1.925	0.00	-0.04	14.75	-14.45	310.82	310.82	379.15	-60.73	
	3.85	-28.62	-5.94	176.86	-175.13	310.82	310.82	379.15	-771.80	
28	0	-0.64	-0.17	-124.84	124.51	310.82	310.82	379.15	522.09	KOL LT 3
	1.925	0.03	0.04	17.85	-17.73	310.82	310.82	379.15	-74.37	
	3.85	0.69	0.25	160.55	-159.96	310.82	310.82	379.15	-670.83	
29	0	-0.84	-0.14	-123.42	123.78	310.82	310.82	379.15	518.84	KOL LT 3
	1.925	0.01	0.01	17.44	-17.54	310.82	310.82	379.15	-73.64	
	3.85	0.86	0.16	158.30	-158.85	310.82	310.82	379.15	-666.12	
30	0	-1.48	-0.34	-123.85	124.90	310.82	310.82	379.15	522.67	KOL LT 3
	1.925	0.02	-0.04	17.59	-17.93	310.82	310.82	379.15	-75.31	
	3.85	1.51	0.25	159.03	-160.75	310.82	310.82	379.15	-673.29	
31	0	-28.84	-6.48	-137.97	140.02	310.82	310.82	379.15	551.00	KOL LT 3
	1.925	0.18	0.10	14.91	-15.43	310.82	310.82	379.15	-64.50	
	3.85	29.20	6.68	167.79	-170.88	310.82	310.82	379.15	-680.02	
32	0	-12.05	-1.96	-77.73	79.25	310.82	0	189.58	318.13	KOL LT 3
	1.925	-0.11	-0.06	10.07	-10.48	310.82	0	189.58	-44.21	
	3.85	11.83	1.83	97.87	-100.21	310.82	0	189.58	-406.55	

Tabel Gaya 4.14.d Geser Rencana Kolom Portal As.A-D (K=1)

Elm	Jarak (m)	VD,k (KN)	VL,k (KN)	VE,k (KN)	MU, k bawah (KN)	MU, k atas (KN)	VU, k = (MU, k a + MU, k b) // h _k (KN)	VU, k = 1,05(VD, k + VL, k ± 4/1. VE, k) (KN)	KET
1	0	7.68	2.24	86.78	459.96	459.96	301.61	374.89	Kol Basmen
	1.925	7.68	2.24	86.78	459.96	459.96	301.61	374.89	
	3.85	7.68	2.24	86.78	459.96	459.96	301.61	374.89	
2	0	4.17	1.49	125.21	459.96	459.96	301.61	531.83	Kol Basmen
	1.925	4.17	1.49	125.21	459.96	459.96	301.61	531.83	
	3.85	4.17	1.49	125.21	459.96	459.96	301.61	531.83	
3	0	7.39	1.49	123.43	459.96	459.96	301.61	527.72	Kol Basmen
	1.925	7.39	1.49	123.43	459.96	459.96	301.61	527.72	
	3.85	7.39	1.49	123.43	459.96	459.96	301.61	527.72	
4	0	0.55	0.13	110.29	459.96	459.96	301.61	463.93	Kol Basmen
	1.925	0.55	0.13	110.29	459.96	459.96	301.61	463.93	
	3.85	0.55	0.13	110.29	459.96	459.96	301.61	463.93	
5	0	0.32	0.08	110.79	459.96	459.96	301.61	465.74	Kol Basmen
	1.925	0.32	0.08	110.79	459.96	459.96	301.61	465.74	
	3.85	0.32	0.08	110.79	459.96	459.96	301.61	465.74	
6	0	0.19	0.06	110.07	459.96	459.96	301.61	462.56	Kol Basmen
	1.925	0.19	0.06	110.07	459.96	459.96	301.61	462.56	
	3.85	0.19	0.06	110.07	459.96	459.96	301.61	462.56	
7	0	7.26	1.60	119.80	459.96	459.96	301.61	512.47	Kol Basmen
	1.925	7.26	1.60	119.80	459.96	459.96	301.61	512.47	
	3.85	7.26	1.60	119.80	459.96	459.96	301.61	512.47	
8	0	2.58	0.37	74.62	459.96	459.96	301.61	316.50	Kol Basmen
	1.925	2.58	0.37	74.62	459.96	459.96	301.61	316.50	
	3.85	2.58	0.37	74.62	459.96	459.96	301.61	316.50	
9	0	13.04	3.85	59.13	459.96	610.58	351.00	266.07	KOL LT.1
	1.925	13.04	3.85	59.13	459.96	610.58	351.00	266.07	
	3.85	13.04	3.85	59.13	459.96	610.58	351.00	266.07	
10	0	7.49	2.68	128.55	459.96	610.58	351.00	550.60	KOL LT.1
	1.925	7.49	2.68	128.55	459.96	610.58	351.00	550.60	
	3.85	7.49	2.68	128.55	459.96	610.58	351.00	550.60	
11	0	13.66	2.74	125.73	459.96	610.58	351.00	545.28	KOL LT.1
	1.925	13.66	2.74	125.73	459.96	610.58	351.00	545.28	
	3.85	13.66	2.74	125.73	459.96	610.58	351.00	545.28	
12	0	0.89	0.20	102.62	459.96	610.58	351.00	432.16	KOL LT.1
	1.925	0.89	0.20	102.62	459.96	610.58	351.00	432.16	
	3.85	0.89	0.20	102.62	459.96	610.58	351.00	432.16	
13	0	0.36	0.08	103.19	459.96	610.58	351.00	433.65	KOL LT.1
	1.925	0.36	0.08	103.19	459.96	610.58	351.00	433.65	
	3.85	0.36	0.08	103.19	459.96	610.58	351.00	433.65	
14	0	0.02	0.03	102.27	459.96	610.58	351.00	429.58	KOL LT.1
	1.925	0.02	0.03	102.27	459.96	610.58	351.00	429.58	
	3.85	0.02	0.03	102.27	459.96	610.58	351.00	429.58	
15	0	13.21	2.93	118.91	459.96	610.58	351.00	516.35	KOL LT.1
	1.925	13.21	2.93	118.91	459.96	610.58	351.00	516.35	
	3.85	13.21	2.93	118.91	459.96	610.58	351.00	516.35	
16	0	4.77	0.68	71.26	459.96	610.58	351.00	305.01	KOL LT.1
	1.925	4.77	0.68	71.26	459.96	610.58	351.00	305.01	
	3.85	4.77	0.68	71.26	459.96	610.58	351.00	305.01	

Lanjutan Tabel 4.14.d Gaya Geser Rencana Kolom Portal As.A-D (K=1)

Elm	Jarak (m)	VD,k (KN)	VL,k (KN)	VE,k (KN)	MU,k bawah (KN)	MU,k atas (KN)	VU,k = (MU,k a + MU,k b) / h _k (KN)	VU,k = 1.05(VD,k + VL,k + 4/1. VE,k) (KN)	KET
17	0	12.44	3.66	53.00	459.96	610.58	351.00	239.51	Kol Basmen
	1.925	12.44	3.66	53.00	459.96	610.58	351.00	239.51	
	3.85	12.44	3.66	53.00	459.96	610.58	351.00	239.51	
18	0	5.68	2.22	111.30	459.96	610.58	351.00	475.77	Kol Basmen
	1.925	5.68	2.22	111.30	459.96	610.58	351.00	475.77	
	3.85	5.68	2.22	111.30	459.96	610.58	351.00	475.77	
19	0	13.98	2.80	109.32	459.96	610.58	351.00	476.76	Kol Basmen
	1.925	13.98	2.80	109.32	459.96	610.58	351.00	476.76	
	3.85	13.98	2.80	109.32	459.96	610.58	351.00	476.76	
20	0	0.44	0.09	91.67	459.96	610.58	351.00	385.58	Kol Basmen
	1.925	0.44	0.09	91.67	459.96	610.58	351.00	385.58	
	3.85	0.44	0.09	91.67	459.96	610.58	351.00	385.58	
21	0	0.36	0.06	91.43	459.96	610.58	351.00	384.42	Kol Basmen
	1.925	0.36	0.06	91.43	459.96	610.58	351.00	384.42	
	3.85	0.36	0.06	91.43	459.96	610.58	351.00	384.42	
22	0	0.44	0.13	91.26	459.96	610.58	351.00	383.90	Kol Basmen
	1.925	0.44	0.13	91.26	459.96	610.58	351.00	383.90	
	3.85	0.44	0.13	91.26	459.96	610.58	351.00	383.90	
23	0	13.83	3.05	103.35	459.96	610.58	351.00	451.78	Kol Basmen
	1.925	13.83	3.05	103.35	459.96	610.58	351.00	451.78	
	3.85	13.83	3.05	103.35	459.96	610.58	351.00	451.78	
24	0	5.68	0.91	60.59	459.96	610.58	351.00	261.38	Kol Basmen
	1.925	5.68	0.91	60.59	459.96	610.58	351.00	261.38	
	3.85	5.68	0.91	60.59	459.96	610.58	351.00	261.38	
25	0	12.40	3.59	46.45	610.58	459.96	351.00	211.89	KOL LT.1
	1.925	12.40	3.59	46.45	610.58	459.96	351.00	211.89	
	3.85	12.40	3.59	46.45	610.58	459.96	351.00	211.89	
26	0	4.43	1.91	85.33	610.58	459.96	351.00	365.03	KOL LT.1
	1.925	4.43	1.91	85.33	610.58	459.96	351.00	365.03	
	3.85	4.43	1.91	85.33	610.58	459.96	351.00	365.03	
27	0	14.87	3.07	84.21	610.58	459.96	351.00	372.51	KOL LT.1
	1.925	14.87	3.07	84.21	610.58	459.96	351.00	372.51	
	3.85	14.87	3.07	84.21	610.58	459.96	351.00	372.51	
28	0	0.34	0.11	74.13	610.58	459.96	351.00	311.81	KOL LT.1
	1.925	0.34	0.11	74.13	610.58	459.96	351.00	311.81	
	3.85	0.34	0.11	74.13	610.58	459.96	351.00	311.81	
29	0	0.44	0.08	73.18	610.58	459.96	351.00	307.88	KOL LT.1
	1.925	0.44	0.08	73.18	610.58	459.96	351.00	307.88	
	3.85	0.44	0.08	73.18	610.58	459.96	351.00	307.88	
30	0	0.78	0.15	73.47	610.58	459.96	351.00	309.57	KOL LT.1
	1.925	0.78	0.15	73.47	610.58	459.96	351.00	309.57	
	3.85	0.78	0.15	73.47	610.58	459.96	351.00	309.57	
31	0	15.07	3.42	79.42	610.58	459.96	351.00	352.97	KOL LT.1
	1.925	15.07	3.42	79.42	610.58	459.96	351.00	352.97	
	3.85	15.07	3.42	79.42	610.58	459.96	351.00	352.97	
32	0	6.20	0.99	45.61	610.58	459.96	351.00	199.12	KOL LT.1
	1.925	6.20	0.99	45.61	610.58	459.96	351.00	199.12	
	3.85	6.20	0.99	45.61	610.58	459.96	351.00	199.12	

Lanjutan Tabel 4.14.d Gaya Geser Rencana Kolom Portal As.A-D (K=1)

Elm	Jarak (m)	VD,k (KN)	VL,k (KN)	VE,k (KN)	MU, k bawah (KN)	MU, k atas (KN)	VU, k = (MU, k a + MU, k b) / h _k (KN)	VU, k = 1,05(VD, k + VL, k ± 4/1. VE, k) (KN)	KET
33	0	21.25	6.48	23.03	459.96	310.82	252.71	125.83	Kol Basmen
	1.34	21.25	6.48	23.03	459.96	310.82	252.71	125.83	
	2.68	21.25	6.48	23.03	459.96	310.82	252.71	125.83	
34	0	6.90	2.47	54.18	459.96	310.82	252.71	237.40	Kol Basmen
	1.34	6.90	2.47	54.18	459.96	310.82	252.71	237.40	
	2.68	6.90	2.47	54.18	459.96	310.82	252.71	237.40	
35	0	20.64	3.81	53.50	459.96	310.82	252.71	250.36	Kol Basmen
	1.34	20.64	3.81	53.50	459.96	310.82	252.71	250.36	
	2.68	20.64	3.81	53.50	459.96	310.82	252.71	250.36	
36	0	1.16	0.43	51.62	459.96	310.82	252.71	218.48	Kol Basmen
	1.34	1.16	0.43	51.62	459.96	310.82	252.71	218.48	
	2.68	1.16	0.43	51.62	459.96	310.82	252.71	218.48	
37	0	0.75	0.15	50.42	459.96	310.82	252.71	212.72	Kol Basmen
	1.34	0.75	0.15	50.42	459.96	310.82	252.71	212.72	
	2.68	0.75	0.15	50.42	459.96	310.82	252.71	212.72	
38	0	2.63	1.18	51.81	459.96	310.82	252.71	221.59	Kol Basmen
	1.34	2.63	1.18	51.81	459.96	310.82	252.71	221.59	
	2.68	2.63	1.18	51.81	459.96	310.82	252.71	221.59	
39	0	20.41	4.12	52.05	459.96	310.82	252.71	244.37	Kol Basmen
	1.34	20.41	4.12	52.05	459.96	310.82	252.71	244.37	
	2.68	20.41	4.12	52.05	459.96	310.82	252.71	244.37	
40	0	12.37	2.81	24.67	459.96	310.82	252.71	119.56	Kol Basmen
	1.34	12.37	2.81	24.67	459.96	310.82	252.71	119.56	
	2.68	12.37	2.81	24.67	459.96	310.82	252.71	119.56	
41	0	4.79	1.29	18.52	310.82	310.82	203.82	84.17	KOL LT 1
	1.1	4.79	1.29	18.52	310.82	310.82	203.82	84.17	
	2.2	4.79	1.29	18.52	310.82	310.82	203.82	84.17	
42	0	34.37	9.92	34.35	310.82	310.82	203.82	190.80	KOL LT 1
	1.1	34.37	9.92	34.35	310.82	310.82	203.82	190.80	
	2.2	34.37	9.92	34.35	310.82	310.82	203.82	190.80	
43	0	10.39	3.30	41.40	310.82	310.82	203.82	188.25	KOL LT 1
	1.1	10.39	3.30	41.40	310.82	310.82	203.82	188.25	
	2.2	10.39	3.30	41.40	310.82	310.82	203.82	188.25	
44	0	0.77	0.65	35.56	310.82	310.82	203.82	150.83	KOL LT 1
	1.1	0.77	0.65	35.56	310.82	310.82	203.82	150.83	
	2.2	0.77	0.65	35.56	310.82	310.82	203.82	150.83	
45	0	7.82	4.72	39.36	310.82	310.82	203.82	178.49	KOL LT 1
	1.1	7.82	4.72	39.36	310.82	310.82	203.82	178.49	
	2.2	7.82	4.72	39.36	310.82	310.82	203.82	178.49	
46	0	37.35	13.29	23.92	310.82	310.82	203.82	153.65	KOL LT 1
	1.1	37.35	13.29	23.92	310.82	310.82	203.82	153.65	
	2.2	37.35	13.29	23.92	310.82	310.82	203.82	153.65	

Tabel 4.15.a Momen Rencana Balok Portal As A - D (K=2)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
54	0	-45.96	-13.51	408.90	-404.00	-76.77	326.65	-404.96	372.58	-480.97	blk. tt.1
	1.5	10.47	2.95	217.85	-215.09	17.27	205.48	-184.16	241.58	-213.00	
	3	35.24	10.97	26.79	-26.18	59.83	55.82	8.15	72.03	16.42	
55	0	35.24	10.97	26.79	-26.18	59.83	55.82	8.15	72.03	16.42	blk. tt.1
	1.5	8.17	2.41	-164.27	162.73	13.65	-140.49	153.81	-162.39	180.96	
	3	-50.56	-14.59	-355.33	351.63	-84.01	-365.30	270.97	-435.38	306.94	
56	0	-24.47	-5.38	530.02	-525.84	-37.98	454.99	-495.28	527.43	-581.22	blk. tt.1
	1.5	0.91	-0.64	-2.11	2.27	0.06	-1.08	2.86	-1.67	2.94	
	3	-27.02	-4.34	-534.24	530.39	-39.36	-505.14	453.03	-592.06	525.80	
57	0	-72.96	-13.62	330.30	-328.63	-109.34	231.61	-361.43	261.63	-430.25	blk. tt.1
	1.5	14.33	2.53	160.81	-159.87	21.25	157.63	-130.99	185.50	-151.22	
	3	48.31	10.24	-8.68	8.88	74.36	35.67	51.47	48.07	66.50	
58	0	48.31	10.24	-8.68	8.88	74.36	35.67	51.47	48.07	66.50	blk. tt.1
	1.5	8.79	1.37	-178.17	177.63	12.75	-152.44	167.79	-176.98	196.62	
	2.5	-84.04	-15.93	-347.66	346.39	-126.33	-388.52	236.12	-463.31	265.43	
59	0	-80.93	-15.23	356.84	-356.51	-121.48	248.32	-393.70	280.11	-469.91	blk. tt.1
	1.5	8.94	1.44	178.73	-178.44	13.03	168.90	-152.54	197.96	-177.06	
	3	45.50	9.67	0.62	-0.36	70.07	41.51	40.63	54.51	53.49	
60	0	45.50	9.67	0.62	-0.36	70.07	41.51	40.63	54.51	53.49	blk. tt.1
	1.5	8.56	1.32	-177.50	177.72	12.38	-152.04	167.65	-176.55	196.43	
	3	-81.69	-15.47	-355.61	355.80	-122.78	-393.57	246.70	-468.91	278.07	
61	0	-80.54	-15.20	355.03	-356.24	-120.97	247.04	-393.10	278.64	-468.19	blk. tt.1
	1.5	9.15	1.46	177.35	-177.82	13.31	167.85	-151.80	196.75	-176.19	
	3	45.53	9.68	-0.33	0.59	70.12	40.68	41.51	53.56	54.52	
62	0	45.53	9.68	-0.33	0.59	70.12	40.68	41.51	53.56	54.52	blk. tt.1
	1.5	8.41	1.32	-178.01	179.00	12.21	-152.63	168.67	-177.24	197.62	
	3	-82.02	-15.48	-355.68	357.41	-123.18	-393.93	247.85	-469.34	279.41	
63	0	-82.28	-15.45	347.30	-349.92	-123.46	238.52	-388.98	268.53	-463.54	blk. tt.1
	1.25	9.52	1.60	177.03	-178.25	13.99	167.90	-151.85	196.89	-176.15	
	2.5	48.01	10.22	6.77	-6.58	73.96	49.30	37.29	63.95	49.94	
64	0	48.01	10.22	6.77	-6.58	73.96	49.30	37.29	63.95	49.94	blk. tt.1
	1.5	13.01	2.25	-163.50	165.08	19.21	-135.44	160.28	-156.60	188.41	
	3	-75.31	-14.15	-333.77	336.75	-113.02	-368.17	235.29	-438.45	265.59	
65	0	-31.58	-4.51	487.28	-492.98	-45.10	410.13	-472.10	475.65	-553.62	blk. tt.1
	1.5	8.81	0.73	-4.36	4.67	11.73	4.01	12.13	5.13	14.61	
	3	-15.22	-2.08	-495.99	502.32	-21.59	-460.09	438.39	-538.08	510.15	
66	0	-48.02	-14.11	412.81	-408.56	-80.19	328.32	-410.92	374.15	-488.29	blk. tt.2
	1.75	8.96	2.51	217.43	-215.09	14.76	203.75	-185.52	239.29	-214.86	
	3.5	34.27	10.69	22.06	-21.61	58.22	50.69	11.39	65.87	20.02	
67	0	34.27	10.69	22.06	-21.61	58.22	50.69	11.39	65.87	20.02	blk. tt.2
	1.5	7.74	2.29	-173.32	171.86	12.95	-149.02	161.64	-172.42	190.03	
	3	-50.44	-14.55	-368.70	365.33	-83.81	-377.23	283.40	-449.27	321.47	
68	0	-26.11	-5.40	532.97	-529.11	-39.98	456.17	-499.70	528.80	-586.39	blk. tt.2
	1.25	2.30	-0.28	-1.07	1.24	2.31	1.11	3.19	1.11	3.54	
	2.5	-22.61	-3.59	-535.11	531.60	-32.88	-501.94	458.09	-587.86	532.17	
69	0	-75.11	-14.10	349.10	-347.46	-112.69	246.59	-380.32	278.81	-452.58	blk. tt.2
	1.5	13.11	2.26	171.15	-170.23	19.34	165.83	-141.41	194.89	-163.56	
	3	48.02	10.17	-6.60	6.99	73.90	37.10	49.51	49.69	64.17	

Lanjutan Tabel 4.15.a Momen Rencana Balok Portal As A-D (K=2)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
70	0	48.02	10.17	-6.80	6.99	73.90	37.10	49.51	49.69	64.17	blk. tt.2
	1.5	9.43	1.51	-184.74	184.22	13.73	-157.78	174.28	-183.13	204.28	
	3	-82.48	-15.59	-362.69	361.45	-123.92	-400.65	251.07	-477.25	283.10	
71	0	-80.18	-15.07	364.61	-364.27	-120.34	255.98	-400.00	289.15	-476.17	blk. tt.2
	1.5	9.38	1.53	182.39	-182.11	13.70	172.59	-155.46	202.32	-180.40	
	3	45.63	9.69	0.18	0.05	70.26	41.23	41.11	54.20	54.07	
72	0	45.63	9.69	0.18	0.05	70.26	41.23	41.11	54.20	54.07	blk. tt.2
	1.5	8.38	1.28	-182.03	182.21	12.09	-156.29	171.52	-181.54	200.92	
	3	-82.19	-15.58	-364.25	364.37	-123.55	-401.79	253.96	-478.57	286.47	
73	0	-80.61	-15.31	363.77	-364.60	-121.47	254.67	-401.05	287.46	-477.54	blk. tt.2
	1.5	9.08	1.41	181.91	-182.31	13.15	171.88	-155.91	201.42	-181.00	
	3	45.64	9.70	0.04	0.19	70.30	41.12	41.25	54.08	54.24	
74	0	45.64	9.70	0.04	0.19	70.30	41.12	41.25	54.08	54.24	blk. tt.2
	1.5	8.72	1.41	-181.82	182.69	12.72	-155.80	172.26	-180.87	201.86	
	3	-81.53	-15.31	-363.69	365.18	-122.33	-400.69	255.29	-477.12	288.19	
75	0	-80.53	-14.99	360.74	-363.08	-120.61	252.19	-399.24	284.78	-475.22	blk. tt.2
	1.5	10.32	1.82	182.87	-183.95	15.30	173.87	-156.26	204.00	-181.16	
	3	47.86	10.18	5.01	-4.82	73.73	47.58	38.74	61.93	51.61	
76	0	47.86	10.18	5.01	-4.82	73.73	47.58	38.74	61.93	51.61	blk. tt.2
	1.5	11.90	1.97	-172.86	174.31	17.43	-144.86	167.58	-167.77	196.76	
	3	-77.38	-14.68	-350.72	353.43	-116.34	-385.29	248.45	-458.75	280.61	
77	0	-25.74	-3.26	483.79	-488.74	-36.11	412.24	-463.03	478.90	-542.26	blk. tt.2
	1.75	9.25	0.85	-5.06	5.33	12.46	3.77	13.13	4.94	15.85	
	3.5	-20.17	-3.08	-493.91	499.41	-29.12	-462.67	431.32	-541.72	501.26	
78	0	-48.63	-14.24	355.43	-350.81	-81.14	276.12	-359.50	313.16	-428.38	blk. tt.3
	1.5	8.70	2.46	187.12	-184.53	14.37	176.23	-158.25	207.16	-183.08	
	3	34.36	10.73	16.82	-16.26	58.40	47.86	14.50	62.60	23.67	
79	0	34.36	10.73	16.82	-16.26	58.40	47.86	14.50	62.60	23.67	blk. tt.3
	1.5	8.19	2.41	-149.49	148.02	13.69	-127.17	140.59	-146.84	165.54	
	3	-49.64	-14.34	-317.79	314.29	-82.51	-330.69	238.19	-394.84	268.85	
80	0	-29.64	-6.26	425.62	-422.05	-45.57	356.39	-406.52	411.84	-478.22	blk. tt.3
	1.5	1.93	-0.36	-1.10	1.25	1.73	0.75	2.86	0.64	3.11	
	3	-19.82	-2.91	-427.81	424.56	-28.44	-402.87	364.26	-471.85	423.14	
81	0	-75.73	-14.23	302.63	-301.02	-113.64	204.21	-339.08	229.28	-404.55	blk. tt.3
	1.5	12.90	2.21	148.86	-147.96	19.02	145.58	-121.55	171.23	-140.42	
	3	48.21	10.21	-4.92	5.11	74.20	38.97	47.99	51.90	62.43	
82	0	48.21	10.21	-4.92	5.11	74.20	38.97	47.99	51.90	62.43	blk. tt.3
	1.5	10.03	1.64	-158.68	158.18	14.66	-133.79	151.39	-155.05	177.65	
	3	-81.46	-15.38	-312.45	311.25	-122.36	-354.53	206.81	-423.30	231.59	
83	0	-79.99	-15.03	308.67	-308.38	-120.03	205.82	-349.53	230.65	-417.25	blk. tt.3
	1.5	9.47	1.55	154.34	-154.08	13.84	147.43	-130.15	172.98	-150.86	
	3	45.62	9.69	0.01	0.22	70.24	41.06	41.26	54.01	54.23	
84	0	45.62	9.69	0.01	0.22	70.24	41.06	41.26	54.01	54.23	blk. tt.3
	1.5	8.27	1.25	-154.32	154.52	11.91	-131.45	146.51	-152.58	171.71	
	3	-82.40	-15.63	-308.66	308.83	-123.89	-351.95	203.78	-420.46	227.90	
85	0	-80.88	-15.39	308.34	-309.44	-121.68	204.71	-351.29	229.14	-419.53	blk. tt.3
	1.5	9.04	1.38	154.23	-154.66	13.06	146.94	-131.06	172.30	-152.03	
	3	45.65	9.71	0.12	0.12	70.32	41.19	41.19	54.17	54.18	

Lanjutan Tabel 4.15.a Momen Rencana Balok Portal As A-D (K=2)

ELM	Jarak (m)	MD (kNm)	ML (kNm)	M.GEMPA (kNm)		1.2MD + 1.6ML (kNm)	0.9(MD ± ME) (kNm)		1.05(MD+0.6ML ± ME) (kNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
86	0	45.65	9.71	0.12	0.12	70.32	41.19	41.19	54.17	54.18	blk. It. 3
	1.5	8.76	1.46	-154.00	154.90	12.86	-130.71	147.30	-151.57	172.77	
	3	-81.44	-15.22	-308.11	309.68	-122.08	-350.59	205.42	-418.61	230.06	
87	0	-79.12	-14.62	309.26	-311.67	-118.34	207.12	-351.72	232.43	-419.55	blk. It. 3
	1.5	11.11	2.02	156.38	-157.49	16.55	150.73	-131.75	177.13	-152.44	
	3	48.02	10.22	3.50	-3.31	73.97	46.37	40.24	60.53	53.38	
88	0	48.02	10.22	3.50	-3.31	73.97	46.37	40.24	60.53	53.38	blk. It. 3
	1.5	11.44	1.84	-149.39	150.87	16.67	-124.15	146.08	-143.69	171.58	
	3	-78.45	-14.98	-302.27	305.05	-118.11	-342.65	203.94	-409.19	228.49	
89	0	-22.68	-2.55	386.94	-392.03	-31.30	327.63	-373.24	360.66	-437.05	blk. It. 3
	1.75	9.27	0.86	-5.63	5.93	12.50	3.28	13.88	4.37	16.51	
	3.5	-23.19	-3.77	-398.20	403.89	-33.85	-379.25	342.63	-444.83	397.37	
90	0	-50.01	-14.77	227.78	-227.73	-83.65	158.99	-249.96	177.35	-300.93	blk. It. 4
	1.5	7.71	2.10	118.11	-118.08	12.61	113.23	-99.34	133.43	-114.57	
	3	33.77	10.54	8.44	-8.44	57.39	37.98	22.80	50.96	33.24	
91	0	33.77	10.54	8.44	-8.44	57.39	37.98	22.80	50.96	33.24	blk. It. 4
	1.5	7.99	2.40	-101.23	101.21	13.43	-83.92	98.28	-96.39	116.17	
	3	-49.45	-14.17	-210.90	210.85	-82.02	-234.32	145.26	-282.30	160.54	
92	0	-31.07	-6.72	248.26	-248.96	-48.03	195.48	-252.02	223.82	-298.26	blk. It. 4
	1.5	2.51	-0.27	-0.05	0.02	2.58	2.21	2.27	2.41	2.48	
	3	-17.23	-2.26	-248.35	248.99	-24.29	-239.03	208.58	-280.29	241.92	
93	0	-76.76	-14.38	207.34	-207.22	-115.12	117.53	-255.58	128.06	-307.24	blk. It. 4
	1.5	12.39	2.16	102.81	-102.74	18.32	103.68	-81.31	122.32	-93.51	
	3	48.23	10.25	-1.72	1.74	74.28	41.86	44.97	55.29	58.93	
94	0	48.23	10.25	-1.72	1.74	74.28	41.86	44.97	55.29	58.93	blk. It. 4
	1.5	10.56	1.78	-106.25	106.22	15.51	-86.12	105.11	-99.35	123.74	
	3	-80.42	-15.14	-210.78	210.70	-120.73	-262.06	117.26	-315.30	127.26	
95	0	-79.87	-15.06	200.79	-200.82	-119.95	108.83	-252.63	117.48	-304.22	blk. It. 4
	1.5	9.52	1.52	100.19	-100.20	13.96	98.74	-81.61	116.15	-94.26	
	3	45.61	9.67	-0.42	0.42	70.20	40.67	41.42	53.54	54.42	
96	0	45.61	9.67	-0.42	0.42	70.20	40.67	41.42	53.54	54.42	blk. It. 4
	1.5	8.19	1.24	-101.02	101.04	11.82	-83.55	98.31	-96.69	115.47	
	3	-82.54	-15.63	-201.63	201.66	-124.05	-255.75	107.21	-308.22	115.23	
97	0	-81.04	-15.43	201.81	-201.83	-121.93	108.70	-254.58	117.10	-306.73	blk. It. 4
	1.5	8.97	1.35	101.04	-101.06	12.92	99.01	-82.88	116.36	-95.84	
	3	45.66	9.69	0.27	-0.28	70.28	41.34	40.84	54.33	53.75	
98	0	45.66	9.69	0.27	-0.28	70.28	41.34	40.84	54.33	53.75	blk. It. 4
	1.5	8.85	1.44	-100.50	100.50	12.93	-82.49	98.41	-95.32	115.72	
	3	-81.27	-15.24	-201.27	201.28	-121.91	-254.29	108.00	-306.27	116.41	
99	0	-78.00	-14.24	207.75	-207.73	-116.38	116.78	-257.16	127.27	-308.99	blk. It. 4
	1.5	11.71	2.24	104.24	-104.24	17.64	104.36	-83.27	123.16	-95.74	
	3	48.11	10.29	0.73	-0.74	74.19	43.95	42.63	57.76	56.22	
100	0	48.11	10.29	0.73	-0.74	74.19	43.95	42.63	57.76	56.22	blk. It. 4
	1.5	11.00	1.75	-102.78	102.76	16.01	-82.60	102.38	-95.26	120.55	
	3	-79.41	-15.22	-206.29	206.25	-119.64	-257.13	114.16	-309.57	123.60	
101	0	-19.76	-1.85	224.58	-224.09	-26.67	184.34	-219.46	213.90	-257.21	blk. It. 4
	1.75	8.96	0.69	-4.29	4.38	11.85	4.20	12.01	5.34	14.44	
	3.5	-26.74	-4.81	-233.16	232.85	-39.78	-233.91	185.50	-275.92	213.39	

Lanjutan Tabel 4.15.a Momen Rencana Balok Portal As A-D (K=2)

ELM	Jarak (m)	MD (KNm)	ML (KNm)	M.GEMPA (KNm)		1.2MD + 1.6ML (KNm)	0.9(MD ± ME) (KNm)		1.05(MD+0.6ML ± ME) (KNm)		KET
				KIRI	KANAN		KIRI	KANAN	KIRI	KANAN	
102	0	-28.52	-8.75	119.44	-122.01	-48.22	81.83	-135.47	89.96	-163.56	blk. nok
	3	18.76	5.79	1.18	-1.90	31.77	17.94	15.18	24.58	21.35	
	6	-32.25	-10.06	-117.09	118.22	-54.78	-134.40	77.37	-163.14	83.94	
103	0	-25.69	-7.79	95.63	-96.11	-43.30	62.95	-109.62	68.53	-132.80	blk. nok
	1.5	1.96	1.34	-1.15	1.21	4.51	0.73	2.85	1.70	4.17	
	3	2.61	0.36	-97.94	98.52	3.71	-85.79	91.02	-99.87	106.41	
104	0	-49.92	-10.69	126.51	-126.40	-77.00	68.94	-158.68	73.69	-191.86	blk. nok
	3	25.33	5.06	0.59	-0.58	38.49	23.33	22.28	30.41	29.18	
	6	-47.20	-9.57	-125.33	125.25	-71.95	-155.28	70.24	-187.19	75.92	
105	0	-46.18	-9.40	110.46	-110.46	-70.45	57.85	-140.98	61.57	-170.39	blk. nok
	3	25.19	5.22	-0.24	0.23	38.58	22.46	22.87	29.49	29.98	
	6	-51.22	-10.53	-110.93	110.92	-78.32	-145.94	53.72	-176.89	56.04	
106	0	-50.78	-11.01	111.11	-111.08	-78.56	54.30	-145.67	56.41	-176.89	blk. nok
	3	25.12	5.28	0.09	-0.09	38.58	22.68	22.52	29.79	29.60	
	6	-46.77	-8.81	-110.94	110.90	-70.22	-141.93	57.72	-171.14	61.79	
107	0	-45.04	-9.01	120.12	-120.11	-68.46	67.58	-148.63	73.16	-179.09	blk. nok
	3	25.16	4.92	-0.56	0.58	38.06	22.14	23.17	28.93	30.13	
	6	-52.43	-11.53	-121.24	121.28	-81.36	-156.30	61.97	-189.61	65.03	
108	0	-0.32	0.50	84.17	-82.42	0.42	75.46	-74.47	88.36	-86.56	blk. nok
	1.75	6.84	2.11	-3.36	2.61	11.58	3.13	8.50	4.99	11.25	
	3.5	-17.23	-4.32	-90.89	87.63	-27.60	-97.31	63.36	-116.25	71.19	
109	0	-9.56	-2.58	48.34	-46.08	-15.60	34.90	-50.08	39.09	-60.05	blk. atap
	1.5	0.03	0.19	10.46	-10.13	0.34	9.44	-9.09	11.13	-10.49	
	3	-19.00	-8.85	-27.42	25.81	-36.96	-41.78	6.13	-54.32	1.58	
110	0	-64.85	-23.37	75.48	-74.63	-115.20	9.57	-125.53	-3.56	-161.17	blk. atap
	1.5	9.52	3.94	38.91	-38.43	17.74	43.59	-26.02	53.34	-27.87	
	3	55.27	19.44	2.34	-2.24	97.43	51.65	47.73	72.74	67.94	
111	0	55.27	19.44	2.34	-2.24	97.43	51.85	47.73	72.74	67.94	blk. atap
	1.5	10.59	3.58	-34.23	33.96	18.44	-21.28	40.09	-22.57	49.03	
	3	-62.72	-24.09	-70.80	70.16	-113.81	-120.16	6.70	-155.37	-7.36	
112	0	-43.75	-17.96	53.54	-53.39	-81.23	8.81	-87.43	-1.04	-113.31	blk. atap
	3	17.42	7.33	-0.74	0.85	32.63	15.02	16.44	22.13	23.80	
	6	-35.89	-14.64	-55.01	55.09	-66.49	-81.81	17.27	-104.67	10.93	
113	0	-36.74	-15.26	54.78	-55.29	-68.50	16.24	-82.83	9.33	-106.25	blk. atap
	3	18.02	7.28	0.80	-0.69	33.26	16.93	15.60	24.34	22.78	
	6	-41.70	-17.44	-53.19	53.92	-77.95	-85.40	10.99	-110.62	1.84	
114	0	-55.55	-25.29	67.38	-68.90	-107.14	10.65	-112.01	-3.52	-146.61	blk. atap
	1.5	9.54	3.54	31.74	-32.34	17.11	37.15	-20.52	45.57	-21.71	
	3	46.02	20.55	-3.91	4.22	88.11	37.90	45.21	57.16	65.70	
115	0	46.02	20.55	-3.91	4.22	88.11	37.90	45.21	57.16	65.70	blk. atap
	1.5	10.98	5.57	-39.56	40.77	22.08	-25.72	46.57	-26.50	57.64	
	3	-52.69	-21.23	-75.20	77.33	-97.20	-115.10	22.18	-147.66	12.50	

Tabel 4.15.b Gaya Geser Rencana Balok Portal As.A-D (K=2)

E/m	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	VUB=1,05(VD,B + VL,B ± 4/2. VE,B) (KN)	KETERANGAN
54	0	48.17	13.78	127.37	61.96	332.54	blk. lt.1
	1.5	27.07	8.16	127.37	35.23	304.47	
	3	5.96	2.53	127.37	8.50	276.40	
55	0	7.49	2.89	127.37	10.39	278.39	blk. lt.1
	1.5	28.60	8.52	127.37	37.12	306.46	
	3	49.70	14.14	127.37	63.85	334.52	
56	0	34.69	5.97	354.76	40.67	787.69	blk. lt.1
	1.5	0.85	0.35	354.76	1.20	746.24	
	3	36.39	5.28	354.76	41.67	788.74	
57	0	75.97	13.58	112.99	89.55	331.31	blk. lt.1
	1.5	40.43	7.95	112.99	48.38	298.09	
	3	4.88	2.33	112.99	7.21	244.86	
58	0	8.57	3.10	112.99	11.67	249.54	blk. lt.1
	1.5	44.12	8.73	112.99	52.84	292.77	
	3	79.66	14.35	112.99	94.01	335.99	
59	0	77.69	13.92	118.74	91.61	345.55	blk. lt.1
	1.5	42.14	6.30	118.74	50.44	302.32	
	3	6.60	2.67	118.74	9.28	259.09	
60	0	6.86	2.75	118.74	9.61	259.45	blk. lt.1
	1.5	42.40	8.38	118.74	50.78	302.67	
	3	77.94	14.00	118.74	91.94	345.90	
61	0	77.57	13.92	118.74	91.48	345.41	blk. lt.1
	1.5	42.02	8.29	118.74	50.32	302.19	
	3	6.48	2.67	118.74	9.15	258.96	
62	0	6.97	2.76	118.74	9.73	259.58	blk. lt.1
	1.5	42.52	8.39	118.74	50.90	302.80	
	3	78.06	14.01	118.74	92.07	346.03	
63	0	78.97	14.18	113.51	93.15	336.18	blk. lt.1
	1.5	43.43	8.56	113.51	51.99	292.96	
	3	7.89	2.93	113.51	10.82	249.73	
64	0	5.57	2.50	113.51	8.06	246.84	blk. lt.1
	1.5	41.11	8.12	113.51	49.23	290.07	
	3	76.65	13.75	113.51	90.40	333.29	
65	0	41.48	5.29	280.94	46.77	639.07	blk. lt.1
	1.5	4.67	0.69	280.94	5.37	595.60	
	3	32.13	3.90	280.94	36.03	627.80	
66	0	48.53	13.89	130.25	62.42	339.08	blk. lt.2
	1.5	27.43	8.27	130.25	35.69	311.01	
	3	6.32	2.64	130.25	8.96	282.94	
67	0	7.13	2.79	130.25	9.92	283.95	blk. lt.2
	1.5	28.24	8.41	130.25	36.65	312.01	
	3	49.34	14.04	130.25	63.38	340.08	
68	0	36.71	6.23	356.03	42.94	792.74	blk. lt.2
	1.5	1.17	0.60	356.03	1.77	749.51	
	3	34.36	5.02	356.03	39.40	789.02	
69	0	76.59	13.72	118.63	90.30	343.94	blk. lt.2
	1.5	41.04	8.09	118.63	49.13	300.72	
	3	5.50	2.47	118.63	7.97	257.49	
70	0	7.96	2.96	118.63	10.92	260.59	blk. lt.2
	1.5	43.50	8.59	118.63	52.09	303.82	
	3	79.04	14.21	118.63	93.25	347.04	
71	0	77.48	13.88	121.48	91.96	351.02	blk. lt.2
	1.5	41.94	8.26	121.48	50.19	307.80	
	3	6.39	2.63	121.48	9.02	264.57	

Lanjutan Tabel 4.15.b Gaya Geser Rencana Balok Portal As.A-D (K=2)

Etm	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	VUB=1,05(VD,B + VL,B ± 4/2 VE,B) (KN)	KETERANGAN
72	0	7.06	2.80	121.48	9.86	265.45	blk. tt.2
	1.5	42.60	8.42	121.48	51.03	308.68	
	3	78.15	14.05	121.48	92.20	351.90	
73	0	77.69	13.96	121.24	91.66	350.85	blk. tt.2
	1.5	42.15	8.34	121.24	50.49	307.62	
	3	6.61	2.71	121.24	9.32	264.40	
74	0	6.85	2.71	121.24	9.56	264.65	blk. tt.2
	1.5	42.39	8.34	121.24	50.73	307.88	
	3	77.93	13.96	121.24	91.90	351.10	
75	0	78.34	14.02	118.58	92.35	345.96	blk. tt.2
	1.5	42.80	8.39	118.58	51.19	302.75	
	3	7.25	2.77	118.58	10.02	259.53	
76	0	6.20	2.66	118.58	8.87	258.32	blk. tt.2
	1.5	41.75	8.29	118.58	50.03	301.54	
	3	77.29	13.91	118.58	91.20	344.77	
77	0	38.40	4.65	279.34	43.05	631.82	blk. tt.2
	1.75	1.59	0.05	279.34	1.65	588.35	
	3.5	35.22	4.54	279.34	39.76	628.36	
78	0	48.77	13.95	112.20	62.72	301.48	blk. tt.3
	1.5	27.67	8.32	112.20	35.99	273.41	
	3	6.56	2.70	112.20	9.26	245.35	
79	0	6.90	2.73	112.20	9.63	245.73	blk. tt.3
	1.5	28.00	8.36	112.20	36.36	273.80	
	3	49.11	13.99	112.20	63.09	301.87	
80	0	38.81	6.74	284.48	45.56	645.24	blk. tt.3
	1.5	3.27	1.12	284.48	4.39	602.01	
	3	32.27	4.51	284.48	36.78	636.02	
81	0	76.86	13.77	102.51	90.63	310.44	blk. tt.3
	1.5	41.32	8.15	102.51	49.46	267.21	
	3	5.77	2.52	102.51	8.29	223.99	
82	0	7.68	2.91	102.51	10.59	226.40	blk. tt.3
	1.5	43.23	8.53	102.51	51.76	269.62	
	3	78.77	14.16	102.51	92.92	312.85	
83	0	77.41	13.86	102.89	91.27	311.90	blk. tt.3
	1.5	41.87	8.24	102.89	50.11	268.68	
	3	6.33	2.61	102.89	8.94	225.45	
84	0	7.13	2.82	102.89	9.95	226.51	blk. tt.3
	1.5	42.67	8.44	102.89	51.11	269.73	
	3	79.22	14.07	102.89	92.28	312.96	
85	0	77.72	13.99	102.74	91.71	312.06	blk. tt.3
	1.5	42.18	8.37	102.74	50.55	268.83	
	3	6.64	2.74	102.74	9.38	225.60	
86	0	6.82	2.69	102.74	9.51	225.74	blk. tt.3
	1.5	42.36	8.31	102.74	50.67	268.96	
	3	77.91	13.94	102.74	91.84	312.19	
87	0	77.93	13.90	101.92	91.83	310.45	blk. tt.3
	1.5	42.38	8.28	101.92	50.66	267.23	
	3	6.84	2.65	101.92	9.49	224.00	
88	0	6.62	2.77	101.92	9.39	223.89	blk. tt.3
	1.5	42.16	8.40	101.92	50.56	267.12	
	3	77.70	14.02	101.92	91.73	310.35	
89	0	36.66	4.25	224.32	40.91	514.03	blk. tt.3
	1.75	0.15	0.35	224.32	0.49	471.60	
	3.5	36.95	4.94	224.32	41.89	515.07	

Lanjutan Tabel 4.15.b Gaya Geser Rencana Balok Portal As.A-D (K=2)

Elm	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	VUB=1,05(VD,B + VL,B ± 4/2. VE,B) (KN)	KETERANGAN
90	0	49.03	14.06	73.11	63.10	219.79	blk. It.4
	1.5	27.93	8.44	73.11	36.37	191.72	
	3	6.62	2.81	73.11	9.64	163.65	
91	0	6.63	2.81	73.11	9.25	163.25	blk. It.4
	1.5	27.74	8.24	73.11	35.96	191.31	
	3	48.84	13.86	73.11	62.71	219.38	
92	0	40.15	7.11	165.54	47.27	397.26	blk. It.4
	1.5	4.61	1.49	165.54	6.10	354.03	
	3	30.93	4.14	165.54	35.07	384.45	
93	0	77.20	13.64	69.69	91.04	241.93	blk. It.4
	1.5	41.66	8.21	69.69	49.87	198.71	
	3	6.12	2.59	69.69	8.71	155.48	
94	0	7.34	2.84	69.69	10.18	157.03	blk. It.4
	1.5	42.88	8.47	69.69	51.35	200.26	
	3	78.42	14.09	69.69	92.51	243.48	
95	0	77.37	13.87	67.07	91.24	236.65	blk. It.4
	1.5	41.83	8.25	67.07	50.07	193.42	
	3	6.28	2.62	67.07	8.90	150.20	
96	0	7.17	2.81	67.07	9.98	151.33	blk. It.4
	1.5	42.71	8.43	67.07	51.15	194.55	
	3	78.26	14.06	67.07	92.32	237.78	
97	0	77.77	14.00	67.18	91.77	237.44	blk. It.4
	1.5	42.23	8.37	67.18	50.60	194.21	
	3	6.69	2.75	67.18	9.43	150.98	
98	0	6.77	2.68	67.18	9.45	151.00	blk. It.4
	1.5	42.31	8.31	67.18	50.62	194.23	
	3	77.85	13.93	67.18	91.79	237.45	
99	0	77.58	13.80	69.01	91.38	240.86	blk. It.4
	1.5	42.04	8.18	69.01	50.21	197.64	
	3	6.49	2.55	69.01	9.04	154.41	
100	0	6.96	2.88	69.01	9.84	155.25	blk. It.4
	1.5	42.51	8.50	69.01	51.01	198.47	
	3	78.05	14.13	69.01	92.18	241.70	
101	0	34.81	3.75	130.78	38.56	315.13	blk. It.4
	1.75	1.99	0.85	130.78	2.84	277.62	
	3.5	38.80	5.44	130.78	44.24	321.10	
102	0	32.14	9.91	39.42	42.05	126.93	blk. nok
	3	0.62	0.22	39.42	0.64	63.67	
	6	33.38	10.34	39.42	43.73	128.70	
103	0	27.43	9.47	64.53	36.90	174.25	blk. nok
	1.5	9.43	2.72	64.53	12.15	148.26	
	3	8.57	4.03	64.53	12.60	148.73	
104	0	49.71	10.31	41.97	60.02	151.17	blk. Nok
	3	0.45	0.19	41.97	0.64	88.81	
	6	48.81	9.94	41.97	58.75	149.83	
105	0	48.42	9.94	36.90	58.36	138.76	blk. Nok
	3	0.84	0.19	36.90	1.03	78.57	
	6	50.10	10.32	36.90	60.42	140.92	
106	0	49.93	10.49	37.01	60.42	141.16	blk. Nok
	3	0.67	0.37	37.01	1.04	78.80	
	6	48.59	9.76	37.01	58.35	138.98	
107	0	48.03	9.71	40.23	57.73	145.10	blk. Nok
	3	1.23	0.42	40.23	1.65	86.21	
	6	50.49	10.54	40.23	61.04	148.56	

Lanjutan Tabel 4.15.b Gaya Geser Rencana Balok Portal As.A-D (K=2)

Elm	Jarak (m)	VD,B (KN)	VL,B (KN)	VE,B (KN)	VG,B (KN)	VUB=1,05(VD,B + VL,B ± 4/2. VE,B) (KN)	KETERANGAN
108	0	13.02	3.22	50.02	16.23	122.08	blk. Nok
	1.75	4.83	1.38	50.02	6.21	111.55	
	3.5	22.68	5.97	50.02	28.66	135.12	
109	0	15.94	5.78	25.26	21.72	75.84	blk. atap
	1.5	3.15	2.09	25.26	5.24	59.53	
	3	22.23	9.97	25.26	32.19	86.84	
110	0	59.12	22.15	24.38	81.26	136.52	blk. Atap
	1.5	40.04	14.27	24.38	54.31	108.22	
	3	20.96	6.40	24.38	27.35	79.92	
111	0	20.25	6.64	24.38	26.89	79.43	blk. Atap
	1.5	39.33	14.51	24.38	53.64	107.73	
	3	58.41	22.39	24.38	80.80	136.03	
112	0	39.47	16.30	18.09	55.77	96.55	blk. Atap
	3	1.31	0.55	18.09	1.86	39.95	
	6	36.85	15.20	18.09	52.05	92.64	
113	0	37.33	15.39	17.99	52.72	93.14	blk. atap
	3	0.83	0.36	17.99	1.19	39.04	
	6	38.99	16.11	17.99	55.10	95.64	
114	0	52.94	23.16	23.76	76.10	129.80	blk. Atap
	1.5	33.86	15.28	23.76	49.14	101.50	
	3	14.78	7.41	23.76	22.19	73.20	
115	0	13.82	6.05	23.76	19.88	70.77	blk. atap
	1.5	32.90	13.93	23.76	46.83	99.08	
	3	51.98	21.80	23.76	73.79	127.38	

Tabel 4.15.c Momen Rencana Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	ME,k (KNm)	MU,k = 1.05(MD,k + ML,k ± od.ME,k) (KNm)	KET
1	0	9.54	2.77	-484.68	664.35	Kol. Basmen
	1.925	-5.23	-1.53	-150.57	195.70	
	3.85	-20.01	-5.83	183.55	-272.95	
2	0	-5.40	-1.92	-580.64	776.83	Kol. Basmen
	1.925	2.62	0.94	-98.56	137.14	
	3.85	10.63	3.79	383.52	-502.55	
3	0	9.09	1.81	-575.82	791.53	Kol. Basmen
	1.925	-5.13	-1.05	-100.61	129.93	
	3.85	-19.35	-3.91	374.60	-531.66	
4	0	-0.99	-0.25	-542.08	736.75	Kol. Basmen
	1.925	0.07	0.01	-117.46	160.04	
	3.85	1.13	0.27	307.16	-416.68	
5	0	-0.81	-0.20	-542.80	741.72	Kol. Basmen
	1.925	-0.19	-0.05	-116.26	159.76	
	3.85	0.43	0.11	310.28	-424.20	
6	0	-0.75	-0.20	-540.67	742.64	Kol. Basmen
	1.925	-0.39	-0.08	-116.90	160.07	
	3.85	-0.03	0.03	306.88	-422.50	
7	0	-9.73	-2.16	-565.00	768.55	Kol. Basmen
	1.925	4.24	0.93	-103.76	148.54	
	3.85	18.20	4.01	357.48	-471.48	
8	0	-3.66	-0.55	-357.16	490.76	Kol. Basmen
	1.925	1.32	0.15	-69.88	98.19	
	3.85	6.29	0.85	217.41	-294.39	
9	0	25.95	7.68	-225.36	340.96	KOL LT.1
	1.925	0.84	0.27	2.28	-1.51	
	3.85	-24.27	-7.14	229.92	-343.97	
10	0	-15.45	-5.42	-501.84	658.14	KOL LT.1
	1.925	-1.04	-0.27	-6.91	8.07	
	3.85	13.38	4.89	488.03	-642.01	
11	0	26.59	5.37	-489.95	696.87	KOL LT.1
	1.925	0.31	0.09	-5.89	8.49	
	3.85	-25.98	-5.19	476.17	-661.69	
12	0	-1.98	-0.44	-397.34	538.78	KOL LT.1
	1.925	-0.26	-0.05	-2.24	2.75	
	3.85	1.48	0.34	392.86	-533.29	
13	0	-0.72	-0.16	-400.36	546.24	KOL LT.1
	1.925	-0.02	0.00	-3.08	4.13	
	3.85	0.67	0.16	394.19	-537.97	
14	0	0.24	0.00	-396.10	543.26	KOL LT.1
	1.925	0.19	0.05	-2.37	3.36	
	3.85	0.15	0.10	391.37	-536.54	
15	0	-25.53	-5.64	-463.57	-659.58	KOL LT.1
	1.925	-0.10	-0.01	-5.78	-5.98	
	3.85	25.32	5.62	452.01	647.62	
16	0	-8.93	-1.23	-278.58	-389.64	KOL LT.1
	1.925	0.26	0.08	-4.23	-3.48	
	3.85	9.44	1.39	270.12	382.67	
17	0	23.75	6.97	-182.89	-224.71	KOL LT.2
	1.925	-0.21	-0.06	21.16	30.69	
	3.85	-24.16	-7.10	225.22	286.10	
18	0	-10.96	-4.26	-413.65	-576.14	KOL LT.2
	1.925	-0.02	0.01	14.88	22.31	
	3.85	10.91	4.28	443.40	620.74	
19	0	26.53	5.31	-406.03	-526.38	KOL LT.2
	1.925	-0.38	-0.07	14.85	21.89	
	3.85	-27.29	-5.46	435.73	570.16	

Lanjutan Tabel 4.15.c Momen Rencana Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	ME,k (KNm)	MU,k= 1.05(MD,k + ML,k ± md.ME,k) (KNm)	KET
20	0	-0.84	-0.17	-334.44	-457.39	KOL LT.2
	1.925	0.00	0.01	18.50	27.27	
	3.85	0.84	0.19	371.44	511.94	
21	0	-0.71	-0.11	-333.82	-456.41	KOL LT.2
	1.925	-0.02	0.00	18.17	26.81	
	3.85	0.68	0.11	370.15	510.01	
22	0	-0.85	-0.23	-333.06	-455.52	KOL LT.2
	1.925	-0.01	0.02	18.31	27.01	
	3.85	0.84	0.26	369.68	509.53	
23	0	-26.32	-5.80	-382.50	-549.74	KOL LT.2
	1.925	0.31	0.08	15.38	23.34	
	3.85	26.93	5.95	413.26	596.42	
24	0	-10.73	-1.69	-223.79	-316.73	KOL LT.2
	1.925	0.21	0.06	9.47	15.17	
	3.85	11.14	1.81	242.73	347.07	
25	0	24.47	7.14	-130.21	-152.04	KOL LT.3
	1.925	0.60	0.23	48.64	69.04	
	3.85	-23.27	-6.69	227.48	290.12	
26	0	-9.09	-3.81	-300.02	-419.07	KOL LT.3
	1.925	-0.56	-0.13	28.48	40.31	
	3.85	7.97	3.56	356.99	499.70	
27	0	28.62	5.86	-294.71	-372.23	KOL LT.3
	1.925	0.00	-0.04	29.50	42.29	
	3.85	-28.62	-5.94	353.72	456.82	
28	0	-0.64	-0.17	-124.84	339.06	KOL LT.3
	1.925	0.03	0.04	17.85	-48.32	
	3.85	0.69	0.25	160.55	-435.69	
29	0	-0.84	-0.14	-123.42	336.69	KOL LT.3
	1.925	0.01	0.01	17.44	-47.86	
	3.85	0.86	0.16	158.30	-432.61	
30	0	-1.48	-0.34	-123.85	339.07	KOL LT.3
	1.925	0.02	-0.04	17.59	-48.96	
	3.85	1.51	0.25	159.03	-436.99	
31	0	-12.05	-1.96	-77.73	201.64	KOL LT.3
	1.925	-0.11	-0.06	10.07	-28.80	
	3.85	11.83	1.83	97.87	-259.24	
32	0	-12.05	-1.96	-77.73	201.64	KOL LT.3
	1.925	-0.11	-0.06	10.07	-28.80	
	3.85	11.83	1.83	97.87	-259.24	
33	0	26.74	8.09	-0.15	45.84	KOL LT.4
	1.34	-0.89	-0.33	29.79	-79.92	
	2.68	-28.52	-8.75	59.72	-205.67	
34	0	-10.42	-3.90	-51.09	132.11	KOL LT.4
	1.34	-1.45	-0.69	19.35	-55.20	
	2.68	7.53	2.53	89.78	-242.52	
35	0	30.91	6.19	-50.99	183.59	KOL LT.4
	1.34	4.08	1.23	18.55	-45.06	
	2.68	-22.76	-3.73	88.10	-273.71	
36	0	0.15	0.17	-45.24	125.39	KOL LT.4
	1.34	-1.36	-0.39	21.87	-61.56	
	2.68	-2.87	-0.94	88.98	-248.51	
37	0	-0.65	-0.04	-43.42	116.37	KOL LT.4
	1.34	0.32	0.15	22.13	-59.97	
	2.68	1.29	0.34	87.69	-236.31	
38	0	-1.76	-0.74	-45.48	116.82	KOL LT.4
	1.34	1.66	0.79	21.87	-57.15	
	2.68	5.08	2.32	89.22	-231.12	

Lanjutan Tabel 4.15.c Momen Rencana Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	MD,k (KNm)	ML,k (KNm)	MEki (KNm)	MU,k= 1.05(MD,k +ML,k + 0.5ME,k) (KNm)	KET
39	0	-30.46	-6.69	-47.65	81.92	KOL LT.4
	1.34	-3.92	-1.33	20.02	-60.05	
	2.68	22.61	4.02	67.68	-202.03	
40	0	-14.91	-2.98	-18.71	25.48	KOL LT.4
	1.34	1.16	0.67	13.37	-35.75	
	2.68	17.23	4.32	45.44	-96.98	
41	0	0.97	0.26	-16.58	40.78	KOL LT.4
	1.1	-4.30	-1.16	3.80	-17.43	
	2.2	-9.56	-2.58	24.17	-75.65	
42	0	29.77	7.32	-24.13	100.05	KOL LT.4
	1.1	-8.04	-3.60	13.66	-50.22	
	2.2	-45.85	-14.52	51.45	-200.49	
43	0	-3.89	-1.12	-28.91	71.98	KOL LT.4
	1.1	7.54	2.51	16.63	-35.15	
	2.2	18.97	6.13	62.17	-142.29	
44	0	0.85	0.82	-23.33	66.76	KOL LT.4
	1.1	0.00	0.10	15.78	-42.72	
	2.2	-0.85	-0.62	54.89	-152.21	
45	0	3.35	2.53	-26.32	82.61	KOL LT.4
	1.1	-5.25	-2.66	16.99	-53.91	
	2.2	-13.85	-7.85	60.29	-190.44	
46	0	-29.49	-8.01	-15.02	-51.47	KOL LT.4
	1.1	11.60	6.61	11.29	29.74	
	2.2	52.69	21.23	37.60	108.96	

Tabel 4.15.d Gaya Geser Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	VD,k (KNm)	VL,k (KNm)	VE,k (KNm)	VU,k= 1.05 (VD,k +VL,k ± mod VE,k) (KNm)	KET
1	0	7.68	2.24	173.56	247.32	Kol Basmen
	1.925	7.68	2.24	173.56	247.32	
	3.85	7.68	2.24	173.56	247.32	
2	0	4.17	1.48	250.43	347.76	Kol Basmen
	1.925	4.17	1.48	250.43	347.76	
	3.85	4.17	1.48	250.43	347.76	
3	0	7.39	1.49	246.86	346.28	Kol Basmen
	1.925	7.39	1.49	246.86	346.28	
	3.85	7.39	1.49	246.86	346.28	
4	0	0.55	0.13	220.58	301.81	Kol Basmen
	1.925	0.55	0.13	220.58	301.81	
	3.85	0.55	0.13	220.58	301.81	
5	0	0.32	0.08	221.58	302.88	Kol Basmen
	1.925	0.32	0.08	221.58	302.88	
	3.85	0.32	0.08	221.58	302.88	
6	0	0.19	0.06	220.14	300.75	Kol Basmen
	1.925	0.19	0.06	220.14	300.75	
	3.85	0.19	0.06	220.14	300.75	
7	0	7.26	1.60	239.60	336.36	Kol Basmen
	1.925	7.26	1.60	239.60	336.36	
	3.85	7.26	1.60	239.60	336.36	
8	0	2.58	0.37	149.24	206.81	Kol Basmen
	1.925	2.58	0.37	149.24	206.81	
	3.85	2.58	0.37	149.24	206.81	
9	0	13.04	3.85	118.25	179.15	KOL LT.1
	1.925	13.04	3.85	118.25	179.15	
	3.85	13.04	3.85	118.25	179.15	
10	0	7.49	2.68	257.11	361.63	KOL LT.1
	1.925	7.49	2.68	257.11	361.63	
	3.85	7.49	2.68	257.11	361.63	
11	0	13.66	2.74	251.46	360.46	KOL LT.1
	1.925	13.66	2.74	251.46	360.46	
	3.85	13.66	2.74	251.46	360.46	
12	0	0.89	0.20	205.24	281.31	KOL LT.1
	1.925	0.89	0.20	205.24	281.31	
	3.85	0.89	0.20	205.24	281.31	
13	0	0.36	0.08	206.38	282.17	KOL LT.1
	1.925	0.36	0.08	206.38	282.17	
	3.85	0.36	0.08	206.38	282.17	
14	0	0.02	0.03	204.54	279.24	KOL LT.1
	1.925	0.02	0.03	204.54	279.24	
	3.85	0.02	0.03	204.54	279.24	
15	0	13.21	2.93	237.81	341.56	KOL LT.1
	1.925	13.21	2.93	237.81	341.56	
	3.85	13.21	2.93	237.81	341.56	
16	0	4.77	0.68	142.52	200.26	KOL LT.1
	1.925	4.77	0.68	142.52	200.26	
	3.85	4.77	0.68	142.52	200.26	
17	0	12.44	3.66	106.00	161.60	KOL LT.2
	1.925	12.44	3.66	106.00	161.60	
	3.85	12.44	3.66	106.00	161.60	
18	0	5.68	2.22	222.61	312.15	KOL LT.2
	1.925	5.68	2.22	222.61	312.15	
	3.85	5.68	2.22	222.61	312.15	
19	0	13.98	2.80	218.64	316.06	KOL LT.2
	1.925	13.98	2.80	218.64	316.06	
	3.85	13.98	2.80	218.64	316.06	

Lanjutan Tabel 4.15.d Gaya Geser Rencana Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	VD,k (KNm)	VL,k (KNm)	VE,k (KNm)	VU,k= 1.05.(VD,k +VL,k ± mod.VE,k) (KNm)	KET
20	0	0.44	0.09	183.35	250.82	KOL LT.2
	1.925	0.44	0.09	183.35	250.82	
	3.85	0.44	0.09	183.35	250.82	
21	0	0.36	0.06	182.85	250.03	KOL LT.2
	1.925	0.36	0.06	182.85	250.03	
	3.85	0.36	0.06	182.85	250.03	
22	0	0.44	0.13	182.53	249.74	KOL LT.2
	1.925	0.44	0.13	182.53	249.74	
	3.85	0.44	0.13	182.53	249.74	
23	0	13.83	3.05	206.69	299.86	KOL LT.2
	1.925	13.83	3.05	206.69	299.86	
	3.85	13.83	3.05	206.69	299.86	
24	0	5.68	0.91	121.17	172.32	KOL LT.2
	1.925	5.68	0.91	121.17	172.32	
	3.85	5.68	0.91	121.17	172.32	
25	0	12.40	3.59	92.91	143.61	KOL LT.3
	1.925	12.40	3.59	92.91	143.61	
	3.85	12.40	3.59	92.91	143.61	
26	0	4.43	1.91	170.65	239.60	KOL LT.3
	1.925	4.43	1.91	170.65	239.60	
	3.85	4.43	1.91	170.65	239.60	
27	0	14.87	3.07	168.42	248.72	KOL LT.3
	1.925	14.87	3.07	168.42	248.72	
	3.85	14.87	3.07	168.42	248.72	
28	0	-0.64	-0.17	-124.84	202.85	KOL LT.3
	1.925	0.03	0.04	17.85	202.85	
	3.85	0.69	0.25	160.55	202.85	
29	0	-0.64	-0.14	-123.42	200.31	KOL LT.3
	1.925	0.01	0.01	17.44	200.31	
	3.85	0.86	0.16	158.30	200.31	
30	0	-1.48	-0.34	-123.85	201.56	KOL LT.3
	1.925	0.02	-0.04	17.59	201.56	
	3.85	1.51	0.25	159.03	201.56	
31	0	-28.84	-6.48	-137.97	236.23	KOL LT.3
	1.925	0.18	0.10	14.91	236.23	
	3.85	29.20	6.68	167.79	236.23	
32	0	-12.05	-1.96	-77.73	132.06	KOL LT.3
	1.925	-0.11	-0.06	10.07	132.06	
	3.85	11.83	1.83	97.87	132.06	
33	0	26.74	8.09	-0.15	91.98	KOL LT.4
	1.34	-0.69	-0.33	29.79	91.98	
	2.68	-28.52	-8.75	59.72	91.98	
34	0	-10.42	-3.90	-51.09	157.75	KOL LT.4
	1.34	-1.45	-0.69	19.35	157.75	
	2.68	7.53	2.53	89.78	157.75	
35	0	30.91	6.19	-50.99	171.72	KOL LT.4
	1.34	4.08	1.23	16.55	171.72	
	2.68	-22.76	-3.73	88.10	171.72	
36	0	0.15	0.17	-45.24	142.60	KOL LT.4
	1.34	-1.36	-0.39	21.87	142.60	
	2.68	-2.87	-0.94	88.98	142.60	
37	0	-0.65	-0.04	-43.42	138.59	KOL LT.4
	1.34	0.32	0.15	22.13	138.59	
	2.68	1.29	0.34	87.69	138.59	
38	0	-1.76	-0.74	-45.48	145.43	KOL LT.4
	1.34	1.66	0.79	21.87	145.43	
	2.68	5.08	2.32	89.22	145.43	

Lanjutan Tabel 4.15.d Gaya Geser Rencana Kolom Portal As.A-D (K=2)

Elm	Jarak (m)	VD,k (KNm)	VL,k (KNm)	VE,k (KNm)	VU,k=1.05.(VD,k +VL,k ± sed.VE,k) (KNm)	KET
39	0	-30.46	-6.69	-47.65	167.85	KOL LT.4
	1.34	-3.92	-1.33	20.02	167.85	
	2.68	22.61	4.02	87.68	167.85	
40	0	-14.91	-2.98	-18.71	83.29	KOL LT.4
	1.34	1.16	0.67	13.37	83.29	
	2.68	17.23	4.32	45.44	83.29	
41	0	0.97	0.26	-16.58	56.95	KOL LT.4
	1.1	-4.30	-1.16	3.80	56.95	
	2.2	-9.56	-2.58	24.17	56.95	
42	0	34.37	9.92	68.71	140.30	KOL LT.4
	1.1	34.37	9.92	68.71	140.30	
	2.2	34.37	9.92	68.71	140.30	
43	0	10.39	3.30	82.80	127.39	KOL LT.4
	1.1	10.39	3.30	82.80	127.39	
	2.2	10.39	3.30	82.80	127.39	
44	0	0.77	0.65	71.12	98.56	KOL LT.4
	1.1	0.77	0.65	71.12	98.56	
	2.2	0.77	0.65	71.12	98.56	
45	0	7.82	4.72	78.73	120.63	KOL LT.4
	1.1	7.82	4.72	78.73	120.63	
	2.2	7.82	4.72	78.73	120.63	
46	0	37.35	13.29	47.84	118.48	KOL LT.4
	1.1	37.35	13.29	47.84	118.48	
	2.2	37.35	13.29	47.84	118.48	

Tabel 4.16.a Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. 9-10 (K=1)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235,5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Sendi	Luar sendi
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	5318.74	10D25 = 4909	2659.37	5D25 = 2454.8	1193.37	3D25 = 1472.7	1799.79	6D25 = 2945.2	φ10-90	φ10-150
2	5368.41	10D25 = 4909	2734.45	5D25 = 2454.8	1428.84	3D25 = 1472.7	2057.28	6D25 = 2945.2	φ10-90	φ10-150
3	5160.23	10D25 = 4909	2580.11	5D25 = 2454.8	1456.21	3D25 = 1472.7	1910.23	6D25 = 2945.2	φ10-90	φ10-150
4	5291.42	10D25 = 4909	2645.71	5D25 = 2454.8	1453.40	3D25 = 1472.7	1859.26	6D25 = 2945.2	φ10-90	φ10-150
Atap	2404.89	5D25 = 2454.8	-	2D25 = 981.80	-	-	-	-	φ10-200	φ10-300

Tabel 4.16.b Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. 9-10 (K=2)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235,5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Di dalam	Di luar
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	6660.00	13D25 = 6381,7	3454.34	11D25 = 5399,9	2224.75	5D25 = 2454.8	3414.87	10D25 = 4909	φ10-110	φ10-140
2	6660.00	13D25 = 6381,7	3605.73	11D25 = 5399,9	2749.28	5D25 = 2454.8	4262.96	10D25 = 4909	φ10-110	φ10-140
3	6660.00	13D25 = 6381,7	3857.46	11D25 = 5399,9	2508.04	5D25 = 2454.8	3696.58	10D25 = 4909	φ10-110	φ10-140
4	6660.00	13D25 = 6381,7	3579.47	11D25 = 5399,9	1962.97	5D25 = 2454.8	3567.29	10D25 = 4909	φ10-110	φ10-140
Atap	2404.89	5D25 = 2454.8	-	2D25 = 981.80	-	-	-	-	φ10-200	φ10-300

Tabel 4.17.a Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. 9-10 (K=1)

It	kolom eksterior kanan															
	Lentur					Geser 4P10					Geser 4P10					
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang
Bm	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90
1	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90
2	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90
3	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90
4	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90	910.59	1081.76	841.77	9836	22D25=10799,8	1875.79	P10-70	P10-90

Tabel 4.17.b Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. 9-10 (K=2)

It	kolom eksterior kanan															
	Lentur					Geser 4P10					Geser 4P10					
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang
Bm	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-60	P10-100	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-100	P10-200
1	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-60	P10-100	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-100	P10-200
2	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-60	P10-100	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-100	P10-200
3	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-60	P10-100	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-100	P10-200
4	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-60	P10-100	1291.32	2592.6	841.77	12600	26D25=12763,4	3481.2	P10-100	P10-200

Tabel 4.18.a Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. 6,7-8 (K=1)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235.5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Sendi	Luar sendi
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	3012.63	8D25 = 3927.0	737.18	4D25 = 1963.6	1193.37	3D25 = 1472.7	1536.12	4D25 = 1963.6	φ10-100	φ10-200
2	3428.48	8D25 = 3927.0	1221.67	4D25 = 1963.6	1428.84	3D25 = 1472.7	1121.77	4D25 = 1963.6	φ10-100	φ10-200
3	3478.48	8D25 = 3927.0	1196.83	4D25 = 1963.6	1456.21	3D25 = 1472.7	981.86	4D25 = 1963.6	φ10-100	φ10-200
4	3478.18	8D25 = 3927.0	955.22	4D25 = 1963.6	1453.40	3D25 = 1472.7	1094.14	4D25 = 1963.6	φ10-100	φ10-200
Atap	2303.36	5D25 = 2454.8	-	2D25 = 819.8	-	-	-	-	φ10-250	φ10-300

Tabel 4.18.b Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. 6,7-8 (K=2)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235.5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Di dalam	Di luar
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	3573.72	10D25 = 4909.0	1916.48	6D25 = 2945.2	1873.19	4D25 = 1963.6	2121.57	6D25 = 2945.2	φ10-110	φ10-150
2	4335.97	10D25 = 4909.0	2900.29	6D25 = 2945.2	877.46	4D25 = 1963.6	2379.70	6D25 = 2945.2	φ10-110	φ10-150
3	4431.92	10D25 = 4909.0	2933.70	6D25 = 2945.2	922.57	4D25 = 1963.6	2446.23	6D25 = 2945.2	φ10-110	φ10-150
4	4393.81	10D25 = 4909.0	2225.10	6D25 = 2945.2	859.40	4D25 = 1963.6	2566.75	6D25 = 2945.2	φ10-110	φ10-150
Atap	1985.80	5D25 = 2454.8	-	2D25 = 819.8	-	-	-	-	φ10-250	φ10-300

Tabel 4.19.a Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. 6,7-8 (K=1)

It	kolom eksterior kiri dan kanan										kolom interior									
	Lentur					Geser 4P10					lentur					Geser 4P10				
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang				
Bm	819.06	1127.20	575	6300	14D25=6872.6	1984.91	P10-70	P10-90	1295.42	2957.36	440	10080	22D25=10799.8	3308.78	P10-70	P10-90				
1	819.06	1127.20	575	6300	14D25=6872.6	1984.91	P10-70	P10-90	1295.42	2957.36	440	10080	22D25=10799.8	3308.78	P10-70	P10-90				
2	819.06	1127.20	575	6300	14D25=6872.6	1984.91	P10-70	P10-90	1295.42	2957.36	440	10080	22D25=10799.8	3308.78	P10-70	P10-90				
3	819.06	1127.20	575	6300	14D25=6872.6	1984.91	P10-70	P10-90	1295.42	2957.36	440	10080	22D25=10799.8	3308.78	P10-70	P10-90				
4	819.06	1127.20	575	6300	14D25=6872.6	1984.91	P10-70	P10-90	1295.42	2957.36	440	10080	22D25=10799.8	3308.78	P10-70	P10-90				

Tabel 4.19.b Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. 6,7-8 (K=2)

It	kolom eksterior kiri dan kanan										kolom interior									
	Lentur					Geser 4P10					lentur					Geser 4P10				
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang				
Bm	1390.81	1683.93	841.77	12600	26D25=12763.4	3481.2	P10-60	P10-100	1429.69	3213.25	841.77	12600	26D25=12763.4	3481.2	P10-100	P10-200				
1	1390.81	1683.93	841.77	12600	26D25=12763.4	3481.2	P10-60	P10-100	1429.69	3213.25	841.77	12600	26D25=12763.4	3481.2	P10-100	P10-200				
2	1390.81	1683.93	841.77	12600	26D25=12763.4	3481.2	P10-60	P10-100	1429.69	3213.25	841.77	12600	26D25=12763.4	3481.2	P10-100	P10-200				
3	1390.81	1683.93	841.77	12600	26D25=12763.4	3481.2	P10-60	P10-100	1429.69	3213.25	841.77	12600	26D25=12763.4	3481.2	P10-100	P10-200				
4	1390.81	1683.93	841.77	12600	26D25=12763.4	3481.2	P10-60	P10-100	1429.69	3213.25	841.77	12600	26D25=12763.4	3481.2	P10-100	P10-200				

Tabel 4.20.a Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. A-D (K=1)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235.5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Sendi plastis	Luar sendi plastis
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	1200.00	4D22 = 1520.4	1233.37	4D22 = 1520.4	225.05	2D22 = 760.2	491.99	2D22 = 760.2	φ10-100	φ10-250
2	1200.00	4D22 = 1520.4	1221.10	4D22 = 1520.4	229.28	2D22 = 760.2	483.86	2D22 = 760.2	φ10-100	φ10-250
3	1200.00	4D22 = 1520.4	1200.00	4D22 = 1520.4	193.51	2D22 = 760.2	420.32	2D22 = 760.2	φ10-100	φ10-250
4	1200.00	4D22 = 1520.4	1200.00	4D22 = 1520.4	117.54	2D22 = 760.2	274.37	2D22 = 760.2	φ10-100	φ10-250
Nok	1200.00	4D22 = 1520.4	1200.00	4D22 = 1520.4	-	2D22 = 760.2	147.64	2D22 = 760.2	φ10-200	φ10-300
Atap	1200.00	4D22 = 1520.4	1200.00	4D22 = 1520.4	62.14	2D22 = 760.2	375.46	2D22 = 760.2	φ10-200	φ10-300

Tabel 4.20.b Tulangan Lentur dan Geser Terpasang Balok 400/800 Portal As. A-D (K=2)

Lantai	Tulangan Tumpuan (mm ²)				Tulangan Lapangan (mm ²)				Jarak tulangan geser Av 3φ10 = 235.5 (mm ²)	
	Luas tulangan atas		Luas tulangan bawah		Luas tulangan atas		Luas tulangan bawah		Di dalam Darah d	Di luar Darah d
	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang	Perlu	Terpasang		
1	2436.54	7D22 = 2660.7	2150.28	6D22 = 2280.6	485.67	2D22 = 760.2	947.70	4D22 = 1520.4	φ10-110	φ10-180
2	2417.76	7D22 = 2660.7	2156.29	6D22 = 2280.6	490.02	2D22 = 760.2	938.44	4D22 = 1520.4	φ10-110	φ10-180
3	1936.12	7D22 = 2660.7	1652.17	6D22 = 2280.6	415.89	2D22 = 760.2	809.15	4D22 = 1520.4	φ10-110	φ10-180
4	1248.70	7D22 = 2660.7	949.07	6D22 = 2280.6	258.08	2D22 = 760.2	516.44	4D22 = 1520.4	φ10-110	φ10-180
Nok	747.96	4D22 = 1520.4	410.51	4D22 = 1520.4	-	2D22 = 760.2	147.64	4D22 = 1520.4	φ10-200	φ10-300
Atap	625.93	4D22 = 1520.4	149.60	4D22 = 1520.4	62.14	2D22 = 760.2	375.46	4D22 = 1520.4	φ10-200	φ10-300

Tabel 4.21.a Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. A-D (K=1)

It	kolom eksterior kiri dan kanan						kolom interior									
	Lentur			geser			lentur			geser						
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang
Bm	372.41	370.23	1005	3150	8D25=3927.2	568.39	P10-70	P10-90	744.61	1931.84	385	6300	14D25=6872.6	2289.56	P10-70	P10-90
1	372.41	370.23	1005	3150	8D25=3927.2	568.39	P10-70	P10-90	744.61	1931.84	385	6300	14D25=6872.6	2289.56	P10-70	P10-90
2	372.41	370.23	1005	3150	8D25=3927.2	568.39	P10-70	P10-90	744.61	1931.84	385	6300	14D25=6872.6	2289.56	P10-70	P10-90
3	372.41	370.23	1005	3150	8D25=3927.2	568.39	P10-70	P10-90	744.61	1931.84	385	6300	14D25=6872.6	2289.56	P10-70	P10-90
4	372.41	370.23	1005	3150	8D25=3927.2	568.39	P10-70	P10-90	744.61	1931.84	385	6300	14D25=6872.6	2289.56	P10-70	P10-90

Tabel 4.21.b Tulangan Lentur dan Geser Terpasang Kolom 450/700 Portal As. A-D (K=2)

It	kolom eksterior kiri dan kanan						kolom interior									
	Lentur			Geser 4P10			lentur			Geser 4P10						
	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang	Mu,k (KNm)	Nu,k (KN)	e (mm)	As perlu (mm ²)	As pasang (mm ²)	φPn (KN)	Dalam d=450 mm	Tengah Bentang
Bm	863.62	1245.31	453	6750	14D25=6872.6	1547.6	P10-60	P10-100	891.76	2709.18	330	6750	14D25=6872.6	3081.2	P10-100	P10-200
1	863.62	1245.31	453	6750	14D25=6872.6	1547.6	P10-60	P10-100	891.76	2709.18	330	6750	14D25=6872.6	3081.2	P10-100	P10-200
2	863.62	1245.31	453	6750	14D25=6872.6	1547.6	P10-60	P10-100	891.76	2709.18	330	6750	14D25=6872.6	3081.2	P10-100	P10-200
3	863.62	1245.31	453	6750	14D25=6872.6	1547.6	P10-60	P10-100	891.76	2709.18	330	6750	14D25=6872.6	3081.2	P10-100	P10-200
4	863.62	1245.31	453	6750	14D25=6872.6	1547.6	P10-60	P10-100	891.76	2709.18	330	6750	14D25=6872.6	3081.2	P10-100	P10-200

Tabel 4.22.a Perbandingan Berat Tulangan Lentur Balok 400/800 Portal As.9-10

Lt	Tulangan Tumpuan										Tulangan Lapangan									
	Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)					Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)				
	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)
1	10D25	5D25	3.85	4.88	281.53	13D25	11D25	3.85	4.88	450.45	3D25	6D25	3.85	8.75	303.19	5D25	10D25	3.85	8.75	505.31
2	10D25	5D25	3.85	4.88	281.53	13D25	11D25	3.85	4.88	450.45	3D25	6D25	3.85	8.75	303.19	5D25	10D25	3.85	8.75	505.31
3	10D25	5D25	3.85	4.88	281.53	13D25	11D25	3.85	4.88	450.45	3D25	6D25	3.85	8.75	303.19	5D25	10D25	3.85	8.75	505.31
4	10D25	5D25	3.85	4.88	281.53	13D25	11D25	3.85	4.88	450.45	3D25	6D25	3.85	8.75	303.19	5D25	10D25	3.85	8.75	505.31
Atap	5D25	2D25	3.85	3.25	87.59	5D25	2D25	3.85	3.25	87.59	-	-	3.85	8.75	303.19	5D25	10D25	3.85	8.75	505.31
	Jumlah					Jumlah					Jumlah					Jumlah				
	1213.71					1899.39					1212.75					2021.25				
	Perbandingan					Perbandingan					Perbandingan					Perbandingan				
	1.00					1.56					1.00					1.67				

Tabel 4.22.b Perbandingan Berat Tulangan Lentur Balok 400/800 Portal As.6,7,8

Lt	Tulangan Tumpuan										Tulangan Lapangan									
	Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)					Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)				
	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)
1	8D25	4D25	3.85	6.5	300.3	10D25	6D25	3.85	6.5	400.4	3D25	4D25	3.85	11.5	309.93	4D25	6D25	3.85	11.5	442.75
2	8D25	4D25	3.85	6.5	300.3	10D26	6D26	3.85	6.5	400.4	3D25	4D25	3.85	11.5	309.93	4D25	6D25	3.85	11.5	442.75
3	8D25	4D25	3.85	6.5	300.3	10D27	6D27	3.85	6.5	400.4	3D25	4D25	3.85	11.5	309.93	4D25	6D25	3.85	11.5	442.75
4	8D25	4D25	3.85	6.5	300.3	10D28	6D28	3.85	6.5	400.4	3D25	4D25	3.85	11.5	309.93	4D25	6D25	3.85	11.5	442.75
Atap	5D25	2D25	3.85	3.25	87.58	5D25	2D25	3.85	3.25	87.59	-	-	3.85	8.75	309.93	4D25	6D25	3.85	11.5	442.75
	Jumlah					Jumlah					Jumlah					Jumlah				
	1288.79					1689.19					1239.7					1771.0				
	Perbandingan					Perbandingan					Perbandingan					Perbandingan				
	1					1.31					1					1.43				

Tabel 4.22.c Perbandingan Berat Tulangan Lentur Balok 250/400 Portal As.A-D

Lt	Tulangan Tumpuan										Tulangan Lapangan									
	Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)					Daktiilitas Penuh (K=1)					Daktiilitas Terbatas (K=2)				
	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)	Atas	Bawah	BS (kg/m)	L (m)	Berat (kg)
1	4D22	4D22	2.98	11.2	267.01	7D22	6D22	2.98	11.2	433.89	2D22	2D22	2.98	25.3	301.58	2D22	4D22	2.98	25.3	452.36
2	4D22	4D22	2.98	11.2	267.01	7D22	6D22	2.98	11.2	433.89	2D22	2D22	2.98	25.3	301.58	2D22	4D22	2.98	25.3	452.36
3	4D22	4D22	2.98	11.2	267.01	7D22	6D22	2.98	11.2	433.89	2D22	2D22	2.98	25.3	301.58	2D22	4D22	2.98	25.3	452.36
4	4D22	4D22	2.98	11.2	267.01	7D22	6D22	2.98	11.2	433.89	2D22	2D22	2.98	25.3	301.58	2D22	4D22	2.98	25.3	452.36
Nok	4D22	4D22	2.98	11.2	267.01	4D22	4D22	2.98	11.2	267.01	2D22	2D22	2.98	25.3	301.58	2D22	4D22	2.98	25.3	452.36
Atap	4D22	4D22	2.98	8	190.72	4D22	4D22	2.98	8	190.72	2D22	2D22	2.98	19	226.48	2D22	4D22	2.98	19	339.72
	Jumlah					Jumlah					Jumlah					Jumlah				
	1525.76					2193.28					2599.56					2601.54				
	Perbandingan					Perbandingan					Perbandingan					Perbandingan				
	1					1.438					1					1.5				

Tabel 4.23.a Perbandingan Berat Tulangan Lentur Kolom 450/700 Portal As.9-10

Lt	Tinggi (m)	Daktilitas Penuh (K=1)				Daktilitas Terbatas (K=2)				
		Kol ekst kiri		Kol ekst kanan		Berat (kg)	Kol ekst kiri		Kol ekstr kanan	
		tulangan	BS (kg/m)	tulangan	tulangan		tulangan	BS (kg/m)	tulangan	Berat (kg)
Bm	3.85	22D25	3.85	22D25	326.10	26D25	3.85	26D25	385.39	
1	3.85	22D25	3.85	22D25	326.10	26D25	3.85	26D25	385.39	
2	3.85	22D25	3.85	22D25	326.10	26D25	3.85	26D25	385.39	
3	3.85	22D25	3.85	22D25	326.10	26D25	3.85	26D25	385.39	
4	4.88	22D25	3.85	22D25	413.34	26D25	3.85	26D25	488.49	
Jumlah					1717.72	Jumlah				
Perbandingan					1.00	Perbandingan				
						2030.03				
						1.18				

Tabel 4.23.b Perbandingan Berat Tulangan Lentur Kolom 450/700 Portal As.6,7,8

Lt	Tinggi (m)	Daktilitas Penuh (K=1)					Daktilitas Terbatas (K=2)						
		eksterior kiri		Eksterior kanan		interior	Berat (kg)	Eksterior kiri		Eksterior kanan		interior	Berat (kg)
		tulangan	BS (kg/m)	tulangan	tulangan			tulangan	BS (kg/m)	tulangan	tulangan		
Bm	3.85	14D25	3.85	14D25	22D25	741.13	26D25	3.85	26D25	26D25	26D25	1156.16	
1	3.85	14D25	3.85	14D25	22D25	741.13	26D25	3.85	26D25	26D25	26D25	1156.16	
2	3.85	14D25	3.85	14D25	22D25	741.13	26D25	3.85	26D25	26D25	26D25	1156.16	
3	3.85	14D25	3.85	14D25	22D25	741.13	26D25	3.85	26D25	26D25	26D25	1156.16	
4	4.88	14D25	3.85	14D25	22D25	939.40	26D25	3.85	26D25	26D25	26D25	1465.46	
Jumlah						3903.9	Jumlah						
Perbandingan						1	Perbandingan						
							6090.1						
							1.56						

Tabel 4.23.c Perbandingan Berat Tulangan Lentur Kolom 450/700 Portal As.A-D

Lt	tinggi (m)	Daktilitas Penuh (K=1)					Daktilitas Terbatas (K=2)						
		eksterior kiri		Eksterior kanan		interior	Berat (kg)	eksterior kiri		Eksterior kanan		interior	Berat (kg)
		tulangan	BS (kg/m)	tulangan	tulangan			tulangan	BS (kg/m)	tulangan	tulangan		
Bmt	3.85	8D25	3.85	8D25	14D25	1482.25	14D25	3.85	14D25	14D25	14D25	1660.12	
1	3.85	8D25	3.85	8D25	14D25	1482.25	14D25	3.85	14D25	14D25	14D25	1660.12	
2	3.85	8D25	3.85	8D25	14D25	1482.25	14D25	3.85	14D25	14D25	14D25	1660.12	
3	3.85	8D25	3.85	8D25	14D25	1482.25	14D25	3.85	14D25	14D25	14D25	1660.12	
4	4.88	8D25	3.85	8D25	14D25	1878.80	14D25	3.85	14D25	14D25	14D25	2104.26	
Jumlah						7807.8	Jumlah						
Perbandingan						1	Perbandingan						
							8744.74						
							1.12						

Tabel 4.24.a Perbandingan Berat Tulangan Geser Balok 400/800 Portal As.9-10

Lt	Daktilitas Penuh (K=1)				Daktilitas Terbatas (K=2)			
	Sendi plastis	diluar sendi plastis	BS (kg/m)	Berat (kg)	Sendi plastis	diluar sendi plastis	BS (kg/m)	Berat (kg)
1	3P10-90	3P10-150	0.617	184.76	3P10-110	3P10-140	0.617	174.95
2	3P10-90	3P10-150	0.617	184.76	3P10-110	3P10-140	0.617	174.95
3	3P10-90	3P10-150	0.617	184.76	3P10-110	3P10-140	0.617	174.95
4	3P10-90	3P10-150	0.617	184.76	3P10-110	3P10-140	0.617	174.95
Atap	3P10-200	3P10-300	0.617	47.42	3P10-200	3P10-300	0.617	47.42
Jumlah				786.46	Jumlah			
Perbandingan				1.0525	Perbandingan			
					1			

Tabel 4.24.b Perbandingan Berat Tulangan Geser Balok 400/800 Portal As.6,7,8

Lt	Daktilitas Penuh (K=1)				Daktilitas Terbatas (K=2)			
	Sendi Plastik	Diluar sendi plastis	BS (kg/m)	Berat (kg)	Sendi plastis	Diluar sendi plastis	BS (kg/m)	Berat (kg)
1	3P10-100	3P10-200	0.617	199.48	3P10-110	3P10-150	0.617	217.46
2	3P10-100	3P10-200	0.617	199.48	3P10-110	3P10-150	0.617	217.46
3	3P10-100	3P10-200	0.617	199.48	3P10-110	3P10-150	0.617	217.46
4	3P10-100	3P10-200	0.617	199.48	3P10-110	3P10-150	0.617	217.46
Atap	3P10-250	3P10-300	0.617	42.51	3P10-250	3P10-300	0.617	42.51
Jumlah				840.42	Jumlah			
Perbandingan				1	Perbandingan			
					1.086			

Tabel 4.24.c Perbandingan Berat Tulangan Geser Balok 250/400 Portal As.A-D

Lt	Daktilitas Penuh (K=1)				Daktilitas Terbatas (K=2)			
	Sendi Plastik	Diluar sendi plastis	BS (kg/m)	Berat (kg)	Sendi plastis	Diluar sendi plastis	BS (kg/m)	Berat (kg)
1	3P10-100	3P10-250	0.617	349.90	3P10-110	3P10-180	0.617	397.32
2	3P10-100	3P10-250	0.617	349.90	3P10-110	3P10-180	0.617	397.32
3	3P10-100	3P10-250	0.617	349.90	3P10-110	3P10-180	0.617	397.32
4	3P10-100	3P10-250	0.617	349.90	3P10-110	3P10-180	0.617	397.32
Nok	3P10-200	3P10-300	0.617	230.54	3P10-200	3P10-300	0.617	230.54
Atap	3P10-200	3P10-300	0.617	170.05	3P10-200	3P10-300	0.617	170.05
Jumlah				1800.19	Jumlah			
Perbandingan				1	Perbandingan			
					1.109			

Tabel 4.25.a Perbandingan Berat Tulangan Geser Kolom 450/700 Portal As.9-10

Lantai	Daktiilitas Penuh (K=1)				Daktiilitas Terbatas (K=2)			
	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)
1	4P10-70	4P10-90	0.617	214.85	4P10-100	4P10-200	0.617	112.10
2	4P10-70	4P10-90	0.617	214.85	4P10-100	4P10-200	0.617	112.10
3	4P10-70	4P10-90	0.617	214.85	4P10-100	4P10-200	0.617	112.10
4	4P10-70	4P10-90	0.617	270.90	4P10-100	4P10-200	0.617	112.10
Jumlah Perbandingan				1130.31	Jumlah Perbandingan			
				1.936	583.54			

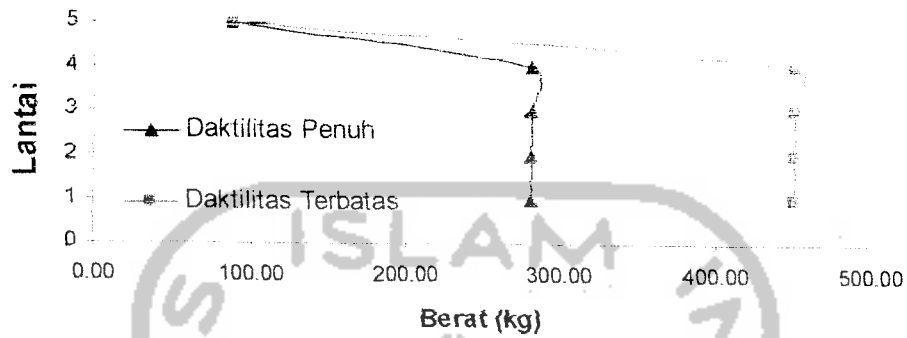
Tabel 4.25.b Perbandingan Berat Tulangan Geser Kolom 450/700 Portal As.6,7,8

Lantai	Daktiilitas Penuh (K=1)						Daktiilitas Terbatas (K=2)					
	eksterior kiri			interior			eksterior kiri			interior		
Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	
1	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	322.28	4P10-60	4P10-100	4P10-100	4P10-200	
2	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	322.28	4P10-60	4P10-100	4P10-100	4P10-200	
3	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	322.28	4P10-60	4P10-100	4P10-100	4P10-200	
4	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	406.35	4P10-60	4P10-100	4P10-100	4P10-200	
Jumlah Perbandingan				1695.46				Jumlah Perbandingan				
				1.220				1389.53				

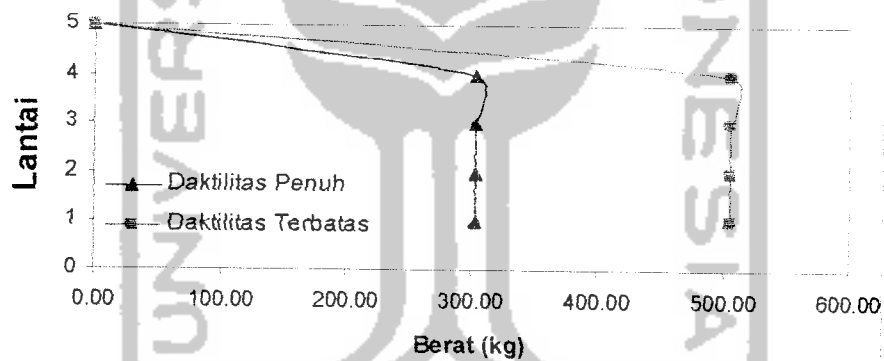
Tabel 4.25.c Perbandingan Berat Tulangan Geser Kolom 450/700 Portal As.A-D

Lantai	Daktiilitas Penuh (K=1)						Daktiilitas Terbatas (K=2)					
	eksterior kiri			interior			eksterior kiri			interior		
Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	Ujung kolom	Tengah kolom	BS (kg/m)	Berat (kg)	
1	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	859.41	4P10-60	4P10-100	4P10-100	4P10-200	
2	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	859.41	4P10-60	4P10-100	4P10-100	4P10-200	
3	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	859.41	4P10-60	4P10-100	4P10-100	4P10-200	
4	4P10-70	4P10-90	4P10-70	4P10-90	4P10-100	4P10-200	597.85	4P10-60	4P10-100	4P10-100	4P10-200	
Jumlah Perbandingan				4035.48				Jumlah Perbandingan				
				1.560				2587.56				

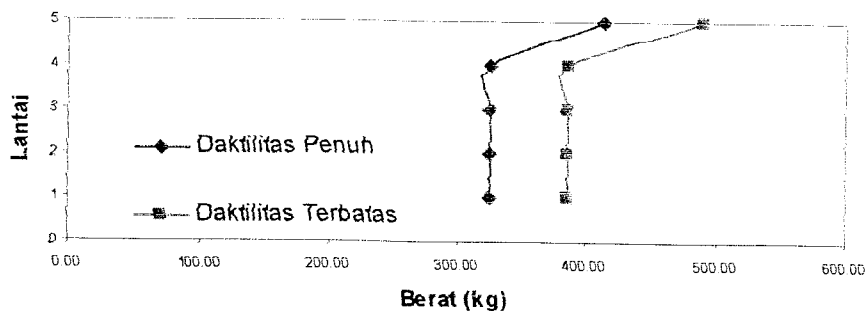
Perbandingan Berat Tulangan Lentur Balok Portal As 9-10 pada Tumpuan



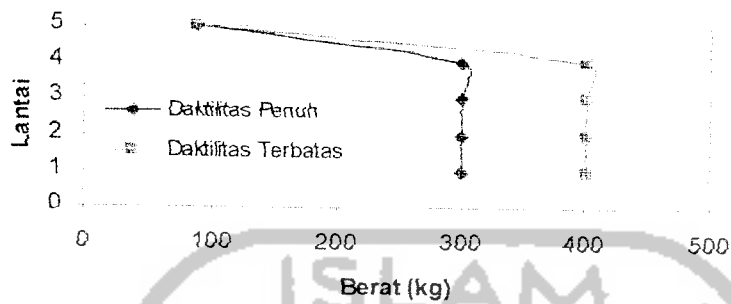
Perbandingan Berat Tulangan Lentur Balok Portal As 9-10 pada Lapangan



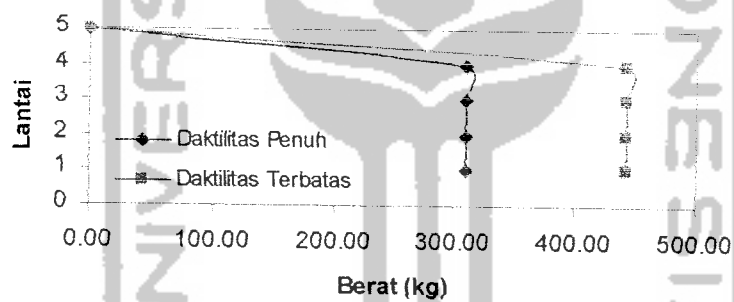
Perbandingan Berat Tulangan Lentur Kolom Portal As 9-10



Perbandingan Berat Tulangan Lentur Balok Portal As 6,7,8 pada Tumpuan



Perbandingan Volume Tulangan Lentur Balok Portal As 6,7,8 pada Lapangan



Perbandingan Berat Tulangan Lentur Kolom Portal As 6,7,8

