

**TUGAS AKHIR**

**PERENCANAAN GEDUNG MAGISTER**  
**MANAJEMEN UNIVERSITAS GADJAH MADA**  
**YOGYAKARTA**



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**FAKULTAS TEKNIK SIPIL DAN PERENCANAAN**  
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LEMBAR PENGESAHAN TUGAS AKHIR

PERENCANAAN GEDUNG MEGISTER MANAJEMEN  
UNIVERSITAS GAJAH MADA  
YOGYAKARTA

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Yogyakarta, April 1998

Penulis



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## DAFTAR NOTASI

$A_s$	=	luas tulangan tarik non-pratekan, $\text{mm}^2$
$A_s'$	=	luas tulangan tekan, $\text{mm}^2$
$A_{s_{\text{min}}}$	=	luas tulangan minimum, $\text{mm}^2$
$b$	=	lebar dari muka tekan komponen struktur, mm
$d$	=	jarak dari serat tekan terluar ke pusat tulangan tarik, mm
$d'$	=	jarak dari serat tekan terluar ke pusat tulangan tekan, mm
$p_b$	=	penutup beton
$f_c'$	=	kuat tekan beton yang disyaratkan, MPa
$f_y$	=	tegangan leleh yang disyaratkan dari tulangan non-pratekan, MPa
$f_s'$	=	tegangan leleh tulangan tekan dari tulangan non-pratekan, Mpa
$q_u$	=	kekuatan yang diperlukan untuk menahan beban terfaktor atau momen dan gaya dalam yang berhubungan dengannya.
$q_d$	=	Beban mati adalah berat dari semua bagian dari suatu gedung yang bersifat tetap, termasuk segala tambahan, penyelesaian mesin-mesin serta peralatan tetap yang merupakan bagian yang tak terpisahkan dari gedung tersebut.
$q_l$	=	beban hidup adalah semua beban yang terjadi akibat pemakaian dan penghunian suatu gedung, termasuk beban-beban pada lantai yang berasal dari barang-barang yang dapat berpindah dan atau beban akibat air hujan pada atap.
$M_u$	=	Momen terfaktor pada penampang
$M_n$	=	Momen nominal pada penampang
$L_x$	=	panjang sisi terpendek dari pelat
$\rho$	=	rasio tulangan tarik non-pratekan
$\rho'$	=	rasio tulangan tekan non-pratekan
$\rho_b$	=	rasio tulangan yang memberikan kondisi regangan yang seimbang.
$\beta_1$	=	faktor kekuatan beton
$\Phi$	=	faktor reduksi kekuatan
$a$	=	tinggi balok tegangan persegi ekuivalen.
$V_u$	=	gaya geser terfaktor pada penampang

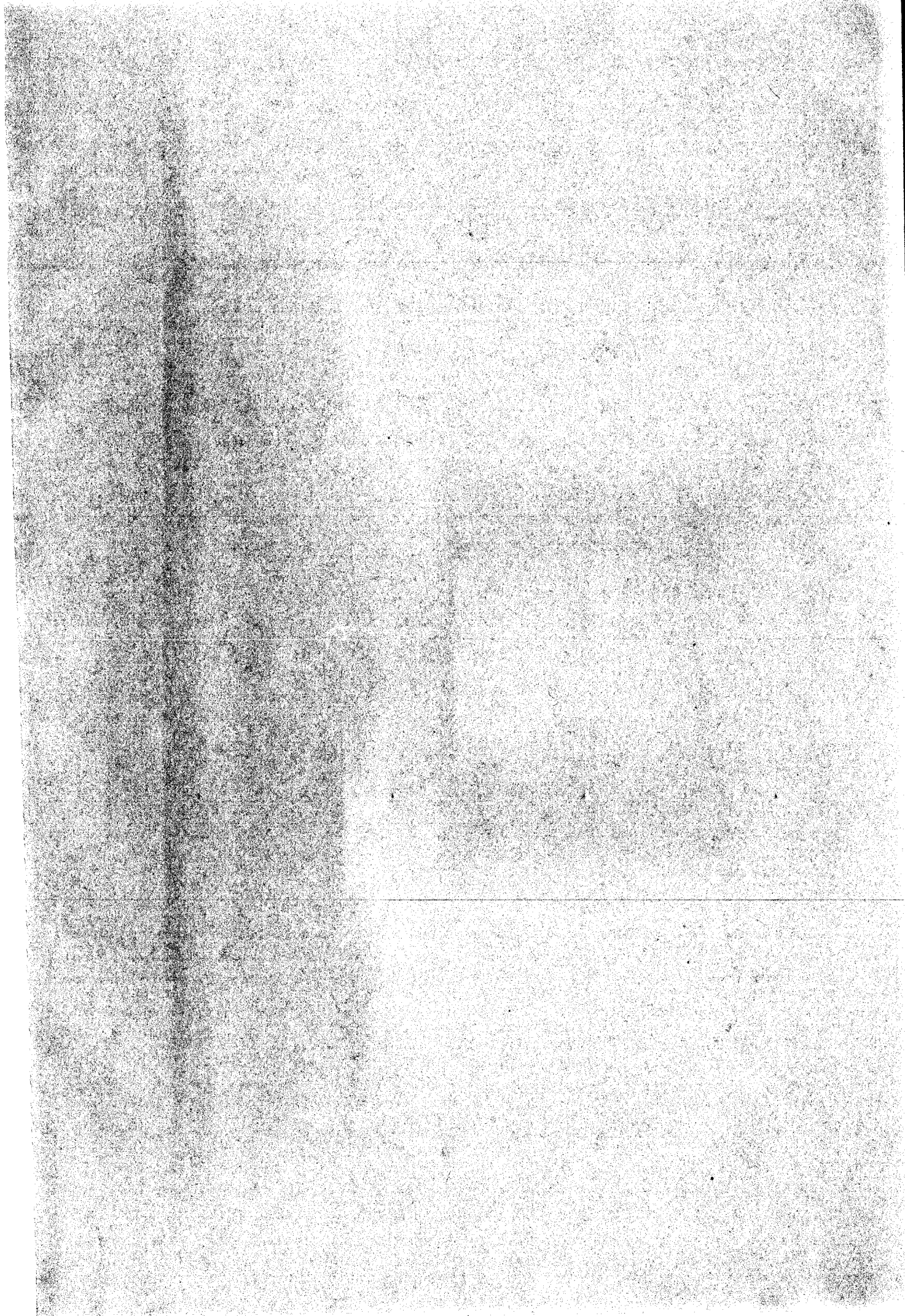
$V_c$	=	kuat geser nominal yang disumbangkan oleh beton.
$V_n$	=	kuat geser nominal
$V_s$	=	kuat geser nominal yang disumbangkan oleh tulangan geser.
$A_v$	=	luas tulangan geser yang tegak lurus terhadap tulangan lentur tarik
$T_u$	=	momen torsi terfaktor pada penampang
$T_n$	=	kuat momen torsi nominal
$T_c$	=	kuat momen torsi nominal yang disumbangkan oleh beton
$x$	=	dimensi pendek dari bagian berbentuk persegi dari penampang
$y$	=	dimensi panjang dari bagian berbentuk persegi dari penampang
$T_s$	=	kuat momen torsi nominal yang disumbangkan oleh tulangan torsi
$\alpha$	=	sudut antara tulangan geser -friksi dengan bidang geser
$A_t$	=	luas satu kaki dari sengkang tertutup dalam daerah sejarak $s$ yang menahan torsi, $\text{mm}^2$
$s$	=	spasi dari tulangan geser
$X_1$	=	dimensi pusat ke pusat yang pendek dari sengkang persegi tertutup
$Y_1$	=	dimensi pusat ke pusat yang panjang dari sengkang persegi tertutup

## ABSTRAKSI

Proyek Pembangunan Komplek Gedung II Program Studi Magister Manajemen Universitas Gadjah Mada terletak di Berek, Depok, Sleman, Lokasi proyek menempati areal 7150 m<sup>2</sup>. Untuk dasar-dasar perencanaan yang dipakai dalam merencanakan gedung adalah Tata cara perhitungan Struktur Beton untuk Bangunan Gedung ( SK-SNI T-15-1991-03). Dan spesifikasi bangunan yang direncanakan untuk beton menggunakan  $f_c' = 20$  MPa dan Mutu baja tulangan yang digunakan untuk  $\leq \varnothing 12$  mm dipakai  $f_y = 240$  MPa, untuk  $> \varnothing 12$  mm dipakai  $f_y = 400$  MPa, sedangkan analisis struktur dengan menggunakan program SAP 90.

Dimensi kolom terbesar 70/60 cm, dimensi kolom terkecil 50/50. Struktur atap menggunakan rangka baja dengan bentang 24 m, dimensi batang atas 2L3x3x3/16, dimensi batang bawah 2Lx2x2x1/4, dimensi batang horizontal dan vertikal 2Lx2x2x1/8. Dan untuk ukuran pondasi yang terkecil 300x300 cm, yang terbesar 350x350 m.





# BAB I

## PENDAHULUAN

### 1.1. Latar Belakang

Pendidikan merupakan suatu wadah transfer ilmu pengetahuan yang diadakan secara terkoordinir dan rutin, dengan semakin maraknya pembangunan kampus bagi perguruan-perguruan tinggi di Yogyakarta, yang tentunya dengan pembangunan lokal kelas baru akan dapat menampung mahasiswa dengan jumlah yang lebih proporsional terhadap kelas yang ada. Dengan demikian proses belajar mengajar diharapkan akan lebih baik.

Suatu usaha mencapai tujuan selalu menghadapi berbagai permasalahan khususnya berkaitan dengan kendala waktu maupun sumber daya dan sumber dana yang tersedia. Kendala-kendala tersebut akan menimbulkan permasalahan bagi Program Magister Manajemen Universitas Gadjah Mada dalam pelaksanaan misinya. Permasalahan daya tampung, menjadi masalah yang cukup serius dan menjadi faktor penghambat bagi pengembangan, Demikian juga keadaan dan kemampuan penyediaan fasilitas belajar mengajar yang terbatas, sehingga perlu perencanaan yang cermat untuk mengembangkan fasilitas belajar mengajar bagi program Magister Manajemen Universitas Gadjah Mada, didasari oleh berbagai kondisi diatas maka sarana dan prasarana fisik yang telah ada perlu ditingkatkan lagi dengan jalan membangun kampus yang baru dengan luas tanah yang jauh lebih besar dari pada kampus yang ada sekarang.

## 1.2. Lokasi Proyek

Proyek Pembangunan Komplek Gedung II Program Studi Magister Manajemen Universitas Gadjah Mada terletak di Berek, Depok, Sleman. Lokasi proyek menempati areal 7150 m<sup>2</sup>. Lokasi ini sebelumnya berupa lahan bekas Jurusan Teknik Arsitektur, Fakultas Teknik, UGM dengan permukaan tanah lokasi relatif mendatar.

Adapun batas-batas lokasi proyek adalah sebagai berikut :

1. Sebelah Utara : Pemukiman Penduduk
2. Sebelah Timur : Jalan Kaliurang,
3. Sebelah Selatan : Selokan, bersebelahan dengan jalan. Teknika Utara,
4. Sebelah Barat : Gedung Universitas Terbuka.

## 1.3. Data Teknis Proyek

Proyek Pembangunan Komplek Gedung II Program Studi Magister Manajemen Universitas Gadjah Mada terdiri dari lima lantai, dengan menggunakan struktur beton bertulang konvensional. Elevasi lantai satu, lantai dua, lantai tiga, lantai empat dan lantai lima berturut-turut adalah +0.00 m, +4.50 m, +9.00 m, +13.50 m dan +18.00 m.

Fungsi bangunan secara umum adalah sebagai berikut :

1. Lantai Satu : Ruang Bookstack, Counter Pelayanan, Gudang, Ruang Mahasiswa, Mushola,
2. Lantai Dua : Ruang Baca, Audio Visual, Ruang Kelas, Ruang Diskusi, Void, Workstation,

3. Lantai Tiga : Ruang Lab. Bahasa, Lab. Komputer, Mushola, Ruang Kelas, Ruang Diskusi,
4. Lantai Empat : Ruang Kelas, Ruang Diskusi dan
5. Lantai Lima : Ruang Sekretariat, Ruang Rapat, Ruang Administrasi, Common Facilities, Mushola.

Bangunan pelengkap pada gedung ini adalah bangunan lavatory, bangunan tangga yang terdiri dari satu tangga utama , dua tangga darurat dari lantai satu sampai lantai lima dan dua lift pada setiap lantai serta satu tangga layang pada lantai pertama yang menuju ke ruang perpustakaan di lantai dua.



## BAB II

### PERENCANAAN

#### 2.1. Tinjauan Pustaka

Pada penggunaan komponen struktur bangunan gedung yang terbuat dari beton konvensional, umumnya beton diperkuat dengan tulangan baja sebagai bahan yang dapat bekerja sama dan mampu membantu kelemahannya. Terutama pada bagian yang menahan gaya tarik. Dengan demikian tersusun pembagian tugas, dimana tulangan baja memperkuat dan menahan gaya tarik, sedangkan beton hanya diperhitungkan untuk menahan gaya tekan. (Istimawan Dipohusodo, 1994, Struktur Beton Bertulang, PT. Gramedia Pustaka Utama, Jakarta).

Struktur gedung terdiri dari bagian-bagian seperti atap, pelat lantai, balok, kolom, tangga dan pondasi.

Atap mempunyai fungsi sebagai penutup bagian atas dari suatu bangunan. Atap pada gedung ini direncanakan menggunakan genteng keramik berglazur.

Pelat lantai berfungsi sebagai pendukung beban akibat berat sendiri dan beban berguna yang ada di atasnya, dalam perencanaan pelat lantai ini yang ditinjau adalah beban pengaruh beban gravitasi.

Balok merupakan bagian horizontal dari portal yang menahan gaya vertikal dari pelat lantai dan berat dinding.

Kolom berfungsi meneruskan beban ke pondasi bangunan yang ada di bawahnya, bentuk kolom gedung ini direncanakan berbentuk persegi.

Tangga berfungsi sebagai sarana penghubung dari masing-masing lantai, direncanakan terdiri dari bordes dan pelat tangga disamping itu disediakan lift yang fungsinya sama dengan tangga, hanya saja lebih efisien.

Pondasi merupakan struktur bawah yang berada di dalam tanah yang merupakan tempat bertumpunya seluruh struktur bangunan yang berada di atasnya dan meneruskan ke tanah.

## 2.2. Perencanaan Konstruksi

### 2.2.1. Dasar Perencanaan

Dalam perencanaan ini menggunakan metode kekuatan batas dimana beban kerja dinaikkan dengan memberikan suatu faktor beban sehingga diperoleh suatu beban pada akhir keruntuhan.

Menurut SK-SNI T-15-1991-03 subbab 3.2.2 faktor beban ditentukan sebagai berikut ini :

1. Beban mati = D
2. Beban Hidup = L
3. Beban Gempa = E

$$U = 1,2 D + 1,6 L \quad (2.1)$$

$$U = 0,9 (D \pm E) \quad (2.2)$$

$$U = 1,05 (D + \phi. L \pm E) \quad (2.3)$$

dimana :

U = Kuat perlu adalah kekuatan suatu komponen struktur atau penampang yang diperlukan untuk menahan beban terfaktor atau momen dan gaya dalam yang berkaitan dengan beban tersebut dalam suatu kombinasi .

D = Beban mati adalah berat dari semua bagian dari suatu gedung yang bersifat tetap, termasuk segala tambahan, penyelesaian mesin-mesin serta peralatan tetap yang merupakan bagian yang tak terpisahkan dari gedung tersebut.

L = Beban hidup adalah semua beban yang terjadi akibat pemakain dan penghunian suatu gedung, termasuk beban-beban pada lantai yang beraal dari barang-barang yang dapat berpindah dan atau beban akibat air hujan pada atap.

E = Beban gempa .

Kepastian kekuatan beban terhadap pembebanan dianggap sebagai faktor reduksi kekuatan ( $\Phi$ ). Menurut SK-SNI T-15-1991-03 subbab 3.2.3 faktor reduksi kekuatan ditentukan sebagai berikut ini :

- |  |               |
|--|---------------|
| 1. Lentur, tanpa beban aksial                  | $\Phi = 0.8$  |
| 2. Aksial tarik dan aksial tarik dengan lentur | $\Phi = 0.8$  |
| 3. Aksial tekan dan aksial tekan dengan lentur | $\Phi = 0.65$ |
| 4. Geser dan torsi                             | $\Phi = 0.6$  |

Faktor reduksi kekuatan diatas juga dipakai untuk mereduksi kekuatan beton dan baja berikut ini :

1. Untuk beton :  $f'c$  (kuat tekan beton yang disyaratkan)
2. Untuk baja :  $f_y$  (tegangan leleh baja ).

### 2.2.2. Perencanaan dan Standar yang digunakan

Perencanaan pada hakekatnya adalah penuangan ide-ide pemikiran secara konseptual yang dituangkan dalam bentuk rencana proyek. Sasaran yang hendak dicapai dalam perencanaan ini adalah :

1. Perencanaan sesuai dengan data proyek



2. Fungsi proyek terpenuhi,
3. Proyek tidak merusak lingkungan,
4. Proyek mempunyai ciri tersendiri sesuai dengan keinginan pemilik,
5. Jadwal yang tepat,
6. Biaya ekonomis,
7. Kualitas maksimal.

Untuk dasar-dasar perencanaan yang dipakai dalam merencanakan gedung adalah :

1. Tata cara perhitungan Struktur Beton untuk Bangunan Gedung ( SK-SNI T-15-1991-03).
2. Peraturan Pembebanan untuk Gedung 1983.
3. Peraturan Perencanaan Bangunan Tahan Gempa Indonesia untuk Gedung 1983.
4. Peraturan Perencanaan Bangunan Baja Indonesia (PPBBI-1984).
5. Hasil Penyelidikan Tanah di Lokasi.
6. Peraturan lain yang ada hubungannya dengan Perencanaan bangunan untuk Gedung yang berlaku di Indonesia.

Metode yang digunakan dalam perencanaan Gedung Magister Manajemen Universitas Gadjah Mada ini adalah SK-SNI T-15-1991-03 dan untuk membantu perhitungan mekanika dengan menggunakan program paket SAP-90.

## 2.3. Kriteria Perencanaan Struktur

### 2.3.1. Pembebanan Atap

- a. Beban penutup atap genteng =  $50 \text{ kg/m}^2$
- b. Beban gording =  $10 \text{ kg/m}^2$
- c. Beban pekerja =  $(40 - (0,8 \times 24,9^\circ)) = 20 \text{ kg/m}^2$
- d. Beban plafond ( asbes ) =  $18 \text{ kg/m}^2$
- e. Beban kuda-kuda =  $(10 + (12/3 \times 5)) \times 4 = 120 \text{ kg/m}^2$
- f. Beban angin =  $25 \text{ kg/m}^2$

### 2.3.2. Pembebanan Pelat

#### a. Beban Mati

Tebal Pelat diambil 0,12m

- 1). Berat sendiri =  $0,12 \times 23 = 2,76 \text{ KN/m}^2$
- 2). Ubin 2 cm =  $0,02 \times 23 = 0,46 \text{ KN/m}^2$
- 3). Spesi 1 cm =  $0,01 \times 23 = 0,23 \text{ KN/m}^2$
- 4). Pasir 5 cm =  $0,05 \times 23 = 1,15 \text{ KN/m}^2$
- 5). Langit<sup>2</sup> + Penggantung =  $18 \times 0,00981 = 0,1766 \text{ KN/m}^2$

$$q_d = 4,7766 \text{ KN/m}^2$$

#### b. Beban Hidup (ql)

- 1). Kantor/ruang kuliah =  $250 \text{ kg/m}^2$
- 2). Perpustakaan =  $400 \text{ kg/m}^2$
- 3). Selasar =  $300 \text{ kg/m}^2$
- 4). Tangga =  $300 \text{ kg/m}^2$

### 2.3.3. Pembebanan Balok

a. Beban Mati (Pada bagian balok yang ada temboknya)

- 1). Beban plat
- 2). Berat tembok
- 3). Berat Balok

b. Beban Hidup

### 2.4 Perencanaan Pelat

Untuk perencanaan penulangan pelat digunakan ketentuan sebagai berikut :

$$A_s = \frac{M_n}{f_y \cdot (0,9)d}$$

Cek rasio penulangan ( $\rho$ ) :

1. Bila  $A_s < A_{s \text{ min}}$

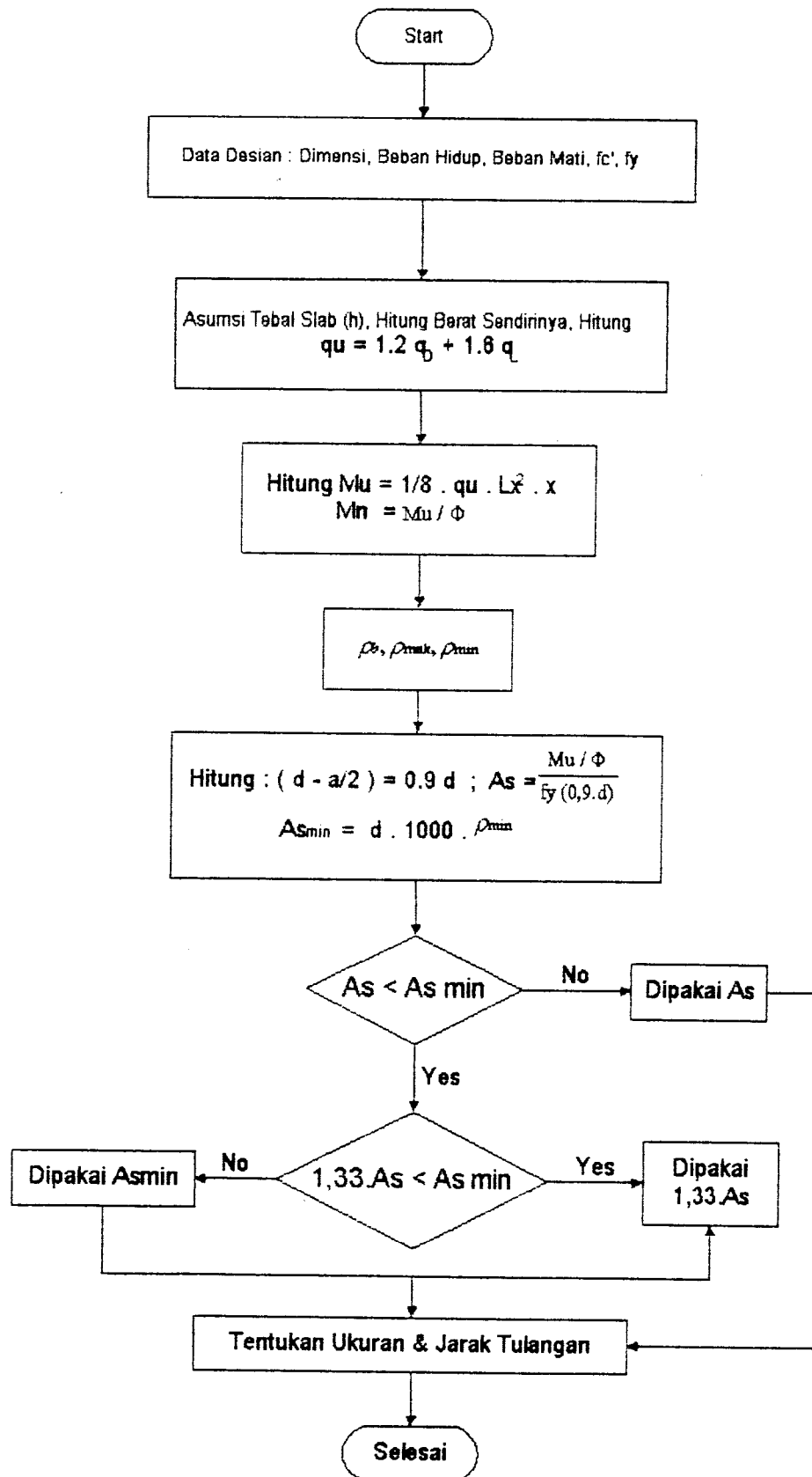
$$A_{s \text{ min}} = \frac{1,4}{f_y} \cdot b \cdot d$$

dan

2. a).  $1,33 \cdot A_s > A_{s \text{ min}} \Rightarrow$  dipakai  $A_{s \text{ min}}$

b).  $1,33 A_s < A_{s \text{ min}} \Rightarrow$  dipakai  $1,33 A_s$

Untuk perencanaan pelat ini akan lebih mudah dengan memakai flowchart seperti gambar 2.1



Gambar 2.1 Diagram Alir Desain Pelat Lantai

## 2.5. Perencanaan Balok

### 2.5.1. Perencanaan Balok Lentur

#### A. Perencanaan Balok Tulangan Rangkap

$$\text{Diketahui : } M_n = \frac{M_u}{\phi} ; b, h, f_c', f_y, d', d$$

$$\rho - \rho' \text{ diambil} = 0,5 \rho_b = \rho_1$$

#### B. Menentukan $M_{n1}$

$$A_{s1} = \rho_1 \cdot b \cdot d$$

$$a = \frac{A_{s1} \cdot f_y}{0,85 \cdot f'_c \cdot b}$$

$$M_{n1} = A_{s1} \cdot f_y \left( d - \frac{a}{2} \right) < M_u / \phi \Rightarrow \text{dipakai tulangan rangkap}$$

#### C. Kontrol $M_n$

$$\rho = \frac{A_s}{b \cdot d} ; \rho' = \frac{A_{s'}}{b \cdot d}$$

$$f_s' = 600 \cdot \left( 1 - \frac{0,85 \cdot f'_c \cdot \beta_1 \cdot d'}{(\rho - \rho') \cdot f_y \cdot d} \right) \leq f_y$$

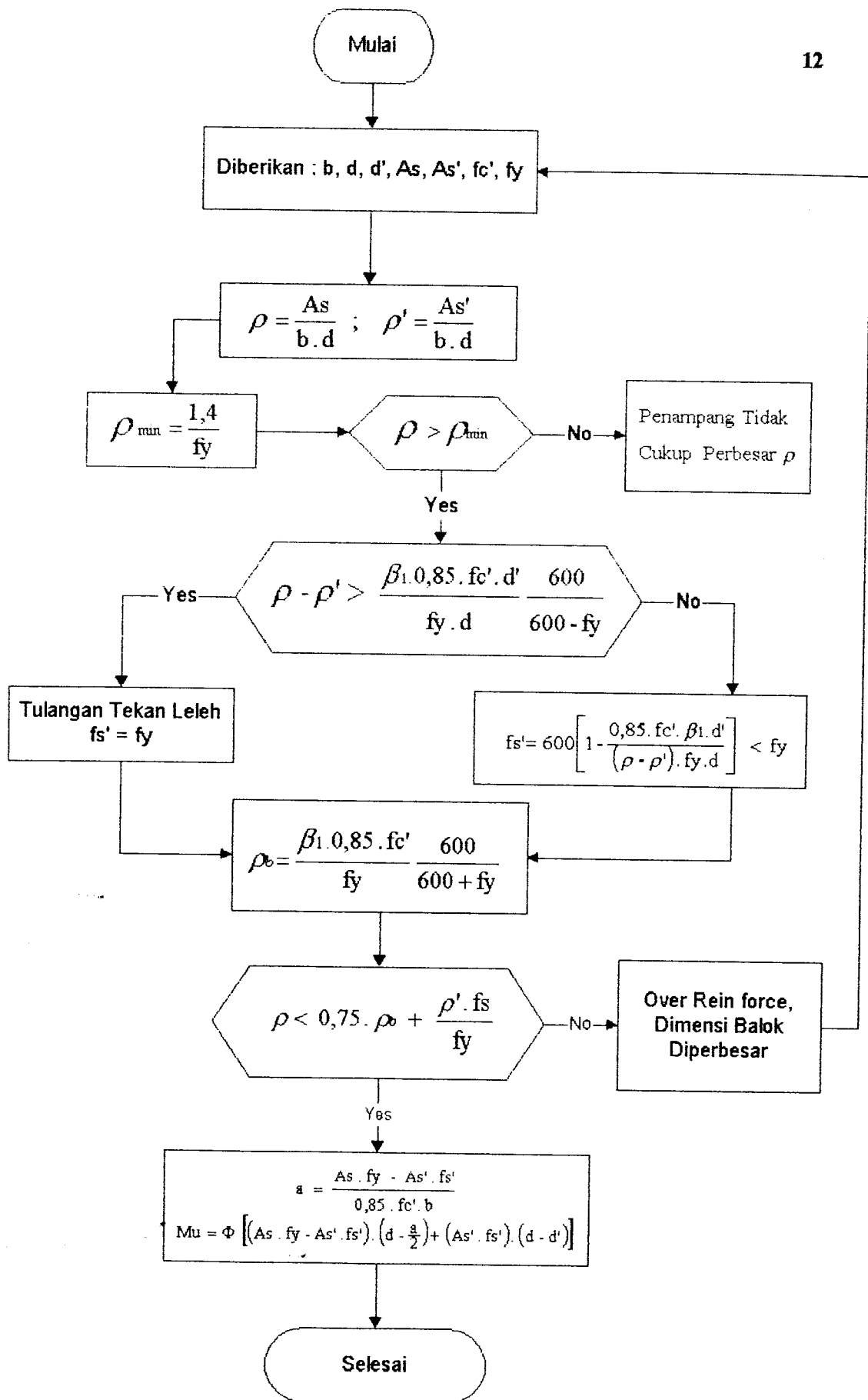
$$a = \frac{A_s \cdot f_y - A_{s'} \cdot f_s'}{0,85 \cdot f'_c \cdot d}$$

$$M_{n1} = (A_s - A_{s'}) \cdot f_y \left( d - a / 2 \right)$$

$$M_{n2} = A_{s'} \cdot f_y \left( d - d' \right)$$

$$M_{n1} + M_{n2} > M_n$$

Untuk perencanaan balok dengan desain tulangan rangkap ini akan lebih mudah dengan memakai flowchart seperti gambar 2.2



Gambar 2.2. Diagram Alir Desain Balok Tulangan Rangkap

### 2.5.2. Penulangan Geser dan Torsi

Pada analisis struktur tiga dimensi dengan kondisi beban yang berbeda serta letak komponen struktur yang tidak simetris dapat menimbulkan torsi dan geser pada masalah ini dipakai tulangan geser dan torsi. Perencanaan tulangan sebagai berikut :

#### A. Penulangan geser

Syarat-syarat tulangan geser berdasarkan SK SNI T-15-1991-03

$$1. V_u \leq \frac{1}{2} \Phi V_c$$

Tidak perlu tulangan geser, dipakai tulangan geser minimum

$$2. \frac{1}{2} \Phi V_c \leq V_u \leq \Phi V_c$$

dipakai tulangan geser dimana  $\Phi V_{s \min} = \Phi A_v \cdot f_y \cdot d / S_{\max}$

$$\text{jarak (s)} = S_{\max} \leq \frac{1}{2} d$$

$$S_{\max} \leq 600 \text{ mm}$$

$$S_{\max} = A_v \cdot f_y \cdot d / V_{s \min}$$

Syarat ini tidak berlaku untuk pelat lantai, pelat pondasi, balok dengan  $d \leq 250$  mm atau  $d \leq 2,5$  x tebal sayap.

$$3. \Phi V_c \leq V_u \leq \Phi (V_c + V_{s \min})$$

dipakai syarat no. 2 dan dihitung jarak sengkang berdasarkan

$$V_{s \text{ perlu}} = V_u / \Phi - V_c$$

$$4. \Phi (V_c + V_{s \min}) \leq V_u \leq 3 \Phi V_c$$

dipakai syarat no. 2 dan dihitung jarak sengkang berdasarkan

$$V_{s \text{ perlu}} = V_u / \Phi - V_c$$

$$5. 3 \Phi V_c \leq V_u \leq 5 \Phi V_c$$

$$\frac{A_t}{s} =$$

$$\text{jarak (s)} = S_{\max} \leq \frac{1}{4} d$$

$$S_{\max} \leq 300 \text{ mm}$$

Luas tul

$$S_{\max} = A_v \cdot f_y \cdot d / V_{S \min}$$

A1 = ;

dipakai syarat no. 2 dan dihitung jarak sengkang berdasarkan

N = A

$$V_{S \text{ perlu}} = V_u / \Phi - V_c$$

Tulangan Geser :

2.6 Perenca

$V_u$  dari hitungan SAP 90.

$$V_c = \frac{1}{6} \cdot (\sqrt{f_c'}) \cdot b \cdot d$$

$$A_v = 2 \cdot A_{1\emptyset \text{ geser}}$$

Perhitung

serta bebar

dan tabel.

$$\text{jarak (s)} = S_{\max} \leq \frac{1}{2} d$$

Pembag

$$S_{\max} \leq 600 \text{ mm}$$

dua cara :

$$S_{\max} = A_v \cdot f_y \cdot d / V_{S \min}$$

1. Tulanga

$S_{\max}$  dipakai yang terkecil

arah ler

2. Tulanga

$$\Phi V_{S \min} = \Phi A_v \cdot f_y \cdot d / S_{\max}$$

=  $A_{s'}$  =

Tulangan geser yang dipakai disesuaikan dengan syarat-syarat diatas.

A. Perenca

B. Penulangan torsi

Dari analisis program SAP 90 didapatkan gaya torsi ( $T_u$ ).

$$T_n = T_u / 0,6$$

Mu =

Pu =

$$T_c = \frac{\sqrt{f_c'}}{15} \cdot \sum x^2 \cdot y$$

Ag =

$$T_s = T_n - T_c$$

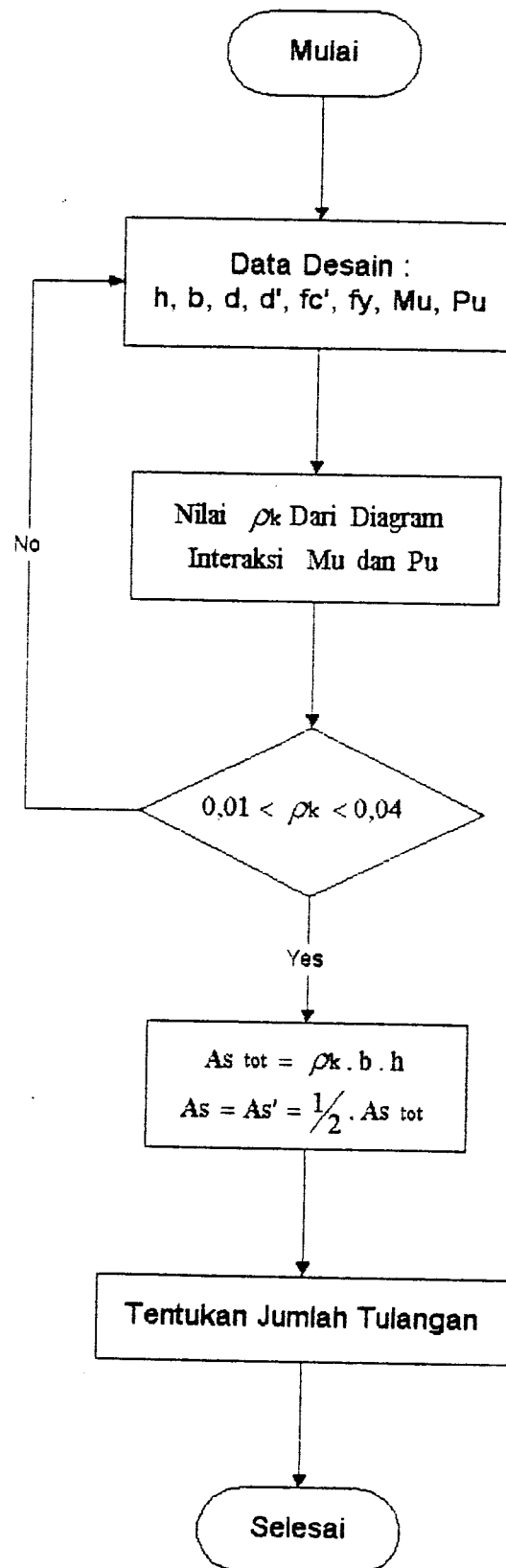
Untuk p

$$X1 = x \cdot 2 (pb + ts / 2) ; Y1 = y - 2 (pb + ts / 2)$$

memakai fl

$$\alpha_t = 1/3 \cdot (2 + (Y1 / X1)) < 1,5$$





Gambar 2.3 Diagram Alir Desain Kolom Persegi

## 2.7 Perencanaan Pelat Pondasi

Pondasi yang direncanakan dalam perhitungan gedung ini adalah pondasi telapak, kondisi tanah pada lokasi adalah tanah baik. Cara perencanaan pondasi pelat ini sama dengan perencanaan pelat dua arah pada umumnya hanya pada bagian bawah dari pelat pondasi ini diberi balok sloof yang berfungsi sebagai pengikat kolom.

Perencanaan pondasi telapak :

### 1. Pemeriksaan terhadap kuat geser

Gaya geser total terfaktor yang bekerja pada penampang kritis :

$$V_u = p_u (A^2 - B^2)$$

$p_u$  = Tekanan tanah terfaktor yang diakibatkan beban yang bekerja

$A$  = Luas telapak pondasi empat persegi panjang

$B$  = Lebar kolom + tinggi efektif

Kuat geser beton :

$$V_c = (1 + (2/\beta_c)) \cdot (2 \cdot \sqrt{f'_c} \cdot b_o \cdot d) \leq 4 \sqrt{f'_c} \cdot b_o \cdot d$$

$\beta_c$  = rasio panjang / lebar kolom = 1

$$b_o = 4B$$

sehingga kuat geser maksimum adalah :

$$V_c = 4 \sqrt{f'_c} \cdot b_o \cdot d$$

$$\phi V_n = \phi V_c \Rightarrow V_u < \phi V_n$$

### 2. Pemulangan Momen

Untuk perencanaan pemulangan pondasi sama dengan pemulangan pelat digunakan ketentuan sebagai berikut :

$$A_s = \frac{M_n}{f_y \cdot (0,9)d}$$

Cek rasio penulangan ( $\rho$ ) :

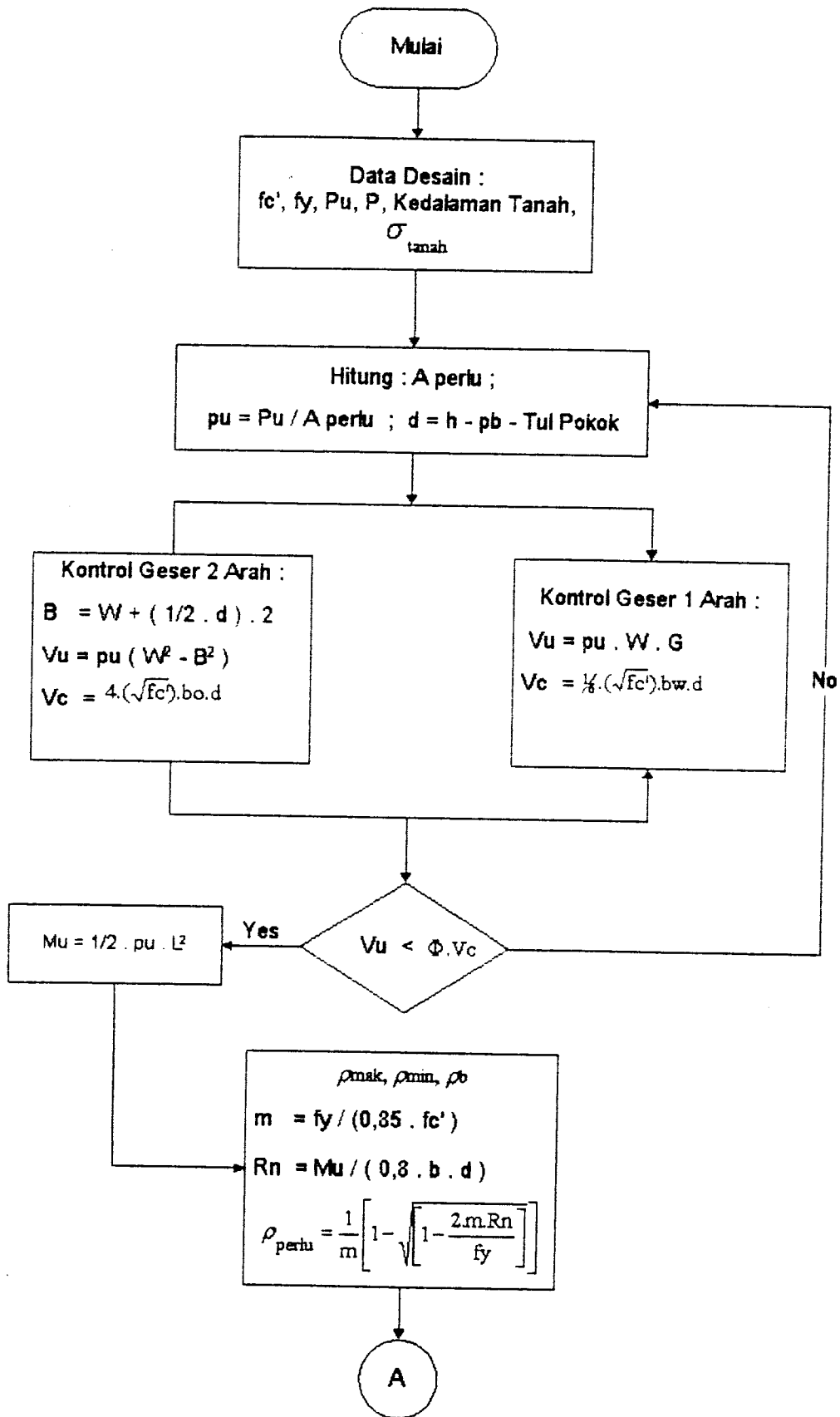
a. Bila  $A_s < A_{s \text{ min}}$

$$A_{s \text{ min}} = \frac{1,4}{f_y} \cdot b \cdot d$$

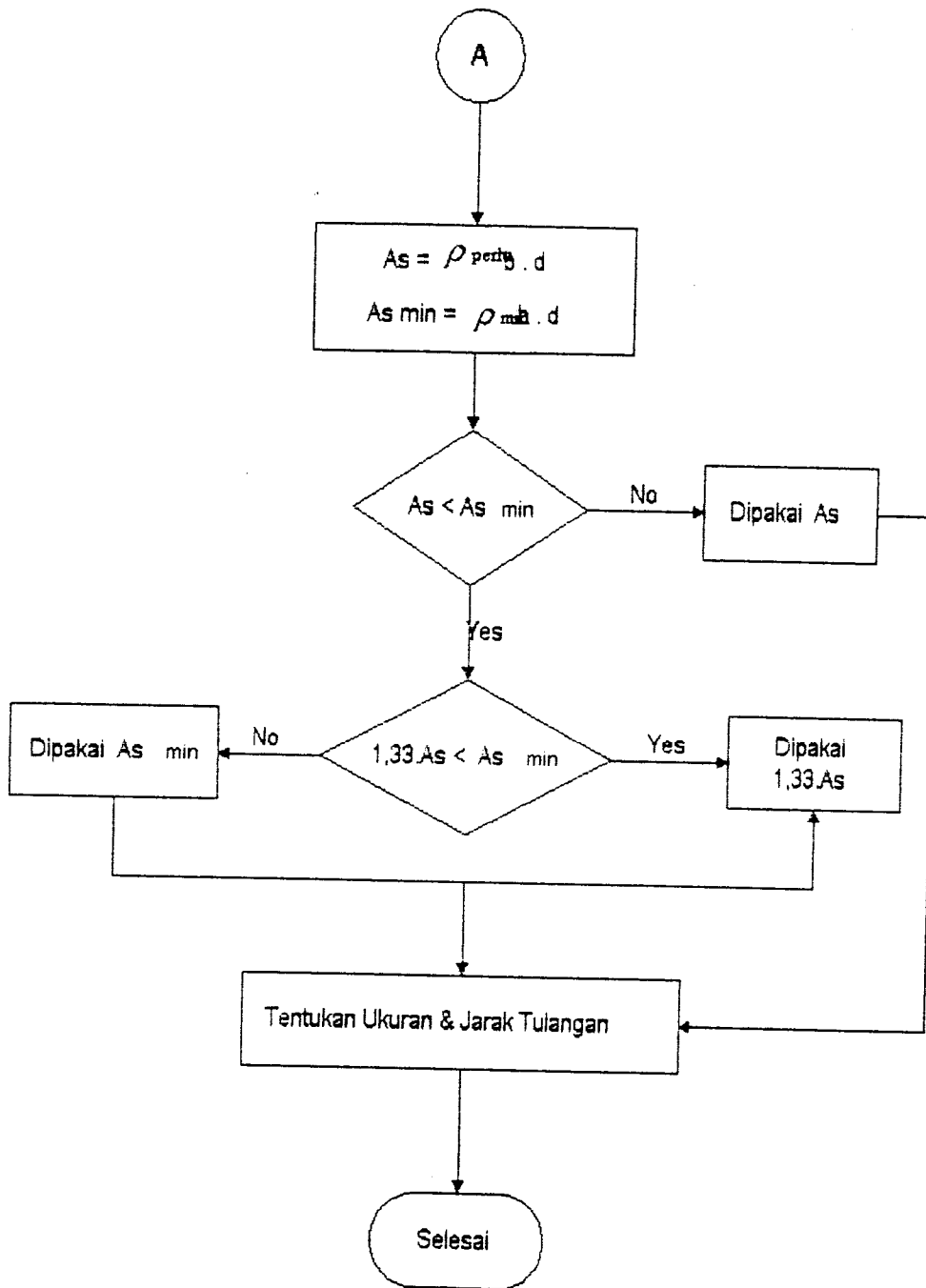
dan

b. 1).  $1,33 \cdot A_s > A_{s \text{ min}} \Rightarrow$  dipakai  $A_{s \text{ min}}$

2).  $1,33 \cdot A_s < A_{s \text{ min}} \Rightarrow$  dipakai  $1,33 \cdot A_s$



Gambar 2.4 Diagram Alir Desain Pondasi



Sambungan Gambar 2.4

## 2.8. Perencanaan Kuda-kuda

Ketentuan umum perencanaan kuda-kuda baja menggunakan rumus-rumus AISC ( American Institute of Steel Construction ).

### A. Perencanaan Gording

$$\frac{fb_{\perp}}{0,66 \cdot Fy} + \frac{fb_{//}}{0,75 \cdot Fy} \leq 1$$

Lendutan :

$$\delta_{\perp} = \frac{5}{384} \cdot \frac{q_{\perp} \cdot (Ly)^4}{E \cdot Ix} \leq L/360$$

$$\delta_{//} = \frac{5}{384} \cdot \frac{q_{//} \cdot (Lx / (a+1))^4}{E \cdot Iy} \leq L/360$$

a = jumlah sagrod dalam satu lubang

### B. Perencanaan Sagrod

$$A_{\text{sagrod}} = \frac{P}{0,33 \cdot Fu} = \frac{1}{4} \cdot \pi \cdot D_{\text{sagrod}}^2$$

D yang dipakai =  $D_{\text{sagrod}} + 3 \text{ mm}$

### C. Perencanaan Tierod

$$T = P_{//} \cdot \cos \alpha ; \quad A_{\text{tierod}} = \frac{T}{0,33 \cdot Fu} = \frac{1}{4} \cdot \pi \cdot D_{\text{tierod}}^2$$

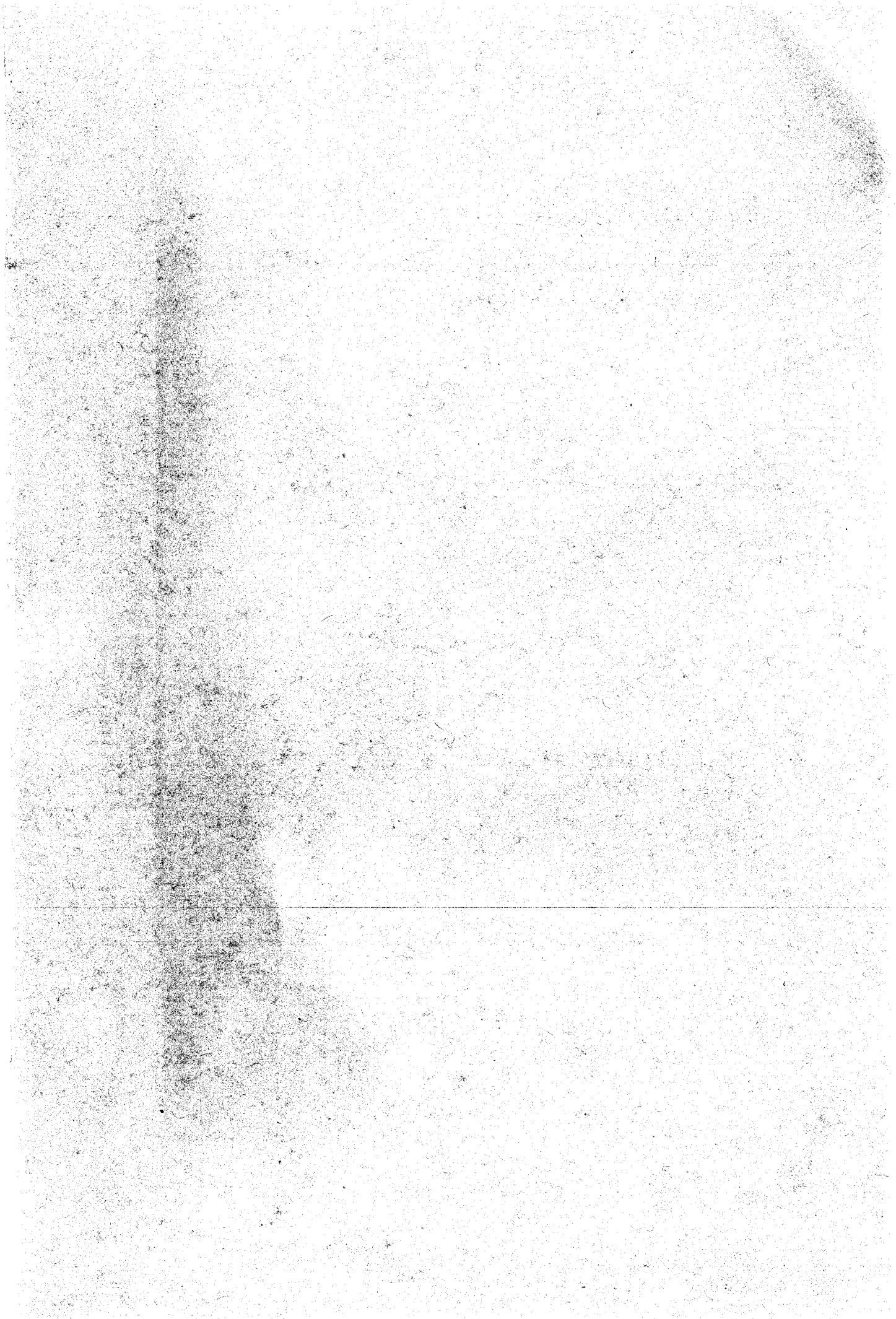
D yang dipakai =  $D_{\text{tierod}} + 3 \text{ mm}$

### D. Perencanaan Sambungan

$$P_{\text{tumpuan}} = tp \cdot D_{\text{baut}} \cdot 1,2 \cdot Fu \cdot n$$

$$P_{\text{geser}} = \frac{1}{4} \cdot \pi \cdot D^2 \cdot 0,33 \cdot Fu \cdot 2n$$

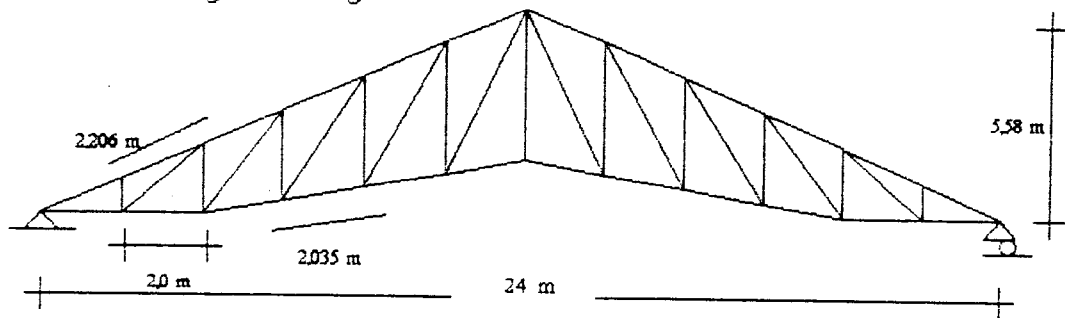
Diambil P yang terkecil.



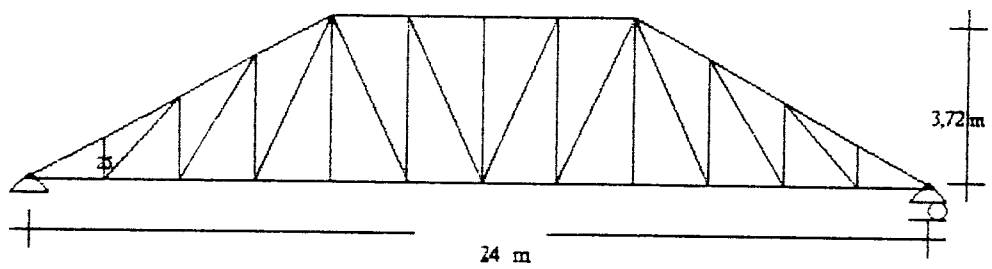
**BAB III**  
**PERHITUNGAN KONSTRUKSI**

**3.1. Perancangan Kuda-Kuda**

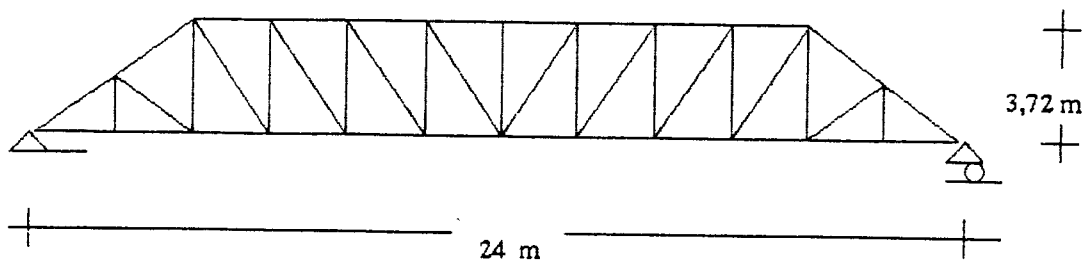
**3.1.1. Perancangan Gording**



Gambar 3.1 Kuda - Kuda KT



Gambar 3.2 Kuda-kuda KT1



Gambar 3.3 Kuda-kuda KT2



## 1. Pembebanan Gording

## A. Beban tetap

$$1. \text{ berat penutup atap} = 50 \cdot 2,206 = 110,3 \text{ kg/m}$$

$$2. \text{ beban hidup} = 20 \cdot 2,206 = 44,12 \text{ kg/m}$$

$$3. \text{ beban gording} = \underline{6,76 \text{ kg/m}}$$

$$q = 161,18 \text{ kg/m}$$

$$\text{Beban Sejajar Atap (q //)} = q \cdot \sin \alpha$$

$$= 161,18 \cdot \sin 25^\circ$$

$$= 68,12 \text{ kg/m}$$

$$\text{Beban Tegak Lurus Atap (q \perp)} = q \cdot \cos \alpha$$

$$= 161,08 \cdot \cos 25^\circ$$

$$= 146,08 \text{ kg/m}$$

## B. Beban angin (w)

## 1. angin tekan ( wt )

$$C_1 = 0,02 \cdot \alpha - 0,4$$

$$= 0,02 \cdot 25^\circ - 0,4$$

$$= 0,1$$

$$wt = C_1 \times w \times \text{jarak gording}$$

$$= 0,1 \cdot 25 \cdot 2,206$$

$$= 5,515 \text{ kg/m}$$

## 2. angin hisap ( wh )

$$C_2 = - 0,4$$

$$wt = C_2 \times w \times \text{jarak gording}$$

$$= - 0,4 \cdot 25 \cdot 2,206$$

$$= - 22,06 \text{ kg/m}$$

## 2. Perhitungan Momen

### A. akibat beban tetap

$$q_{//} = 68,12 \text{ kg/m}$$

$$q_{\perp} = 146,08 \text{ kg/m}$$

$$1. M_{\perp} = \frac{1}{8} \cdot q_{\perp} \cdot L_{\perp}^2 = \frac{1}{8} \cdot 146,08 \cdot 4^2 = 292,16 \text{ kgm}$$

$$2. M_{//} = \frac{1}{90} \cdot q_{//} \cdot L_{//}^2 = \frac{1}{90} \cdot 68,12 \cdot 4^2 = 12,11 \text{ kgm}$$

### B. akibat beban angin

$$w_h = -22,06 \text{ kg/m}$$

$$w_t = 5,515 \text{ kg/m}$$

$$M_{\max} = \frac{1}{8} \cdot 5,515 \cdot 4^2 = 11,03 \text{ kgm}$$

## 3. Dimensi Gording

### A. Data profil C 150x50x20x3,2 dengan berat 6,76 kg/m

$$I_x = 280 \text{ cm}^4 \quad I_y = 28,3 \text{ cm}^4$$

$$w_x = 37,4 \text{ cm}^3 \quad w_y = 8,19 \text{ cm}^3$$

akibat beban tetap

$$f_{bx} = \frac{M_{\perp}}{w_x} = \frac{292,16 \cdot 10^2}{37,4} = 781,176 \text{ kg/cm}^2$$

$$f_{by} = \frac{M_{//}}{w_y} = \frac{12,11 \cdot 10^2}{8,19} = 147,86 \text{ kg/cm}^2$$

$$\begin{aligned} \frac{f_{bx}}{0,66 \cdot F_y} + \frac{f_{by}}{0,75 \cdot F_y} &= \frac{781,176}{0,66 \cdot 2500} + \frac{147,86}{0,75 \cdot 2500} \\ &= 0,55 < 1,0 \end{aligned}$$

#### 4. Kontrol Lendutan

$$\delta_{\perp} = \frac{5}{384} \cdot \frac{q_{\perp} L^4}{E \cdot I_x} = \frac{5}{384} \cdot \frac{146,08 \cdot 10^{-2} \cdot 400^4}{2,1 \cdot 10^6 \cdot 280} = 0,828 \text{ cm} \leq 400/360 = 1,11 \text{ cm}$$

$$\delta_{//} = \frac{5}{384} \cdot \frac{q_{//} L^4}{E \cdot I_y} = \frac{5}{384} \cdot \frac{68,12 \cdot 10^{-2} \cdot [400 / (2+1)]^4}{2,1 \cdot 10^6 \cdot 28,3} = 0,0344 \text{ cm} \leq 1,11 \text{ cm}$$

#### 5. Perencanaan Sagrod dan Tierod

##### A. Beban Sagrod

1. berat penutup atap x (1/2.L/cos α)

$$= 50 \cdot 1/2 \cdot 24 / \cos 25^\circ = 662,027 \text{ kg/m}$$

2. beban hidup x (1/2.L/cos α)

$$= 20 \cdot 1/2 \cdot 24 / \cos 25^\circ = 264,81 \text{ kg/m}$$

3. jumlah gording satu sisi x berat gording

$$= 7,6,76 = \underline{47,32 \text{ kg/m}}$$

$$P = 974,157 \text{ kg/m}$$

$P_{//} = P \cdot \sin \alpha \cdot \text{jarak antar sagrod}$

$$= 974,157 \sin 25^\circ \cdot 4/3$$

$$= 548,928 \text{ kg}$$

##### B. Dimensi Sagrod.

$$A_{\text{sagrod}} = \frac{P_{//}}{0,33 \cdot F_u} = \frac{548,928}{0,33 \cdot 3600} = 0,46 \text{ cm}^2$$

$$D = \sqrt{\frac{4 \cdot P}{0,33 \cdot F_u \cdot \pi}}$$

$$D = \sqrt{\frac{4 \cdot 548,928}{0,33 \cdot 3600 \cdot \pi}}$$

$$D = 0,767 \text{ cm}$$

dipakai diameter sagrod

$$D \text{ sagrod} = D + 3 \text{ mm}$$

$$= 0,767 + 0,3$$

$$= 1,067 \text{ cm} = 10,67 \text{ mm} \approx 12 \text{ mm} \rightarrow \varnothing 12$$

$$A \varnothing 12 = \frac{1}{4} \pi D^2 = \frac{1}{4} \pi 1,2^2$$

$$= 1,13 \text{ cm}^2 > 0,46 \text{ cm}^2$$

chek :

$$\frac{P_{//}}{A \text{ sagrod}} \leq 0,33 \cdot F_u$$

$$\frac{548,928}{1,13} \leq 1188$$

$$485,77 \leq 1188 \text{ kg/cm}^2$$

### C. Dimensi Tierod

$$T = P_{//} \cos \alpha$$

$$= 548,928 \cos 25^\circ$$

$$= 497,498 \text{ kg}$$

$$A \text{ tierod} = \frac{T}{0,33 \cdot F_u} = \frac{497,498}{0,33 \cdot 3600} = 0,418 \text{ cm}^2$$

$$D = \sqrt{\frac{4 \cdot T}{0,33 \cdot F_u \cdot \pi}}$$

$$= \sqrt{\frac{4 \cdot 497,498}{0,33 \cdot 3600 \cdot \pi}}$$

$$= 0,73 \text{ cm}$$

dipakai diameter tierod

$$D \text{ tierod} = D + 3 \text{ mm}$$

$$= 0,73 + 0,3$$

$$= 1,03 \text{ cm} = 10,3 \text{ mm} \approx 12 \text{ mm} \rightarrow \varnothing 12$$

$$A \varnothing 12 = \frac{1}{4} \pi \cdot D^2 = \frac{1}{4} \pi \cdot 1,2^2$$

$$= 1,13 \text{ cm}^2 > 0,46 \text{ cm}^2$$

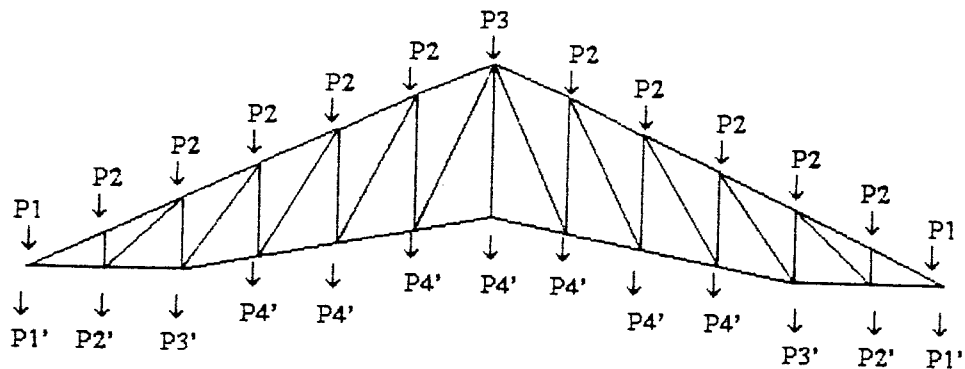
chek :

$$\frac{T}{A \text{ tierod}} \leq 0,33 \cdot F_u$$

$$\frac{497,498}{1,13} \leq 1188$$

$$440,26 \leq 1188 \text{ kg/cm}^2$$

### 3.1.2. Perencanaan Kuda-kuda KT



Gambar 3.4 Kuda - Kuda KT Akibat Beban Tetap

## 1. Pembebanan kuda-kuda KT

## A. Beban tetap

1. berat gording = 6,76 kg / m
2. berat eternit + penggantung = 18 kg / m<sup>2</sup>
3. berat penutup atap = 50 kg / m<sup>2</sup>
4. beban hidup = 20 kg / m<sup>2</sup>
5. taksiran berat kuda-kuda = 120 kg / m

## a. Beban masing-masing joint

## 1. P1

$$\text{berat gording} = 6,76 \cdot 4 = 27,04 \text{ kg}$$

$$\text{berat penutup atap} = 50 \cdot 4 \cdot \frac{1}{2} \cdot 2,206 = 220,6 \text{ kg}$$

$$\text{beban hidup} = 20 \cdot 4 \cdot \frac{1}{2} \cdot 2,206 = \underline{88,24 \text{ kg}}$$

$$P1 = 335,08 \text{ kg}$$

## 2. P2

$$\text{berat gording} = 6,76 \cdot 4 = 27,04 \text{ kg}$$

$$\text{berat penutup atap} = 50 \cdot 4 \cdot 2,206 = 441,2 \text{ kg}$$

$$\text{beban hidup} = 20 \cdot 4 \cdot 2,206 = \underline{176,48 \text{ kg}}$$

$$P2 = 644,72 \text{ kg}$$

## 3. P3

$$\text{berat gording} = 2 \cdot 6,76 \cdot 4 = 54,08 \text{ kg}$$

$$\text{berat penutup atap} = 50 \cdot 4 \cdot 2,206 = 441,2 \text{ kg}$$

$$\text{beban hidup} = 20 \cdot 4 \cdot 2,206 = \underline{176,48 \text{ kg}}$$

$$P3 = 671,76 \text{ kg}$$

## 4. P1'

$$\text{berat eternit + plafond} = 18 \cdot 4 \cdot \frac{1}{2} \cdot 2 = 72 \text{ kg}$$

$$\text{berat taksiran kuda-kuda} = 120 \cdot \frac{1}{2} \cdot 2 = \underline{120 \text{ kg}}$$

$$P1' = 192 \text{ kg}$$

5. P2'

$$\text{berat eternit + plafond} = 18 \cdot 4 \cdot 2 = 144 \text{ kg}$$

$$\text{berat taksiran kuda-kuda} = 120 \cdot 2 = \underline{240 \text{ kg}}$$

$$P2' = 384 \text{ kg}$$

6. P3'

$$\text{berat eternit + plafond} = 18 \cdot 4 \cdot \frac{1}{2} \cdot (2 + 2,035) = 145,26 \text{ kg}$$

$$\text{berat taksiran kuda-kuda} = 120 \cdot \frac{1}{2} \cdot (2 + 2,035) = \underline{242,1 \text{ kg}}$$

$$P3' = 387,36 \text{ kg}$$

7. P4'

$$\text{berat eternit + plafond} = 18 \cdot 4 \cdot 2,035 = 146,52 \text{ kg}$$

$$\text{berat taksiran kuda-kuda} = 120 \cdot 2,035 = \underline{244,2 \text{ kg}}$$

$$P1' = 390,72 \text{ kg}$$

B. Beban angin

$$\text{muatan angin} = 25 \text{ kg/m}^2$$

a. beban yang bekerja

$$C1 = 0,02 \cdot 25 = 0,5$$

$$C2 = -0,4$$

$$w_t = 0,5 \cdot 25 = 12,5 \text{ kg/m}^2$$

$$w_h = -0,4 \cdot 25 = -10 \text{ kg/m}^2$$

b. angin kiri

1. sisi kiri

$$w_1 = 12,5 \cdot \frac{1}{2} \cdot 2,206 \cdot 4 = 11,03 \text{ kg}$$

$$w_2 = 12,5 \cdot 2,206 \cdot 4 = 22,06 \text{ kg}$$

2. sisi kanan

$$w1 = -10 \cdot \frac{1}{2} \cdot 2,206 \cdot 4 = -44,12 \text{ kg}$$

$$w2 = -10 \cdot 2,206 \cdot 4 = -88,24 \text{ kg}$$

c. angin kanan

1. sisi kanan

$$w1 = 2,5 \cdot \frac{1}{2} \cdot 2,206 \cdot 4 = 11,03 \text{ kg}$$

$$w2 = 2,5 \cdot 2,206 \cdot 4 = 22,06 \text{ kg}$$

2. sisi kiri

$$w1 = -10 \cdot \frac{1}{2} \cdot 2,206 \cdot 4 = -44,12 \text{ kg}$$

$$w2 = -10 \cdot 2,206 \cdot 4 = -88,24 \text{ kg}$$

### 3.1.3. Perencanaan Plat Tumpuan Kuda - Kuda

$$P = 6225,48 \text{ kg}$$

$$f_c' = 20 \text{ Mpa} = 200 \text{ kg/cm}^2$$

$$\begin{aligned} A_{\text{perlu}} &= \frac{P}{0,33 \cdot f_c'} \\ &= \frac{6225,48}{0,33 \cdot 200} \\ &= 94,325 \text{ cm}^2 \end{aligned}$$

dipakai B = 15 cm ; L = 20 cm

$$\begin{aligned} q &= \frac{P}{B \cdot L} \\ &= \frac{6225,48}{15 \cdot 20} \\ &= 20,75 \text{ kg/cm}^2 \end{aligned}$$



diambil  $b = 1$  satuan

maka,  $q = 20,75 \cdot 1$

$$= 20,75 \text{ kg/cm}$$

$$M = \frac{1}{2} \cdot q \cdot x^2$$

$$= \frac{1}{2} \cdot 20,75 \cdot 2,88^2$$

$$= 86,05 \text{ kgcm}$$

$$tp = \sqrt{10 \frac{M}{F_y}}$$

$$= \sqrt{10 \frac{86,05}{2500}}$$

$$= 0,587 \text{ cm} \approx 6 \text{ mm}$$

dipakai plat tumpu :  $150 \times 200 \times 6$

#### 3.1.4. Perencanaan Dukungan Arah Lateral

$$L_c = \text{Jarak kuda-kuda} = 4,0 \text{ m}$$

$$L_b = \text{Panjang satu batang atas} = 2,206 \text{ m}$$

$$L = \sqrt{L_c^2 + L_b^2}$$

$$L = \sqrt{4,0^2 + 2,206^2}$$

$$L = 4,56 \text{ m}$$

Dipakai Profil  $\varnothing 16$

#### 3.1.5. Perencanaan Baut dan Sambungan

##### 1. Perhitungan kekuatan baut ijin

$$\text{Dipakai baut } \phi 1/2'' = 1,27 \text{ cm}$$

$$\text{Tebal plat sambung (tp)} = 1 \text{ cm}$$

$$\text{Tegangan luluh baja profil (Fy)} = 2500 \text{ kg/cm}^2$$

$$\text{Kuat Tarik Baja Profil (Fu)} = 3700 \text{ kg/cm}^2$$

$$\text{Fa tarik} = 0,33 \cdot \text{Fu}$$

$$= 0,33 \cdot 3700$$

$$= 1227 \text{ kg/cm}^2$$

$$\text{Fa tumpuan} = 1,2 \cdot \text{Fu}$$

$$= 1,2 \cdot 3700$$

$$= 4440 \text{ kg/cm}^2$$

$$\text{P tumpuan} = t_p \cdot D \text{ baut} \cdot 1,2 \cdot \text{Fu} \cdot n$$

$$= 1 \cdot 1,27 \cdot 1,2 \cdot 3700 \cdot n$$

$$= 5638,8 n$$

$$\text{P geser} = \frac{1}{4} \cdot \pi \cdot D^2 \cdot 0,33 \cdot \text{Fu} \cdot 2n$$

$$= \frac{1}{4} \cdot \pi \cdot 1,27^2 \cdot 0,33 \cdot 3700 \cdot 2n$$

$$= 3093,449 n$$

Diambil P yang kecil  $P = 3093,449 n$

## 3.1.6. HITUNGAN KUDA - KUDA KT

HITUNGAN KUDA-KUDA TON-METER  
SYSTEM

L=3

## JOINTS

1	X=0	Y=0	Z=0	
3		Y=4	Z=0	G=1,3.1
4		Y=6	Z=0.375	
5		Y=8	Z=0.75	
6		Y=10	Z=1.125	
7		Y=12	Z=1.5	
8		Y=14	Z=1.125	
9		Y=16	Z=0.75	
10		Y=18	Z=0.375	
11		Y=20	Z=0	
13		Y=24	Z=0	G=11.13.1
14		Y=2	Z=0.93	
15		Y=4	Z=1.86	
16		Y=6	Z=2.79	
17		Y=8	Z=3.7	
18		Y=10	Z=4.65	
19		Y=12	Z=5.5	
20		Y=14	Z=4.65	
21		Y=16	Z=3.7	
22		Y=18	Z=2.79	
23		Y=20	Z=1.86	
24		Y=22	Z=0.93	

:

## RESTRAINTS

1			R=1,1,1,0,1,1
13			R=1,0,1,0,1,1
2	12	1	R=1,0,0,0,1,1
14	24	1	R=1,0,0,0,1,1

:

## FRAME

NM=4 Z=-1

## C MATERIAL PROPERTIES

1	SH=2L2X2X1/4	W=2*5.42E-3	E=2.1E7
2	SH=2L2X2X1/8	W=2*4.95E-3	
3	SH=2L2X2X1/8	W=2*4.95E-3	
4	SH=2L3X3X3/16	W=2*7.94e-3	

## C ELEMEN LOCATION DATA

## C BATANG BAWAH

1	1	2	LR=1,1,0,1,1,1	G=1,1,1,1	M=1	LP=3,0
3	3	4	LR=1,1,0,1,1,1	G=7,1,1,1		
11	11	12	LR=1,1,0,1,1,1	G=1,1,1,1		

## C BATANG VERTIKAL

13	2	14	LR=1,1,0,1,1,1	G=1,20,10,10	M=2	LP=3,0
15	3	15	LR=1,1,0,1,1,1	G=1,16,8,8		
17	4	16	LR=1,1,0,1,1,1	G=1,12,6,6		
19	5	17	LR=1,1,0,1,1,1	G=1,8,4,4		
21	6	18	LR=1,1,0,1,1,1	G=1,4,2,2	M=4	
23	7	19	LR=1,1,0,1,1,1		M=2	

## C BATANG DIAGONAL

14	2	15	LR=1,1,0,1,1,1	G=1,18,10,8	M=3	LP=3,0
16	3	16	LR=1,1,0,1,1,1	G=1,14,8,6		

```

18  4  17  LR=1,1,0,1,1,1      G=1,10,6,4
20  5  18  LR=1,1,0,1,1,1      G=1,6,4,2
22  6  19  LR=1,1,0,1,1,1      G=1,2,2,0
C BATANG ATAS
34  1  14  LR=1,1,0,1,1,1      M=4      LP=3,0
35  14 15  LR=1,1,0,1,1,1      G=9,1,1,1
45  24 13  LR=1,1,0,1,1,1
:
LOADS
C BEBAN PADA JOINT
14  18  1      F=0,0,-0.64472,0,0,0      L=1
19      F=0,0,-0.67176,0,0,0
20  24  1      F=0,0,-0.64472,0,0,0
2      F=0,0,-0.384,0,0,0
3      F=0,0,-0.38736,0,0,0
4  10  1      F=0,0,-0.39072,0,0,0
11     F=0,0,-0.38736,0,0,0
12     F=0,0,-0.384,0,0,0
C AKIBAT BEBAN ANGIN KIRI
14  18  1      F=0,0,0.00932,-0.01999,0,0,0      L=2
19     F=0,0,0.02331,0.03,0,0,0
20  24  1      F=0,0,0.03729,0.07997,0,0,0
C AKIBAT BEBAN ANGIN KANAN
14  18  1      F=0,-0.03729,0.07997,0,0,0      L=3
19     F=0,-0.02331,0.03,0,0,0
20  24  1      F=0,-0.00932,-0.01999,0,0,0
:
COMBO
1  C=1,0,0
2  C=1,1,0
3  C=1,0,1

```

## HITUNGAN SAPSTL KUDA - KUDA KT

DATA BAJA UNTUK TRUSS KUDAS

CONTROL

IX=0 IP=0 IU=M IT=1

COMBO

```

1  C=1000
2  C=1000,1000,0
3  C=1000,0,1000
:

```

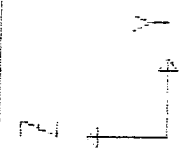
## SECTIONS

C MATERIAL PROPERTIES

```

1 SH=2L2X2X1/4  E=2.1E10 MN=S FY=2.5E7
F=1.7E7,1.7E7,1.7E7,1.25E7,1.42E7
2 SH=2L2X2X1/8  E=2.1E10 MN=S FY=2.5E7
F=1.7E7,1.7E7,1.7E7,1.25E7,1.42E7
3 SH=2L2X2X1/8  E=2.1E10 MN=S FY=2.5E7
F=1.7E7,1.7E7,1.7E7,1.25E7,1.42E7
4 SH=2L3X3X3/16 E=2.1E10 MN=S FY=2.5E7
F=1.7E7,1.7E7,1.7E7,1.25E7,1.42E7

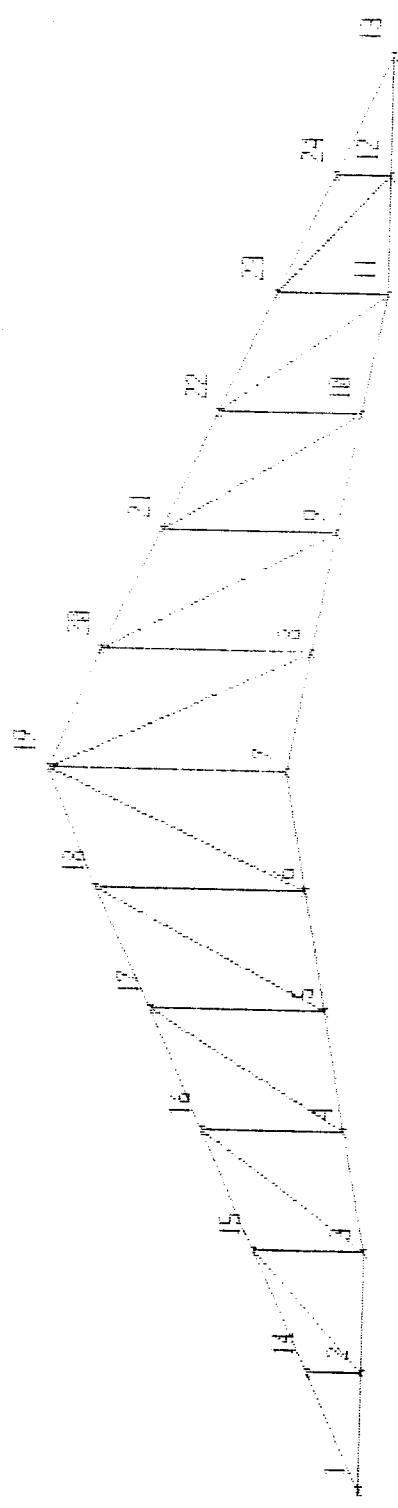
```



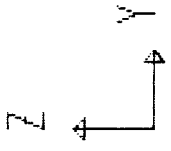
Kuda  
 UNDEFORMED  
 SHAPE

OPTIONS

JOINT IDS  
 ALL JOINTS  
 WIRE FRAME

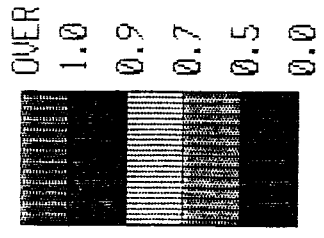


Gambar 3.5. Nomer Joint Struktur Rangka “ Kuda-Kuda KT “



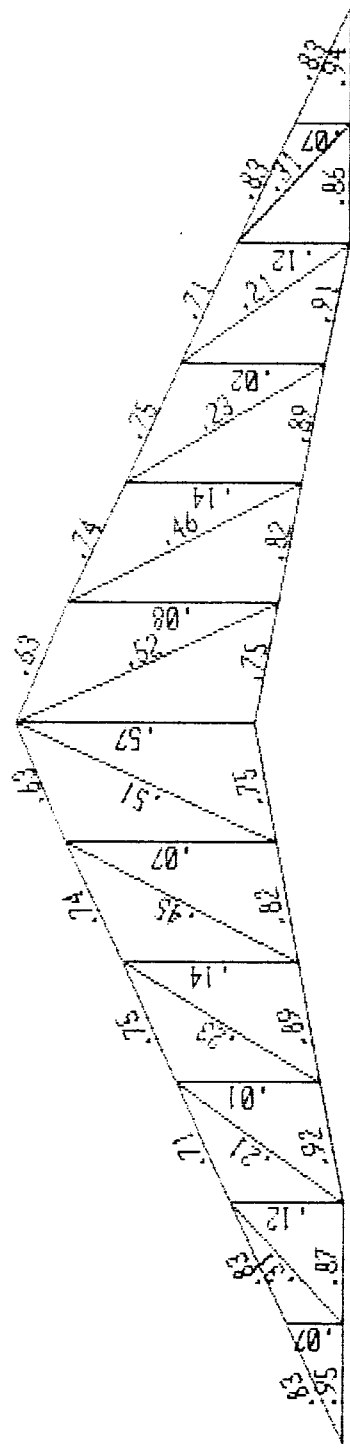
KUDA  
SAPSTL  
RATIOS

LEGEND



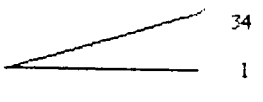
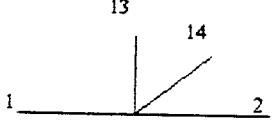
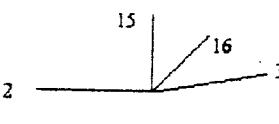
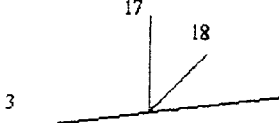
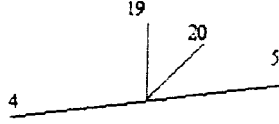
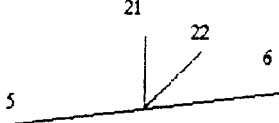
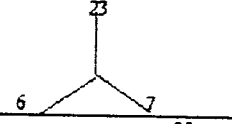
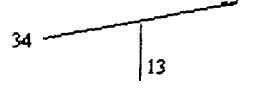
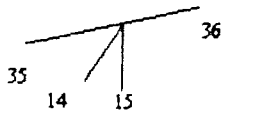
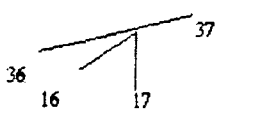
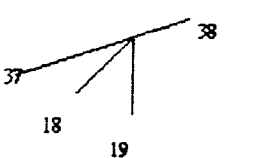
OTHER

SAP90



Gambar 3.5.b. Rasio Tegangan Struktur Rangka "Kuda-Kuda KT"

TABEL 3.1 PERHITUNGAN KEBUTUHAN BOUT KUDA-KUDA KT

Titik	ARAH GAYA	Ø BOUT cm	Gaya Batang kg	Daya Dukung Bout kg	Jumlah Bout
1		1,27	13510	3093,45	5
		1,27	12360		4
2		1,27	12360	3093,45	4
		1,27	11220		4
		1,27	670		2
		1,27	1550		2
3		1,27	11220	3093,45	4
		1,27	11860		4
		1,27	1200		2
		1,27	790		2
4		1,27	11860	3093,45	4
		1,27	11480		4
		1,27	150		2
		1,27	720		2
5		1,27	11480	3093,45	4
		1,27	10490		4
		1,27	1340		2
		1,27	2150		2
6		1,27	10490	3093,45	4
		1,27	9520		4
		1,27	1610		2
		1,27	2400		2
7		1,27	9520	3093,45	4
		1,27	9520		4
		1,27	3900		2
14		1,27	670	3093,45	2
		1,27	13510		5
		1,27	13510		5
15		1,27	1550	3093,45	2
		1,27	1200		2
		1,27	13510		5
		1,27	12290		4
16		1,27	790	3093,45	2
		1,27	150		2
		1,27	12290		4
		1,27	12740		5
17		1,27	720	3093,45	2
		1,27	1340		2
		1,27	12740		5
		1,27	12470		5

18		20 21 38 39	1,27 1,27 1,27 1,27	2150 1610 12470 11200	3093,45	2 2 5 4
19		22 23 24 39 40	1,27 1,27 1,27 1,27 1,27	2400 3900 2440 11200 11200	3093,45	2 2 2 4 4

TABEL 3.2 PERHITUNGAN KEBUTUHAN BAUT KUDA-KUDA KT1

Titik	ARAH GAYA	φBAUT cm	Gaya Batang kg	Daya Dukung Baut kg	Jumlah Baut
1		1,27 1,27	12200 13280	3093,45	4 5
2		1,27 1,27 1,27 1,27	12200 11070 670 1550	3093,45	4 4 2 2
3		1,27 1,27 1,27 1,27	11070 9940 1200 1940	3093,45	4 4 2 2
4		1,27 1,27 1,27 1,27	9940 8850 1620 2280	3093,45	4 3 2 2
5		1,27 1,27 1,27	8850 8850 380	3093,45	3 3 2
6		1,27 1,27 1,27 1,27	8850 9590 1790 1190	3093,45	3 4 2 2
7		1,27 1,27 1,27 1,27 1,27	9590 9470 690 590 670	3093,45	4 4 2 2 2
14		1,27 1,27 1,27	670 13280 13290	3093,45	2 5 5



15		14 15 35 36	1,27 1,27 1,27 1,27	1550 1200 13290 12050	3093,45	2 2 5 4
16		16 17 36 37	1,27 1,27 1,27 1,27	1940 1620 12050 10770	3093,45	2 2 4 4
17		18 19 20 37 38	1,27 1,27 1,27 1,27 1,27	2280 380 1790 10770 9460	3093,45	2 2 2 4 4
18		21 22 38 39	1,27 1,27 1,27 1,27	1190 690 9460 9720	3093,45	2 2 4 4
19		23 39 40	1,27 1,27 1,27	590 9720 9720	3093,45	2 4 4

TABEL 3.3 PERHITUNGAN KEBUTUHAN BAUT KUDA-KUDA KT2

Titik	ARAH GAYA	φBAUT cm	Gaya Batang kg	Daya Dukung Baut kg	Jumlah Baut
1		1,27 1,27	11920 13070	3093,45	4 5
2		1,27 1,27 1,27	11920 11920 380	3093,45	4 4 2
3		1,27 1,27 1,27 1,27	11920 10730 1210 740	3093,45	4 4 2 2
4		1,27 1,27 1,27 1,27	10730 14330 5030 3050	3093,45	4 6 2 2
5		1,27 1,27 1,27 1,27	14330 16890 3610 2080	3093,45	5 6 2 2

6		5 6 20 21	1,27 1,27 1,27 1,27	16890 18410 2180 1110	3093,45	6 6 2 2
7		6 7 22 23 24	1,27 1,27 1,27 1,27 1,27	18410 18340 760 590 760	3093,45	6 6 2 2 2
14		13 14 34 35	1,27 1,27 1,27 1,27	380 1310 13070 11790	3093,45	2 5 5 4
15		14 15 35 36	1,27 1,27 1,27 1,27	940 5030 11790 14280	3093,45	2 2 4 5
16		17 18 36 37	1,27 1,27 1,27 1,27	3050 3610 14280 16830	3093,45	2 2 5 6
17		19 20 37 38	1,27 1,27 1,27 1,27	2080 2180 16830 18350	3093,45	2 2 6 6
18		21 22 38 39	1,27 1,27 1,27 1,27	1110 760 18350 18840	3093,45	2 2 6 7
19		23 39 40	1,27 1,27 1,27	590 18840 18840	3093,45	2 7 7

### 3.2. Perhitungan Pelat

#### 3.2.1. Pembebanan

##### 1. Beban Mati Pelat

Tebal Pelat diambil 0,12 m

$$\text{Berat sendiri} = 0,12 \times 23 = 2,76 \text{ KN/m}^2$$

$$\text{Ubin 2 cm} = 0,02 \times 23 = 0,46 \text{ KN/m}^2$$

$$\text{Spesi 1 cm} = 0,01 \times 23 = 0,23 \text{ KN/m}^2$$

$$\text{Pasir 5 cm} = 0,05 \times 23 = 1,15 \text{ KN/m}^2$$

Langit-langit +

$$\text{Penggantung} = 18 \times 0,00981 = \underline{0,1766 \text{ KN/m}^2}$$

$$q_d = 4,766 \text{ KN/m}^2$$

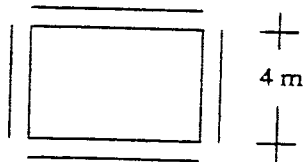
##### 2. Beban Hidup Pelat

Tabel 3-1 Data beban hidup pelat

Pelat AC	=	100 kg/m <sup>2</sup>
R. Baca, A. Visual, Gudang Kelas, Kelas Kecil, R. Diskusi, Kantor, Musolla, Toilet.	=	250 kg/m <sup>2</sup>
Selasar, Kelas Kecil Tingkat, Kelas Besar Tingkat, Hall.	=	300 kg/m <sup>2</sup>
Common Facilities, Perpustakaan, Laboratorium, Gudang Lift.	=	400 kg/m <sup>2</sup>

#### 3.2.2. Perhitungan Pelat

##### 1. Pelat C



$$q_l = 250 \text{ kg/m}^2 = 2,5 \text{ KN/m}^2$$

$$q_d = 4,766 \text{ KN/m}^2$$

$$+ \quad 6 \text{ m} \quad +$$

Tebal pelat lantai = 12 cm

Penutup beton = 25 mm

Diameter tulangan pokok =  $\varnothing 10 \Rightarrow F_y = 240 \text{ Mpa}$

$$d = 120 - 25 - 10/2 = 90 \text{ mm}$$

Konstruksi dianggap terjepit elastis

$$L_y / L_x = 6 / 4 = 1,5$$

$$\begin{aligned} q_u &= 1,2 \cdot q_d + 1,6 \cdot q_l \\ &= 1,2 \cdot 4,766 + 1,6 \cdot 2,5 \\ &= 9,719 \text{ KN/m} \end{aligned}$$

Koefisien momen

$$M_{lx} = - (M_{tx}) = 0,001 \cdot q_u \cdot L_x^2 \cdot x = 0,001 \cdot 9,719 \cdot 4^2 \cdot 56 = 8,708 \text{ KN m}$$

$$(M_{ly}) = 0,001 \cdot q_u \cdot L_x^2 \cdot x = 0,001 \cdot 9,719 \cdot 4^2 \cdot 37 = 5,754 \text{ KN m}$$

$$- (M_{ty}) = 0,001 \cdot q_u \cdot L_x^2 \cdot x = 0,001 \cdot 9,719 \cdot 4^2 \cdot 37 = 5,754 \text{ KN m}$$

a. Perhitungan pemulangan arah  $l_x$  dan  $t_x$  ( diambil  $b = 1000 \text{ mm} = 1 \text{ m}$  )

$$M_{ulx} = 8,708 \text{ KN m}$$

$$M_{ulx} / \Phi = 8,708 / 0,8 = 10,885 \text{ KN m}$$

$$\rho_b = \frac{0,85 \cdot F'_c \cdot \beta_1 \left( \frac{600}{600 + F_y} \right)}{F_y} = \frac{0,85 \cdot 20 \cdot 0,85 \left( \frac{600}{600 + 240} \right)}{240} = 0,043$$

$$\rho_{\text{mak}} = 0,75 \cdot \rho_b = 0,75 \cdot 0,043 = 0,03225$$

$$\rho_{\text{min}} = \frac{1,4}{F_y} = \frac{1,4}{240} = 0,00583$$

$$\text{Misal } (d - a/2) = 0,9 \cdot d = 0,9 \cdot 90 = 81 \text{ mm}$$

$$A_s = \frac{M_u / \Phi}{F_y \cdot (d - a/2)} = \frac{10,885 \cdot 10^6}{240 \cdot 81} = 559,954 \text{ mm}^2$$

$$A_{s \text{ min}} = \rho_{\text{min}} \cdot 1000 \cdot d = 0,00583 \cdot 1000 \cdot 90 = 524,7 \text{ mm}^2 < A_s$$

Dipakai  $A_s = 559,945 \text{ mm}^2$  ; dengan  $\varnothing 10 = \frac{1}{4} \cdot \pi \cdot 10^2 = 78,539 \text{ mm}^2$

$$X = \frac{A_{\varnothing 10} \cdot 1000}{A_s} = \frac{78,539 \cdot 1000}{559,954} = 140,259 \text{ mm}$$

Maka dipakai  $\varnothing 10 - 100$

Kontrol kekuatan

$$A_s = \frac{A_{\varnothing 10} \cdot 1000}{100} = \frac{78,539 \cdot 1000}{100} = 785,39 \text{ mm}^2$$

$$a = \frac{A_s \cdot F_y}{0,85 \cdot F'_c \cdot 1000} = \frac{785,39 \cdot 240}{0,85 \cdot 20 \cdot 1000} = 11,088 \text{ mm}$$

$$\begin{aligned} M_n &= A_s \cdot F_y \cdot (d - a/2) \\ &= 785,39 \cdot 240 (90 - 11,088/2) \\ &= 15,919 \text{ KN m} > M_{u_x} = 10,885 \text{ KN m} \end{aligned}$$

Penulangan bagi dibagian tumpuan  $t_x$

$$A_{s \text{ bagi}} = 20 \% A_s \text{ tulangan pokok} = 20 \% \cdot 559,954 = 111,991 \text{ mm}^2$$

$$A_{\varnothing 8} = \frac{1}{4} \cdot \pi \cdot 8^2 = 50,265 \text{ mm}^2$$

$$X = \frac{A_{\varnothing 8} \cdot 1000}{A_{s \text{ bagi}}} = \frac{50,265 \cdot 1000}{111,991} = 448,831 \text{ mm}$$

Maka dipakai  $\varnothing 8 - 200$

b. Perhitungan penulangan arah  $l_y$  dan  $t_y$  (diambil  $b = 1000 \text{ mm} = 1 \text{ m}$ )

$$M_{u_y} = 5,754 \text{ KN m}$$

$$M_u / \Phi = 5,754 / 0,8 = 7,193 \text{ KN m}$$

$$\rho_b = \frac{0,85 \cdot F'_c \cdot \beta_1 \left( \frac{600}{600 + F_y} \right)}{F_y} = \frac{0,85 \cdot 20 \cdot 0,85 \left( \frac{600}{600 + 240} \right)}{240} = 0,043$$

$$\rho_{\text{mak}} = 0,75 \cdot \rho_b = 0,75 \cdot 0,043 = 0,03225$$

$$\rho_{\min} = \frac{1,4}{F_y} = \frac{1,4}{240} = 0,00583$$

Misal  $(d - a/2) = 0,9 \cdot d = 0,9 \cdot 90 = 81 \text{ mm}$

$$A_s = \frac{M_u / \Phi}{F_y \cdot (d - a/2)} = \frac{7,19310^6}{240 \cdot 81} = 369,969 \text{ mm}^2$$

$$A_{s \min} = \rho_{\min} \cdot 1000 \cdot d = 0,00583 \cdot 1000 \cdot 90 = 524,7 \text{ mm}^2 > A_s$$

$$\text{Chek : } 1,33 \cdot A_s = 1,33 \cdot 369,969 = 492,059 \text{ mm}^2 < A_{s \min}$$

Dipakai  $1,33 \cdot A_s = 492,059 \text{ mm}^2$  ; dengan  $\emptyset 10 = \frac{1}{4} \cdot \pi \cdot 10^2 = 78,539 \text{ mm}^2$

$$X = \frac{A_{\emptyset 10} \cdot 1000}{A_s \cdot 1,33} = \frac{78,539 \cdot 1000}{492,059} = 159,595 \text{ mm}$$

Maka dipakai  $\emptyset 10 - 100$

Kontrol kekuatan

$$A_s = \frac{A_{\emptyset 10} \cdot 1000}{100} = \frac{78,539 \cdot 1000}{100} = 785,39 \text{ mm}^2$$

$$a = \frac{A_s \cdot F_y}{0,85 \cdot F'_c \cdot 1000} = \frac{785,39 \cdot 240}{0,85 \cdot 20 \cdot 1000} = 11,088 \text{ mm}$$

$$\begin{aligned} M_n &= A_s \cdot F_y \cdot (d - a/2) \\ &= 785,39 \cdot 240 (90 - 11,088/2) \\ &= 15,919 \text{ KN m} > M_u = 7,193 \text{ KN m} \end{aligned}$$

Penulangan bagi dibagian tumpuan ty

$$A_{s \text{ bagi}} = 20 \% A_{s \text{ tulangan pokok}} = 20 \% \cdot 492,059 = 98,412 \text{ mm}^2$$

$$A_{\emptyset 8} = \frac{1}{4} \cdot \pi \cdot 8^2 = 50,265 \text{ mm}^2$$

$$X = \frac{A_{\emptyset 8} \cdot 1000}{A_{s \text{ bagi}}} = \frac{50,265 \cdot 1000}{98,412} = 510,76 \text{ mm}$$

Maka dipakai  $\emptyset 8 - 200$

## 3.2.3. HITUNGAN PELAT TANGGA 1

ANALISA PLAT TANGGA [KN-M]  
SYSTEM

L=1

## JOINTS

66	X=1.5	Y=3.9	Z=0	
67	X=1.5		Z=2.29	
1	X=3			A=66.67,1,12,1,15
14	X=2.6375			A=66.67,14,12,1,15
27	X=2.275			A=66.67,27,12,1,15
40	X=1.9125			A=66.67,40,12,1,15
53	X=1.55			A=66.67,53,12,1,15
68	X=0	Y=0	Z=0	
72	X=1.45			
93	X=0	Y=3.25	Z=1.908	
97	X=1.45			Q=68,72,93,97,1.5
98	X=1.55	Y=0	Z=4.5	
102	X=3			
123	X=1.55	Y=3.25	Z=2.6717	
127	X=3			Q=98,102,123,127,1.5

:

## RESTRAINTS

66	67	1	R=1,1,1,1,1,1
68	72	1	R=1,1,1,1,1,1
98	102	1	R=1,1,1,1,1,1

:

## FRAME

## C CONTROL DATA

NM=1 NSEC=5 Z=-1

## C MATERIAL PROPERTIES BALOK

1 SH=R T=0.4,0.25 W=0.4\*0.25\*23 E=2.1E7

## C ELEMENT LOCATION DATA BALOK

9	1	14	M=1	G=3,1,13,13	LP=1,0
13	13	26	M=1	G=3,1,13,13	LP=1,0

:

## SHELL

## C MATERIAL PROPERTIES

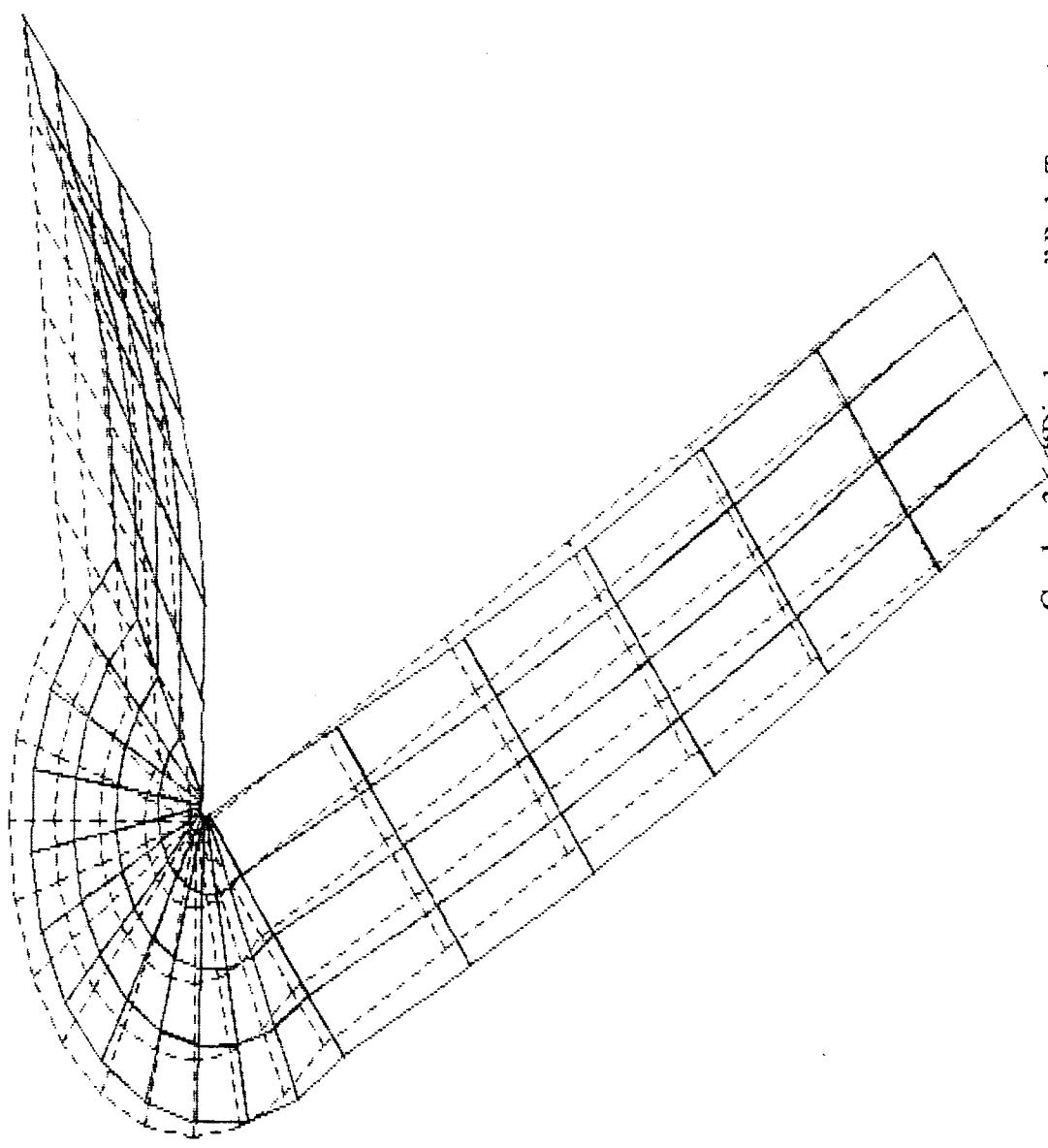
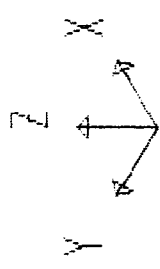
NM=1 Z=-1

## C INFORMATION DATA

1 E=2.1E7 U=0.2 W=12.528/0.2

## C PEMBAGIAN ELEMEN TANGGA

1	JQ=1,2,14,15	M=1	ETYPE=0	TH=0.2	G=12,1
13	JQ=14,15,27,28	M=1	ETYPE=0	TH=0.2	G=12,1
25	JQ=27,28,40,41	M=1	ETYPE=0	TH=0.2	G=12,1
37	JQ=40,41,53,54	M=1	ETYPE=0	TH=0.2	G=12,1
49	JQ=68,69,73,74	M=1	ETYPE=0	TH=0.2	G=4,5
70	JQ=93,94,13,26	M=1	ETYPE=0	TH=0.2	
71	JQ=94,95,26,39	M=1	ETYPE=0	TH=0.2	
72	JQ=95,96,39,52	M=1	ETYPE=0	TH=0.2	
73	JQ=96,97,52,65	M=1	ETYPE=0	TH=0.2	
74	JQ=98,99,103,104	M=1	ETYPE=0	TH=0.2	G=4,5
94	JQ=123,124,53,40	M=1	ETYPE=0	TH=0.2	
95	JQ=124,125,40,27	M=1	ETYPE=0	TH=0.2	
96	JQ=125,126,27,14	M=1	ETYPE=0	TH=0.2	
97	JQ=126,127,14,1	M=1	ETYPE=0	TH=0.2	

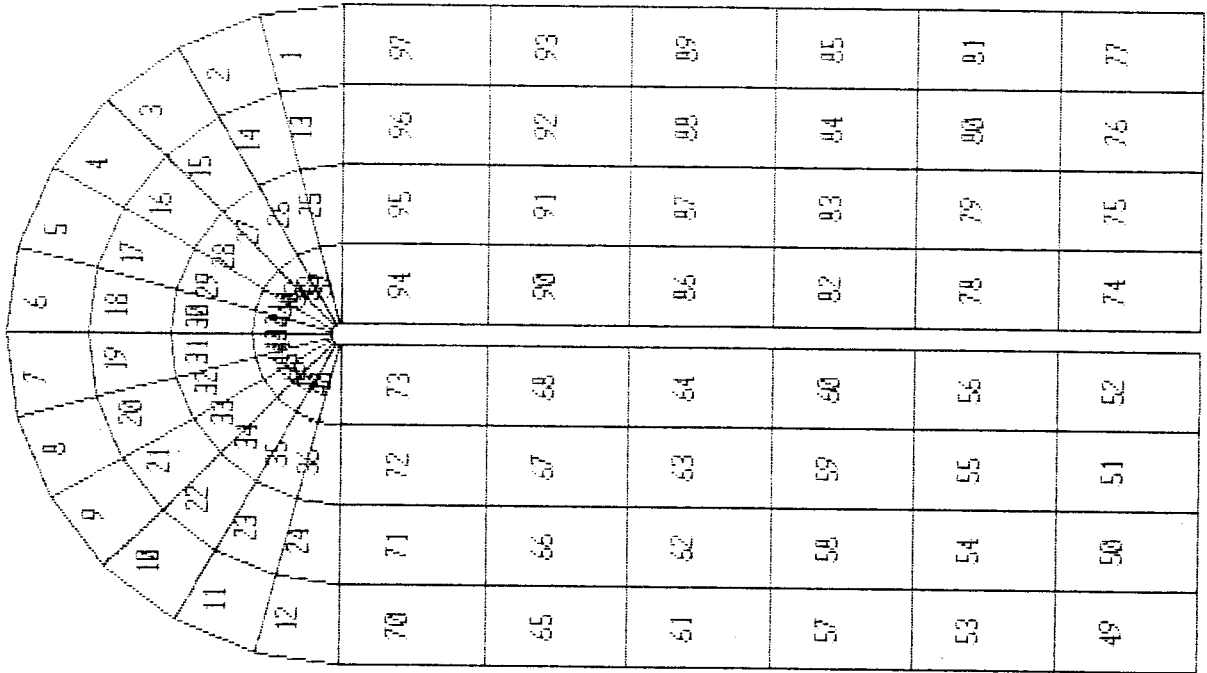
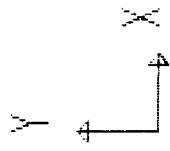


tangga I  
DEFORMED  
SHAPE  
LOAD I

MINIMA  
X .00000E+00  
Y -.1407E-02  
Z -.2200E-02  
MAXIMA  
X .2209E-02  
Y .1436E-02  
Z .00000E+00

Gambar 3.6. "Displacement" Pada Tangga I



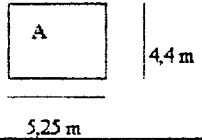
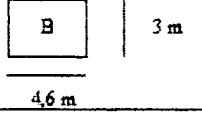
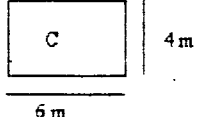
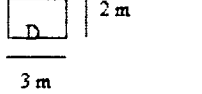
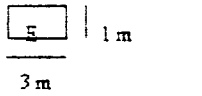
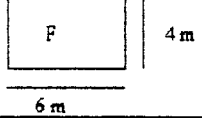
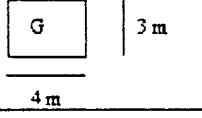
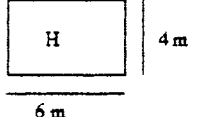
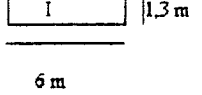
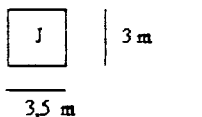
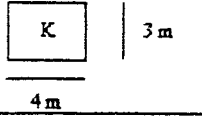


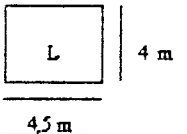
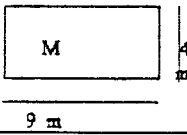
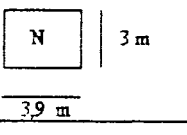
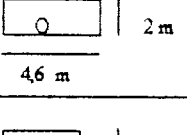
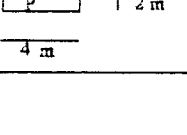
TANGGI  
UNDEFORMED  
SHAPE

OPTIONS  
ELEMENT IDS  
WIRE FRAME

Gambar 3.7. Nomor Elemen Pelat Tangga 1

Tabel 3.5 Penulangan Pelat Lantai

Bentuk Pelat	Mu/φ	As	As min	1,33.As	As Terpakai	Tul. Pokok	Mn	As Bagi	Tul Bagi
	(KN-m)	(mm <sup>2</sup> )	(mm <sup>2</sup> )	(mm <sup>2</sup> )	(mm <sup>2</sup> )		(KN-m)	(mm <sup>2</sup> )	
 A	4,4 m	11.709	602.364	524.7	—	Ø10-100	15,919	—	—
		11.709	602.364	524.7	—	Ø10-100	15,919	120,473	Ø8-200
		9.673	497.605	524.7	661.816	Ø10-100	15,919	—	—
		9.673	497.606	524.7	661.816	Ø10-100	15,919	104,94	Ø8-200
 B	3 m	6.627	340.9	524.7	453.397	Ø10-100	15,919	—	—
		6.627	340.9	524.7	453.397	Ø10-100	15,919	90,679	Ø8-200
		4.379	225.238	524.7	299.566	Ø10-100	15,919	—	—
		4.379	225.238	524.7	299.566	Ø10-100	15,919	59,913	Ø8-200
 C	4 m	10.885	559.954	524.7	—	Ø10-100	15,919	—	—
		10.885	559.954	524.7	—	Ø10-100	15,919	111,991	Ø8-200
		7.193	369.969	524.7	492.059	Ø10-100	15,919	—	—
		7.193	369.969	524.7	492.059	Ø10-100	15,919	98,412	Ø8-200
 D	2 m	2.721	179.985	408,1	239.380	Ø10-100	15,919	—	—
		2.721	179.985	408,1	239.380	Ø10-100	15,919	47,876	Ø8-200
		1.798	118,919	408,1	158,162	Ø10-100	15,919	—	—
		1.798	118,919	408,1	158,162	Ø10-100	15,919	31,832	Ø8-200
 E	1 m	0.828	42.613	524.7	56.675	Ø10-100	15,919	—	—
		0.828	42.613	524.7	56.675	Ø10-100	15,919	11,335	Ø8-200
		0.171	8.793	524.7	11.695	Ø10-100	15,919	—	—
		0.499	25.703	524.7	34.185	Ø10-100	15,919	6,837	Ø8-200
 F	4 m	13.574	698.226	524.7	—	Ø10-100	15,919	—	—
		13.574	698.226	524.7	—	Ø10-100	15,919	139,845	Ø8-200
		8.968	461.328	524.7	613.566	Ø10-100	15,919	—	—
		8.968	461.328	524.7	613.566	Ø10-100	15,919	104,94	Ø8-200
 G	3 m	6.817	350.671	524.7	466.393	Ø10-100	15,919	—	—
		6.817	350.671	524.7	466.393	Ø10-100	15,919	93,278	Ø8-200
		5.181	266.510	524.7	354.458	Ø10-100	15,919	—	—
		5.181	266.510	524.7	354.458	Ø10-100	15,919	70,892	Ø8-200
 H	4 m	11.782	606.044	524.7	—	Ø10-100	15,919	—	—
		11.782	606.044	524.7	—	Ø10-100	15,919	121,208	Ø8-200
		7.784	400.422	524.7	532.562	Ø10-100	15,919	—	—
		7.784	400.422	524.7	532.562	Ø10-100	15,919	104,94	Ø8-200
 I	1,3 m	0.835	42.949	524.7	57.123	Ø10-100	15,919	—	—
		0.835	42.949	524.7	57.123	Ø10-100	15,919	11,425	Ø8-200
		0.294	15,112	524.7	20,099	Ø10-100	15,919	—	—
		0.866	44,540	524.7	59,238	Ø10-100	15,919	11,848	Ø8-200
 J	3 m	6.272	322.618	524.7	429.081	Ø10-100	15,919	—	—
		6.272	322.618	524.7	429.081	Ø10-100	15,919	85,818	Ø8-200
		5.181	266.510	524.7	354.459	Ø10-100	15,919	—	—
		5.181	266.510	524.7	354.459	Ø10-100	15,919	70,892	Ø8-200
 K	3 m	5.467	281.227	524.7	374.031	Ø10-100	15,919	—	—
		5.467	281.227	524.7	374.031	Ø10-100	15,919	74,808	Ø8-200
		4.155	213.732	524.7	284.264	Ø10-100	15,919	—	—
		4.155	213.732	524.7	284.264	Ø10-100	15,919	58,853	Ø8-200

 4,5 m	8.836	454.533	524.7	604.529	524.7	Ø10-100	15,919	—	—
	8.836	454.533	524.7	604.529	524.7	Ø10-100	15,919	104,94	Ø8-200
	7.784	400.422	524.7	532.562	524.7	Ø10-100	15,919	—	—
	7.784	400.422	524.7	532.562	524.7	Ø10-100	15,919	104,94	Ø8-200
 9 m	13.254	681.8	524.7	—	681.8	Ø10-100	15,919	—	—
	13.254	681.8	524.7	—	681.8	Ø10-100	15,919	136,36	Ø8-200
	7.153	367.956	524.7	489.381	489.381	Ø10-100	15,919	—	—
	7.153	367.956	524.7	489.381	489.381	Ø10-100	15,919	97,876	Ø8-200
 3,9 m	6.817	350.671	524.7	466.393	466.393	Ø10-100	15,919	—	—
	6.817	350.671	524.7	466.393	466.393	Ø10-100	15,919	93,279	Ø8-200
	5.181	266.510	524.7	354.459	354.459	Ø10-100	15,919	—	—
	5.181	266.510	524.7	345.459	345.459	Ø10-100	15,919	70,891	Ø8-200
 4,6 m	3.314	170.45	524.7	226.699	226.699	Ø10-100	15,919	—	—
	3.314	170.45	524.7	226.699	226.699	Ø10-100	15,919	45,339	Ø8-200
	1.788	91.989	524.7	122.345	122.345	Ø10-100	15,919	—	—
	1.788	91.989	524.7	122.345	122.345	Ø10-100	15,919	24,489	Ø8-200
 4 m	3.013	154.987	524.7	206.133	206.133	Ø10-100	15,919	—	—
	3.013	154.987	524.7	206.133	206.133	Ø10-100	15,919	41,227	Ø8-200
	1.701	87.493	524.7	116.365	116.365	Ø10-100	15,919	—	—
	1.701	87.493	524.7	116.365	116.365	Ø10-100	15,919	23,273	Ø8-200

Tabel 3.6 Penulangan Pelat Tangga

Momen	Mu $\phi$ (kN-m)	As (mm <sup>2</sup> )	As min (mm <sup>2</sup> )	1,33As (mm <sup>2</sup> )	As Terpakai (mm <sup>2</sup> )	Tul. Pokok	Mn (kN-m)	As Bagi (mm <sup>2</sup> )	Tul Bagi	
Tangga 1	Mmax	11.0775	234.157	524.7	311.428	311.428	Ø10-250	50.680	—	
		12.4838	207.647	524.7	276.171	276.171	Ø10-250	—	55.234	Ø8-250
	Mmin	0.435	7.2355	524.7	9.623	9.623	Ø10-250	50.680	—	—
		40.7438	677.707	524.7	—	677.707	Ø10-250	—	180.27	Ø8-250
Tangga 2	Mmax	13.9363	231.8072	524.7	308.304	308.304	Ø10-250	50.680	—	—
		16.5788	275.761	524.7	366.762	366.762	Ø10-250	—	73.352	Ø8-250
	Mmin	0.1575	2.6198	524.7	3.4843	3.4843	Ø10-250	50.680	—	—
		43.9025	730.2478	524.7	—	730.2478	Ø10-250	—	194.246	Ø8-250
Tangga 3		3.5235	58.6078	584.5	77.948	77.948	Ø16-250	50.680	—	—
		2.7836	46.3002	584.5	61.579	61.579	Ø16-250	50.680	—	—
		2.7836	46.3002	584.5	61.579	61.579	Ø16-250	—	41,227	Ø8-250
		2.4348	40.4993	584.5	53.8641	53.8641	Ø16-250	50.680	—	—
		2.4348	40.4993	584.5	53.8641	53.8641	Ø16-250	—	27.135	Ø8-250
		0.4235	7.0434	584.5	9.3677	9.3677	Ø16-250	50.680	—	—

### 3.3. Perhitungan Portal Akibat Beban Gempa

#### 3.3.1. Luas Tiap Lantai

##### 1. Luas lantai 2

$$\begin{aligned}
 21 \times 60 - (12.4) - (6 \times 4.6) - (6 \times 4) - (3 \times 4) &= 1148.6 \text{ m}^2 \\
 1.3 \times 24 \times 2 - (1.3 \times 2 \times 3) &= 54.6 \text{ m}^2 \\
 1.3 \times 4 \times 7 &= 36.4 \text{ m}^2 \\
 3 \times 4 \times 6 &= 72 \text{ m}^2 \\
 2.75 \times 12 &= 33 \text{ m}^2 \\
 \text{Total} &= 1344.4 - (2.7 \times 3 \times 2) = 1328.2 \text{ m}^2
 \end{aligned}$$

##### 2. Luas lantai 3

$$\begin{aligned}
 21 \times 60 - (4.6 \times 6) - (3 \times 4) - (2 \times 12) &= 1196.4 \text{ m}^2 \\
 3 \times 4 \times 6 &= 72 \text{ m}^2 \\
 1.3 \times 26.6 \times 2 - (3 \times 1.3 \times 2) &= 61.36 \text{ m}^2 \\
 1.3 \times 4 \times 16 &= 83.2 \text{ m}^2 \\
 4.3 \times 1.3 \times 2 &= 11.18 \text{ m}^2 \\
 2.75 \times 12 &= 33 \text{ m}^2 \\
 \text{Total} &= 1457.14 - (2.7 \times 3 \times 2) = 1440.94 \text{ m}^2
 \end{aligned}$$

$$3. \text{ Lantai 3} = \text{lantai 4} = 1440.94 \text{ m}^2$$

##### 4. Lantai 5

$$\begin{aligned}
 26.6 \times 62.6 - (4.6 \times 6) - (3 \times 4) - (1.3 \times 3 \times 2) &= 1617.70 \text{ m}^2 \\
 1.45 \times 12 &= 17.4 \text{ m}^2 \\
 \text{Total} &= 1635.16 - (2.7 \times 3 \times 2) = 1618.96 \text{ m}^2
 \end{aligned}$$

##### 5. Ring Balok (25/55)

$$((60 \times 2) + (24 \times 3) + (3 \times 3) + (2 \times 2)) \times 0.25 = 51.25 \text{ m}^2$$

$$2 \times 2 \times 0.35 = 1.4 \text{ m}^2$$

$$\text{Plat} \quad 3 \times 4 = 12 \text{ m}^2$$

$$\text{Total} = 51.25 + 1.4 + 12 = 64.65 \text{ m}^2$$

### 3.3.2. Pembebanan Tiap Lantai

#### A. Berat balok atap dan rangka baja

##### 1. Beban mati

Balok atap (25/55)

$$(0.25 \times 0.55) \times (60 + 60 + 24 + 24 + 24 + 3 + 3 + 2 + 2) \times 23 = 638,825 \text{ KN}$$

$$(0,35 \times 0,60) \times (2+2) \times 23 = 19,32 \text{ KN}$$

Plat

$$3 \times 4 \times 4,766 = 57,192 \text{ m}^2$$

kolom

$$60/70 = (0.6 \times 0.7 \times 1/2 \times 4.5 \times 30 \times 23) = 652.05 \text{ KN}$$

$$40/70 (0.4 \times 0.7 \times 1/2 \times 4.5 \times 6 \times 23) = 173.88 \text{ KN}$$

$$\text{Kaca} \Rightarrow ((27 \times 4 \times 4.5) + (6 \times 6 \times 4.5)) \times 0.1 \times 1/2 = 32.4 \text{ KN}$$

$$\text{Dinding } \frac{1}{2} \text{ batu} \Rightarrow (69.35 \times 4.5 \times 2.5) \times 1/2 = 390.094 \text{ KN}$$

$$\text{Beban rangka atap} = 215,114 \text{ KN}$$

$$\text{Total wmati} = 2151,875 \text{ KN}$$

##### 2. Beban hidup

$$q_h = 1 \text{ KN/m}^2 \text{ koefred} = 1$$

$$w_h = 1 \times 1 \times 58,65 = 64,65 \text{ KN}$$

$$w_{\text{atap}} = w_{\text{mati}} + w_{\text{hidup}} = 2216,525 \text{ KN}$$

#### B. Lantai 5

##### 1. Beban mati

$$\text{Pelat} = 1618.96 \times 4.766 = 7715.9634 \text{ KN}$$

Balok lantai :

$$25/55 = 0.25 \times 0.55 \times 27 \times 4 \times 23 = 341.55 \text{ KN}$$

$$\begin{aligned}
25/45 &= 0.25 \times 0.45 \times 39 \times 4 \times 23 &= 403.65 \text{ KN} \\
30/65 &= 0.3 \times 0.65 \times 4 \times 6 \times 23 &= 107.64 \text{ KN} \\
30/95 &= 0.3 \times 0.95 \times 24 \times 6 \times 23 &= 943.92 \text{ KN} \\
30/70 &= 0.3 \times 0.7 \times 20 \times 23 &= 96.6 \text{ KN} \\
30 \times 55 &= 0.3 \times 0.55 \times 12 \times 3 \times 23 &= 124.2 \text{ KN} \\
30 \times 75 &= 0.3 \times 0.75 \times 45 \times 23 &= 232.875 \text{ KN} \\
30/80 &= 0.3 \times 0.8 \times 12 \times 23 &= 66.24 \text{ KN} \\
20/25 &= 0.2 \times 0.25 \times 19.2 \times 23 &= 22.08 \text{ KN} \\
20/40 &= 0.2 \times 0.4 \times 4 \times 23 &= 7.36 \text{ KN} \\
30 \times 90 &= 0.3 \times 0.9 \times 20 \times 23 &= 124.2 \text{ KN} \\
25/85 &= 0.25 \times 0.85 \times 12 \times 23 &= 58.65 \text{ KN} \\
0.25 \times 0.6 \times 12 \times 23 & &= 41.4 \text{ KN} \\
0.4 \times 1 \times 9 \times 23 & &= 82.8 \text{ KN} \\
0.2 \times 0.6 \times 6 \times 23 & &= 16.56 \text{ KN} \\
0.25 \times 0.8 \times 24 \times 23 & &= 110.4 \text{ KN} \\
0.3 \times 0.5 \times 9 \times 23 & &= 31.05 \text{ KN} \\
0.3 \times 0.5 \times 14 \times 23 & &= 48.3 \text{ KN} \\
0.3 \times 0.45 \times 3.5 \times 23 & &= 10.867 \text{ KN} \\
1.3 \times 42 \times 0.1 \times 0.7 \times 23 & &= 87.906 \text{ KN} \\
6 \times 0.5 \times 0.2 \times 23 & &= 13.8 \text{ KN}
\end{aligned}$$

#### Kolom

$$\begin{aligned}
60/70 &\Rightarrow 0.6 \times 0.7 \times 30 \times 4.5 \times 23 &= 1304.1 \text{ KN} \\
10/70 &= 0.4 \times 0.7 \times 8 \times 1.5 \times 23 &= 231.84 \text{ KN} \\
40/40 &\Rightarrow 0.4 \times 0.4 \times 5 \times 4.5 \times 23 &= 82.8 \text{ KN} \\
40/70 &\Rightarrow 0.4 \times 0.7 \times 7 \times 1/2 \times 4.5 \times 23 &= 101.43 \text{ KN} \\
60/70 &\Rightarrow 0.6 \times 0.7 \times 18 \times 4.5 \times 23 \times 1/2 &= 391.23 \text{ KN} \\
\text{Kaca} &\Rightarrow ((27 \times 4 \times 4.5) + (6 \times 6 \times 4.5)) \times 0.1 &= 64.8 \text{ KN} \\
\text{Dinding } \frac{1}{2} \text{ batu} &\Rightarrow (69.35 \times 4.5 \times 2.5) &= 780.188 \text{ KN}
\end{aligned}$$

$$\text{Total beban mati} = 13713.3994 \text{ KN}$$

## 2. Beban hidup

$$Q_h = 3 \text{ KN/m}^2 \text{ koef. Red.} = 1$$

$$W_h = 1 \times 3 \times 1618.96 = 4856.88$$

$$W \text{ lantai 5} = 4856.88 + 13713.3994 = 18570.2794 \text{ KN}$$

## C. Lantai 4

## 1. Beban mati

## Material

$$26 \quad 1.3 \times 0.1 \times 0.7 \times 23 \times 38 = 79.543 \text{ KN}$$

$$6. \quad = 945.92 \text{ KN}$$

$$7. \quad = 403.65 \text{ KN}$$

$$9. \quad = 27 \times 0.25 \times 0.6 \times 23 = 93.15 \text{ KN}$$

$$16. \quad = 110.4 \text{ KN}$$

$$8. \quad = 124.2 \text{ KN}$$

$$17. \quad = 58.56 \text{ KN}$$

$$5. \quad = 107.64 \text{ KN}$$

$$18. \quad = 22.08 \text{ KN}$$

$$11. \quad = 232.875 \text{ KN}$$

$$12. \quad = 66.24 \text{ KN}$$

$$13. \quad = 7.36 \text{ KN}$$

$$25. \quad = 10.867 \text{ KN}$$

$$14. \quad = 124.2 \text{ KN}$$

$$10. \quad = 96.6 \text{ KN}$$

$$19. \quad = 41.4 \text{ KN}$$

$$20. \quad = 13.8 \text{ KN}$$

$$21. \quad = 16.56 \text{ KN}$$

## Kolom

$$60/70 \Rightarrow 0.6 \times 0.7 \times 48 \times 4.5 \times 23 = 2086.56 \text{ KN}$$

$$40/70 \Rightarrow 0.4 \times 0.7 \times 15 \times 4.5 \times 23 = 434.7 \text{ KN}$$

$$\begin{aligned}
 40/40 &\Rightarrow 0.4 \times 0.4 \times 5 \times 4.5 \times 23 &= & 82.8 \text{ KN} \\
 \text{Kaca} &= (27 \times 4 \times 4.5 \times 0.1) + (8 \times 4 \times 4.5 \times 0.1) &= & 70.2 \text{ KN} \\
 \text{Dinding} &&= & 780.188 \text{ KN} \\
 \text{Total} &&= & 12875.094 \text{ KN}
 \end{aligned}$$

## 2. Beban Hidup

$$q_h = 3 \text{ KN/M}^2 \quad \text{koef.red} = 1$$

$$w_h = 1 \times 3 \times 1440.94 = 3890.538$$

$$w_{\text{lantai 4}} = 3890.538 + 12875.094 = 16765.632 \text{ KN}$$

## D. Lantai 3

### 1. Beban mati

$$\text{Pelat} = 6867.52 \text{ KN}$$

#### Material

$$\begin{aligned}
 15 &= 26 \times 0.3 \times 0.45 \times 23 = 80.73 \text{ KN} \\
 14 &= 0.3 \times 0.55 \times 8 \times 3 \times 23 = 91.08 \text{ KN} \\
 26 &= 1.3 \times 0.1 \times 0.7 \times 23 \times 38 = 79.543 \text{ KN} \\
 6. &= 945.92 \text{ KN} \\
 7. &= 403.65 \text{ KN} \\
 9. &= 27 \times 0.25 \times 0.6 \times 23 = 93.15 \text{ KN} \\
 16. &= 110.4 \text{ KN} \\
 8. &= 124.2 \text{ KN} \\
 17. &= 58.56 \text{ KN} \\
 5. &= 107.64 \text{ KN} \\
 18. &= 22.08 \text{ KN} \\
 11. &= 232.875 \text{ KN} \\
 12. &= 66.24 \text{ KN} \\
 13. &= 7.36 \text{ KN} \\
 25. &= 10.867 \text{ KN} \\
 14. &= 124.2 \text{ KN}
 \end{aligned}$$



10. = 96.6 KN  
 19. = 41.4 KN  
 20. = 13.8 KN  
 21. = 16.56 KN  
 22 =  $0.3 \times 0.4 \times 4.5 \times 2 \times 23 = 24.84$  KN

#### Kolom

- 60/70  $\Rightarrow 0.6 \times 0.7 \times 48 \times 4.5 \times 23 = 2086.56$  KN  
 40/70  $\Rightarrow 0.4 \times 0.7 \times 15 \times 4.5 \times 23 = 434.7$  KN  
 40/40  $\Rightarrow 0.4 \times 0.4 \times 5 \times 4.5 \times 23 = 82.8$  KN  
 Kaca =  $(27 \times 4 \times 4.5 \times 0.1) + (8 \times 4 \times 4.5 \times 0.1) = 70.2$  KN

Dinding = 780.188 KN

Total beban mati = 12875.094 KN

#### 2. Beban Hidup

$$Q_h = 3 \text{ KN/m}^2 \text{ koef.red} = 1$$

$$W_h = 1 \times 3 \times 1440.94 = 3458.256 \text{ KN}$$

$$W_{\text{lantai 3}} = 3458.256 + 12875.094 = 16333.35 \text{ KN}$$

#### E. Lantai 2

##### 1. Beban mati

$$\text{Pelat} = 1328.2 \times 4.766 = 6330.2 \text{ KN}$$

##### Balok

- 26  $1.3 \times 0.1 \times 0.7 \times 23 \times 22 = 46.046$  KN  
 6. = 945.92 KN  
 7. = 403.65 KN  
 9. = 341.55 KN  
 16. = 110.4 KN  
 8. = 124.2 KN  
 17. = 58.56 KN

5. = 107.64 KN  
 18. = 22.08 KN  
 11. =  $0.3 \times 0.75 \times 6 \times 6 \times 23 = 186.3$  KN  
 12. = 66.24 KN  
 13. = 7.36 KN  
 25. = 10.867 KN  
 14. = 124.2 KN  
 10. = 96.6 KN  
 19. = 41.4 KN  
 20. = 13.8 KN  
 21. = 16.56 KN

#### Kolom

$$60/70 \Rightarrow 0.6 \times 0.7 \times 48 \times 5.875 \times 23 = 2724.12 \text{ KN}$$

$$40/70 \Rightarrow 0.4 \times 0.7 \times 15 \times 5.875 \times 23 = 567.525 \text{ KN}$$

$$40/40 \Rightarrow 0.4 \times 0.4 \times 5 \times 5.875 \times 23 = 108.1 \text{ KN}$$

$$\text{Kaca} = (27 \times 4 \times 5.875 \times 0.1) + (8 \times 4 \times 5.875 \times 0.1) = 82.25 \text{ KN}$$

$$\text{Dinding } \frac{1}{2} \text{ batu} \Rightarrow (69.35 \times 5.875 \times 2.5) = 1018.578 \text{ KN}$$

$$\text{Total beban mati} = 13903.836 \text{ KN}$$

#### 2. Beban hidup

$$Q_h = 3 \text{ KN/m}^2 \text{ koefred} = 1$$

$$W_h = 1 \times 3 \times 1328.2 = 2789.22 \text{ KN}$$

$$W_{\text{lantai 2}} = 2789.22 + 13903.836 = 16693.056 \text{ KN}$$

#### Jumlah total semua lantai ( Wt)

$$= 2216.525 \text{ KN} + 18570.2794 \text{ KN} + 16765.632 \text{ KN} + 16333.35 \text{ KN} +$$

$$16693.056 \text{ KN} = 70934.0414 \text{ KN}$$

### 3.3.3. Perhitungan gaya horizontal pada tiap lantai

a.  $T = 0.06 H^{3/4}$

Dimana H = Tinggi bangunan gedung diukur dari penjepitan

$$H = 7.25 + (4 \times 4.5) = 25.25 \text{ m}$$

$$T_x = T_y = 0,06 ( 25.25 )^{3/4} = 0.6758 \text{ dt}$$

b. Koefisien gempa geser ( c )

Daerah yogyakarta termasuk wilayah III, tanah keras  $T_x = T_y = 0.6758 \text{ dt}$

( dari gambar 2-5, buku Gideon), koefisien gempa dasar diperoleh  $C_x = C_y = 0.04$

c. Faktor kekakuan dari tabel 1 PPTGIUG 1983 didapat  $I = 1.5$

d. Faktor jenis kontruksi (K) untuk bangunan gedung bertulang  $K=1$

e. Gaya geser dasar horisontal total akibat gempa ( V )

$$\begin{aligned} V_x = V_y &= C.L.K.Wt \\ &= 0.04 \times 1.5 \times 1 \times 70934.0414 \text{ KN} \\ &= 4256.042 \text{ KN} \end{aligned}$$

f. Pembagian gaya geser dasar horisontal total akibat gempa ke sepanjang tinggi gedung

$$\text{Arah x } H/A = 25.25/60 = 0.421 < 3$$

$$\text{Arah y } H/B = 25.25/24 = 1.052 < 3$$

$H/A \ \& \ H/B < 3$ , maka dipakai rumus :

$$F_i = \frac{W_i \cdot h_i}{\sum W_i \cdot h_i} \cdot V$$

Dimana :

$F_i$  = beban horisontal terpusat pada lantai i

$W_i$  = berat lantai i

$H_i$  = tinggi lantai i

$V$  = gaya geser dasar horisontal akibat gempa

Lantai	$H_i$ (m)	$W_i$ (KN)	$W_i.H_i$ (KN)	$F_i$ (KN)
Atap	25.25	2216.525	64936.031	266.856
5	20.75	18570.2794	385333.298	1583.538
4	16.25	16765.632	272441.52	1119.606
3	11.75	16333.35	191916.863	788.688
2	7.25	16693.056	121024.656	497.354
			$\Sigma W_i.H_i = 1035652.368$	

### 3.3.4. Beban Gempa Pada Tiap Lantai

Beban Gempa Pada balok atap

Gempa arah - x

$$\text{Joint 693} = (4.5/24) \times 266.856 = 50.036 \text{ KN}$$

$$\text{Joint 730} = ((1.5+4.5)/24) \times 266.856 = 66.714 \text{ KN}$$

$$\text{Joint 729} = ((1.5+3)/24) \times 266.856 = 50.036 \text{ KN}$$

$$\text{Joint 728} = (6/24) \times 266.856 = 66.714 \text{ KN}$$

$$\text{Joint 727} = (3/24) \times 266.856 = 33.357 \text{ KN}$$

Gempa arah - y

$$\text{Joint 693} \rightarrow 708 = (2/60) \times 266.856 = 8.895 \text{ KN}$$

$$\text{Joint 694} \rightarrow 707 = (4/60) \times 266.856 = 17.790 \text{ KN}$$

Beban gempa pada lantai 5

Gempa arah – x

$$\text{Joint 100} = (4.5/24) \times 1583.538 = 296.913 \text{ KN}$$

$$\text{Joint 285} = ((1.5+4.5)/24) \times 1583.538 \text{ KN} = 395.885 \text{ KN}$$

$$\text{Joint 327} = ((1.5+3)/24) \times 1583.538 = 296.913 \text{ KN}$$

$$\text{Joint 469} = (6/24) \times 1583.538 = 395.885 \text{ KN}$$

$$\text{Joint 582} = (3/24) \times 1583.538 = 197.942 \text{ KN}$$

Arah – y

$$\text{Joint 100} = 115 = (2/60) \times 1583.538 = 52.785 \text{ KN}$$

$$\text{Joint 101} \rightarrow 114 = (4/60) \times 1583.538 = 105.569 \text{ KN}$$

Beban gempa lantai 4

Arah – x

$$\text{Joint 68} = (4.5/24) \times 1119.606 = 209.926 \text{ KN}$$

$$\text{Joint 267} = ((1.5+4.5)/24) \times 1119.606 = 279.902 \text{ KN}$$

$$\text{Joint 354} = ((1.5+3)/24) \times 1119.606 = 209.926 \text{ KN}$$

$$\text{Joint 453} = (6/24) \times 1119.606 = 279.902 \text{ KN}$$

$$\text{Joint 563} = (3/24) \times 1119.606 = 139.951 \text{ KN}$$

Arah – y

$$\text{Joint 68} = 81 = (2/60) \times 1119.606 = 37.32 \text{ KN}$$

$$\text{Joint 69} \rightarrow 71 = 73 \rightarrow 74 = 82 = 222 = 223 = 183 = 75 = 76 = 78 \rightarrow 80 = (4/60) \times 1119.606 =$$

$$74.64 \text{ KN}$$

## Beban gempa lantai 3

Arah - x

$$\text{Joint 40} = (4.5/24) \times 788.688 = 147.879 \text{ KN}$$

$$\text{Joint 250} = ((1.5+4.5)/24) \times 788.688 = 197.172 \text{ KN}$$

$$\text{Joint 336} = ((1.5+3)/24) \times 788.688 = 147.879 \text{ KN}$$

$$\text{Joint 437} = (6/24) \times 788.688 = 197.172 \text{ KN}$$

$$\text{Joint 544} = (3/24) \times 788.688 = 98.586 \text{ KN}$$

Arah - y

$$\text{Joint 40} = 57 = (2/60) \times 788.688 = 26.290 \text{ KN}$$

$$\text{Joint 41} = 43 = 45 \rightarrow 52 = 54 \rightarrow 56 = (4/60) \times 788.688 \text{ KN}$$

## Beban gempa lantai 2

Arah - x

$$\text{Joint 18} = (4.5/24) \times 497.354 = 93.254 \text{ KN}$$

$$\text{Joint 232} = ((1.5+4.5)/24) \times 497.354 = 124.339 \text{ KN}$$

$$\text{Joint 318} = ((1.5+3)/24) \times 497.354 = 93.254 \text{ KN}$$

$$\text{Joint 421} = ((3+3)/24) \times 497.354 = 124.339 \text{ KN}$$

$$\text{Joint 525} = (3/24) \times 497.354 = 62.169 \text{ KN}$$

Arah - y

$$\text{Joint 18} = 29 = (2/60) \times 497.354 = 16.579 \text{ KN}$$

$$\text{Joint 19} \rightarrow 21 = 138 \rightarrow 139 = 22 \rightarrow 25 = 144 \rightarrow 145 = 26 \rightarrow 28 = (4/60) \times 497.354$$

$$= 33.157 \text{ KN}$$

## 3.3.5. HITUNGAN STRUKTUR TIGA DIMENSI

HITUNGAN STRUKTUR TIGA DIMENSI - UNITS: KN - M  
SYSTEM

L=6

JOINTS

1	X=0	Y=0	Z=0		: PORTAL 1
16	X=60	Y=0	Z=0	G=1,16,1	
17	X=-1.3	Y=0	Z=7.25		
18	X=0	Y=0	Z=7.25		
21	X=12	Y=0	Z=7.25	G=18.21,1	
22	X=24	Y=0	Z=7.25		
25	X=36	Y=0	Z=7.25	G=22,25,1	
26	X=48	Y=0	Z=7.25		
29	X=60	Y=0	Z=7.25	G=26,29,1	
30	X=61.3	Y=0	Z=7.25		
31	X=0	Y=-1.3	Z=11.75		
34	X=12	Y=-1.3	Z=11.75	G=31,34,1	
35	X=48	Y=-1.3	Z=11.75		
38	X=60	Y=-1.3	Z=11.75	G=35,38,1	
39	X=-1.3	Y=0	Z=11.75		
40	X=0	Y=0	Z=11.75		
43	X=12	Y=0	Z=11.75	G=40,43,1	
44	X=13.3	Y=0	Z=11.75		
45	X=16	Y=0	Z=11.75		
52	X=44	Y=0	Z=11.75	G=45,52,1	
53	X=46.7	Y=0	Z=11.75		
54	X=48	Y=0	Z=11.75		
57	X=60	Y=0	Z=11.75	G=54,57,1	
58	X=61.3	Y=0	Z=11.75		
59	X=0	Y=-1.3	Z=16.25		
62	X=12	Y=-1.3	Z=16.25	G=59,62,1	
63	X=48	Y=-1.3	Z=16.25		
66	X=60	Y=-1.3	Z=16.25	G=63,66,1	
67	X=-1.3	Y=0	Z=16.25		
68	X=0	Y=0	Z=16.25		
71	X=12	Y=0	Z=16.25	G=68,71,1	
72	X=13.5	Y=0	Z=16.25		
73	X=16	Y=0	Z=16.25		
74	X=20	Y=0	Z=16.25		
75	X=40	Y=0	Z=16.25		
76	X=44	Y=0	Z=16.25		
77	X=46.7	Y=0	Z=16.25		
78	X=48	Y=0	Z=16.25		
81	X=60	Y=0	Z=16.25	G=78,81,1	
82	X=61.3	Y=0	Z=16.25		
83	X=0	Y=-2	Z=20.75		
98	X=60	Y=-2	Z=20.75	G=83,98,1	
99	X=-2	Y=0	Z=20.75		
100	X=0	Y=0	Z=20.75		
115	X=60	Y=0	Z=20.75	G=100,115,1	
116	X=62	Y=0	Z=20.75		
117	X=12	Y=3	Z=0		: PORTAL 2
120	X=24	Y=3	Z=0	G=117,120,1	
121	X=36	Y=3	Z=0		
124	X=48	Y=3	Z=0	G=121,124,1	
125	X=16	Y=1.7	Z=7.25		

126	X=20			G=125,126,1
131	X=40			
132	X=44	Y=1.7	Z=7.25	G=131,132,1
133	X=-1.3	Y=3	Z=7.25	
134	X=0	Y=3	Z=7.25	
149	X=60	Y=3	Z=7.25	G=134,149,1
150	X=61.3	Y=3	Z=7.25	
155	X=-1.3	Y=3	Z=11.75	
156	X=0	Y=3	Z=11.75	
162	X=24	Y=3	Z=11.75	G=162,163,1
163	X=36	Y=3	Z=11.75	
164	X=60	Y=3	Z=11.75	G=163,169,1
165	X=61.3	Y=3	Z=11.75	
175	X=-1.3	Y=3	Z=16.25	
176	X=0	Y=3	Z=16.25	
182	X=24	Y=3	Z=16.25	G=176,182,1
183	X=36	Y=3	Z=16.25	
189	X=60	Y=3	Z=16.25	G=183,189,1
190	X=61.3	Y=3	Z=16.25	
193	X=-2	Y=3	Z=20.75	
194	X=0	Y=3	Z=20.75	
200	X=24	Y=3	Z=20.75	G=194,200,1
203	X=36	Y=3	Z=20.75	
209	X=60	Y=3	Z=20.75	G=203,209,1
210	X=62	Y=3	Z=20.75	
211	X=28	Y=5	Z=0	
212	X=32	Y=5	Z=0	
213	X=24	Y=5	Z=7.25	
216	X=36	Y=5	Z=7.25	G=213,216,1
217	X=24	Y=5	Z=11.75	
220	X=36	Y=5	Z=11.75	G=217,220,1
221	X=24	Y=5	Z=16.25	
224	X=36	Y=5	Z=16.25	G=221,224,1
225	X=24	Y=5	Z=20.75	
228	X=36	Y=5	Z=20.75	G=225,228,1
229	X=0	Y=9	Z=0	
230	X=60	Y=9	Z=0	
231	X=-1.3	Y=9	Z=7.25	
232	X=0	Y=9	Z=7.25	
233	X=2.7	Y=9	Z=7.25	
233	X=4	Y=9	Z=7.25	
238	X=24	Y=9	Z=7.25	G=233,238,1
239	X=28.6			
240	X=33			
241	X=36	Y=9	Z=7.25	
246	X=56	Y=9	Z=7.25	G=241,246,1
646	X=57.3	Y=9	Z=7.25	
247	X=60	Y=9	Z=7.25	
248	X=61.3	Y=9	Z=7.25	
249	X=-1.3	Y=9	Z=11.75	
250	X=0	Y=9	Z=11.75	
648	X=2.7	Y=9	Z=11.75	
251	X=4	Y=9	Z=11.75	
257	X=28	Y=9	Z=11.75	G=251,257,1
618	X=28.6			
619	X=32	Y=9	Z=11.75	
258	X=33			

: PORTAL 3



259	X=36	Y=9	Z=11.75	
264	X=56	Y=9	Z=11.75	G=259,264,1
650	X=57.3	Y=9	Z=11.75	
265	X=60			
660	X=61.3			
266	X=-1.3		Z=16.25	
267	X=0			
652	X=2.7			
268	X=4			
274	X=28			G=268,274,1
620	X=28.6			
621	X=32			
275	X=33			
276	X=36			
281	X=56			G=276,281,1
654	X=57.3			
282	X=60			
283	X=61.3			
284	X=-2	Z=20.75		
285	X=0			
656	X=2.7			
286	X=4			
292	X=28			G=286,292,1
622	X=28.6			
623	X=32			
293	X=33			
294	X=36			
299	X=56			G=294,299,1
658	X=57.3			: PORTAL 4
300	X=60			
301	X=62			
302	X=0	Y=12	Z=0	: PORTAL 5
308	X=24			G=302,308,1
309	X=33			
310	X=36			
316	X=60			G=310,316,1
317	X=-1.3		Z=7.25	
318	X=0			
644	X=2.7			
319	X=4			
324	X=24			G=319,324,1
325	X=28.6			
326	X=33			
327	X=36			
332	X=56			G=327,332,1
647	X=57.3			
333	X=60			
334	X=61.3			
335	X=-1.3		Z=11.75	
336	X=0			
649	X=2.7			
337	X=4			
342	X=24			G=337,342,1
343	X=28.6			
344	X=33			
345	X=36			
350	X=56			G=345,350,1

651	X=57.3			
351	X=60			
352	X=61.3			
353	X=-1.3		Z=16.25	
354	X=0			
653	X=2.7			
355	X=4			
360	X=24			G=355,360,1
361	X=28.6			
362	X=33			
363	X=36			
368	X=56			G=363,368,1
655	X=57.3			
369	X=60			
370	X=61.3			
371	X=-2		Z=20.75	
372	X=0			
657	X=2.7			
373	X=4			
378	X=24			G=373,378,1
379	X=28.6			
380	X=33			
381	X=36			
386	X=56			G=381,386,1
659	X=57.3			
387	X=60			
388	X=62			
389	X=24	Y=14	Z=0	: PORTAL 6
390	X=33			
391	X=36			
392	X=24		Z=7.25	
393	X=28.6			
394	X=33			
395	X=36			
396	X=24		Z=11.75	
397	X=28.6			
398	X=33			
399	X=36			
400	X=24		Z=16.25	
401	X=28.6			
402	X=33			
403	X=36			
404	X=24		Z=20.75	
405	X=28.6			
406	X=33			
407	X=36			
408	X=33	Y=16	Z=0	: PORTAL 7
409	X=36			
410	X=33		Z=7.25	
411	X=36			
412	X=33		Z=11.75	
413	X=36			
414	X=33		Z=16.25	
415	X=36			
416	X=33		Z=20.75	
417	X=36			
747	X=24	Y=16	Z=0	

748		Z=7.25	
749		Z=11.75	
750		Z=16.25	
751		Z=20.75	
418 X=0	Y=18	Z=0	: PORTAL 8
419 X=60			
420 X=-1.3		Z=7.25	
421 X=0			
427 X=24			G=421,427,1
428 X=36			
434 X=60			G=428,434,1
435 X=61.3			
436 X=-1.3		Z=11.75	
437 X=0			
443 X=24			G=437,443,1
444 X=36			
450 X=60			G=444,450,1
451 X=61.3			
452 X=-1.3		Z=16.25	
453 X=0			
459 X=24			G=453,459,1
460 X=36			
466 X=60			G=460,466,1
467 X=61.3			
468 X=-2		Z=20.75	
469 X=0			
475 X=24			G=469,475,1
476 X=36			
482 X=60			G=476,482,1
483 X=62			
484 X=24	Y=20	Z=0	: PORTAL 9
485 X=36			
486 X=24		Z=7.25	
487 X=27			
488 X=28.6			
489 X=30			
490 X=33			
491 X=36			
492 X=24		Z=11.75	
493 X=27			
494 X=28.6			
495 X=30			
496 X=33			
497 X=36			
498 X=24		Z=16.25	
499 X=27			
500 X=28.6			
501 X=30			
502 X=33			
503 X=36			
504 X=24		Z=20.75	
505 X=27			
506 X=28.6			
507 X=30			
508 X=33			
509 X=36			
510 X=0	Y=24	Z=0	: PORTAL 10

516	X=24			G=510,516,1
517	X=36			
523	X=60			G=517,523,1
524	X=-1.3		Z=7.25	
525	X=0			
531	X=24			G=525,531,1
532	X=27			
535	X=36			G=532,535,1
536	X=40			
541	X=60			G=536,541,1
542	X=61.3			
191	X=16	Y=25.3		
192	X=20			
201	X=40			
202	X=44			
543	X=-1.3	Y=24	Z=11.75	
544	X=0			
550	X=24			G=544,550,1
551	X=27			
554	X=36			G=551,554,1
555	X=40			
560	X=60			G=555,560,1
561	X=61.3			
151	X=0	Y=25.3		
154	X=12			G=151,154,1
171	X=16			
172	X=20			
600	X=40			
605	X=60			G=600,605,1
562	X=-1.3	Y=24	Z=16.25	
563	X=0			
569	X=24			G=563,569,1
570	X=27			
573	X=36			G=570,573,1
574	X=40			
579	X=60			G=574,579,1
580	X=61.3			
606	X=0	Y=25.3		
611	X=20			G=606,611,1
612	X=40			
617	X=60			G=612,617,1
581	X=-2	Y=24	Z=20.75	
582	X=0			
588	X=24			G=582,588,1
589	X=27			
592	X=36			G=589,592,1
593	X=40			
598	X=60			G=593,598,1
599	X=62			
173	X=0	Y=26		
174	X=4			
683	X=8			
686	X=20			G=683,686,1
687	X=40			
692	X=60			G=687,692,1
661	X=24	Y=25.75	Z=0	
662	X=36			

: PORTAL 11

663	X=24		Z=7.25		
667	X=36			G=663,667,1	
668	X=24		Z=11.75		
672	X=36			G=668,672,1	
673	X=24		Z=16.25		
677	X=36			G=673,677,1	
678	X=24		Z=20.75		
682	X=36			G=678,682,1	
624	X=24	Y=21.6	Z=7.25		: PORTAL 12
628	X=36			G=624,628,1	
629	X=24		Z=11.75		
633	X=36			G=629,633,1	
634	X=24		Z=16.25		
638	X=36			G=634,638,1	
639	X=24		Z=20.75		
643	X=36			G=639,643,1	
693	X=0	Y=0	Z=25.25		: RING BALOK ATAP
708	X=60			G=693,708,1	
709		Y=9			
710		Y=12			
711		Y=18			
712		Y=24			
727	X=0			G=712,727,1	
728		Y=18			
729		Y=12			
730		Y=9			
731	X=24	Y=26.75	Z=7.25		
732	X=36				
733	X=24		Z=11.75		
734	X=36				
735	X=24		Z=16.25		
736	X=36				
737	X=24		Z=20.75		
738	X=36				
739	X=60	Y=4	Z=25.25		
740		Y=8			
741		Y=16			
742		Y=20			
743	X=0	Y=20			
744		Y=16			
745		Y=8			
746		Y=4			
C JOINT PADA LIFT					
851	X=33	Y=12	Z=25.25		
853	X=36			G=851,853,1	
854	X=33	Y=14	Z=25.25		
856	X=36			G=854,856,1	
857	X=33	Y=16	Z=25.25		
859	X=36			G=857,859,1	
C JOINT BALOK BORDES					
835	X=0	Y=9	Z=5		
836		Y=12			
837		Y=9	Z=9.5		
838		Y=12			
839		Y=9	Z=14		
840		Y=12			
841		Y=9	Z=18.5		

842		Y=12		
843	X=60	Y=9	Z=5	
844		Y=12		
845		Y=9	Z=9.5	
846		Y=12		
847		Y=9	Z=14	
848		Y=12		
849		Y=9	Z=18.5	
850		Y=12		
752	X=0	Y=0	Z=2.67	: JOINT BALOK SLOOF
767	X=60	Y=0	Z=2.67	G=752,767,1
768	X=12	Y=3		
777	X=48			G=768,777,1
778	X=28	Y=5		
779	X=32			
780	X=0	Y=9		
781	X=60			
782	X=0	Y=12		
783	X=4			
788	X=24			G=783,788,1
789	X=33			
790	X=36			
795	X=56			G=790,795,1
796	X=60			
797	X=24	Y=14		
798	X=33			
799	X=36			
800	X=24	Y=16		
801	X=33			
802	X=36			
803	X=0	Y=18		
804	X=60			
805	X=24	Y=20		
806	X=36			
807	X=0	Y=24		
813	X=24			G=807,813,1
814	X=36			
820	X=60			G=814,820,1
821	X=24	Y=25.75		
822	X=36			
823	X=2.7	Y=9	Z=0	: BALOK SLOOF TANGGA
824	X=12			
825	X=16			
826	X=57.3			
827	X=2.7		Z=2.67	
828	X=12			
829	X=16			
830	X=57.3			
831	X=2.7	Y=12	Z=0	
832	X=2.7		Z=2.67	
833	X=57.3	Y=12	Z=0	
834	X=57.3		Z=2.67	

:

## RESTRAINTS

1	16	1	R=1,1,1,0,0,1
117	120	1	R=1,1,1,0,0,1
121	124	1	R=1,1,1,0,0,1

211	212	1	R=1,1,1,0,0,1
229	230	1	R=1,1,1,0,0,1
302	308	1	R=1,1,1,0,0,1
309	316	1	R=1,1,1,0,0,1
389			R=1,1,1,0,0,1
390	391	1	R=1,1,1,0,0,1
408	409	1	R=1,1,1,0,0,1
418	419	1	R=1,1,1,0,0,1
510	516	1	R=1,1,1,0,0,1
517	523	1	R=1,1,1,0,0,1
484	485	1	R=1,1,1,0,0,1
661	662	1	R=1,1,1,0,0,1
747			R=1,1,1,0,0,1
823	826	1	R=1,1,1,0,0,1
831			R=1,1,1,0,0,1
833			R=1,1,1,0,0,1

:

## FRAME

## C CONTROL DATA

NM=29 NSEC=5 NL=108 Z=-1

## C MATERIAL PROPERTIES

1	SH=R	T=0.7,0.6	E=2.1E7	W=0.7*0.6*23	: KOLOM
2	SH=R	T=0.7,0.5	E=2.1E7	W=0.7*0.5*23	: KOLOM
3	SH=R	T=0.5,0.5	E=2.1E7	W=0.5*0.5*23	: KOLOM
4	SH=R	T=1.0,0.4	E=2.1E7	W=1.0*0.4*23	: BALOK
5	SH=R	T=0.65,0.3	E=2.1E7	W=0.65*0.3*23	: BALOK
6	SH=R	T=0.95,0.3	E=2.1E7	W=0.95*0.3*23	: BALOK
7	SH=R	T=0.50,0.30	E=2.1E7	W=0.45*0.25*23	: BALOK
8	SH=R	T=0.9,0.3	E=2.1E7	W=0.9*0.3*23	: BALOK
9	SH=R	T=0.6,0.3	E=2.1E7	W=0.6*0.3*23	: BALOK
10	SH=R	T=0.7,0.3	E=2.1E7	W=0.7*0.3*23	: BALOK
11	SH=R	T=0.75,0.3	E=2.1E7	W=0.75*0.3*23	: BALOK
12	SH=R	T=0.8,0.3	E=2.1E7	W=0.8*0.3*23	: BALOK
13	SH=R	T=0.4,0.2	E=2.1E7	W=0.4*0.2*23	: BALOK
14	SH=R	T=0.55,0.3	E=2.1E7	W=0.55*0.3*23	: BALOK
15	SH=R	T=0.5,0.3	E=2.1E7	W=0.5*0.3*23	: BALOK
16	SH=R	T=0.8,0.25	E=2.1E7	W=0.8*0.25*23	: BALOK
17	SH=R	T=0.85,0.25	E=2.1E7	W=0.85*0.25*23	: BALOK
18	SH=R	T=0.3,0.25	E=2.1E7	W=0.25*0.2*23	: BALOK
19	SH=R	T=0.6,0.25	E=2.1E7	W=0.6*0.25*23	: BALOK
20	SH=R	T=0.5,0.2	E=2.1E7	W=0.5*0.2*23	: BALOK
21	SH=R	T=0.6,0.2	E=2.1E7	W=0.6*0.2*23	: BALOK
22	SH=R	T=0.5,0.3	E=2.1E7	W=0.5*0.3*23	: BALOK
23	SH=R	T=0.4,0.2	E=2.1E7	W=0.4*0.2*23	: BALOK
24	SH=R	T=0.4,0.3	E=2.1E7	W=0.4*0.3*23	: BALOK
25	SH=R	T=0.45,0.3	E=2.1E7	W=0.45*0.3*23	: BALOK
26	SH=R	T=0.7,0.1	E=2.1E7	W=0.7*0.1*23	: BALOK
27	SH=R	T=0.45,0.45	E=2.1E7	W=0.4*0.4*23	: BALOK
28	SH=R	T=0.5,0.35	E=2.1E7	W=0.5*0.35*23	: BALOK
29	SH=R	T=0.6,0.35	E=2.1E7	W=0.6*0.35*23	: BALOK

## C SPAN LOADING DATA

1	TRAP=0,0,0,2.199,-4.766,0,3.799,-4.766,0,5.99,0,0	: A
2	TRAP=0,0,0,2.199,-4.766,0,4.399,0,0	
3	TRAP=0,0,0,2.199,-3,0,3.799,-3,0,5.99,0,0	
4	TRAP=0,0,0,2.199,-3,0,4.399,0,0	
5	TRAP=0,0,0,1.499,-4.766,0,3.099,-4.766,0,4.599,0,0	: B
6	TRAP=0,0,0,1.499,-4.766,0,2.99,0,0	

7 TRAP=0,0,0,1.499,-3,0,3.099,-3,0,4.599,0,0  
 8 TRAP=0,0,0,1.499,-3,0,2.99,0,0  
 9 TRAP=0,0,0,1.99,-4.766,0,3.99,-4.766,0,5.99,0,0 :C  
 10 TRAP=0,0,0,1.99,-4.766,0,3.99,0,0  
 11 TRAP=0,0,0,1.99,-2.5,0,3.99,-2.5,0,5.99,0,0  
 12 TRAP=0,0,0,1.99,-2.5,0,3.99,0,0  
 13 TRAP=0,0,0,1.199,-4.766,0,1.799,-4.766,0,2.99,0,0 :D  
 14 TRAP=0,0,0,1.199,-4.766,0,2.399,0,0  
 15 TRAP=0,0,0,1.199,-2.5,0,1.799,-2.5,0,2.99,0,0  
 16 TRAP=0,0,0,1.199,-2.5,0,2.399,0,0  
 17 TRAP=0,0,0,0.799,-4.766,0,2.199,-4.766,0,2.99,0,0 :E  
 18 TRAP=0,0,0,0.799,-4.766,0,1.599,0,0  
 19 TRAP=0,0,0,0.799,-3,0,2.199,-3,0,2.99,0,0  
 20 TRAP=0,0,0,0.799,-3,0,1.599,0,0  
 21 TRAP=0,0,0,1.99,-4.766,0,3.99,-4.766,0,5.99,0,0 :F  
 22 TRAP=0,0,0,1.99,-4.766,0,3.99,0,0  
 23 TRAP=0,0,0,1.99,-4,0,3.99,-4,0,5.99,0,0  
 24 TRAP=0,0,0,1.99,-4,0,3.99,0,0  
 25 TRAP=0,0,0,1.499,-4.766,0,2.499,-4.766,0,3.99,0,0 :G  
 26 TRAP=0,0,0,1.499,-4.766,0,2.99,0,0  
 27 TRAP=0,0,0,1.499,-4,0,2.499,-4,0,3.99,0,0  
 28 TRAP=0,0,0,1.499,-4,0,2.99,0,0  
 29 TRAP=0,0,0,1.99,-4.766,0,3.99,-4.766,0,5.99,0,0 :H  
 30 TRAP=0,0,0,1.99,-4.766,0,3.99,0,0  
 31 TRAP=0,0,0,1.99,-3,0,3.99,-3,0,5.99,0,0  
 32 TRAP=0,0,0,1.99,-3,0,3.99,0,0  
 33 TRAP=0,0,0,1.299,-4.766,0,4.699,-4.766,0,5.99,0,0 :I  
 34 TRAP=0,0,0,1.299,-4.776,0  
 35 TRAP=0,0,0,1.299,-1,0,4.699,-1,0,5.99,0,0  
 36 TRAP=0,0,0,1.299,-1,0  
 37 TRAP=0,0,0,1.499,-4.766,0,1.99,-4.766,0,3.499,0,0 :J  
 38 TRAP=0,0,0,1.499,-4.766,0,2.99,0,0  
 39 TRAP=0,0,0,1.499,-4,0,1.99,-4,0,3.499,0,0  
 40 TRAP=0,0,0,1.499,-4,0,2.99,0,0  
 41 TRAP=0,0,0,1.499,-4.766,0,2.499,-4.766,0,3.99,0,0 :K  
 42 TRAP=0,0,0,1.499,-4.766,0,2.99,0,0  
 43 TRAP=0,0,0,1.499,-2.5,0,2.499,-2.5,0,3.99,0,0  
 44 TRAP=0,0,0,1.499,-2.5,0,2.99,0,0  
 45 TRAP=0,0,0,1.99,-4.766,0,3.99,0,0,0 :L  
 46 TRAP=0,0,0,1.99,-4.766,0,2.99,-4.766,0,4.99,0,0  
 47 TRAP=0,0,0,1.99,-3,0,3.99,0,0  
 48 TRAP=0,0,0,1.99,-3,0,2.99,-3,0,4.99,0,0  
 49 TRAP=0,0,0,1.99,-4.766,0,6.99,-4.766,0,8.99,0,0 :M  
 50 TRAP=0,0,0,1.99,-4.766,0,3.99,0,0  
 51 TRAP=0,0,0,1.99,-3,0,6.99,-3,0,8.99,0,0  
 52 TRAP=0,0,0,1.99,-3,0,3.99,0,0  
 53 TRAP=0,0,0,1.499,-4.766,0,2.399,-4.766,0,3.9,0,0 :N  
 54 TRAP=0,0,0,1.499,-4.766,0,2.99,0,0  
 55 TRAP=0,0,0,1.499,-4,0,2.399,-4,0,3.899,0,0  
 56 TRAP=0,0,0,1.499,-4,0,2.99,0,0  
 57 TRAP=0,0,0,0.99,-4.766,0,3.599,-4.766,0,4.599,0,0 :O  
 58 TRAP=0,0,0,0.99,-4.766,0,1.99,0,0  
 59 TRAP=0,0,0,0.99,-3,0,3.599,-3,0,5.599,0,0  
 60 TRAP=0,0,0,1,-3,0,3.599,0,0  
 61 TRAP=0,0,0,0.99,-4.766,0,2.99,-4.766,0,3.99,0,0 :P  
 62 TRAP=0,0,0,1,-4.766,0,2,0,0  
 63 TRAP=0,0,0,0.99,-2.5,0,2.99,-2.5,0,3.999,0,0



64 TRAP=0,0,0,1,-2.5,0,2,0,0  
 65 TRAP=0,0,0,1.3,-4.766,0,1.7,-4.766,0,2.99,0,0 :Q = I  
 66 TRAP=0,-4.766,0,1.2999,0,0  
 67 TRAP=0,0,0,1.3,-1,0,1.7,-1,0,2.99,0,0  
 68 TRAP=0,-1,0,1.2999,0,0  
 69 WG=0,0,0  
 70 WG=0,0,-6.355 : RATA C MATI  
 71 WG=0,0,-3.333 : RATA C HIDUP  
 72 WG=0,0,-4 : RATA H HIDUP  
 73 TRAP=0,0,0,1.5,-3,0,2.5,-3,0,4,0,0 :B' = K  
 74 TRAP=0,0,0,0.99,-4.766,0,3.399,-4.766,0,4.399,0,0 :O'  
 75 TRAP=0,0,0,0.99,-3,0,3.399,-3,0,4.399,0,0  
 76 WG=0,0,-6.999 : A MT  
 77 WG=0,0,-4.4 : A HD  
 78 WG=0,0,-2.54 : E BALOK BK  
 79 WG=0,0,-1.6 : E  
 80 WG=0,0,-4.766 : J=B RATA MATI  
 81 WG=0,0,-4 : J  
 82 WG=0,0,-3 : B RATA HD  
 83 TRAP=0,0,0,0.65,-4.766,0,2.35,-4.766,0,3,0,0 :PLAT DKT TANGGA  
 84 TRAP=0,0,0,0.6499,-4.766,0,1.299,0,0  
 85 TRAP=0,0,0,0.65,-3,0,2.35,-3,0,3,0,0  
 86 TRAP=0,0,0,0.6499,-3,0,1.299,0,0  
 87 WG=0,0,-3.177 : PLAT UJUNG 1M  
 88 WG=0,0,-0.667 : PLAT UJUNG 1M  
 89 WG=0,0,-5.33 : RATA HIDUP F  
 90 TRAP=0,0,0,0.87499,-4.766,0,2.12499,-4.766,0,2.999,0,0 :D  
 91 TRAP=0,0,0,0.87499,-4.766,0,1.7499,0,0  
 92 TRAP=0,0,0,0.87499,-2.5,0,2.12499,-2.5,0,2.99,0,0  
 93 TRAP=0,0,0,0.87499,-2.5,0,1.7499,0,0  
 94 TRAP=0,0,0,1.299,-4.766,0,2.699,-4.766,0,3.99,0,0 :Q = I  
 95 TRAP=0,0,0,1.299,-1,0,2.699,-1,0,3.999,0,0  
 96 WG=0,0,-4.29  
 97 WG=0,0,-2  
 98 WG=0,0,-8.372  
 99 WG=0,0,-3.9  
 100 TRAP=0,-4.766,0,1.999,0,0  
 101 TRAP=0,-1,0,1.999,0,0  
 102 TRAP=0,0,0,2,-4.766,0,3.999,0,0  
 103 TRAP=0,0,0,2,-1,0,3.999,0,0  
 104 TRAP=0,0,0,2,-4.766,0,4,-4.766,0,6,0,0  
 105 TRAP=0,0,0,2,-1,0,4,-1,0,6,0,0  
 106 TRAP=0,0,0,1.5,-4.766,0,3,0,0  
 107 TRAP=0,0,0,1.5,-1,0,3,0,0  
 108 WG=0,0,-2.383 PLD=1,-6.8,0  
 C ELEMEN LOCATION DATA  
 C KOLOM  
 1418 1 752 M=1 G=15,1,1,1 LP=3,0 : PORTAL 1  
 1 752 18 M=1 G=3,1,1,1 LP=3,0  
 5 756 45 M=1 G=1,1,1,1 LP=3,0  
 7 758 22 M=1 G=3,1,1,1 LP=3,0  
 11 762 51 M=1 G=1,1,1,1 LP=3,0  
 13 764 26 M=1 G=3,1,1,1 LP=3,0  
 17 18 40 M=1 G=3,1,1,1 LP=3,0  
 23 22 47 M=1 G=3,1,1,1 LP=3,0  
 29 26 54 M=1 G=3,1,1,1 LP=3,0  
 33 40 68 M=1 G=3,1,1,1 LP=3,0

37	45	73	M=1	G=1,1,1,1	LP=3,0	
39	47	106	M=1	G=3,1,1,1	LP=3,0	
43	51	75	M=1	G=1,1,1,1	LP=3,0	
45	54	78	M=1	G=3,1,1,1	LP=3,0	
49	68	100	M=1	G=3,1,1,1	LP=3,0	
53	73	104	M=1	G=1,1,1,1	LP=3,0	
59	75	110	M=1	G=1,1,1,1	LP=3,0	
61	78	112	M=1	G=3,1,1,1	LP=3,0	
1434	117	768	M=1	G=3,1,1,1	LP=3,0	: PORTAL 2
1438	121	774	M=1	G=3,1,1,1	LP=3,0	
145	768	137	M=1	G=3,1,1,1	LP=3,0	
149	774	143	M=1	G=3,1,1,1	LP=3,0	
153	137	159	M=1	G=3,1,1,1	LP=3,0	
157	143	163	M=1	G=3,1,1,1	LP=3,0	
161	159	179	M=1	G=3,1,1,1	LP=3,0	
165	163	183	M=1	G=3,1,1,1	LP=3,0	
1293	179	197	M=1	G=3,1,1,1	LP=3,0	
1297	183	203	M=1	G=3,1,1,1	LP=3,0	
1442	211	778	M=2	G=1,1,1,1	LP=3,0	: PORTAL 3
249	778	214	M=2	G=1,1,1,1	LP=3,0	
251	214	218	M=2	G=1,1,1,1	LP=3,0	
253	218	222	M=2	G=1,1,1,1	LP=3,0	
1301	222	226	M=2	G=1,1,1,1	LP=3,0	
1487	823	827	M=3		LP=3,0	: PORTAL 4
1488	824	828	M=3		LP=3,0	
1489	825	829	M=3		LP=3,0	
1490	826	830	M=3		LP=3,0	
1444	229	780	M=2	G=1,1,1,1	LP=3,0	
267	780	232	M=2		LP=3,0	
1507	229	835	M=2		LP=3,0	
268	230	247	M=2		LP=3,0	
1569	230	843	M=2		LP=3,0	
269	232	250	M=2		LP=3,0	
1509	232	837	M=2		LP=3,0	
270	247	265	M=2		LP=3,0	
1571	247	845	M=2		LP=3,0	
271	250	267	M=2		LP=3,0	
1561	250	839	M=2		LP=3,0	
272	265	282	M=2		LP=3,0	
1573	265	847	M=2		LP=3,0	
273	267	285	M=2		LP=3,0	
1563	267	841	M=2		LP=3,0	
274	282	300	M=2		LP=3,0	
1575	282	849	M=2		LP=3,0	
1446	302	782	M=2		LP=3,0	
1491	831	832	M=3		LP=3,0	: PORTAL 5
1492	833	834	M=3		LP=3,0	
1447	303	783	M=1	G=4,1,1,1	LP=3,0	
1452	308	788	M=2		LP=3,0	
1453	309	789	M=3		LP=3,0	
1454	310	790	M=2		LP=3,0	
1455	311	791	M=1	G=4,1,1,1	LP=3,0	
1460	316	796	M=2		LP=3,0	
343	782	318	M=2		LP=3,0	: PORTAL 5
1508	302	836	M=2		LP=3,0	
344	783	319	M=1	G=4,1,1,1	LP=3,0	
349	788	324	M=2		LP=3,0	

350	789	326	M=3		LP=3,0	
351	790	327	M=2		LP=3,0	
352	791	328	M=2	G=4,1,1,1	LP=3,0	
357	796	333	M=2		LP=3,0	
1570	316	844	M=2		LP=3,0	
358	318	336	M=2		LP=3,0	
1560	318	838	M=2		LP=3,0	
359	319	337	M=1	G=4,1,1,1	LP=3,0	
364	324	342	M=2		LP=3,0	
365	326	344	M=3		LP=3,0	
366	327	345	M=2		LP=3,0	
367	328	346	M=1	G=4,1,1,1	LP=3,0	
372	846	351	M=2		LP=3,0	
1572	333	846	M=2		LP=3,0	
373	840	354	M=2		LP=3,0	
1562	336	840	M=2		LP=3,0	
374	337	355	M=1	G=4,1,1,1	LP=3,0	
379	342	360	M=2		LP=3,0	
380	344	362	M=3		LP=3,0	
381	345	363	M=2		LP=3,0	
382	346	364	M=1	G=4,1,1,1	LP=3,0	
387	848	369	M=2		LP=3,0	
1574	351	848	M=2		LP=3,0	
388	842	372	M=2		LP=3,0	
1564	354	842	M=2		LP=3,0	
389	360	378	M=2		LP=3,0	
390	362	380	M=3		LP=3,0	
391	363	381	M=2		LP=3,0	
392	850	387	M=2		LP=3,0	
1576	369	850	M=2		LP=3,0	
1303	355	373	M=1	G=4,1,1,1	LP=3,0	
1308	364	382	M=1	G=4,1,1,1	LP=3,0	
1461	389	797	M=2		LP=3,0	: PORTAL 6
1462	390	798	M=3	G=1,1,1,1	LP=3,0	
463	797	392	M=2		LP=3,0	: PORTAL 6
464	798	394	M=3	G=1,1,1,1	LP=3,0	
466	392	396	M=2		LP=3,0	
467	394	398	M=3	G=1,1,1,1	LP=3,0	
469	396	400	M=2		LP=3,0	
470	398	402	M=3	G=1,1,1,1	LP=3,0	
472	400	404	M=2		LP=3,0	
1464	747	800	M=2		LP=3,0	: PORTAL 7
1465	408	801	M=3	G=1,1,1,1	LP=3,0	
473	402	406	M=3	G=1,1,1,1	LP=3,0	: PORTAL 7
487	801	410	M=3	G=1,1,1,1	LP=3,0	
489	410	412	M=3	G=1,1,1,1	LP=3,0	
491	412	414	M=3	G=1,1,1,1	LP=3,0	
493	414	416	M=3	G=1,1,1,1	LP=3,0	
1313	747	748	M=3		LP=3,0	
1314	748	749	M=3	G=2,1,1,1	LP=3,0	
1467	418	803	M=2	G=1,1,1,1	LP=3,0	: PORTAL 8
499	803	421	M=2		LP=3,0	: PORTAL 8
500	419	434	M=2		LP=3,0	
501	421	437	M=2		LP=3,0	
502	434	450	M=2		LP=3,0	
503	437	453	M=2		LP=3,0	
504	450	466	M=2		LP=3,0	

505	453	469	M=2		LP=3,0	
506	466	482	M=2		LP=3,0	
1469	484	805	M=2	G=1,1,1,1	LP=3,0	: PORTAL 9
563	805	486	M=2		LP=3,0	: PORTAL 9
564	485	491	M=2		LP=3,0	
565	486	492	M=2		LP=3,0	
566	491	497	M=2		LP=3,0	
567	492	498	M=2		LP=3,0	
568	497	503	M=2		LP=3,0	
569	498	504	M=2		LP=3,0	
570	503	509	M=2		LP=3,0	
1471	510	807	M=1	G=6,1,1,1	LP=3,0	: PORTAL 10
1478	517	814	M=1	G=6,1,1,1	LP=3,0	
591	807	525	M=1	G=6,1,1,1	LP=3,0	: PORTAL 10
598	814	535	M=1	G=6,1,1,1	LP=3,0	
605	525	544	M=1	G=6,1,1,1	LP=3,0	
612	535	554	M=1	G=6,1,1,1	LP=3,0	
619	544	563	M=1	G=6,1,1,1	LP=3,0	
626	554	573	M=1	G=6,1,1,1	LP=3,0	
633	563	582	M=1	G=6,1,1,1	LP=3,0	
640	573	592	M=1	G=6,1,1,1	LP=3,0	
1485	661	821	M=2		LP=3,0	: PORTAL 11
1486	662	822	M=2		LP=3,0	
731	821	663	M=2		LP=3,0	: PORTAL 11
732	822	667	M=2		LP=3,0	
733	663	668	M=2		LP=3,0	
734	667	672	M=2		LP=3,0	
735	668	673	M=2		LP=3,0	
736	672	677	M=2		LP=3,0	
737	673	678	M=2		LP=3,0	
738	677	682	M=2		LP=3,0	
1249	100	693	M=1	G=15,1,1,1	LP=3,0	
1265	300	709	M=2		LP=3,0	
1266	387	710	M=2		LP=3,0	
1267	482	711	M=2		LP=3,0	
1268	598	712	M=1	G=6,1,-1,1	LP=3,0	
1275	588	721	M=1	G=6,1,-1,1	LP=3,0	
1282	469	728	M=2		LP=3,0	
1283	372	729	M=2		LP=3,0	
1284	285	730	M=2		LP=3,0	
C ELEMEN KOLOM LT-5 & RING BALOK						
1591	380	851	M=3		LP=3,0	
1592	381	853	M=3		LP=3,0	
1593	406	854	M=3		LP=3,0	
1594	407	856	M=3		LP=3,0	
1595	416	857	M=3		LP=3,0	
1596	417	859	M=3		LP=3,0	
C ELEMEN BALOK SLOOF SEARAH SUMBU - X						
1321	752	753	M=27	G=14,1,1,1	LP=2,0	
1346	768	769	M=27	G=8,1,1,1	LP=2,0	
1355	778	779	M=27		LP=2,0	
1356	782	783	M=27		LP=2,0	
1357	783	784	M=27	G=4,1,1,1	LP=2,0	
1362	788	789	M=27		LP=2,0	
1363	789	790	M=27		LP=2,0	
1364	790	791	M=27	G=4,1,1,1	LP=2,0	
1369	795	796	M=27		LP=2,0	

1370	798	799	M=27		LP=2,0
1371	801	802	M=27		LP=2,0
1372	807	808	M=27	G=5,1,1,1	LP=2,0
1378	813	814	M=27		LP=2,0
1379	814	815	M=27	G=5,1,1,1	LP=2,0
1597	821	822	M=27		LP=2,0
C ELEMEN BALOK SLOOF SEARAH SUMBU - Y					
1385	755	768	M=27	G=9,1,1,1	LP=3,0
1395	772	778	M=27	G=1,1,1,1	LP=3,0
1397	788	797	M=27		LP=3,0
1398	797	800	M=27		LP=3,0
1399	800	805	M=27		LP=3,0
1400	805	813	M=27		LP=3,0
1401	789	798	M=27		LP=3,0
1402	798	801	M=27		LP=3,0
1403	790	799	M=27		LP=3,0
1404	799	802	M=27		LP=3,0
1405	802	806	M=27		LP=3,0
1406	806	814	M=27		LP=3,0
1407	752	780	M=27		LP=3,0
1408	780	782	M=27		LP=3,0
1409	782	803	M=27		LP=3,0
1410	803	807	M=27		LP=3,0
1411	767	781	M=27		LP=3,0
1412	781	796	M=27		LP=3,0
1413	796	804	M=27		LP=3,0
1414	804	820	M=27		LP=3,0
1416	813	821	M=9		LP=3,0
1417	814	822	M=10		LP=3,0
1495	826	830	M=27		LP=3,0
1497	827	832	M=27		LP=3,0
1498	830	834	M=27		LP=3,0
1499	780	827	M=27		LP=2,0
1500	830	781	M=27		LP=2,0
1501	828	785	M=27		LP=3,0
1502	829	786	M=27		LP=3,0
1493	828	829	M=27	NSL=25,69,24,69	LP=2,0
1496	797	798	M=27		LP=2,0
C ELEMEN BALOK SEJAJAR SUMBU - X					
65	17	18	M=26	NSL=66,66,68,68	LP=-2,0 : PORTAL 1
66	18	19	M=9	NSL=25,94,27,95	G=2,1,1,1 LP=-2,0
69	22	23	M=9	NSL=78,78,79,79	G=2,1,1,1 LP=-2,0
72	26	27	M=9	NSL=41,94,43,95	G=2,1,1,1 LP=-2,0
75	30	29	M=26	NSL=66,66,68,68	LP=2,0
76	31	40	M=26	NSL=66,66,68,68	G=3,1,1,1 LP=3,0
80	35	54	M=26	NSL=66,66,68,68	G=3,1,1,1 LP=3,0
84	39	40	M=26	NSL=66,66,68,68	LP=-2,0
85	40	41	M=9	NSL=25,94,27,95	G=2,1,1,1 LP=-2,0
88	44	43	M=26	NSL=66,66,68,68	LP=2,0
89	53	54	M=26	NSL=66,66,68,68	LP=-2,0
90	54	55	M=9	NSL=41,94,43,95	G=2,1,1,1 LP=-2,0
93	58	57	M=26	NSL=66,66,68,68	LP=2,0
94	59	68	M=26	NSL=66,66,68,68	G=3,1,1,1 LP=3,0
98	63	78	M=26	NSL=66,66,68,68	G=3,1,1,1 LP=3,0
102	67	68	M=26	NSL=66,66,68,68	LP=-2,0
103	68	69	M=9	NSL=41,94,43,95	G=2,1,1,1 LP=-2,0
106	72	71	M=26	NSL=66,66,68,68	LP=2,0

107	77	78	M=26	NSL=66,66,68,68		LP=-2,0
108	78	79	M=9	NSL=41,94,43,95	G=2,1,1,1	LP=-2,0
111	82	81	M=26	NSL=66,66,68,68		LP=2,0
112	83	100	M=26	NSL=100,100,101,101	G=15,1,1,1	LP=3,0
128	99	100	M=26	NSL=100,100,101,101		LP=-2,0
129	100	101	M=9	NSL=41,102,43,103	G=5,1,1,1	LP=-2,0
135	106	107	M=9	NSL=45,102,47,103	G=2,1,1,1	LP=-2,0
138	109	110	M=9	NSL=25,102,27,103	G=5,1,1,1	LP=-2,0
144	116	115	M=26	NSL=100,100,101,101		LP=2,0
169	138	125	M=26	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
171	140	22	M=15	NSL=34,69,36,69		LP=-3,0 : A PORTAL 2
172	141	23	M=9	NSL=34,34,36,36	G=1,1,1,1	LP=-3,0
174	143	25	M=9	NSL=34,69,36,69		LP=-3,0
175	144	131	M=26	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
177	133	134	M=26	NSL=66,66,68,68		LP=-2,0
178	134	135	M=22	NSL=22,25,24,28	G=2,1,1,1	LP=-2,0
181	137	138	M=9	NSL=69,69,69,69		LP=-2,0
182	138	139	M=9	NSL=22,94,24,95		LP=-2,0
183	139	140	M=9	NSL=22,69,24,69		LP=-2,0
184	140	141	M=9	NSL=94,61,95,63	G=2,1,1,1	LP=-2,0
187	143	144	M=9	NSL=10,69,12,69		LP=-2,0
188	144	145	M=9	NSL=10,94,12,95		LP=-2,0
189	145	146	M=9	NSL=10,69,12,69		LP=-2,0
190	146	147	M=22	NSL=41,10,43,12	G=2,1,1,1	LP=-2,0
193	150	149	M=26	NSL=66,66,68,68		LP=2,0
198	155	156	M=26	NSL=66,66,68,68		LP=-2,0
199	156	157	M=22	NSL=22,25,24,28	G=2,1,1,1	LP=-2,0
202	159	160	M=9	NSL=10,34,12,36		LP=-2,0
203	160	161	M=9	NSL=10,94,12,95		LP=-2,0
204	161	162	M=9	NSL=10,69,12,69		LP=-2,0
205	163	164	M=9	NSL=10,69,12,69		LP=-2,0
206	164	165	M=9	NSL=10,94,12,95		LP=-2,0
207	165	166	M=9	NSL=10,34,12,36		LP=-2,0
1223	166	167	M=13	NSL=10,41,12,43		LP=-2,0
1224	167	168	M=13	NSL=10,41,12,43		LP=-2,0
210	168	169	M=13	NSL=10,41,12,43		LP=-2,0
211	170	169	M=26	NSL=66,66,68,68		LP=-2,0
216	175	176	M=26	NSL=66,66,68,68		LP=-2,0
217	177	177	M=13	NSL=10,41,12,43	G=2,1,1,1	LP=-2,0
220	179	180	M=9	NSL=10,34,12,36		LP=-2,0
221	180	181	M=9	NSL=10,94,12,95		LP=-2,0
222	181	182	M=9	NSL=10,69,12,69		LP=-2,0
223	183	184	M=9	NSL=10,69,12,69		LP=-2,0
224	184	185	M=9	NSL=10,94,12,95		LP=-2,0
225	186	185	M=9	NSL=10,34,12,36		LP=2,0
226	186	187	M=13	NSL=10,41,12,43	G=2,1,1,1	LP=-2,0
229	190	189	M=26	NSL=66,66,68,68		LP=2,0
232	193	194	M=26	NSL=100,100,101,101		LP=-2,0
233	194	195	M=13	NSL=10,41,12,43	G=2,1,1,1	LP=-2,0
236	197	198	M=9	NSL=10,41,12,43	G=2,1,1,1	LP=-2,0
242	203	204	M=9	NSL=10,41,12,43	G=2,1,1,1	LP=-2,0
245	206	207	M=13	NSL=22,25,24,27	G=2,1,1,1	LP=-2,0
248	210	209	M=26	NSL=100,100,101,101		LP=2,0
C PORTAL 3						
255	213	214	M=9	NSL=61,69,63,69	G=2,1,1,1	LP=-2,0
258	217	218	M=9	NSL=45,69,47,69	G=2,1,1,1	LP=-2,0
261	221	222	M=9	NSL=45,69,47,69	G=2,1,1,1	LP=-2,0

264	225	226	M=9	NSL=45,45,47,47	G=2,1,1,1	LP=-2,0
C PORTAL 4						
275	231	232	M=26	NSL=66,69,68,69		LP=-2,0
276	232	645	M=9	NSL=70,69,89,69		LP=-2,0
1181	645	233	M=7	NSL=70,84,89,86		LP=-2,0
277	233	234	M=7	NSL=22,41,24,73	G=1,1,1,1	LP=-2,0
279	235	236	M=7	NSL=41,96,73,97		LP=-2,0
280	236	237	M=7	NSL=22,41,24,73	G=1,1,1,1	LP=-2,0
282	238	239	M=7	NSL=5,69,7,69		LP=-2,0
283	239	240	M=7	NSL=80,69,82,69		LP=-2,0
284	240	241	M=7	NSL=6,69,8,69		LP=-2,0
285	241	242	M=7	NSL=10,41,12,42	G=4,1,1,1	LP=-2,0
1182	246	646	M=7	NSL=70,84,86,71		LP=-2,0
290	646	247	M=9	NSL=70,69,71,69		LP=-2,0
291	248	247	M=26	NSL=66,69,68,69		LP=2,0
292	249	250	M=26	NSL=66,69,68,69		LP=-2,0
293	250	648	M=9	NSL=70,69,89,69		LP=-2,0
1183	648	251	M=7	NSL=70,84,89,86		LP=-2,0
294	251	252	M=7	NSL=22,41,24,73	G=4,1,1,1	LP=-2,0
299	256	257	M=7	NSL=80,45,82,47		LP=-2,0
230	257	618	M=7	NSL=80,70,82,72		LP=-2,0
300	618	619	M=7	NSL=80,70,82,72		LP=-2,0
231	619	258	M=7	NSL=80,70,82,72		LP=-2,0
301	258	259	M=7	NSL=80,70,82,72		LP=-2,0
302	259	260	M=7	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
1184	264	650	M=7	NSL=84,70,86,71		LP=-2,0
307	650	265	M=7	NSL=70,69,71,69		LP=-2,0
308	660	265	M=7	NSL=66,69,68,69		LP=2,0
309	266	267	M=7	NSL=66,69,68,69		LP=-2,0
310	267	652	M=7	NSL=70,69,71,69		LP=-2,0
1185	652	268	M=7	NSL=70,84,71,86		LP=-2,0
311	268	269	M=7	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
316	273	274	M=7	NSL=80,45,82,47		LP=-2,0
239	274	620	M=7	NSL=80,70,82,72		LP=-2,0
317	620	621	M=7	NSL=80,70,82,72		LP=-2,0
240	621	275	M=7	NSL=80,70,82,72		LP=-2,0
318	275	276	M=7	NSL=80,70,82,72		LP=-2,0
319	276	277	M=7	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
1186	281	654	M=7	NSL=84,70,86,71		LP=-2,0
324	654	282	M=7	NSL=70,69,71,69		LP=-2,0
325	283	282	M=26	NSL=66,69,68,69		LP=2,0
326	284	285	M=26	NSL=100,69,101,69		LP=-2,0
327	285	656	M=9	NSL=70,69,71,69		LP=-2,0
1187	656	286	M=7	NSL=70,84,71,86		LP=-2,0
328	286	287	M=7	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
333	291	292	M=7	NSL=80,45,82,47		LP=-2,0
241	292	622	M=7	NSL=80,70,82,72		LP=-2,0
334	622	623	M=7	NSL=80,70,82,72		LP=-2,0
755	623	293	M=7	NSL=80,70,82,72		LP=-2,0
335	293	294	M=7	NSL=80,70,82,72		LP=-2,0
336	294	295	M=7	NSL=22,53,24,55	G=4,1,1,1	LP=-2,0
1188	299	658	M=7	NSL=84,70,86,89		LP=-2,0
341	658	300	M=7	NSL=70,69,89,69		LP=-2,0
342	301	300	M=26	NSL=100,69,101,69		LP=2,0
C PORTAL 5						
393	317	318	M=7	NSL=66,69,68,69		LP=-2,0
394	318	644	M=9	NSL=70,69,71,69		LP=-2,0

1189	644	319	M=11	NSL=84,70,86,71		LP=-2,0
395	319	320	M=10	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
400	324	325	M=19	NSL=57,5,59,7		LP=-2,0
401	325	326	M=19	NSL=80,74,82,75		LP=-2,0
402	326	327	M=19	NSL=6,69,8,69		LP=-2,0
403	327	328	M=7	NSL=41,30,73,32	G=4,1,1,1	LP=-2,0
1190	332	647	M=11	NSL=84,70,86,72		LP=-2,0
408	647	333	M=14	NSL=70,69,72,68		LP=-2,0
409	334	333	M=26	NSL=66,69,68,69		LP=2,0
410	335	336	M=26	NSL=66,69,68,69		LP=-2,0
411	336	649	M=7	NSL=70,69,89,69		LP=-2,0
1191	649	337	M=11	NSL=70,84,89,86		LP=-2,0
412	337	338	M=7	NSL=22,41,24,73	G=4,1,1,1	LP=-2,0
418	342	343	M=19	NSL=57,5,59,7		LP=-2,0
419	343	344	M=19	NSL=80,74,82,75		LP=-2,0
420	344	345	M=19	NSL=6,69,8,69		LP=-2,0
421	345	346	M=7	NSL=41,30,73,32	G=4,1,1,1	LP=-2,0
1192	350	651	M=11	NSL=84,70,86,72		LP=-2,0
426	651	351	M=7	NSL=70,69,72,69		LP=-2,0
427	352	351	M=26	NSL=66,69,68,69		LP=2,0
428	353	354	M=26	NSL=66,69,68,69		LP=-2,0
429	354	653	M=7	NSL=70,69,72,69		LP=-2,0
1193	653	355	M=11	NSL=70,84,72,86		LP=-2,0
430	355	356	M=7	NSL=30,41,32,73	G=4,1,1,1	LP=-2,0
435	360	361	M=19	NSL=57,5,59,7		LP=-2,0
436	361	362	M=19	NSL=80,74,82,75		LP=-2,0
437	362	363	M=19	NSL=6,69,8,69		LP=-2,0
438	363	364	M=7	NSL=30,41,32,73	G=4,1,1,1	LP=-2,0
1194	368	655	M=7	NSL=70,84,72,86		LP=-2,0
443	655	369	M=7	NSL=70,69,72,69		LP=-2,0
444	370	369	M=26	NSL=66,69,68,69		LP=2,0
445	371	372	M=26	NSL=100,69,101,69		LP=-2,0
446	372	657	M=9	NSL=70,69,71,69		LP=-2,0
1195	657	373	M=7	NSL=70,84,71,86		LP=-2,0
447	373	374	M=7	NSL=10,41,12,73	G=4,1,1,1	LP=-2,0
452	378	379	M=19	NSL=57,5,59,7		LP=-2,0
453	379	380	M=19	NSL=80,74,82,75		LP=-2,0
454	380	381	M=19	NSL=6,69,8,69		LP=-2,0
456	381	382	M=19	NSL=22,53,24,73	G=4,1,1,1	LP=-2,0
1196	386	659	M=7	NSL=70,84,89,86		LP=-2,0
461	659	387	M=7	NSL=70,69,89,69		LP=-2,0
462	388	387	M=26	NSL=100,69,101,69		LP=2,0
C PORTAL 6						
475	392	393	M=4	NSL=57,96,59,97		LP=-2,0
476	393	394	M=4	NSL=2,74,4,75		LP=-2,0
477	394	395	M=21	NSL=69,69,69,69		LP=-2,0
478	396	397	M=4	NSL=57,96,59,97		LP=-2,0
479	397	398	M=4	NSL=2,74,4,75		LP=-2,0
480	398	399	M=21	NSL=69,69,69,69		LP=-2,0
481	400	401	M=4	NSL=57,96,59,97		LP=-2,0
482	401	402	M=4	NSL=2,74,4,75		LP=-2,0
483	402	403	M=21	NSL=69,69,69,69		LP=-2,0
484	404	405	M=4	NSL=57,96,59,97		LP=-2,0
485	405	406	M=4	NSL=2,74,4,75		LP=-2,0
486	406	407	M=21	NSL=69,69,69,69		LP=-2,0
C PORTAL 7						
495	410	411	M=21	NSL=38,69,40,69		LP=-2,0



496	412	413	M=21	NSL=38,69,40,69		LP=-2,0
497	414	415	M=21	NSL=38,69,40,69		LP=-2,0
498	416	417	M=21	NSL=38,69,40,69		LP=-2,0
C PORTAL 8						
507	420	421	M=26	NSL=66,66,68,68		LP=-2,0
508	421	422	M=7	NSL=10,10,12,12	G=5,1,1,1	LP=-2,0
514	428	429	M=7	NSL=30,30,32,32	G=5,1,1,1	LP=-2,0
520	435	434	M=26	NSL=66,66,68,68		LP=2,0
521	436	437	M=26	NSL=66,66,68,68		LP=-2,0
522	437	438	M=7	NSL=22,22,24,24	G=5,1,1,1	LP=-2,0
528	444	445	M=7	NSL=30,30,32,32	G=5,1,1,1	LP=-2,0
534	451	450	M=26	NSL=66,66,68,68		LP=2,0
535	452	453	M=26	NSL=66,66,68,68		LP=-2,0
536	453	454	M=7	NSL=10,10,12,12	G=5,1,1,1	LP=-2,0
542	460	461	M=7	NSL=22,22,24,24	G=5,1,1,1	LP=-2,0
548	467	466	M=26	NSL=66,66,68,68		LP=2,0
549	468	469	M=26	NSL=100,100,101,101		LP=-2,0
550	469	470	M=7	NSL=30,30,32,32	G=5,1,1,1	LP=-2,0
556	476	477	M=7	NSL=30,30,32,32	G=5,1,1,1	LP=-2,0
562	483	482	M=26	NSL=100,100,101,101		LP=2,0
C PORTAL 9						
571	486	487	M=17	NSL=17,69,19,69		LP=-2,0
572	487	488	M=17	NSL=78,69,79,69		LP=-2,0
573	488	489	M=8	NSL=78,76,79,77		LP=-2,0
574	489	490	M=17	NSL=17,76,19,77		LP=-2,0
575	490	491	M=17	NSL=17,38,19,40		LP=-2,0
576	492	493	M=17	NSL=17,69,19,69		LP=-2,0
577	493	494	M=17	NSL=78,69,79,69		LP=-2,0
578	494	495	M=17	NSL=78,76,79,77		LP=-2,0
579	495	496	M=17	NSL=17,76,19,77		LP=-2,0
580	496	497	M=17	NSL=17,38,19,40		LP=-2,0
581	498	499	M=17	NSL=17,69,19,69		LP=-2,0
582	499	500	M=17	NSL=78,69,79,69		LP=-2,0
583	500	501	M=17	NSL=78,76,79,77		LP=-2,0
584	501	502	M=17	NSL=17,76,19,77		LP=-2,0
585	502	503	M=17	NSL=17,38,19,40		LP=-2,0
586	504	505	M=17	NSL=17,69,19,69		LP=-2,0
587	505	506	M=17	NSL=78,69,79,69		LP=-2,0
588	506	507	M=8	NSL=78,76,79,77		LP=-2,0
589	507	508	M=17	NSL=17,76,19,77		LP=-2,0
590	508	509	M=17	NSL=17,38,19,40		LP=-2,0
C PORTAL 10						
647	524	525	M=26	NSL=66,66,68,68		LP=-2,0
648	525	526	M=9	NSL=10,94,12,95	G=2,1,1,1	LP=-2,0
651	528	529	M=9	NSL=10,69,12,69		LP=-2,0
652	529	530	M=9	NSL=10,94,12,95		LP=-2,0
653	530	531	M=9	NSL=10,69,12,69		LP=-2,0
654	531	532	M=16	NSL=90,13,92,15	G=3,1,1,1	LP=-2,0
658	535	536	M=9	NSL=30,69,32,69		LP=-2,0
659	536	537	M=9	NSL=30,94,32,95		LP=-2,0
660	537	538	M=9	NSL=30,69,32,69		LP=-2,0
661	538	539	M=9	NSL=30,94,32,95	G=2,1,1,1	LP=-2,0
664	542	541	M=26	NSL=66,66,68,68		LP=2,0
756	191	529	M=26	NSL=66,69,68,69		LP=-3,0
757	192	530	M=26	NSL=66,69,68,69		LP=-3,0
758	201	536	M=26	NSL=66,69,68,69		LP=-3,0
759	202	537	M=26	NSL=66,69,68,69		LP=-3,0

665	543	544	M=26	NSL=66,66,68,68		LP=-2,0
666	544	545	M=9	NSL=22,94,24,95	G=2,1,1,1	LP=-2,0
669	547	548	M=9	NSL=22,69,24,69		LP=-2,0
670	548	549	M=9	NSL=22,94,24,95		LP=-2,0
671	549	550	M=9	NSL=22,69,24,69		LP=-2,0
672	550	551	M=16	NSL=90,13,92,15	G=3,1,1,1	LP=-2,0
676	554	555	M=9	NSL=30,69,32,69		LP=-2,0
677	555	556	M=9	NSL=30,94,32,95		LP=-2,0
678	556	557	M=9	NSL=30,69,32,69		LP=-2,0
679	557	558	M=9	NSL=30,94,32,95	G=2,1,1,1	LP=-2,0
682	561	560	M=26	NSL=66,66,68,68		LP=2,0
760	151	544	M=26	NSL=66,66,68,68	G=2,1,1,1	LP=-3,0
763	154	547	M=26	NSL=66,69,68,69		LP=-3,0
764	171	548	M=26	NSL=66,69,68,69	G=1,1,1,1	LP=-3,0
766	600	555	M=26	NSL=66,69,68,69	G=2,1,1,1	LP=-3,0
769	603	558	M=26	NSL=66,66,68,68	G=2,1,1,1	LP=-3,0
683	562	563	M=26	NSL=66,66,68,68		LP=-2,0
684	563	564	M=9	NSL=30,94,32,95	G=2,1,1,1	LP=-2,0
687	566	567	M=9	NSL=30,69,32,69		LP=-2,0
688	567	568	M=9	NSL=30,94,32,95		LP=-2,0
689	568	569	M=9	NSL=30,69,32,69		LP=-2,0
690	569	570	M=16	NSL=90,13,92,15	G=3,1,1,1	LP=-2,0
694	573	574	M=9	NSL=30,69,32,69		LP=-2,0
695	574	575	M=9	NSL=30,94,32,95		LP=-2,0
696	575	576	M=9	NSL=30,69,32,69		LP=-2,0
697	576	577	M=9	NSL=30,94,32,95	G=2,1,1,1	LP=-2,0
700	580	579	M=26	NSL=66,66,68,68		LP=2,0
772	606	563	M=26	NSL=66,66,68,68	G=2,1,1,1	LP=-3,0
775	609	566	M=26	NSL=66,69,68,69		LP=-3,0
776	610	567	M=26	NSL=66,69,68,69	G=1,1,1,1	LP=-3,0
778	612	574	M=26	NSL=66,69,68,69	G=2,1,1,1	LP=-3,0
781	615	577	M=26	NSL=66,66,68,68	G=2,1,1,1	LP=-3,0
701	581	582	M=26	NSL=100,100,101,101		LP=-2,0
702	582	583	M=9	NSL=10,102,12,103	G=5,1,1,1	LP=-2,0
708	588	589	M=16	NSL=90,13,92,15	G=3,1,1,1	LP=-2,0
712	592	593	M=9	NSL=22,102,24,103		LP=-2,0
713	593	594	M=9	NSL=22,102,24,103	G=4,1,1,1	LP=-2,0
718	599	598	M=26	NSL=100,100,101,101		LP=2,0
719	173	582	M=26	NSL=100,100,101,101	G=1,1,1,1	LP=-3,0
721	683	584	M=26	NSL=100,100,101,101	G=3,1,1,1	LP=-3,0
725	687	593	M=26	NSL=100,100,101,101	G=5,1,1,1	LP=-3,0
C PORTAL 11						
739	663	664	M=16	NSL=90,87,92,88	G=3,1,1,1	LP=-2,0
743	668	669	M=16	NSL=90,87,92,88	G=3,1,1,1	LP=-2,0
747	673	674	M=16	NSL=90,87,92,88	G=3,1,1,1	LP=-2,0
751	678	679	M=16	NSL=90,87,92,88	G=3,1,1,1	LP=-2,0
C PORTAL 12						
1225	624	625	M=8	NSL=13,17,15,19	G=3,1,1,1	LP=-2,0
1229	629	630	M=8	NSL=13,17,15,19	G=3,1,1,1	LP=-2,0
1233	634	635	M=8	NSL=13,17,15,19	G=3,1,1,1	LP=-2,0
1237	639	640	M=8	NSL=13,17,15,19	G=3,1,1,1	LP=-2,0
C ELEMEN BALOK SEJAJAR SUMBU - Y						
C LANTAI 2						
784	18	134	M=14	NSL=65,26,67,28		LP=3,0
785	19	135	M=14	NSL=26,26,28,28	G=1,1,1,1	LP=3,0
787	21	137	M=5	NSL=65,26,67,28		LP=3,0
788	26	146	M=11	NSL=42,65,44,67		LP=3,0

789	27	147	M=14	NSL=42,42,44,44	G=1,1,1,1	LP=3,0
791	29	149	M=14	NSL=42,65,44,67		LP=3,0
792	134	232	M=10	NSL=33,21,35,23		LP=3,0
793	135	233	M=6	NSL=21,21,23,23	G=1,1,1,1	LP=3,0
795	137	235	M=11	NSL=21,69,23,69	G=1,1,1,1	LP=3,0
797	139	237	M=11	NSL=21,21,23,23		LP=3,0
798	213	238	M=12	NSL=70,69,89,69		LP=3,0
799	140	213	M=12	NSL=70,62,89,64		LP=3,0
800	141	214	M=12	NSL=62,62,64,64	G=1,1,1,1	LP=3,0
802	143	216	M=12	NSL=70,62,71,64		LP=3,0
803	216	241	M=12	NSL=70,69,71,69		LP=3,0
804	144	242	M=11	NSL=9,9,11,11	G=2,1,1,1	LP=3,0
807	147	245	M=6	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
809	149	247	M=10	NSL=33,9,35,11		LP=3,0
810	232	318	M=8	NSL=62,69,69,69		LP=3,0
811	645	644	M=4	NSL=83,96,85,97		LP=3,0
811	233	319	M=18	NSL=83,6,85,8		LP=3,0
812	234	320	M=9	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
817	239	325	M=18	NSL=6,6,8,8	G=1,1,1,1	LP=3,0
819	241	327	M=18	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
824	246	332	M=18	NSL=83,6,85,8		LP=3,0
22	646	647	M=4	NSL=83,96,85,97		LP=3,0
825	247	333	M=5	NSL=69,69,69,69		LP=3,0
826	318	421	M=5	NSL=33,9,35,11		LP=3,0
827	319	422	M=6	NSL=9,9,11,11	G=4,1,1,1	LP=3,0
832	324	392	M=12	NSL=70,58,71,60		LP=3,0
833	392	748	M=11	NSL=70,69,71,69		LP=3,0
1317	748	427	M=11	NSL=70,69,71,69		LP=3,0
834	427	486	M=8	NSL=70,69,71,69		LP=3,0
835	325	393	M=18	NSL=58,58,60,60		LP=3,0
836	393	488	M=20	NSL=1,69,3,69		LP=3,0
837	326	394	M=13	NSL=58,69,60,69		LP=3,0
838	394	410	M=13	NSL=76,69,77,69		LP=3,0
839	410	490	M=13	NSL=76,37,77,39		LP=3,0
840	327	395	M=15	NSL=69,70,69,72		LP=3,0
841	395	411	M=15	NSL=69,70,69,72		LP=3,0
842	411	428	M=15	NSL=80,70,81,82		LP=3,0
843	428	491	M=8	NSL=80,70,81,82		LP=3,0
844	328	429	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
849	333	434	M=5	NSL=33,29,35,31		LP=3,0
850	421	525	M=5	NSL=33,9,35,11		LP=3,0
851	422	526	M=6	NSL=9,9,11,11	G=4,1,1,1	LP=3,0
856	486	624	M=8	NSL=70,18,71,20		LP=3,0
857	624	531	M=8	NSL=70,14,71,16		LP=3,0
858	487	625	M=18	NSL=18,18,20,20		LP=3,0
859	625	532	M=18	NSL=14,14,16,16		LP=3,0
860	489	626	M=18	NSL=18,18,20,20		LP=3,0
861	626	533	M=18	NSL=14,14,16,16		LP=3,0
862	490	627	M=18	NSL=18,18,20,20		LP=3,0
863	627	534	M=18	NSL=14,14,16,16		LP=3,0
864	491	628	M=4	NSL=18,70,20,72		LP=3,0
865	628	535	M=8	NSL=14,70,16,72		LP=3,0
866	429	536	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
871	434	541	M=5	NSL=33,29,35,21		LP=3,0
872	531	663	M=10	NSL=91,69,93,69		LP=3,0
873	532	664	M=18	NSL=91,91,93,93	G=2,1,1,1	LP=3,0
876	535	667	M=10	NSL=91,69,93,69		LP=3,0

1241	663	731	M=26	NSL=87,69,88,69		LP=3,0 <sup>81</sup>
1242	667	732	M=26	NSL=87,69,88,69		LP=3,0
1243	668	733	M=26	NSL=87,69,88,69		LP=3,0
1244	672	734	M=26	NSL=87,69,88,69		LP=3,0
1245	673	735	M=26	NSL=87,69,88,69		LP=3,0
1246	677	736	M=26	NSL=87,69,88,69		LP=3,0
1247	678	737	M=26	NSL=87,69,88,69		LP=3,0
1248	682	738	M=26	NSL=87,69,88,69		LP=3,0
C LANTAI 3						
877	40	156	M=14	NSL=65,26,66,28		LP=3,0
878	41	157	M=5	NSL=26,26,28,28	G=1,1,1,1	LP=3,0
880	43	159	M=14	NSL=26,65,28,67		LP=3,0
881	160	45	M=15	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
883	47	162	M=15	NSL=69,69,69,69		LP=3,0
884	48	218	M=24	NSL=69,69,69,69	G=1,1,1,1	LP=3,0
886	50	163	M=15	NSL=69,69,69,69		LP=3,0
887	164	51	M=15	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
889	54	166	M=14	NSL=42,65,44,67		LP=3,0
890	55	167	M=14	NSL=42,42,44,44	G=1,1,1,1	LP=3,0
892	57	169	M=14	NSL=42,65,44,67		LP=3,0
893	156	250	M=10	NSL=33,21,35,23		LP=3,0
894	157	251	M=6	NSL=21,21,23,23	G=1,1,1,1	LP=3,0
896	159	253	M=11	NSL=9,21,11,23		LP=3,0
897	160	254	M=11	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
899	162	217	M=12	NSL=70,69,71,69		LP=3,0
900	217	256	M=12	NSL=70,45,71,47		LP=3,0
901	218	257	M=11	NSL=45,45,47,47		LP=3,0
902	219	619	M=11	NSL=45,45,47,47		LP=3,0
903	220	259	M=12	NSL=70,45,71,47		LP=3,0
904	163	220	M=12	NSL=70,69,71,69		LP=3,0
905	164	260	M=11	NSL=9,9,11,11	G=2,1,1,1	LP=3,0
908	167	263	M=6	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
910	169	265	M=10	NSL=9,33,11,35		LP=3,0
911	250	336	M=5	NSL=69,69,69,69		LP=3,0
27	648	649	M=4	NSL=83,96,85,97		LP=3,0
912	251	337	M=18	NSL=83,6,85,8		LP=3,0
913	252	338	M=9	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
918	618	343	M=18	NSL=6,6,8,8		LP=3,0
919	258	344	M=18	NSL=6,6,8,8		LP=3,0
920	259	345	M=18	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
925	264	350	M=18	NSL=83,6,85,8		LP=3,0
28	650	651	M=4	NSL=83,96,85,97		LP=3,0
926	265	351	M=5	NSL=69,69,69,69		LP=3,0
927	336	437	M=5	NSL=33,21,35,23		LP=3,0
928	337	438	M=6	NSL=21,21,23,23	G=4,1,1,1	LP=3,0
933	342	396	M=10	NSL=70,58,89,60		LP=3,0
934	396	749	M=10	NSL=70,69,89,69		LP=3,0
1318	749	443	M=10	NSL=70,69,89,69		LP=3,0
935	443	492	M=10	NSL=70,69,89,69		LP=3,0
936	343	397	M=18	NSL=58,58,60,60		LP=3,0
937	397	494	M=20	NSL=1,69,3,69		LP=3,0
938	344	398	M=13	NSL=58,69,60,69		LP=3,0
939	398	412	M=13	NSL=76,69,77,69		LP=3,0
940	412	496	M=13	NSL=76,37,77,39		LP=3,0
941	345	399	M=15	NSL=69,70,69,72		LP=3,0
942	399	413	M=15	NSL=69,70,69,72		LP=3,0
943	413	444	M=15	NSL=80,70,81,82		LP=3,0

944	444	497	M=8	NSL=80,70,81,82		LP=3,0
945	346	445	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
950	351	450	M=5	NSL=33,29,35,31		LP=3,0
951	437	544	M=5	NSL=33,21,35,23		LP=3,0
952	438	545	M=6	NSL=21,21,23,23	G=4,1,1,1	LP=3,0
957	492	629	M=8	NSL=70,18,71,20		LP=3,0
958	629	550	M=8	NSL=70,14,89,16		LP=3,0
959	493	630	M=18	NSL=18,18,20,20		LP=3,0
960	630	551	M=18	NSL=14,14,16,16		LP=3,0
961	495	631	M=18	NSL=18,18,20,20		LP=3,0
962	631	552	M=18	NSL=14,14,16,16		LP=3,0
963	496	632	M=18	NSL=18,18,20,20		LP=3,0
964	632	553	M=18	NSL=14,14,16,16		LP=3,0
965	497	633	M=8	NSL=18,70,20,72		LP=3,0
966	633	554	M=8	NSL=14,70,16,72		LP=3,0
967	445	555	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
972	450	560	M=5	NSL=33,29,35,31		LP=3,0
973	550	668	M=25	NSL=91,69,93,69		LP=3,0
974	551	669	M=18	NSL=91,91,93,93	G=2,1,1,1	LP=3,0
977	554	672	M=25	NSL=91,69,93,69		LP=3,0
C LANTAI 4						
978	68	176	M=14	NSL=42,65,44,67		LP=3,0
979	69	177	M=14	NSL=42,42,44,44	G=1,1,1,1	LP=3,0
981	71	179	M=14	NSL=42,65,44,67		LP=3,0
982	180	73	M=15	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
984	184	75	M=15	NSL=34,69,36,69	G=1,1,1,1	LP=-3,0
986	78	186	M=14	NSL=42,65,44,67		LP=3,0
987	79	187	M=14	NSL=42,42,44,44	G=1,1,1,1	LP=3,0
989	81	189	M=14	NSL=42,65,44,67		LP=3,0
990	176	267	M=10	NSL=9,33,11,35		LP=3,0
991	177	268	M=6	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
993	179	270	M=11	NSL=9,9,11,11	G=2,1,1,1	LP=3,0
996	221	273	M=12	NSL=70,45,71,47		LP=3,0
997	222	274	M=11	NSL=45,45,47,47		LP=3,0
998	223	621	M=11	NSL=45,45,47,47		LP=3,0
999	224	276	M=12	NSL=70,45,71,47		LP=3,0
1000	182	221	M=15	NSL=70,69,71,69		LP=3,0
1001	183	224	M=15	NSL=70,69,71,69		LP=3,0
1002	184	277	M=11	NSL=9,9,11,11	G=2,1,1,1	LP=3,0
1005	187	280	M=6	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
1007	189	282	M=10	NSL=9,33,11,35		LP=3,0
1008	267	354	M=5	NSL=69,69,69,69		LP=3,0
1177	652	653	M=4	NSL=83,96,85,97		LP=3,0
1009	268	355	M=18	NSL=83,6,85,8		LP=3,0
1010	269	356	M=18	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
1015	620	361	M=18	NSL=6,6,8,8		LP=3,0
1016	275	362	M=18	NSL=6,6,8,8		LP=3,0
1017	276	363	M=18	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
1023	281	368	M=18	NSL=83,6,85,8		LP=3,0
1178	654	655	M=4	NSL=83,96,85,97		LP=3,0
1024	282	369	M=5	NSL=69,69,69,69		LP=3,0
1025	354	453	M=5	NSL=29,33,31,35		LP=3,0
1026	355	454	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
1031	360	400	M=10	NSL=70,72,58,60		LP=3,0
1032	400	750	M=10	NSL=70,69,72,69		LP=3,0
1319	750	459	M=10	NSL=70,69,72,69		LP=3,0
1033	459	498	M=10	NSL=70,69,72,69		LP=3,0

1034	361	401	M=18	NSL=58,58,60,60		LP=3,0
1035	401	500	M=20	NSL=1,69,3,69		LP=3,0
1036	362	402	M=13	NSL=58,69,60,69		LP=3,0
1037	402	414	M=13	NSL=76,69,77,69		LP=3,0
1038	414	502	M=13	NSL=76,37,77,39		LP=3,0
1039	363	403	M=15	NSL=69,70,69,72		LP=3,0
1040	403	415	M=15	NSL=69,70,69,72		LP=3,0
1041	415	460	M=15	NSL=80,70,81,82		LP=3,0
1042	460	503	M=8	NSL=80,70,81,82		LP=3,0
1043	364	461	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
1048	369	466	M=5	NSL=33,29,35,31		LP=3,0
1049	453	563	M=5	NSL=29,33,31,35		LP=3,0
1050	454	564	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
1055	498	634	M=8	NSL=70,18,72,20		LP=3,0
1056	499	635	M=18	NSL=18,18,20,20		LP=3,0
1057	501	636	M=18	NSL=18,18,20,20	G=1,1,1,1	LP=3,0
1059	503	638	M=8	NSL=18,70,20,72		LP=3,0
1060	634	569	M=8	NSL=70,14,72,16		LP=3,0
1061	635	570	M=18	NSL=14,14,16,16	G=2,1,1,1	LP=3,0
1064	638	573	M=8	NSL=14,70,16,72		LP=3,0
1065	461	574	M=6	NSL=29,29,31,31	G=4,1,1,1	LP=3,0
1070	466	579	M=5	NSL=29,33,31,35		LP=3,0
1071	569	673	M=25	NSL=91,69,93,69		LP=3,0
1072	570	674	M=18	NSL=91,91,93,93	G=2,1,1,1	LP=3,0
1075	573	677	M=25	NSL=91,69,93,69		LP=3,0
C LANTAI 5						
1076	100	194	M=14	NSL=42,106,44,107		LP=3,0
1077	101	195	M=14	NSL=42,42,44,44	G=4,1,1,1	LP=3,0
1082	106	200	M=15	NSL=42,70,44,72		LP=3,0
1083	107	226	M=22	NSL=46,46,48,48	G=1,1,1,1	LP=3,0
1085	109	203	M=15	NSL=26,70,28,72		LP=3,0
1086	110	204	M=14	NSL=26,26,28,28	G=4,1,1,1	LP=3,0
1091	115	209	M=14	NSL=26,106,28,107		LP=3,0
1092	194	285	M=10	NSL=9,104,11,105		LP=3,0
1093	195	286	M=6	NSL=9,9,11,11	G=1,1,1,1	LP=3,0
1095	197	288	M=11	NSL=9,9,11,11	G=2,1,1,1	LP=3,0
1098	200	225	M=12	NSL=70,70,71,72		LP=3,0
1099	225	291	M=12	NSL=70,45,71,47		LP=3,0
1100	226	292	M=11	NSL=45,45,47,47		LP=3,0
1101	227	623	M=11	NSL=45,45,47,47		LP=3,0
1102	228	294	M=12	NSL=70,45,89,47		LP=3,0
1103	203	228	M=12	NSL=70,70,72,89		LP=3,0
1104	204	295	M=11	NSL=21,21,23,23	G=2,1,1,1	LP=3,0
1107	207	298	M=6	NSL=21,21,23,23	G=1,1,1,1	LP=3,0
1109	209	300	M=10	NSL=21,104,23,105		LP=3,0
1110	285	372	M=5	NSL=69,69,69,69		LP=3,0
1179	656	657	M=4	NSL=83,96,85,97		LP=3,0
1111	286	373	M=18	NSL=83,6,85,8		LP=3,0
1112	287	374	M=18	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
1117	622	379	M=18	NSL=6,6,8,8		LP=3,0
1118	293	380	M=18	NSL=6,6,8,8		LP=3,0
1119	294	381	M=29	NSL=6,6,8,8	G=4,1,1,1	LP=3,0
1124	299	386	M=18	NSL=83,6,85,8		LP=3,0
1180	658	659	M=4	NSL=83,96,85,97		LP=3,0
1125	300	387	M=5	NSL=69,69,69,69		LP=3,0
1126	372	469	M=5	NSL=9,104,11,105		LP=3,0
1127	373	470	M=6	NSL=9,9,11,11	G=4,1,1,1	LP=3,0

1132	378	404	M=11	NSL=70,58,71,60		LP=3,0
1133	404	751	M=11	NSL=70,69,71,69		LP=3,0
1320	751	475	M=11	NSL=70,69,71,69		LP=3,0
1134	475	504	M=8	NSL=70,69,71,69		LP=3,0
1135	379	405	M=18	NSL=58,58,60,60		LP=3,0
1136	405	506	M=20	NSL=1,69,3,69		LP=3,0
1137	380	406	M=13	NSL=58,69,60,69		LP=3,0
1138	406	416	M=13	NSL=76,69,77,69		LP=3,0
1139	416	508	M=13	NSL=76,37,77,39		LP=3,0
1140	381	407	M=15	NSL=70,69,89,69		LP=3,0
1141	407	417	M=15	NSL=70,69,89,69		LP=3,0
1142	417	476	M=15	NSL=80,70,81,89		LP=3,0
1143	476	509	M=15	NSL=80,70,81,89		LP=3,0
1144	382	477	M=6	NSL=21,21,23,23	G=4,1,1,1	LP=3,0
1149	387	482	M=5	NSL=21,104,23,105		LP=3,0
1150	469	582	M=5	NSL=9,104,11,105		LP=3,0
1151	470	583	M=6	NSL=9,9,11,11	G=4,1,1,1	LP=3,0
1156	504	639	M=8	NSL=70,18,71,20		LP=3,0
1161	639	588	M=8	NSL=70,14,71,16		LP=3,0
1157	505	640	M=18	NSL=18,18,20,20		LP=3,0
1158	507	641	M=18	NSL=18,18,20,20	G=1,1,1,1	LP=3,0
1160	509	643	M=8	NSL=18,70,20,89		LP=3,0
1162	640	589	M=18	NSL=14,14,16,16	G=2,1,1,1	LP=3,0
1165	643	592	M=8	NSL=14,70,16,89		LP=3,0
1166	477	593	M=6	NSL=21,21,23,23	G=4,1,1,1	LP=3,0
1171	482	598	M=5	NSL=21,104,23,105		LP=3,0
1172	588	678	M=25	NSL=66,91,68,93		LP=3,0
1173	589	679	M=18	NSL=91,91,93,93	G=2,1,1,1	LP=3,0
1176	592	682	M=25	NSL=66,91,68,93		LP=3,0

## C ELEMEN BALOK BORDES

1503	835	836	M=15	NSL=98,98,99,99		LP=3,0
1504	837	838	M=15	NSL=98,98,99,99		LP=3,0
1505	839	840	M=15	NSL=98,98,99,99		LP=3,0
1506	841	842	M=15	NSL=98,98,99,99		LP=3,0
1565	843	844	M=15	NSL=98,98,99,99		LP=3,0
1566	845	846	M=15	NSL=98,98,99,99		LP=3,0
1567	847	848	M=15	NSL=98,98,99,99		LP=3,0
1568	849	850	M=15	NSL=98,98,99,99		LP=3,0

## C ELEMEN RING BALOK

55	693	694	M=28		G=3,1,1,1	LP=2,0
194	697	698	M=28		G=3,1,1,1	LP=2,0
212	701	702	M=28		G=3,1,1,1	LP=2,0
1197	705	706	M=28		G=3,1,1,1	LP=2,0
1204	711	712	M=28		G=14,1,1,1	LP=2,0
1200	708	739	M=28			LP=3,0
1201	739	740	M=28			LP=3,0
1202	740	709	M=28			LP=3,0
1203	709	710	M=28			LP=3,0
1219	710	741	M=28			LP=3,0
1220	741	711	M=28			LP=3,0
1221	711	742	M=28			LP=3,0
1222	742	712	M=28			LP=3,0
1285	727	743	M=28			LP=3,0
1286	743	728	M=28			LP=3,0
1287	728	744	M=28			LP=3,0
1288	744	729	M=28			LP=3,0
1289	729	730	M=28			LP=3,0

1290	730	745	M=28		LP=3,0
1291	745	746	M=28		LP=3,0
1292	746	693	M=28		LP=3,0

## C ELEMEN LIFT

1577	851	852	M=28	NSL=78,69,97,69	LP=2,0
1578	852	853	M=28	NSL=78,69,97,69	LP=2,0
1579	854	855	M=28	NSL=78,78,97,97 G=1,1,1,1	LP=2,0
1581	857	858	M=28	NSL=78,69,97,69 G=1,1,1,1	LP=2,0
1583	851	854	M=28	NSL=78,69,97,69	LP=3,0
1584	852	855	M=29	NSL=78,108,97,97	LP=3,0
1585	853	856	M=28	NSL=78,69,97,69	LP=3,0
1586	854	857	M=28	NSL=78,69,97,69	LP=3,0
1587	855	858	M=29	NSL=78,108,97,97	LP=3,0
1588	856	859	M=28	NSL=78,69,97,69	LP=3,0
1589	859	718	M=28		LP=3,0
1590	702	853	M=28		LP=3,0

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## LOADS

## C BEBAN PADA JOINT RING BALOK

694			F=0,-11.0238,-55.119,0,0,0	L=1
695			F=0,-11.2548,-56.299,0,0,0	
696	705	1	F=0,-11.3968,-56.984,0,0,0	
706			F=0,-11.2548,-56.299,0,0,0	
707			F=0,-11.0238,-55.119,0,0,0	
739			F=5.5734,0,-9.876,0,0,0	
740			F=17.213,0,-20.56,0,0,0	
710			F=17.213,0,-20.56,0,0,0	
741			F=17.213,0,-20.56,0,0,0	
742			F=5.5734,0,-9.876,0,0,0	
713			F=0,11.0238,-55.119,0,0,0	
714			F=0,11.2548,-56.299,0,0,0	
715	724	1	F=0,11.3968,-56.984,0,0,0	
725			F=0,11.2548,-56.299,0,0,0	
726			F=0,11.0238,-55.119,0,0,0	
743			F=-5.5734,0,-9.876,0,0,0	
744			F=-17.213,0,-20.56,0,0,0	
729			F=-17.213,0,-20.56,0,0,0	
745			F=-17.213,0,-20.56,0,0,0	
746			F=-5.5734,0,-9.876,0,0,0	

## C BEBAN GEMPA RING BALOK

693			F=50.036,0,0,0,0,0	L=5
730			F=66.714,0,0,0,0,0	
729			F=50.036,0,0,0,0,0	
728			F=66.714,0,0,0,0,0	
727			F=33.357,0,0,0,0,0	
693			F=0,8.895,0,0,0,0	L=6
708			F=0,8.895,0,0,0,0	
694	707	1	F=0,17.790,0,0,0,0	

## C BEBAN GEMPA LANTAI 5

100			F=296.913,0,0,0,0,0	L=5
285			F=395.885,0,0,0,0,0	
372			F=296.913,0,0,0,0,0	
469			F=395.885,0,0,0,0,0	
582			F=197.942,0,0,0,0,0	
100			F=0,52.785,0,0,0,0	L=6
115			F=0,52.785,0,0,0,0	
101	114	1	F=0,105.569,0,0,0,0	



## C BEBAN GEMPA LANTAI 4

68			F=209.926,0,0,0,0,0	L=5
267			F=279.902,0,0,0,0,0	
354			F=209.926,0,0,0,0,0	
453			F=279.902,0,0,0,0,0	
563			F=139.951,0,0,0,0,0	
68			F=0,37.32,0,0,0,0	L=6
81			F=0,37.32,0,0,0,0	
69	71	1	F=0,74.64,0,0,0,0	
73	74	1	F=0,74.64,0,0,0,0	
222	223	1	F=0,74.64,0,0,0,0	
183			F=0,74.64,0,0,0,0	
75	76	1	F=0,74.64,0,0,0,0	
78	80	1	F=0,74.64,0,0,0,0	

## C BEBAN GEMPA LANTAI 3

40			F=147.879,0,0,0,0,0	L=5
250			F=197.172,0,0,0,0,0	
336			F=147.879,0,0,0,0,0	
437			F=197.172,0,0,0,0,0	
544			F=98.586,0,0,0,0,0	
40			F=0,26.290,0,0,0,0	L=6
57			F=0,26.290,0,0,0,0	
41	43	1	F=0,52.579,0,0,0,0	
45	52	1	F=0,52.579,0,0,0,0	
54	56	1	F=0,52.579,0,0,0,0	

## C BEBAN GEMPA LANTAI 2

18			F=93.254,0,0,0,0,0	L=5
232			F=124.339,0,0,0,0,0	
318			F=93.254,0,0,0,0,0	
421			F=124.339,0,0,0,0,0	
525			F=62.169,0,0,0,0,0	
18			F=0,16.579,0,0,0,0	L=6
29			F=0,16.579,0,0,0,0	
19	21	1	F=0,33.157,0,0,0,0	
138	139	1	F=0,33.157,0,0,0,0	
22	25	1	F=0,33.157,0,0,0,0	
144	145	1	F=0,33.157,0,0,0,0	
26	28	1	F=0,33.157,0,0,0,0	

:

## COMBO

C	DD	DD	LL	LL	EQx	EQy
C (1,2DD + 1,6DL)						
1	C=1.35	1.35	1.8	1.8	0	0
C 1,05 (DD + 0LL ± EQ)						
2	C=1.181	1.181	0.591	0.591	1.181	0.227
3	C=1.181	1.181	0.591	0.591	-1.181	0.227
4	C=1.181	1.181	0.591	0.591	1.181	-0.227
5	C=1.181	1.181	0.591	0.591	-1.181	-0.227
6	C=1.181	1.181	0.591	0.591	1.181	0.227
7	C=1.181	1.181	0.591	0.591	-1.181	0.227
8	C=1.181	1.181	0.591	0.591	1.181	-0.227
9	C=1.181	1.181	0.591	0.591	-1.181	-0.227
C 0,9 (DD ± EQ)						
10	C=1.013	1.013	0	0	1.013	0.304
11	C=1.013	1.013	0	0	-1.013	0.304
12	C=1.013	1.013	0	0	1.013	-0.304
13	C=1.013	1.013	0	0	-1.013	-0.304

14	C=1.013	1.013	0	0	0.304	1.013
15	C=1.013	1.013	0	0	-0.304	1.013
16	C=1.013	1.013	0	0	0.304	-1.013
17	C=1.013	1.013	0	0	-0.304	-1.013

HITUNGAN SAPCON STRUKTUR TIGA DIMENSI  
CONTROL

IX=0 IU=S IP=0 IC=7,21.1

COMBO

C	1.2	DL	=	1.2	x	0.9/0.8	=	1.35		
C	1.6	LL	=	1.6	x	0.9/0.8	=	1.8		
C	1.05	DD	=	1.05	x	0.9/0.8	=	1.181		
C	1.05	LR x LL	=	1.05	x	0.5 x 0.9/0.8	=	0.591		
C	1.05	EQx	=	1.05	x	0.9/0.8	=	1.181		
C	1.05	EQy	=	1.05	x	0.9/0.8	=	1.181		
C	30%	x 1.05 EQx	=	0.3	x	1.05 x 0.9/0.8	=	0.227		
C	30%	x 1.05 EQy	=	0.3	x	1.05 x 0.9/0.8	=	0.227		
C	0.9	DD	=	0.9	x	0.9/0.8	=	1.013		
C	0.9	EQx	=	0.9	x	0.9/0.8	=	1.013		
C	0.9	EQy	=	0.9	x	0.9/0.8	=	1.013		
C	30%	x 0.9 EQx	=	0.3	x	0.9 x 0.9/0.8	=	0.304		
C	30%	x 0.9 EQy	=	0.3	x	0.9 x 0.9/0.8	=	0.304		
C		DD	DD	LL	LL		EQx	EQy		
C	(1.2DD + 1.6DL)									
1	C=1.35	1.35	1.8	1.8			0	0		
C	1,05 ( DD + 0LL ± EQ )									
2	C=1.181	1.181	0.591	0.591			1.181	0.227		
3	C=1.181	1.181	0.591	0.591			-1.181	0.227		
4	C=1.181	1.181	0.591	0.591			1.181	-0.227		
5	C=1.181	1.181	0.591	0.591			-1.181	-0.227		
6	C=1.181	1.181	0.591	0.591			1.181	0.227		
7	C=1.181	1.181	0.591	0.591			-1.181	0.227		
8	C=1.181	1.181	0.591	0.591			1.181	-0.227		
9	C=1.181	1.181	0.591	0.591			-1.181	-0.227		
C	0,9 (DD ± EQ)									
10	C=1.013	1.013	0	0			1.013	0.304		
11	C=1.013	1.013	0	0			-1.013	0.304		
12	C=1.013	1.013	0	0			1.013	-0.304		
13	C=1.013	1.013	0	0			-1.013	-0.304		
14	C=1.013	1.013	0	0			0.304	1.013		
15	C=1.013	1.013	0	0			-0.304	1.013		
16	C=1.013	1.013	0	0			0.304	-1.013		
17	C=1.013	1.013	0	0			-0.304	-1.013		

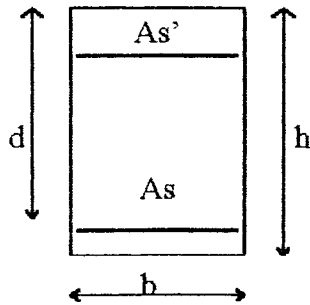
SECTION

1	MN=C	SH=C	T=.7,0.6,0.0625	E=2.1E7	IS=RR-5-5\
	A=3.14*.025*.025	F=4E5,2E4,0.75*4E5,0.65*2E4			
2	MN=C	SH=C	T=0.7,0.5,0.0625	E=2.1E7	IS=RR-4-4\
	A=3.14*.025*.025	F=4E5,2E4,0.75*4E5,0.65*9E5			
3	MN=C	SH=C	T=0.5,0.5,0.0625	E=2.1E7	IS=RR-3-3\
	A=3.14*.025*.025	F=4E5,2E4,0.75*4E5,0.65*9E5			
4	MN=C	SH=B	T=1.0,0.4,0.0625,0.0625	E=2.1E7	
	F=4E5,2E4,0.75*2.4E5				
5	MN=C	SH=B	T=0.65,0.3,0.0625,0.0625	E=2.1E7	
	F=4E5,2E4,0.75*2.4E5				
6	MN=C	SH=B	T=0.95,0.3,0.0625,0.0625	E=2.1E7	

	F=4E5,2E4,0.75*2.4E5	
7	MN=C SH=B T=0.5,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
8	MN=C SH=B T=0.9,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
9	MN=C SH=B T=0.6,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
10	MN=C SH=B T=0.7,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
11	MN=C SH=B T=0.75,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
12	MN=C SH=B T=0.8,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
13	MN=C SH=B T=0.4,0.2,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
14	MN=C SH=B T=0.55,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
15	NN=C SH=B T=0.5,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
16	MN=C SH=B T=0.8,0.25,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
17	MN=C SH=B T=0.85,0.25,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
18	MN=C SH=B T=0.4,0.35,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
19	MN=C SH=B T=0.6,0.25,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
20	MN=C SH=B T=0.5,0.2,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
21	MN=C SH=B T=0.6,0.2,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
22	MN=C SH=B T=0.5,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
23	MN=C SH=B T=0.4,0.2,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
24	MN=C SH=B T=0.4,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
25	MN=C SH=B T=0.45,0.3,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
26	MN=C SH=B T=0.7,0.1,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
27	MN=C SH=B T=0.45,0.45,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
28	MN=C SH=B T=0.5,0.35,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7
29	MN=C SH=B T=0.6,0.35,0.0625,0.0625 F=4E5,2E4,0.75*2.4E5	E=2.1E7

### 3.4. Perhitungan balok (elemen 812)

#### 3.4.1. Perhitungan tulangan lentur



$$d = 800 - 40 - 10 - (25/2) = 737,5 \text{ mm}$$

##### a. Pada Tumpuan

$$M^- = 998 \text{ KN m} ; \quad M^+ = 499 \text{ KN m}$$

Pada SAPCON rumus yang digunakan

$$A_s = \frac{M_u}{\Phi \cdot f_y \cdot (d - \frac{a}{2})} ; \quad (d - \frac{a}{2}) \Rightarrow 0,9 \cdot d$$

$$A_s^- = \frac{998 \times 10^6}{0,8 \times 400 \times 0,9 \times 737,5} = 4698 \text{ mm}^2 = 46,98 \text{ cm}^2$$

$$A_s^+ = \frac{499 \times 10^6}{0,8 \times 400 \times 0,9 \times 737,5} = 2349 \text{ mm}^2 = 23,49 \text{ cm}^2$$

Pada keluaran sapcon dihasilkan

$$A_s^- = 46,02 \text{ cm}^2 ; \quad A_s^+ = 20,47 \text{ cm}^2$$

$$\text{digunakan } \emptyset_{25}, A_1 \emptyset = \frac{1}{4} \times 3,14 \times 2,5^2 = 4,91 \text{ cm}^2$$

$$N = 46,02 / 4,91 = 9,37 \text{ dipakai } 10 \emptyset_{25} \text{ untuk tulangan atas (negatif)}$$

$$N = 20,46 / 4,91 = 4,16 \text{ dipakai } 5 \emptyset_{25} \text{ untuk tulangan bawah (positif)}$$

##### b. Pada Lapangan

$$M^- = 647 \text{ KN m} ; \quad M^+ = 211 \text{ KN m}$$

$$As^- = \frac{647 \times 10^6}{0.8 \times 400 \times 0.9 \times 737,5} = 3046 \text{ mm}^2 = 30,46 \text{ cm}^2$$

$$As^+ = \frac{211 \times 10^6}{0.8 \times 400 \times 0.9 \times 737,5} = 993 \text{ mm}^2 = 9,93 \text{ cm}^2$$

Pada keluaran sapcon dihasilkan

$$As^- = 27,36 \text{ cm}^2 ; \quad As^+ = 10,17 \text{ cm}^2$$

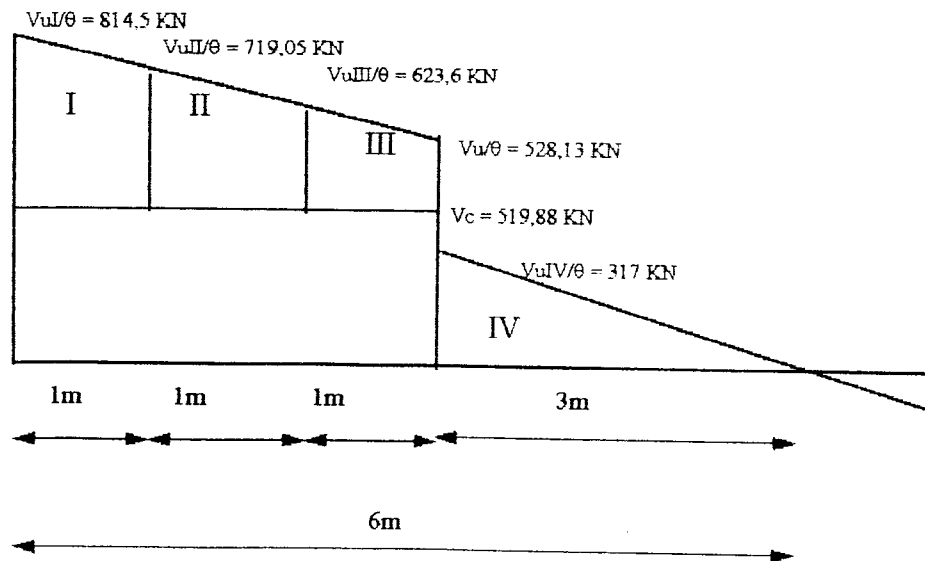
Perbedaan yang cukup kecil ini disebabkan oleh faktor pembulatan, tulangan yang digunakan  $\varnothing_{25}$ ,  $A_{1\varnothing} = \frac{1}{4} \times 3,14 \times 2,5^2 = 4,91 \text{ cm}^2$

$$N = 27,36 / 4,91 = 5,57 \text{ dipakai } 6\varnothing_{25} \text{ untuk tulangan atas (negatif)}$$

$$N = 10,17 / 4,91 = 2,07 \text{ dipakai } 3\varnothing_{25} \text{ untuk tulangan bawah (positif)}$$

### 3.4.2. Perhitungan tulangan geser

Elemen 812 (40/80), dari hitungan SAP'90 didapat  $V_u = 488,70 \text{ KN}$ , pada perhitungan tulangan geser ini di bagi menjadi beberapa daerah



Perhitungan Tulangan geser daerah I

$$d = 800 - 40 - 10 - 12.5 = 737.5 \text{ mm}$$

$$V_{uI} = 488,70 \text{ KN} \quad ; \quad V_{uI}/\theta = 488,70/0,6 = 814,5 \text{ KN}$$

$$V_c = \frac{1}{6} \sqrt{20} \cdot 400 \cdot 737,5 = 519,88 \text{ KN}$$

$$V_s = 814,5 - 519,88 = 294,62 \text{ KN}$$

Tulangan sengkang dipakai  $\varnothing 10$

$$A_{1\varnothing} = \frac{1}{4} \times 3,14 \times 1^2 = 0,785 \text{ cm}^2 \quad ; \quad A_v = 2 \times 0,785 = 1,57 \text{ cm}^2$$

$$s = \frac{A_v \cdot f_y \cdot d}{V_s} \quad ; \quad s = \frac{157 \cdot 240 \cdot 737,5}{294,62 \cdot 10^3} = 106,19 \text{ mm}$$

Dipakai tulangan  $\varnothing 10 - 100$

Perhitungan Tulangan geser daerah II

$$V_{uII} = 431,43 \text{ KN} \quad ; \quad V_{uII}/\theta = 431,43/0,6 = 719,05 \text{ KN}$$

$$V_c = \frac{1}{6} \sqrt{20} \cdot 400 \cdot 737,5 = 519,88 \text{ KN}$$

$$V_s = 719,05 - 519,88 = 199,17 \text{ KN}$$

$$s = \frac{157 \cdot 240 \cdot 737,5}{199,17 \cdot 10^3} = 122,43 \text{ mm}$$

Dipakai tulangan  $\varnothing 10 - 120$

Perhitungan Tulangan geser daerah III

$$V_{uIII} = 374,16 \text{ KN} \quad ; \quad V_{uIII}/\theta = 374,16/0,6 = 623,6 \text{ KN}$$

$$V_c = \frac{1}{6} \sqrt{20} \cdot 400 \cdot 737,5 = 519,88 \text{ KN}$$

$$V_s = 623,6 - 519,88 = 103,72 \text{ KN}$$

$$s = \frac{157.240.737,5}{106,72 \cdot 10^3} = 144,53 \text{ mm}$$

Dipakai tulangan  $\varnothing 10 - 140$

#### Perhitungan Tulangan geser daerah IV

Untuk perhitungan daerah IV, karena gaya lintangnya masih lebih kecil daripada  $V_c$  (kekuatan beton menahan gaya geser), sehingga dipakai tulangan geser minimum, dalam hal ini diambil  $\varnothing 10 - 300$

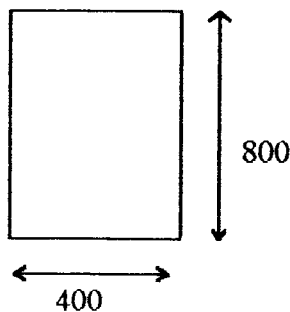
Jarak maksimum tulangan

- $\frac{V_u}{\theta} \leq 3 V_c$  ;  $s \leq 600$  ;  $s \leq d/2$
- $\frac{V_u}{\theta} \leq 5 V_c$  ;  $s \leq 300$  ;  $s \leq d/4$

#### 3.4.3. Perhitungan Tulangan Torsi

Dari keluaran SAP90 didapat

$$T_u = 65,49 \text{ KNm}$$



$$T_c = \frac{\sqrt{f'_c}}{15} \cdot \Sigma x^2 \cdot y$$

$$\Sigma x^2 \cdot y = 400^2 \cdot 800 = 128000000 \text{ mm}^3$$

### 3.5. Perhitungan Kolom

#### 3.5.1 Penulangan lentur

Data hasil dari analisis program SAP90 didapat, untuk kolom 50/50 lantai I

$$P_u = 1810 \text{ KN}$$

$$M_u = 444 \text{ KNm}$$

Dengan memplotkan nilai tersebut ke dalam diagram interaksi M dan P maka didapatkan rasio penulangan kolom = 1,2 %.

$$\text{Maka } A_s \text{ total} = 1,2 \% \times A_g = 1,2 \% \times 250000 = 3000 \text{ mm}^2$$

$$A_s = A_s' = \frac{1}{2} \times A_s \text{ total} = 1500 \text{ mm}^2$$

$$\text{Digunakan tulangan } 4 \text{ } \varnothing 25 = 19963,48 \text{ mm}^2 > 1500 \text{ mm}^2$$

#### 3.5.2. Penulangan geser

Besarnya gaya geser rencana yang bekerja pada kolom ditentukan berdasarkan persamaan berikut yaitu :

$$V_{u,k} = \frac{M_{u,k-a} + M_{u,k-b}}{h_x}$$

Karena pada kolom lantai satu ujung bawahnya menyatu dengan lantai dasar, maka  $M_{u,k-b} = 0$ , sehingga :

$$V_{u,k} = 444 / 4,3 = 103,256 \text{ KN}$$

Panjang daerah dimana beton tidak ikut diperhitungkan dalam menahan gaya geser ditentukan berdasarkan nilai terbesar dari pernyataan berikut, yaitu:

$$1) \ h = 500 \text{ mm} , \ P_u = 1810 \text{ KN} < 0,3 \times A_g \times F_c' = 0,3 \times 250000 \times 20 = 1500 \text{ KN}$$

$$2) \ 1,5 \ h = 1,5 \times 500 = 750 \text{ mm}, \ P_u = 1810 > 0,3 \times A_g \times F_c' = 0,3 \times 250000 \times 20 = 1500 \text{ KN}$$



$$3) \frac{1}{6} \text{ Tinggi bersih kolom} = \frac{1}{6} \times 4300 = 716,67 \text{ mm} \approx 720 \text{ mm}$$

$$4) \text{ sebesar } 450 \text{ mm}$$

Maka nilai yang menentukan 750 mm.

a) di dalam daerah sendi plastis

$$V_{u,k} \text{ desain} = V_{u,k} / \phi$$

$$V_{u,k} \text{ desain} = 103,256 / 0,60 = 172,093 \text{ KN}$$

$$A_v = 2 \left( \frac{1}{4} \cdot \pi \cdot D^2 \right)$$

$$= 2 \left( \frac{1}{4} \cdot \pi \cdot 10^2 \right)$$

$$= 157,08 \text{ mm}^2$$

Jarak sengkang perlu adalah

$$S = (A_v \cdot F_y \cdot d) / (V_{u,k} \text{ desain} \cdot 10^3)$$

$$= (157,08 \times 240 \times 435,5) / (172,093 \times 10^3)$$

$$= 95,402 \text{ mm}$$

Dipakai  $\varnothing 10 - 90$

b) di luar daerah sendi plastis

Pada daerah ini kontribusi  $V_c$  diperhitungkan dalam memberikan tahanan geser. Kemampuan beton dalam menahan gaya geser.

$$V_c = (1 + (P_u / (14 \cdot A_g))) \cdot (\sqrt{f_c'} / 6) \cdot b \cdot d$$

$$V_c = (1 + (1810000 / (14 \times 250000))) \cdot (\sqrt{20} / 6) \cdot 500 \cdot 435,5 \cdot 10^{-3}$$

$$V_c = 246,234 \text{ KN}$$

Karena  $V_c > V_{u,k} \text{ desain}$  maka dipasang sengkang minimum yaitu  $\varnothing 10 - 100$

TABEL 3.8 DIAGRAM INTERAKSI KOLOM 50/50 1%

1%	Po	*****										Mn	
b =	500	500	500	500	500	500	500	500	500	500	500	500	= b
ht =	500	500	500	500	500	500	500	500	500	500	500	500	= ht
d' =		64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	= d'
d =		435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	= d
x =		300	290	270	261.3	200	190	180	150	125	100		= x
Ag =	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	= Ag
Ast =	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	= Ast
As =	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	= As
As' =	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	1250	= As'
fc' =	20	20	20	20	20	20	20	20	20	20	20	20	= fc'
fy =	400	400	400	400	400	400	400	400	400	400	400	400	= fy
a		255	246.5	229.5	222.105	170	161.5	153	127.5	106.25	85	58.82353	a
fs' < fy		400	400	400	400	400	396.3158	385	342	290.4	213		fs' < fy
fs < fy		271	301	368	400	400	400	400	400	400	400	400	fs < fy
Mnb	0	417.165	424.1834	437.9255	443.8761	419.9831	413.0418	403.7168	369.9578	333.9474	288.1163	203.0441	Mnb
Pnb	5207.5	2307.5	2197.707	1969.778	1866.643	1423.75	1346.895	1260.5	990	744.875	467.5	0	Pnb

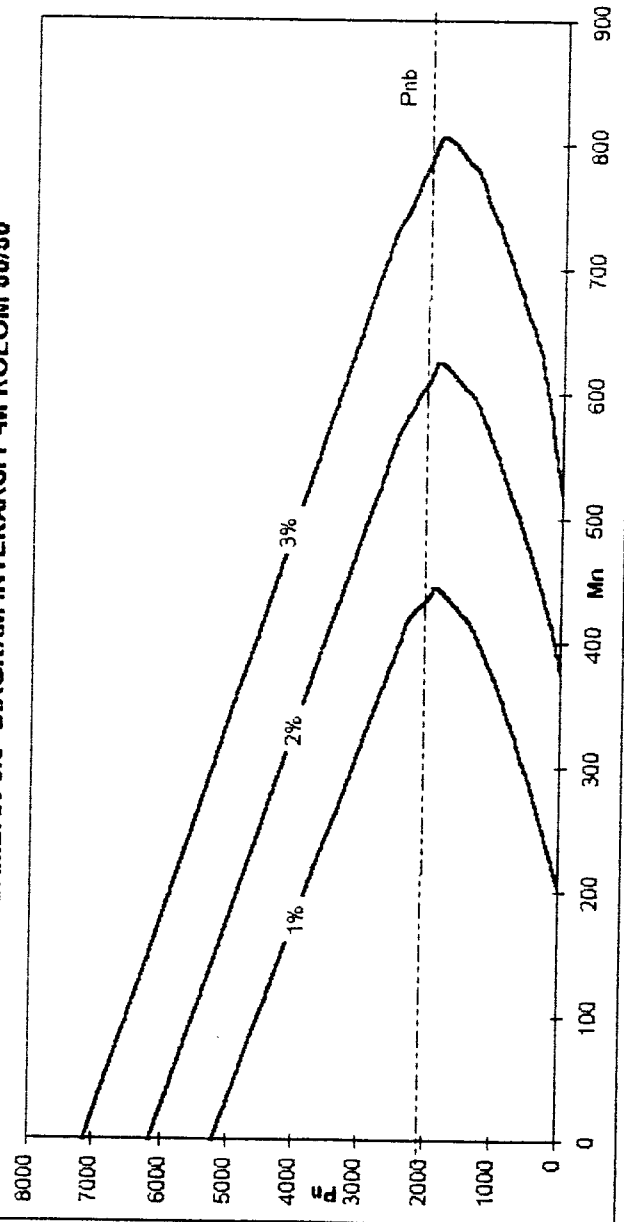
TABEL 3.9 DIAGRAM INTERAKSI KOLOM 50/50 2%

2%	Po	*****										Mn	
b =	500	500	500	500	500	500	500	500	500	500	500	500	= b
ht =	500	500	500	500	500	500	500	500	500	500	500	500	= ht
d' =		64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	= d'
d =		435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	= d
x =		300	290	270	261.3	200	190	180	150	125	100		= x
Ag =	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	= Ag
Ast =	5000	5000	5000	5000	5000	5000	5000	5000	5000	5000	5000	5000	= Ast
As =	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	= As
As' =	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	2500	= As'
fc' =	20	20	20	20	20	20	20	20	20	20	20	20	= fc'
fy =	400	400	400	400	400	400	400	400	400	400	400	400	= fy
a		255	246.5	229.5	222.105	170	161.5	153	127.5	106.25	85	117.6471	a
fs' < fy		400	400	400	400	400	396.3158	385	342	290.4	213		fs' < fy
fs < fy		271	301	368	400	400	400	400	400	400	400	400	fs < fy
Mnb	0	568.811	582.7939	612.0121	625.4342	601.5413	593.7456	581.7968	538.0672	490.092	426.3138	376.6765	Mnb
Pnb	6165	2447.5	2300.164	1988.806	1845.393	1402.5	1321.039	1220.5	896.25	586.625	212.5	0	Pnb

TABEL 3.10 DIAGRAM INTERAKSI KOLOM 50/50 3 %

3% Po	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	Mn	
b =	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	= b
ht =	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	500	= ht
d' =	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	= d'
d =	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	435.5	= d
x =	300	290	270	261.3	200	190	180	150	130	120								= x
Ag =	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	250000	= Ag
Ast =	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	7500	= Ast
As =	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	= AS
As' =	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	3750	= AS'
fc' =	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	= fc'
fy =	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	= fy
a		255	246.5	229.5	222.105	170	161.5	153	127.5	110.5	102	176.4706						a
fs' < fy		400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	fs' < fy
fs < fy		271	301	368	400	400	400	400	400	400	400	400	400	400	400	400	400	fs < fy
Mnb	0	720.458	741.4044	786.0987	806.9923	783.0994	774.4495	759.8768	706.1766	659.6361	631.9933	520.8971						Mnb
Pnb	7122.5	2587.5	2402.621	2007.833	1824.143	1381.25	1295.184	1180.5	802.5	509.1538	343.875	0						Pnb

GAMBAR 3.8 DIAGRAM INTERAKSI P-M KOLOM 60/50







TABEL 3.14 DIAGRAM INTERAKSI KOLOM 70/60 1 %

1%	Po	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	Mn	= b
b =	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	= b
ht =	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	= ht
d' =	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	= d'
d =	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	= d
x =	500	450	400	381.3	340	330	310	290	275	260	246.5	233.75	220	207	194	181	168	= x
Ag =	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	= Ag
Ast =	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	= Ast
As =	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	= As
As' =	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	2100	= As'
fc' =	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	= fc'
fy =	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	= fy
a		425	382.5	340	324.105	289	263.5	255	246.5	233.75	220	207	194	181	168	155	142	a
fs' < fy		400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	fs' < fy
fs < fy		163	247	353	400	400	400	400	400	400	400	400	400	400	400	400	400	fs < fy
Mnb	0	923.177	997.2795	1065.659	1090.778	1075.221	1056.038	1048.17	1039.565	1025.276	1011.000	1000.000	990.000	980.000	970.000	960.000	950.000	Mnb
Pnb	8748.6	4797.84	4186.4	3530.475	3270.171	2912.1	2825.4	2652	2478.6	2348.55	2228.5	2118.5	2008.5	1908.5	1808.5	1708.5	1608.5	Pnb

TABEL 3.15 DIAGRAM INTERAKSI KOLOM 70/60 2 %

2%	Po	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	Mn	= b
b =	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	= b
ht =	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	700	= ht
d' =	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	64.5	= d'
d =	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	635.5	= d
x =	500	450	400	381.3	340	330	310	290	275	260	246.5	233.75	220	207	194	181	168	155	= x
Ag =	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	420000	= Ag
Ast =	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	8400	= Ast
As =	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	= AS
As' =	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	4200	= AS'
fc' =	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	20	= fc'
fy =	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	= fy
a		425	382.5	340	324.105	289	263.5	255	246.5	233.75	220	207	194	181	168	155	142	a	
fs' < fy		400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	fs' < fy
fs < fy		163	247	353	400	400	400	400	400	400	400	400	400	400	400	400	400	400	fs < fy
Mnb	0	1250.29	1375.196	1507.077	1560.225	1544.668	1517.618	1509.013	1494.724	1480.435	1466.146	1451.857	1437.568	1423.279	1408.990	1394.701	1380.412	1366.123	Mnb
Pnb	10357.2	5260.68	4471.3	3592.95	3234.471	2876.4	2616.3	2442.9	2312.85	2188.8	2064.8	1940.8	1816.8	1702.8	1588.8	1474.8	1360.8	1246.8	Pnb



TABEL 3.17 PENULANGAN KOLOM

Lantai	Dimensi mm	Gaya Aksial (KN)	Momen (KN-m)		Geser (KN)		Vc (KN)	Luas Tulangan			Tulangan Lentur Dipakai			Tulangan Geser	
			Bawah	Atas	Tumpuan	Lapangan		As (mm <sup>2</sup> )	As' (mm <sup>2</sup> )	As (mm <sup>2</sup> )	D	As (mm <sup>2</sup> )	D	As'	Tumpuan
I	500 / 500	1810	0	444	172.093	172.093	246.234	1500	1500	1963.48	4D25	1963.48	4D25	P10 - 90	P10 - 300
	500 / 700	2708	0	490	189.922	189.922	367.725	1750	1750	1963.48	4D25	1963.48	4D25	P10 - 100	P10 - 300
	600 / 700	2789	0	613	237.597	237.597	419.008	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 100	P10 - 300
II	500 / 500	796	444	418	334.109	134.896	199.213	1875	1875	1963.48	4D25	1963.48	4D25	P10 - 60	P10 - 300
	500 / 700	1637	490	452	356.116	49.157	315.959	1750	1750	1963.48	4D25	1963.48	4D25	P10 - 60	P10 - 300
	600 / 700	1315	613	499	431.008	83.244	347.764	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 60	P10 - 300
III	500 / 500	484	418	325	287.985	103.24	184.745	1500	1500	1963.48	4D25	1963.48	4D25	P10 - 60	P10 - 300
	500 / 700	1113	452	357	313.566	22.933	290.633	1750	1750	1963.48	4D25	1963.48	4D25	P10 - 70	P10 - 300
	600 / 700	1137	499	364	334.496	334.496	339.16	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 70	P10 - 300
IV	500 / 500	303	325	242	219.767	43.415	176.352	1250	1250	1472.62	3D25	1472.62	3D25	P10 - 70	P10 - 300
	500 / 700	575	357	425	303.101	38.472	264.629	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 70	P10 - 300
	600 / 700	588	364	531	346.899	34.274	312.625	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 60	P10 - 300
V	500 / 500	117	242	61	117.442	117.442	167.726	1250	1250	1472.62	3D25	1472.62	3D25	P10 - 100	P10 - 300
	500 / 700	112	425	469	346.512	104.262	242.25	2450	2450	2454.37	5D25	2454.37	5D25	P10 - 60	P10 - 300
	600 / 700	144	531	439	375.969	84.805	291.164	2100	2100	2454.37	5D25	2454.37	5D25	P10 - 60	P10 - 300



### 3.6. Perhitungan Pondasi

#### 3.6.1. Perencanaan Dimensi Pelat

Beban kolom  $P_u = 1810 \text{ KN}$  ;  $P = 1347 \text{ KN}$  ;  $\sigma_{\text{tanah}} = 200 \text{ KN/m}^2$

Kedalaman tanah = 2 m ; dengan berat rata - rata  $19,6 \text{ KN/m}^3$  ;  $f_c' = 20 \text{ Mpa}$

Tulangan pokok :  $\varnothing_{19}$  dan tulangan susut :  $\varnothing_8$

Tekanan tanah yang timbul dibawah pondasi akibat beban tersebut :

$$2 ( 19,6 ) = 39,2 \text{ KN/m}^2 \text{ Maka tekanan tanah ijin efektif :}$$

$$200 - 39,2 = 160,8 \text{ KN/m}^2$$

$$A_{\text{petu}} = 1347 / 160,8 = 8,377 \text{ m}^2$$

dipakai ukuran  $3 \times 3 = 9 \text{ m}^2$

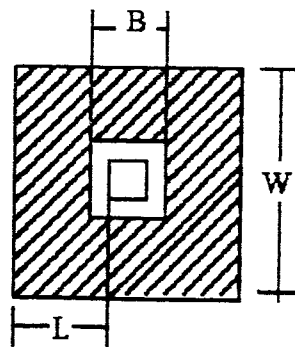
$$p_u = P_u / A = 1810/9 = 201,11 \text{ KN/m}^2$$

Tebal pondasi diambil 700 mm dan tebal selimut beton = 50 mm

$$\text{Maka } d = 700 - 50 - 19 = 631 \text{ mm}$$

Kontrol Geser

Untuk dua arah



$$B = \text{lebar kolom} + (1/2 \cdot d) \cdot 2$$

$$= 500 + 631 = 1131 \text{ mm}$$

$$V_u = p_u ( W^2 - B^2 )$$

$$= 201,11 \cdot ( 3^2 - 1,131^2 )$$

$$= 1552,7 \text{ KN}$$

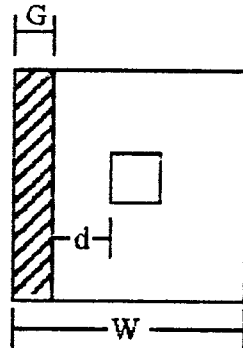
Karena  $\beta_c = 1$  maka , kuat geser maksimum akan menjadi :

$$\begin{aligned} V_c &= 4 (\sqrt{f_c'}) \cdot b_o \cdot d \\ &= 4 (\sqrt{20}) (1131) (4) (631) \\ &= 50579,86 \text{ KN} \end{aligned}$$

$$\begin{aligned} \Phi_{vc} &= 0,6 \cdot 50579,86 \\ &= 30347,9 \text{ KN} \end{aligned}$$

Maka,  $V_u < \Phi V_c$  .....aman

Untuk satu arah



$$\begin{aligned} V_u &= p_u \cdot W \cdot G \\ &= 201,11 \cdot 3 \cdot 0,631 \\ &= 380,7 \text{ KN} \end{aligned}$$

$$\begin{aligned} V_c &= 1/6 (\sqrt{f_c'}) b_w \cdot d \\ &= 1/6 (\sqrt{20}) \cdot 3000 \cdot 631 \\ &= 1410,96 \text{ KN} \end{aligned}$$

$$\begin{aligned} \Phi_{vc} &= 0,6 \cdot 1410,96 \\ &= 846,6 \text{ KN} \end{aligned}$$

Maka,  $V_u < \Phi V_c$  ..... aman

Momen rencana dapat dihitung

$$\begin{aligned} \text{Mu} &= 1/2 \cdot \text{pu} \cdot L^2 \\ &= 1/2 \cdot 201,11 \cdot 1,25^2 \\ &= 157,12 \text{ KNm} \end{aligned}$$

### 3.6.2. Perencanaan tulangan

$$\begin{aligned} \rho_{\min} &= 1,4 / f_y \\ &= 0,00292 \end{aligned}$$

$$\rho_b = \frac{0,85 \cdot f_c \cdot \beta_1}{f_y} \cdot \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \cdot 20 \cdot 0,85}{400} \cdot \frac{600}{600 + 400} = 0,0216$$

$$\begin{aligned} \rho_{\max} &= 0,75 \cdot \rho_b \\ &= 0,0162 \end{aligned}$$

$$\begin{aligned} m &= f_y / (0,85 \cdot f_c) \\ &= 23,53 \end{aligned}$$

$$\begin{aligned} R_n &= \text{Mu} / (\Phi \cdot b \cdot d) \\ &= 157,12 \cdot 10^6 / (0,8 \cdot 1000 \cdot 625^2) \\ &= 0,493 \text{ Mpa} \end{aligned}$$

$$\rho_{\text{perlu}} = \frac{1}{m} \left[ 1 - \sqrt{1 - \frac{2m \cdot R_n}{f_y}} \right]$$

$$\rho_{\text{perlu}} = 0,00123 < \rho_{\min} = 0,00292$$

$$\begin{aligned} A_s &= \rho_{\text{perlu}} \cdot b \cdot d \\ &= 0,00123 \cdot 1000 \cdot 631 = 777,69 \text{ mm}^2 \end{aligned}$$

Cek terhadap  $\rho_{\min}$

$$A_{s_{\min}} = \rho_{\min} \cdot b \cdot d$$

$$= 0,00292 \cdot 1000 \cdot 631 = 1845$$

$$A_{s'} = 1,33 A_s$$

$$= 1,33 \cdot 777,69 = 1034,33 \text{ mm}^2$$

$$A_{s_{\text{terpakai}}} = 1034,33 \text{ mm}^2$$

$$\text{Jarak tulangan} = (0,25 \cdot \pi \cdot D^2 \cdot 1000) / A_s$$

$$= (0,25 \cdot \pi \cdot 19^2 \cdot 1000) / 1034,33$$

$$= 274 \text{ mm}$$

Dipakai  $\varnothing_{19} - 270$

$$\text{Tulangan susut } 20\% A_s = 206,87 \text{ mm}^2$$

$$\text{Jarak tulangan} = (0,25 \cdot \pi \cdot D^2 \cdot 1000) / A_s$$

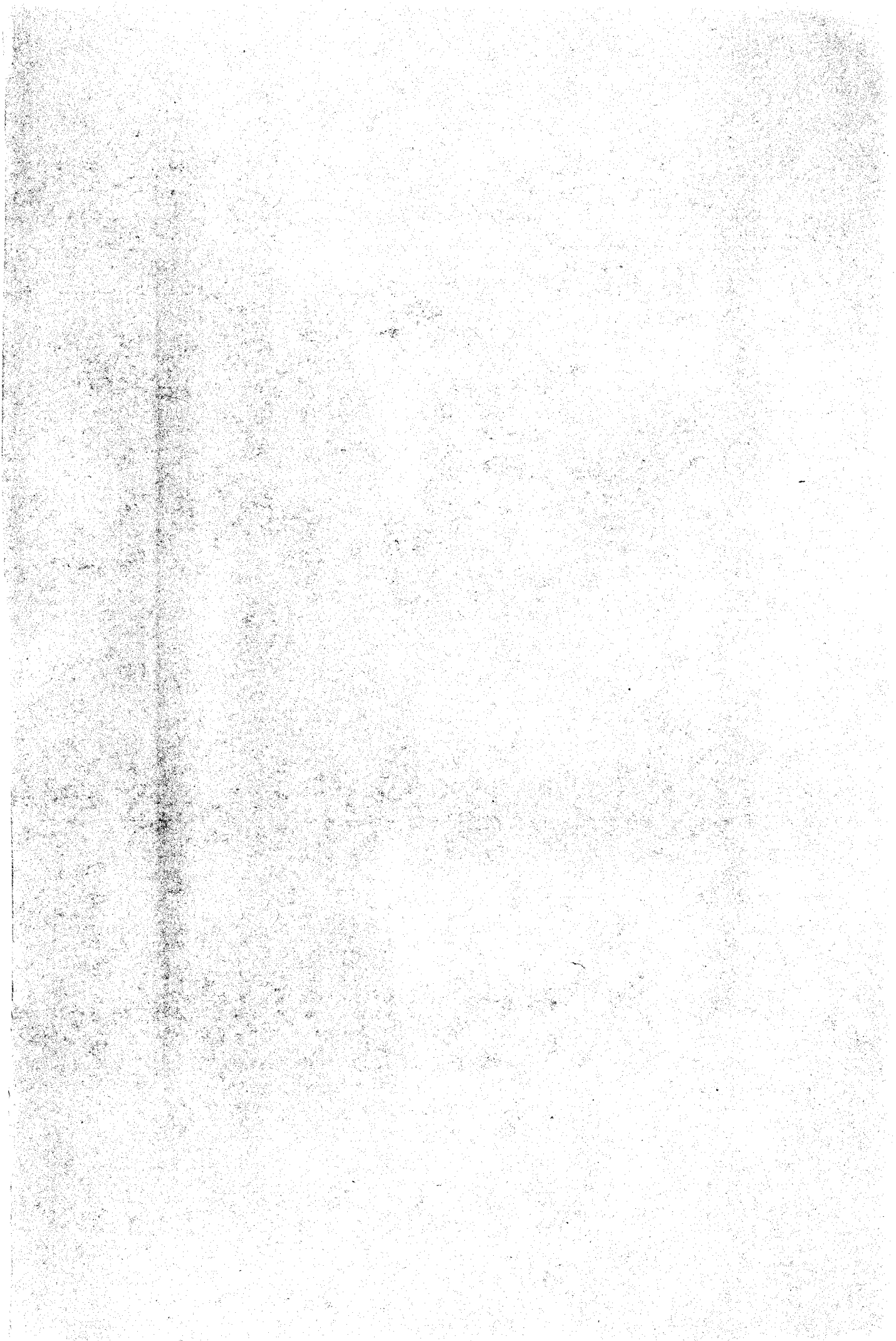
$$= (0,25 \cdot \pi \cdot 8^2 \cdot 1000) / 206,87$$

$$= 242 \text{ mm} = 24 \text{ cm}$$

Dipakai  $\varnothing_8 - 240$

Tabel 3.18 Penulangan pondasi

Dimensi Kolom	B (mm)	A(m)	Tul.Pokok	Tul.susut
500/500	300	8,377	D19-270	D8-240
700/500	350	12,25	D19-240	D8-200
700/600	350	13,47	D19-230	D8-200



## PENUTUP

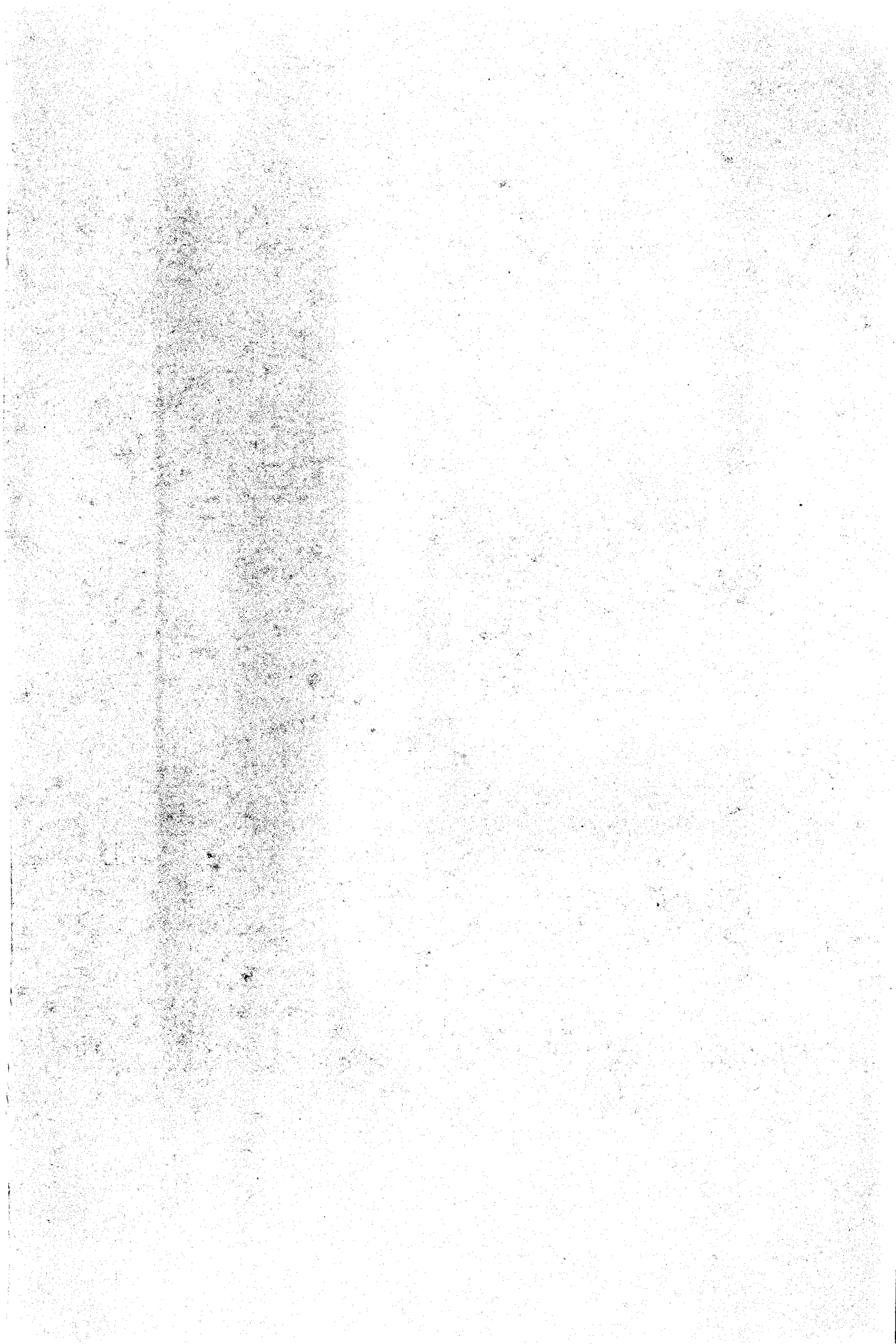
Alhamdulillah wa syukurulillah, hanya dengan Rahmat dan Hidayah Allah SWT penyusun dapat menyelesaikan Tugas Akhir Perencanaan Gedung Megister Manajemen, UGM Yogyakarta ini dengan baik dan penuh perjuangan.

Beberapa hal yang disajikan dalam tugas akhir ini, hanya merupakan sebagian masalah yang ada dalam suatu perencanaan gedung, sehingga belum dapat dijadikan pegangan sepenuhnya tentang seluk-beluk persoalan yang harus diatasi untuk berdirinya sebuah gedung. Masih banyak lagi hal-hal yang harus dibahas dan diketahui, bukan hanya disiplin ilmu teknik sipil saja, tetapi juga dari berbagai disiplin ilmu lain yang memerlukan kerja sama antar berbagai disiplin ilmu. Tugas akhir ini hanya terbatas pada hal-hal yang penyusun ketahui, sehingga sangat disadari bahwa tugas akhir ini jauh dari sempurna seperti yang diharapkan.

Kritik dan saran sangat penyusun harapkan demi menuju kesempurnaan. Penyusun berharap semoga tugas akhir ini dapat bermanfaat bagi yang membacanya khususnya bagi penyusun sendiri.

Akhir kata tidak lupa penyusun mengucapkan terima kasih yang sedalam-dalamnya kepada semua pihak yang telah membantu terselesaikannya laporan ini. Semoga segala amal baiknya mendapat balasan yang setara dari Allah SWT, Amien.

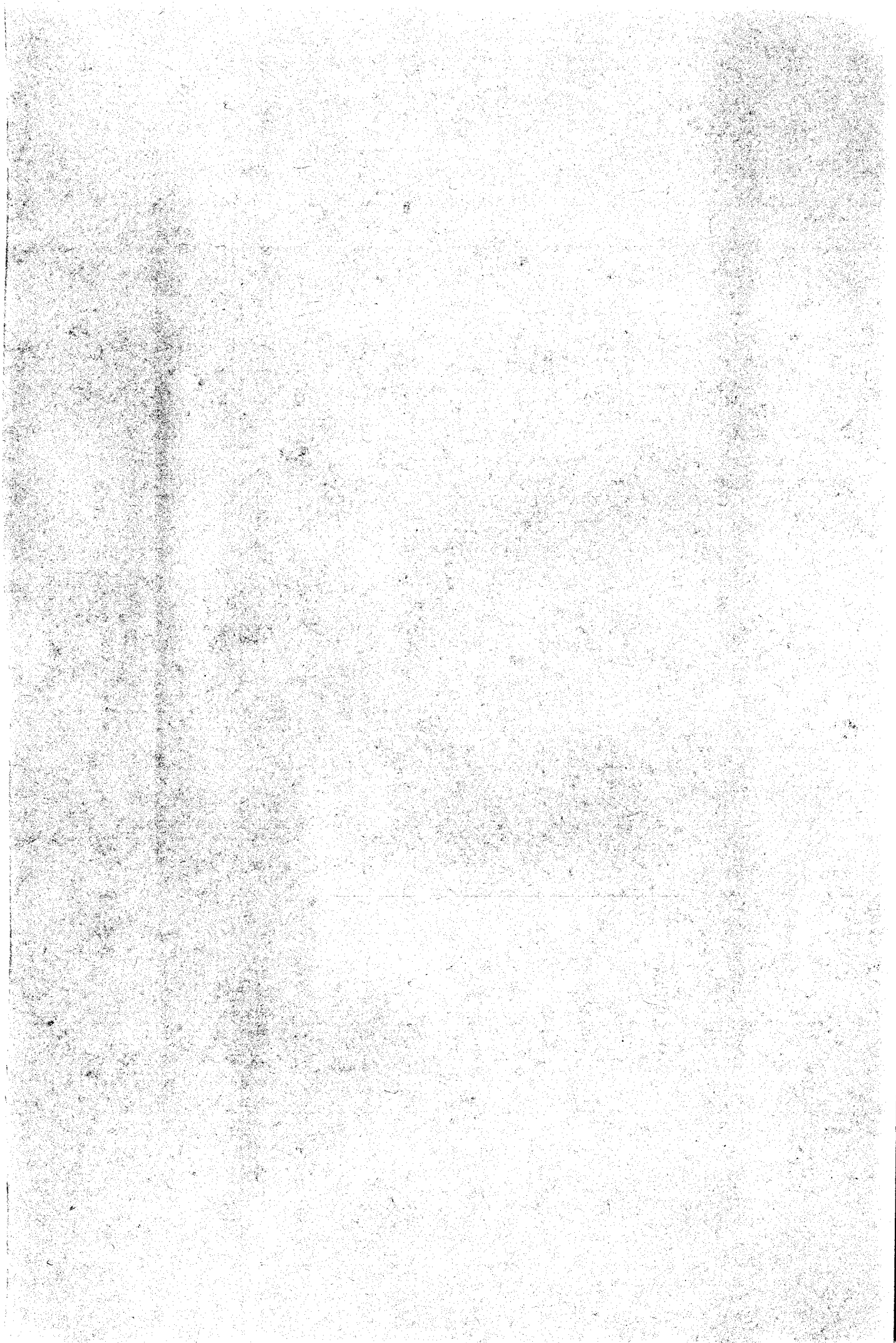
Shodakallahuladzim.



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# LAMPIRAN

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## LAMPIRAN 1. DATA SONDIR LOKASI PROYEK

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## **A. BANGUNAN GEDUNG INDUK**

### **1. HASIL PENYELIDIKAN**

#### **a. Lapisan-lapisan Tanah**

Secara garis besar, kondisi tanah di lokasi ini adalah sebagai berikut.

Jenis tanah di lokasi ini dapat dikatakan hampir seluruhnya berupa tanah pasiran vulkanik sedikit berlanau, yang kondisinya berlapis-lapis dengan masing-masing lapisan: tebal, kepadatan, serta kedalamannya agak bervariasi.

Dari hasil uji sondir sebanyak 6 titik yang dilaksanakan di lokasi ini (lihat gambar: titik-titik sondir T1 sampai dengan T2), ternyata lapisan tanah yang dapat ditembus oleh konis sondir dengan alat sondir 2 ton hanya sampai sedalam 1 sampai 1,80 m. Pada kedalaman tersebut nilai sondir telah melampaui 200 kg/cm<sup>2</sup> sehingga telah melampaui kemampuan alat sondir.

Dari hasil pemboran dan pembuatan sumur uji, diketahui bahwa pada kedalaman tersebut dijumpai lapisan tanah berupa cadas pasir berkerikil yang sangat keras. Untuk mengetahui ketebalan cadas tersebut telah diusahakan dengan pemboran dan menggali sumur uji. Pada dua sumur uji yang digali dengan tenaga orang, hanya berhasil sampai sedalam, yang pertama 4,50 m dan yang kedua 6,0 m. Dan ternyata tanahnya tetap cadas keras dan di bawahnya terdapat krakal dan batu-batu yang terlalu sukar untuk dapat mendalamkan lagi. Beberapa pemboran secara manual telah dicoba dilakukan, dan hanya satu titik yang kebetulan berhasil dan mencapai kedalaman 7 m, dan menunjukkan bahwa seluruh lapisan tanah berupa cadas keras.

Di bawah kedalaman 7 m, belum dapat diketahui secara langsung bagaimana keadaan lapisan-lapisan tanahnya, namun diperkirakan tidak akan dijumpai tanah yang sangat tidak padat atau tanah lunak. Di samping itu, dengan tebal lapisan cadas mulai dari kedalaman sekitar 1 - 1,80 m sampai kedalaman sekitar 7 m pada seluruh luas lokasi, kita sudah bisa yakin bahwa tanah di lokasi ini akan mampu memikul bangunan gedung 5 lantai yang akan dibuat di lokasi ini.

#### **b. Muka Air Tanah**

Pada saat penyelidikan lapangan (Juni 1995), muka air tanah terdapat pada kedalaman sekitar 7,20 - 7,50 m dari permukaan tanah.

## 2. PEMBAHASAN FONDASI

Dengan kondisi tanah seperti yang diuraikan di atas, maka bagi bangunan gedung Induk Proyek MM UGM yang merupakan bangunan 5 lantai yang akan dibuat di lokasi ini, dapat direkomendasikan penggunaan fondasi dangkal berupa *individual spread footing* atau *strip footing* di bawah kolom-kolom yang dasarnya diletakkan pada kedalaman 2 m terhadap permukaan tanah asli.

Kapasitas dukung ijin netto tanah dapat diambil dengan sangat aman sebesar:

$$q_a = 2 \text{ kg/cm}^2 = \underline{\underline{20 \text{ t/m}^2}}$$

dengan penurunan yang sangat kecil dan dapat diabaikan.

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**LAMPIRAN 2. OUT PUT TANGGA 1**

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ANALISA PLAT TANGGA [KN-M]

SHELL ELEMENT FORCES

MEMBRANE FORCES ARE IN FORCE PER UNIT LENGTH  
 BENDING MOMENTS ARE IN MOMENTS PER UNIT LENGTH

ELEMENT ID 30 -----  
 LOAD COND 1 -----

JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
32	4.8327E+00	2.9942E+01	-4.0574E+01	5.9959E+01	-2.5085E+01	-53.60
33	-3.5352E+00	-1.1897E+01	-4.2261E+01	3.4751E+01	-9.0183E+01	-42.18
45	1.4217E+01	6.4241E+01	-1.4333E+02	1.8472E+02	-1.0626E+02	-49.95
46	-7.6578E+00	-4.5144E+01	-1.4474E+02	1.1954E+02	-1.7235E+02	-41.31
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
32	-1.5570E+01	-4.9942E-01	2.3867E-01	-4.9564E-01	-1.5574E+01	89.09
33	-1.3714E+01	-6.7546E-01	-1.1954E+00	-5.8103E-01	-1.5809E+01	-85.48
45	-2.7295E+01	-3.9779E+00	6.7310E-01	-3.9585E+00	-2.7314E+01	88.35
46	-2.7373E+01	-4.0669E+00	-1.8479E+00	-3.9213E+00	-2.7519E+01	-85.49

ELEMENT ID 49 -----  
 LOAD COND 1 -----

JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
68	2.9637E+01	1.4819E+02	1.0791E-01	1.4819E+02	2.9637E+01	89.95
69	4.9861E+00	2.4930E+01	-3.6008E+00	2.5561E+01	4.3559E+00	-50.07
73	5.5478E+00	1.4337E+02	-7.0112E-01	1.4337E+02	5.5443E+00	-89.71
74	-1.9102E+01	2.0113E+01	-4.4097E+00	2.0603E+01	-1.9592E+01	-83.66
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
68	-6.5154E+00	-3.2577E+01	6.7584E-01	-6.4979E+00	-3.2595E+01	1.48
69	-5.9856E+00	-2.9928E+01	1.0444E+00	-5.9401E+00	-2.9974E+01	2.49
73	1.7287E+00	-8.2919E+00	2.3097E+00	2.2353E+00	-8.7986E+00	12.37
74	-1.2404E+00	-7.6572E+00	2.6782E+00	-2.6945E-01	-8.6281E+00	19.93

ELEMENT ID 51 -----  
 LOAD COND 1 -----

JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
70	-1.9511E+01	-9.7556E+01	-2.4204E+00	-1.9436E+01	-9.7631E+01	-1.77
71	-4.4352E+01	-2.2176E+02	4.1010E-01	-4.4351E+01	-2.2176E+02	.13
75	1.3825E+01	-9.0890E+01	-1.9074E+00	1.3860E+01	-9.0924E-01	-1.04
76	-1.1014E+01	-2.1509E+02	9.2299E-01	-1.1010E+01	-2.1510E+02	.26
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
70	-5.2553E+00	-2.6276E+01	1.0912E+00	-5.1988E+00	-2.6333E+01	2.96
71	-4.4455E+00	-2.2227E+01	1.1987E+00	-4.3651E+00	-2.2308E+01	3.84
75	-1.3405E+00	-7.2683E+00	2.7998E+00	-2.2719E-01	-8.3816E+00	21.68
76	-4.4628E-01	-7.0320E+00	2.9073E+00	6.5352E-01	-9.1318E+00	20.72

ELEMENT ID 63 -----  
 LOAD COND 1 -----

JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
85	9.6785E+00	-9.6786E+01	2.5079E+00	9.7437E+00	-8.6851E+01	1.49
86	-1.4017E+01	-2.0526E+02	3.5031E+00	-1.3953E+01	-2.0533E+02	1.05
90	2.2579E+01	-8.4206E+01	2.0984E+00	2.2620E+01	-8.4247E+01	1.13
91	-1.1156E+00	-2.0268E+02	3.0935E+00	-1.0681E+00	-2.0273E+02	.88

## ANALISA PLAT TANGGA (KN-M)

## SHELL ELEMENT FORCES

ELEMENT ID	63 -----					
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
85	7.5388E-01	9.9418E+00	3.4448E+00	1.1090E+01	-3.9424E-01	71.57
86	5.5165E-01	1.0017E+01	3.4428E+00	1.1137E+01	-5.8809E-01	71.98
90	6.4192E-01	9.8886E+00	3.3128E+00	1.0892E+01	-5.6190E-01	71.08
91	5.3064E-01	9.7073E+00	3.5108E+00	1.0896E+01	-6.5842E-01	71.29

ELEMENT ID	72 -----					
LOAD COND	1 -----					
JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
95	-1.5495E+01	-2.8535E+01	1.7562E+00	-1.5262E+01	-2.8768E+01	7.54
96	-3.9164E+01	-1.4688E+02	-2.4966E+01	-3.3659E+01	-1.5239E+02	-12.43
39	3.8498E+00	-2.4666E+01	2.4344E+01	1.7804E+01	-3.8620E+01	29.82
52	-1.9819E+01	-1.4301E+02	-2.3777E+00	-1.9773E+01	-1.4306E+02	-1.11
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
95	-1.2123E+00	3.0689E+00	2.8078E+00	4.4590E+00	-2.6024E+00	63.66
96	-8.7729E-01	3.4152E+00	2.3944E+00	4.4845E+00	-1.9466E+00	65.94
39	-2.4137E+00	-3.0744E+00	-8.4237E-02	-2.4124E+00	-8.0757E+00	-1.85
52	-1.0025E+01	-1.8550E+01	-4.9762E-01	-9.9869E+00	-1.8588E+01	-4.34

ELEMENT ID	76 -----					
LOAD COND	1 -----					
JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
100	1.9247E+01	9.6236E+01	-2.0368E+00	9.6289E+01	1.9193E-01	-88.49
101	-6.3350E+00	-3.1675E+01	-3.2526E+00	-6.9242E+00	-3.2086E+01	-7.20
105	1.5414E+01	9.3468E+01	-2.9149E+00	9.5674E+01	1.5308E-01	-87.92
106	-1.0167E+01	-3.2441E+01	-4.1306E+00	-9.4256E+00	-3.3182E+01	-10.17
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
100	-5.3075E+00	-2.6538E+01	-1.0870E+00	-5.2520E+00	-2.6593E+01	-2.92
101	-8.0415E+00	-3.0207E+01	-1.0397E+00	-5.9968E+00	-3.0232E+01	-2.46
105	-1.3745E+00	-7.4801E+00	-2.7784E+00	-2.9946E-01	-8.5551E+00	-21.15
106	-1.2295E+00	-7.3743E+00	-2.7311E+00	-2.5104E-01	-8.8528E+00	-19.71

ELEMENT ID	88 -----					
LOAD COND	1 -----					
JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
115	1.1487E+01	9.0271E+01	2.3586E+00	9.0341E+01	1.1417E-01	88.29
116	-1.5767E+01	-4.5999E+01	-2.0726E+00	-1.5629E+01	-4.6140E-01	-3.90
120	1.2213E+01	9.0416E+01	-5.3398E-01	9.0419E+01	1.2209E-01	-89.61
121	-1.5040E+01	-4.5853E+01	-4.9651E+00	-1.4259E+01	-4.6633E-01	-3.93
JOINT	M11	M22	M12	MMAX	MMIN	ANGLE
115	7.9558E-01	9.9833E+00	-3.5576E+00	1.1200E+01	-4.2087E-01	-71.12
116	5.6077E-01	1.0056E+01	-3.5924E+00	1.1262E+01	-6.4517E-01	-71.44
120	5.4456E-01	9.9186E+00	-3.4829E+00	1.1071E+01	-6.0786E-01	-71.69
121	4.3339E-01	9.9504E+00	-3.5178E+00	1.1110E+01	-7.2574E-01	-71.76

ELEMENT ID	96 -----					
LOAD COND	1 -----					
JOINT	F11	F22	F12	FMAX	FMIN	ANGLE
125	2.9200E+01	3.2289E+01	1.9801E+01	5.0605E+01	1.0884E-01	47.23
126	1.4044E+01	-4.3494E+01	2.7964E+01	2.5395E+01	-5.4845E+01	22.09
27	1.6801E+00	2.6784E+01	2.1687E+01	3.9290E+01	-1.0825E+01	60.03
14	-1.3476E+01	-4.8998E+01	2.9850E+01	3.4973E+00	-6.5971E+01	29.62

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**LAMPIRAN 3. OUT PUT FRAME**

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DATA UNTUK FORTAL 3DIMENSI KN-M

FLEXURAL AND SHEAR DESIGN OF BEAM-TYPE ELEMENTS

ELEM ID	SECTION DEPTH X WIDTH	STATN LOC	REQUIRED REINFORCING			DESIGN FORCES		
			TOP (LC)	BOT (LC)	SHR (LC)	-M33	+M33	V22
177	.70 X .10	.0	2.20 <15>	2.20 <16>	10.49 <1>	3	1	102
		.3	2.20 <1>	2.20 <17>	10.68 <1>	3	1	104
		.6	2.20 <1>	2.20 <17>	10.81 <1>	3	1	105
		1.0	2.20 <1>	2.20 <17>	10.89 <1>	7	1	106
		1.3	2.20 <1>	2.20 <17>	10.92 <1>	11	6	107
182	.60 X .30	.0	14.03 <7>	11.67 <8>	19.62 <1>	244	207	161
		1.0	5.88 <7>	6.29 <8>	19.32 <1>	109	116	159
		2.0	5.56 <14>	5.56 <1>	18.56 <1>	61	52	153
		3.0	5.92 <6>	6.28 <9>	19.29 <1>	110	116	159
		4.0	14.08 <8>	11.64 <7>	19.59 <1>	244	206	161
395	.70 X .30	.0	17.34 <7>	14.56 <8>	23.87 <1>	355	304	233
		1.0	7.59 <7>	7.60 <8>	23.65 <1>	166	166	231
		2.0	6.59 <15>	6.59 <1>	23.04 <1>	89	77	225
		3.0	7.14 <8>	7.56 <7>	23.49 <1>	157	165	229
		4.0	16.57 <8>	14.75 <7>	23.70 <1>	342	308	231
400	.60 X .25	.0	12.00 <9>	5.65 <10>	14.11 <1>	208	104	116
		1.1	5.58 <7>	4.63 <12>	13.77 <1>	103	29	113
		2.3	4.63 <11>	4.63 <8>	12.90 <1>	52	62	106
		3.5	4.63 <11>	4.78 <1>	12.03 <1>	52	89	99
		4.6	4.63 <15>	6.28 <1>	11.69 <1>	52	115	96
476	1.00 X .40	.0	12.93 <13>	12.93 <1>	19.90 <1>	119	278	285
		1.1	12.93 <12>	12.93 <7>	20.17 <1>	119	224	289
		2.2	12.93 <10>	12.93 <9>	20.66 <1>	119	250	296
		3.3	12.93 <6>	12.93 <9>	21.16 <1>	260	248	303
		4.4	14.75 <6>	12.93 <9>	21.42 <1>	475	237	307
495	.60 X .20	.0	11.17 <9>	10.55 <6>	20.33 <1>	190	181	167
		.8	5.00 <9>	5.18 <6>	20.33 <1>	91	95	167
		1.5	3.71 <13>	3.71 <1>	20.33 <1>	48	45	167
		2.3	4.99 <6>	5.00 <9>	20.33 <1>	91	92	167
		3.0	11.26 <6>	10.25 <9>	20.33 <1>	191	176	167
574	.85 X .25	.0	6.79 <16>	11.65 <1>	18.90 <1>	0	307	228
		.8	6.79 <12>	11.29 <1>	18.91 <1>	0	299	228
		1.5	6.79 <12>	10.18 <1>	19.49 <1>	0	271	235
		2.3	6.79 <12>	8.28 <1>	20.08 <1>	0	223	242
		3.0	6.79 <12>	6.79 <7>	20.67 <1>	0	178	249
654	.80 X .25	.0	18.64 <7>	8.70 <12>	27.27 <1>	436	218	308
		.8	12.96 <7>	6.36 <12>	27.14 <1>	316	55	306
		1.5	8.10 <7>	6.36 <12>	26.76 <1>	204	55	302
		2.3	6.36 <7>	6.36 <8>	26.38 <1>	109	93	298
		3.0	6.36 <11>	6.36 <8>	26.25 <1>	109	134	296
789	.55 X .30	.0	16.38 <1>	7.57 <15>	23.90 <1>	250	125	178
		.8	10.54 <16>	5.04 <15>	23.74 <1>	169	31	177
		1.5	6.16 <17>	5.04 <14>	23.25 <1>	103	39	173
		2.3	5.04 <17>	5.04 <14>	22.77 <1>	62	66	170
		3.0	5.04 <17>	5.42 <6>	22.61 <1>	62	91	169
797	.75 X .30							

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			.0	12.92 <16>	7.11 <15>	12.19 <1>	296	148	128
			1.5	7.11 <16>	7.11 <15>	11.85 <1>	162	142	125
			3.0	7.11 <16>	7.11 <15>	10.97 <1>	74	135	115
			4.5	7.11 <16>	7.11 <1>	10.09 <1>	74	124	106
			6.0	7.11 <15>	7.11 <1>	10.38 <1>	74	99	109
799	.80 X	.30							
			.0	12.85 <17>	7.63 <14>	31.99 <1>	318	159	361
			.5	10.18 <17>	7.63 <14>	31.92 <1>	256	72	360
			1.0	7.73 <17>	7.63 <14>	31.71 <1>	197	84	358
			1.5	7.63 <16>	7.63 <15>	31.49 <1>	148	98	355
			2.0	7.63 <16>	7.63 <15>	31.42 <1>	105	112	355
821	.40 X	.35							
			.0	4.07 <17>	9.70 <1>	22.65 <1>	31	106	117
			.8	4.07 <17>	5.41 <1>	22.88 <1>	31	62	118
			1.5	4.07 <14>	4.07 <17>	23.58 <1>	31	27	122
			2.3	4.60 <14>	4.07 <17>	24.28 <1>	53	27	125
			3.0	11.59 <1>	5.42 <17>	24.51 <1>	125	62	127
836	.50 X	.20							
			.0	3.02 <1>	3.02 <15>	4.30 <1>	74	37	29
			1.5	3.02 <16>	3.02 <15>	4.30 <1>	19	9	29
			3.0	3.02 <17>	3.02 <1>	4.30 <1>	18	18	29
			4.5	3.02 <13>	3.02 <1>	4.30 <1>	18	18	29
			6.0	3.02 <7>	3.02 <12>	4.30 <1>	18	9	29
839	.40 X	.20							
			.0	9.15 <1>	4.14 <14>	10.12 <1>	93	47	32
			1.0	2.94 <17>	2.33 <14>	9.70 <1>	34	12	50
			2.0	2.33 <16>	2.33 <1>	8.56 <1>	23	17	44
			3.0	2.33 <10>	2.33 <1>	7.84 <1>	23	26	40
			4.0	2.33 <14>	2.33 <17>	7.94 <1>	23	19	41
842	.50 X	.30							
			.0	10.44 <17>	6.77 <14>	25.24 <1>	149	100	169
			.5	5.97 <17>	4.95 <14>	24.60 <1>	89	74	165
			1.0	4.53 <17>	4.53 <14>	23.96 <1>	37	45	160
			1.5	4.53 <10>	4.53 <1>	23.32 <1>	37	35	156
			2.0	4.53 <14>	4.59 <17>	22.68 <1>	37	39	152
865	.90 X	.30							
			.0	8.66 <17>	8.66 <14>	33.52 <1>	120	135	430
			.6	8.66 <14>	8.66 <17>	33.93 <1>	120	95	435
			1.2	8.66 <14>	8.66 <17>	34.33 <1>	161	189	440
			1.8	11.10 <14>	9.66 <17>	34.73 <1>	317	278	445
			2.4	17.30 <14>	12.74 <17>	35.13 <1>	479	361	450
870	.95 X	.30							
			.0	9.18 <12>	9.18 <1>	13.74 <1>	131	205	187
			1.5	9.18 <14>	9.18 <17>	14.01 <1>	131	181	190
			3.0	9.18 <14>	9.18 <17>	14.69 <1>	139	217	200
			4.5	10.37 <15>	9.18 <16>	15.37 <1>	316	223	209
			6.0	17.36 <15>	9.18 <16>	15.64 <1>	325	263	212
871	.65 X	.30							
			.0	12.41 <16>	8.90 <15>	13.13 <1>	241	177	118
			1.5	6.08 <16>	6.08 <15>	12.72 <1>	86	103	114
			3.0	6.08 <14>	6.08 <1>	11.84 <1>	73	51	106
			4.5	6.45 <15>	6.69 <16>	12.86 <1>	131	135	116
			6.0	15.32 <15>	10.29 <16>	13.26 <1>	291	203	119

PORTAL TIBA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
177							
1	.00	.0	.00	.00	.00	.00	.00
		.3	-5.39	-1.91	.00	.00	.00
		.7	-9.44	-5.36	.00	.00	.00
		1.0	-12.15	-8.90	.00	.00	.00
		1.3	-13.53	-11.11	.00	.00	.00
2	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
3	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
4	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
5	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
6	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
7	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
8	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00
		.7	-7.30	-2.59	.00	.00	.00
		1.0	-9.43	-3.34	.00	.00	.00
		1.3	-10.56	-3.81	.00	.00	.00
9	.00	.0	.00	.00	.00	.00	.00
		.3	-4.16	-1.70	.00	.00	.00

PORTAL TIBA DINESNSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
10	.7	-7.30	-2.59	.00	.00	.00
	1.0	-8.43	-5.34	.00	.00	
	1.3	-10.56	-8.81	.00	.00	
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
11	1.0	-7.47	-4.22	.00	.00	.00
	1.3	-8.40	-5.82	.00	.00	
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
12	1.3	-8.40	-5.82	.00	.00	.00
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
	1.3	-8.40	-5.82	.00	.00	
13	.0	.00	.00	.00	.00	.00
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
	1.3	-8.40	-5.82	.00	.00	
	.0	.00	.00	.00	.00	
14	.3	-3.28	-1.55	.00	.00	.00
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
	1.3	-8.40	-5.82	.00	.00	
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
15	.7	-5.77	-2.04	.00	.00	.00
	1.0	-7.47	-4.22	.00	.00	
	1.3	-8.40	-5.82	.00	.00	
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
16	1.0	-7.47	-4.22	.00	.00	.00
	1.3	-8.40	-5.82	.00	.00	
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
17	1.3	-8.40	-5.82	.00	.00	.00
	.0	.00	.00	.00	.00	
	.3	-3.28	-1.55	.00	.00	
	.7	-5.77	-2.04	.00	.00	
	1.0	-7.47	-4.22	.00	.00	
	1.3	-8.40	-5.82	.00	.00	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
182								
1	-9.00							-1.59
		.0		35.72	-25.71	-1.18	.43	
		1.0		23.54	5.02	-1.18	.25	
		2.0		-7.28	17.33	-1.18	.07	
		3.0		-23.97	4.52	-1.18	-.11	
		4.0		-56.03	-26.58	-1.18	-.29	
2	-103.78							.02
		.0		-84.53	206.35	3.39	-7.21	
		1.0		-85.82	115.91	3.39	-3.81	
		2.0		-112.74	12.04	3.39	-.42	
		3.0		-129.59	-109.54	3.39	2.97	
		4.0		-138.80	-244.47	3.39	6.37	
3	91.08							-1.25
		.0		133.63	-243.60	-4.47	3.36	
		1.0		129.34	-108.94	-4.47	5.48	
		2.0		112.42	12.36	-4.47	1.01	
		3.0		95.57	115.94	-4.47	-3.47	
		4.0		86.56	206.17	-4.47	-7.94	
4	-103.47							-1.53
		.0		-86.62	206.67	4.20	-9.31	
		1.0		-85.91	116.14	4.20	-5.11	
		2.0		-112.83	12.19	4.20	-.92	
		3.0		-129.67	-109.48	4.20	3.28	
		4.0		-138.89	-244.50	4.20	7.48	
5	91.39							-1.80
		.0		138.54	-243.34	-3.67	7.86	
		1.0		129.25	-108.71	-3.67	4.18	
		2.0		112.53	12.51	-3.67	.31	
		3.0		95.49	115.00	-3.67	-3.16	
		4.0		86.28	206.14	-3.67	-6.80	
6	-103.78							.02
		.0		-84.53	206.35	3.39	-7.21	
		1.0		-85.82	115.91	3.39	-3.81	
		2.0		-112.74	12.04	3.39	-.42	
		3.0		-129.59	-109.54	3.39	2.97	
		4.0		-138.80	-244.47	3.39	6.37	
7	91.08							-1.25
		.0		133.63	-243.60	-4.47	3.36	
		1.0		129.34	-108.94	-4.47	5.48	
		2.0		112.42	12.36	-4.47	1.01	
		3.0		95.57	115.94	-4.47	-3.47	
		4.0		86.56	206.17	-4.47	-7.94	
8	-103.47							-1.53
		.0		-86.62	206.67	4.20	-9.31	
		1.0		-85.91	116.14	4.20	-5.11	
		2.0		-112.83	12.19	4.20	-.92	
		3.0		-129.67	-109.48	4.20	3.28	
		4.0		-138.89	-244.50	4.20	7.48	
9	91.39							-1.80
		.0		138.54	-243.34	-3.67	7.86	
		1.0		129.25	-108.71	-3.67	4.18	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ				
				SHEAR	MOMENT	SHEAR	MOMENT					
10	-88.23		2.0	112.33	12.51	-3.67	.51	.26				
			3.0	95.49	116.00	-3.67	-3.16					
			4.0	86.28	206.14	-3.67	-6.83					
			1.0	-76.99	178.98	2.73	-6.71					
			1.0	-84.17	98.96	2.73	-2.98					
			2.0	-76.66	8.31	2.73	-.25					
			3.0	-109.11	-94.35	2.73	2.48					
11	78.91		4.0	-116.32	-207.38	2.73	3.21	.05				
			1.0	116.23	-207.01	-4.02	9.02					
			1.0	108.96	-93.90	-4.02	5.00					
			2.0	96.47	9.08	-4.02	.99					
			3.0	84.02	99.06	-4.02	-3.05					
			4.0	76.21	178.96	-4.02	-7.07					
			12	-87.81		1.0	-77.02		179.40	3.20	-3.52	-1.47
1.0	-84.29	99.26				3.20	-4.72					
2.0	-76.79	9.00				3.20	-1.92					
3.0	-109.23	-94.27				3.20	2.89					
4.0	-116.44	-207.62				3.20	6.69					
13	79.33					1.0	116.11	-306.59	-2.95	6.20	-1.71	
						1.0	106.85	-93.60	-2.95	3.26		
			2.0	96.35	9.27	-2.95	.51					
			3.0	83.90	99.13	-2.95	-2.63					
			4.0	76.19	178.92	-2.95	-7.08					
			14	-30.22		1.0	-9.18	43.41	-1.89	2.73		1.04
						1.0	-16.45	31.11	-1.89	1.84		
2.0	-28.74	8.68				-1.89	.85					
3.0	-41.39	-26.75				-1.89	.07					
4.0	-48.60	-72.26				-1.89	-.39					
15	19.95					1.0	48.78	-72.43	-2.91	7.15	.97	
						1.0	41.51	-25.77	-2.91	4.23		
			2.0	29.02	9.76	-2.91	1.32					
			3.0	16.57	31.29	-2.91	-1.59					
			4.0	9.36	43.74	-2.91	-4.50					
			16	-28.87		1.0	-9.67	44.82	2.70	-6.65		-1.41
						1.0	-16.83	32.13	2.70	-3.96		
2.0	-29.33	9.32				2.70	-1.26					
3.0	-41.78	-26.50				2.70	1.43					
4.0	-48.99	-72.40				2.70	4.13					
17	21.32					1.0	48.39	-71.02	.67	-2.23	-1.49	
						1.0	41.12	-25.75	.67	-1.56		
			2.0	28.63	9.40	.67	-.39					
			3.0	16.18	31.54	.67	-1.22					
			4.0	8.97	43.60	.67	.45					

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-3 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
395						
1	-17					1.94
	0	41.70	-35.62	.61	-1.22	
	1.0	28.48	.59	.61	-7.71	
	2.0	2.27	17.05	.61	-7.99	
	3.0	-22.64	6.39	.61	.82	
	4.0	-35.75	-23.92	.61	1.13	
2	-101.96					1.52
	0	-132.31	300.97	5.65	-11.14	
	1.0	-142.47	164.18	5.65	-5.49	
	2.0	-160.32	13.29	5.65	.16	
	3.0	-178.10	-156.42	5.65	5.21	
	4.0	-187.98	-340.16	5.65	11.46	
3	113.85					1.04
	0	193.60	-333.47	-3.43	2.00	
	1.0	163.64	-166.14	-3.43	4.58	
	2.0	165.20	9.08	-3.43	1.15	
	3.0	148.02	165.48	-3.43	-2.28	
	4.0	128.13	307.25	-3.43	-3.71	
4	-115.68					1.50
	0	-133.69	304.25	4.25	-9.25	
	1.0	-143.65	166.29	4.25	-5.58	
	2.0	-161.30	14.22	4.25	-1.33	
	3.0	-179.28	-156.68	4.25	2.92	
	4.0	-189.16	-341.61	4.25	7.18	
5	100.12					1.02
	0	192.42	-352.19	-4.83	2.31	
	1.0	182.46	-164.04	-4.83	4.49	
	2.0	164.61	10.00	-4.83	-7.74	
	3.0	146.84	165.22	-4.83	-5.16	
	4.0	126.95	306.41	-4.83	-9.99	
6	-101.96					1.52
	0	-132.31	300.97	5.65	-11.14	
	1.0	-142.47	164.18	5.65	-5.49	
	2.0	-160.32	13.29	5.65	.16	
	3.0	-178.10	-156.42	5.65	5.21	
	4.0	-187.98	-340.16	5.65	11.46	
7	113.85					1.04
	0	193.60	-333.47	-3.43	2.00	
	1.0	163.64	-166.14	-3.43	4.58	
	2.0	165.20	9.08	-3.43	1.15	
	3.0	148.02	165.48	-3.43	-2.28	
	4.0	128.13	307.25	-3.43	-3.71	
8	-115.68					1.50
	0	-133.69	304.25	4.25	-9.25	
	1.0	-143.65	166.29	4.25	-5.58	
	2.0	-161.30	14.22	4.25	-1.33	
	3.0	-179.28	-156.68	4.25	2.92	
	4.0	-189.16	-341.61	4.25	7.18	
9	100.12					1.02
	0	192.42	-352.19	-4.83	2.31	
	1.0	182.46	-164.04	-4.83	4.49	

PORTAL TIGA DIMENSI - UNITS: KM - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL DIST	1-2 PLANE		1-3 PLANE		AXIAL TORG
			SHEAR	MOMENT	SHEAR	MOMENT	
10	-84.54	2.0	164.61	10.00	-4.83	-3.34	1.09
		3.0	146.84	165.22	-4.83	-5.16	
		4.0	136.95	306.41	-4.83	-9.99	
		1.0	-115.74	260.22	5.12	-9.71	
		1.0	-124.46	140.09	5.12	-4.63	
		2.0	-137.41	9.49	5.12	.49	
		3.0	-150.33	-134.71	5.12	5.61	
		4.0	-157.99	-389.34	5.12	10.73	
11	100.56	1.0	152.99	-302.84	-2.67	6.67	.68
		1.0	155.27	-143.24	-2.67	4.01	
		2.0	142.31	5.88	-2.67	1.34	
		3.0	129.40	141.40	-2.67	-1.33	
		4.0	121.73	766.49	-2.67	-3.99	
		1.0	-118.32	264.62	3.25	-7.99	
12	-102.92	1.0	-126.04	142.91	3.25	-4.75	1.06
		2.0	-139.00	10.73	3.25	-1.50	
		3.0	-151.91	-135.06	3.25	1.75	
		4.0	-169.37	-291.27	3.25	4.99	
		1.0	161.40	-398.44	-4.54	8.43	
		1.0	153.68	-140.42	-4.54	3.89	
13	82.18	2.0	140.72	7.12	-4.54	-1.65	.65
		3.0	127.81	141.05	-4.54	-5.19	
		4.0	120.15	264.56	-4.54	-9.73	
		1.0	-17.00	58.04	4.58	-5.04	
		1.0	-24.72	37.65	4.58	-1.47	
		2.0	-37.68	5.79	4.58	3.11	
14	1.66	3.0	-50.59	-37.68	4.58	7.69	.98
		4.0	-59.25	-92.57	4.58	12.27	
		1.0	56.94	-110.93	2.24	-1.12	
		1.0	59.22	-47.38	2.24	1.13	
		2.0	46.27	5.70	2.24	3.37	
		3.0	33.35	45.18	2.24	3.61	
15	57.21	4.0	25.69	74.23	2.24	7.85	.86
		1.0	-22.28	70.72	-1.66	-1.70	
		1.0	-30.00	47.05	-1.66	-1.66	
		2.0	-42.96	10.31	-1.66	-3.53	
		3.0	-55.87	-38.84	-1.66	-5.19	
		4.0	-63.53	-99.01	-1.66	-6.85	
16	-59.57	1.0	61.66	-35.26	-4.00	4.72	.76
		1.0	53.95	-37.98	-4.00	.73	
		2.0	40.99	9.82	-4.00	-3.27	
		3.0	28.08	44.02	-4.00	-7.27	
		4.0	20.41	87.79	-4.00	-11.27	
		1.0	-22.28	70.72	-1.66	-1.70	



PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST ENDB	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
400								
1	15.00							15.21
		.0		116.53	-189.25	-1.0	.22	
		1.2		98.20	-63.35	-1.0	.10	
		2.3		66.11	31.35	-1.0	-.01	
		3.5		34.02	88.70	-1.0	-.12	
		4.6		13.77	114.90	-1.0	-.24	
2	-23.37							8.41
		.0		61.66	-48.67	.28	-.34	
		1.2		48.86	16.41	.28	-.01	
		2.3		27.45	60.43	.28	.31	
		3.5		6.05	79.55	.28	.63	
		4.6		-6.70	77.66	.28	.96	
3	52.41							10.07
		.0		95.63	-206.92	-1.17	-.27	
		1.2		82.84	-102.77	-1.17	-.47	
		2.3		61.43	-19.68	-1.17	-.67	
		3.5		40.02	38.51	-1.17	-.86	
		4.6		27.27	75.68	-1.17	-1.06	
4	-30.11							7.61
		.0		62.02	-49.50	.02	.60	
		1.2		49.72	16.56	.02	.63	
		2.3		28.31	61.36	.02	.65	
		3.5		6.90	81.67	.02	.68	
		4.6		-5.84	80.76	.02	.71	
5	45.67							9.27
		.0		96.49	-207.74	-1.43	.66	
		1.2		83.69	-102.62	-1.43	.17	
		2.3		62.28	-18.55	-1.43	-.32	
		3.5		40.88	40.63	-1.43	-.81	
		4.6		28.13	78.73	-1.43	-1.30	
6	-23.37							8.41
		.0		61.66	-48.67	.28	-.34	
		1.2		48.86	16.41	.28	-.01	
		2.3		27.45	60.43	.28	.31	
		3.5		6.05	79.55	.28	.63	
		4.6		-6.70	77.66	.28	.96	
7	52.41							10.07
		.0		95.63	-206.92	-1.17	-.27	
		1.2		82.84	-102.77	-1.17	-.47	
		2.3		61.43	-19.68	-1.17	-.67	
		3.5		40.02	38.51	-1.17	-.86	
		4.6		27.27	75.68	-1.17	-1.06	
8	-30.11							7.61
		.0		62.02	-49.50	.02	.60	
		1.2		49.72	16.56	.02	.63	
		2.3		28.31	61.36	.02	.65	
		3.5		6.90	81.67	.02	.68	
		4.6		-5.84	80.76	.02	.71	
9	45.67							9.27
		.0		96.49	-207.74	-1.43	.66	
		1.2		83.69	-102.62	-1.43	.17	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
476								
1	24.43							11.57
		.0		-43.24	278.39	-.24	-.21	
		1.1		-47.32	219.47	-.24	-.48	
		2.2		-103.76	125.97	-.24	-.75	
		3.3		-140.20	-8.81	-.24	-1.02	
		4.4		-164.20	-178.11	-.24	-1.29	
2	-32.25							6.28
		.0		-113.92	223.04	-.95	2.20	
		1.1		-132.38	88.75	-.95	1.15	
		2.2		-158.59	-70.91	-.95	.10	
		3.3		-184.78	-260.14	-.95	-.84	
		4.4		-203.20	-474.71	-.95	-1.89	
3	63.75							1.62
		.0		55.26	172.11	.72	-3.77	
		1.1		36.80	223.92	.72	-2.68	
		2.2		10.89	250.36	.72	-3.18	
		3.3		-15.60	247.23	.72	-1.59	
		4.4		-34.82	218.77	.72	-.60	
4	-30.10							15.16
		.0		-113.41	221.99	-1.02	3.46	
		1.1		-131.87	88.26	-1.02	2.34	
		2.2		-158.08	-70.84	-1.02	1.22	
		3.3		-184.27	-259.50	-1.02	.10	
		4.4		-202.69	-473.51	-1.02	-1.02	
5	65.90							10.50
		.0		55.77	171.06	.65	-2.50	
		1.1		37.31	223.44	.65	-1.78	
		2.2		11.11	250.44	.65	-1.07	
		3.3		-15.09	247.87	.65	-.75	
		4.4		-33.51	219.97	.65	.77	
6	-32.25							6.28
		.0		-113.92	223.04	-.95	2.20	
		1.1		-132.38	88.75	-.95	1.15	
		2.2		-158.59	-70.91	-.95	.10	
		3.3		-184.78	-260.14	-.95	-.84	
		4.4		-203.20	-474.71	-.95	-1.89	
7	63.75							1.62
		.0		55.26	172.11	.72	-3.77	
		1.1		36.80	223.92	.72	-2.68	
		2.2		10.89	250.36	.72	-3.18	
		3.3		-15.60	247.23	.72	-1.59	
		4.4		-34.82	218.77	.72	-.60	
8	-30.10							15.16
		.0		-113.41	221.99	-1.02	3.46	
		1.1		-131.87	88.26	-1.02	2.34	
		2.2		-158.08	-70.84	-1.02	1.22	
		3.3		-184.27	-259.50	-1.02	.10	
		4.4		-202.69	-473.51	-1.02	-1.02	
9	65.90							10.50
		.0		55.77	171.06	.65	-2.50	
		1.1		37.31	223.44	.65	-1.78	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORG
				SHEAR	MOMENT	SHEAR	MOMENT	
10	-30.53		2.2	11.11	250.44	.65	-1.07	2.35
			3.3	-15.09	247.87	.65	-.35	
			4.4	-33.31	219.97	.65	.37	
			.0	-93.33	197.30	-.77	1.60	
			1.1	-107.83	57.74	-.77	.75	
			2.2	-127.37	-71.38	-.77	-.09	
			3.3	-146.92	-222.48	-.77	-.94	
11	51.82		4.4	-161.38	-392.81	-.77	-1.78	-1.64
			.0	51.79	123.92	.67	-3.32	
			1.1	37.29	173.88	.67	-3.79	
			2.2	17.74	204.19	.67	-3.03	
			3.3	-1.80	212.72	.67	-1.32	
			4.4	-16.27	202.01	.67	-.89	
			.0	-93.34	166.20	-.85	3.29	
12	-27.64		1.1	-107.14	57.09	-.85	2.35	14.25
			2.2	-126.89	-71.27	-.85	1.41	
			3.3	-146.23	-221.62	-.85	.47	
			4.4	-160.70	-391.20	-.85	-.47	
			.0	52.47	122.32	.68	-1.33	
			1.1	37.97	173.03	.68	-1.19	
			2.2	19.43	204.30	.68	-.33	
13	54.70		3.3	-11.12	213.37	.68	.08	10.23
			4.4	-15.39	203.62	.68	.72	
			.0	-43.34	153.85	-.16	-2.16	
			1.1	-57.84	99.07	-.16	-2.34	
			2.2	-77.39	24.94	-.16	-2.33	
			3.3	-96.93	-71.18	-.16	-3.71	
			4.4	-111.40	-186.53	-.16	-3.99	
14	-5.07		.0	-43.34	153.85	-.16	-2.16	-12.91
			1.1	-57.84	99.07	-.16	-2.34	
			2.2	-77.39	24.94	-.16	-2.33	
			3.3	-96.93	-71.18	-.16	-3.71	
			4.4	-111.40	-186.53	-.16	-3.99	
			.0	52.47	122.32	.68	-1.33	
			1.1	37.97	173.03	.68	-1.19	
15	19.64		2.2	-133.34	107.64	.27	-3.11	-14.11
			3.3	-53.38	59.43	.27	-3.22	
			4.4	-67.85	-3.02	.27	-3.33	
			.0	52.47	122.32	.68	-1.33	
			1.1	-14.29	133.87	.27	-3.41	
			2.2	-33.34	107.64	.27	-3.11	
			3.3	-53.38	59.43	.27	-3.22	
16	4.53		4.4	-67.85	-3.02	.27	-3.33	25.72
			.0	-41.06	119.28	-.45	3.17	
			1.1	-55.56	96.91	-.45	2.57	
			2.2	-73.11	25.29	-.45	2.47	
			3.3	-94.65	-68.33	-.45	1.97	
			4.4	-109.12	-181.17	-.45	1.47	
			.0	2.49	136.17	-.02	1.83	
17	29.25		1.1	-12.01	131.70	-.02	1.91	25.52
			2.2	-31.56	107.98	-.02	1.38	
			3.3	-51.10	52.27	-.02	1.35	
			4.4	-65.37	-2.67	-.02	1.83	
			.0	2.49	136.17	-.02	1.83	
			1.1	-12.01	131.70	-.02	1.91	
			2.2	-31.56	107.98	-.02	1.38	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10	9.11		1.5	121.97	2.95	.21	.00	.58
			2.3	115.04	91.53	.21	.16	
			3.0	111.13	176.16	.21	.32	
			4.0	-98.79	156.30	1.41	-1.79	
			4.8	-101.78	81.21	1.41	-1.73	
			1.5	-106.59	3.18	1.41	.33	
11	-5.35		2.3	-111.40	-78.68	1.41	1.39	.81
			3.0	-114.39	-163.46	1.41	2.45	
			4.0	110.35	-158.73	1.78	-2.49	
			4.8	107.34	-76.98	1.78	-1.16	
			1.5	102.33	1.83	1.78	.17	
			2.3	97.72	76.81	1.78	1.50	
12	7.01		3.0	94.74	149.87	1.78	2.87	-1.70
			4.0	-96.00	132.11	-1.89	1.12	
			4.8	-95.00	79.10	-1.89	.81	
			1.5	-107.82	3.13	-1.89	.09	
			2.3	-108.82	-75.63	-1.89	-1.43	
			3.0	-111.81	-152.32	-1.89	-1.84	
13	-7.45		4.0	113.12	-162.81	-1.33	.42	-1.47
			4.8	110.12	-79.09	-1.33	.17	
			1.5	105.31	1.80	-1.33	-1.07	
			2.3	100.50	75.87	-1.33	-1.31	
			3.0	97.32	133.81	-1.33	-1.56	
			4.0	-98.83	30.95	3.99	-5.43	
14	6.51		4.8	-31.84	28.31	3.99	-2.44	2.15
			1.5	-36.85	3.74	3.99	.55	
			2.3	-41.46	-26.66	3.99	3.55	
			3.0	-44.44	-99.99	3.99	6.34	
			4.0	33.92	-43.59	4.10	-5.64	
			4.8	30.92	-19.15	4.10	-2.57	
15	2.17		1.5	26.11	3.34	4.10	.31	2.22
			2.3	21.30	20.00	4.10	3.58	
			3.0	18.32	34.74	4.10	6.58	
			4.0	-19.59	36.97	-3.01	4.27	
			4.8	-22.58	21.28	-3.01	2.01	
			1.5	-27.40	3.63	-3.01	-1.25	
16	-1.51		2.3	-32.20	-19.81	-3.01	-2.31	-2.11
			3.0	-35.18	-45.20	-3.01	-4.75	
			4.0	43.18	-37.37	-2.90	4.36	
			4.8	40.18	-26.20	-2.90	1.58	
			1.5	35.36	3.24	-2.90	-1.28	
			2.3	30.56	26.85	-2.90	-2.47	
17	-4.85		3.0	27.37	48.33	-2.90	-4.63	-2.04

PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
574								
1	86.17							-0.91
		.0		-1.26	307.30	-0.13	-0.12	
		.8		-23.40	298.57	-0.13	-0.22	
		1.5		-50.23	270.97	-0.13	-0.31	
		2.3		-77.06	223.23	-0.13	-0.41	
		3.0		-99.13	156.63	-0.13	-0.50	
2	14.34							-2.17
		.0		-25.25	214.48	-3.23	4.84	
		.8		-40.34	190.21	-3.23	2.42	
		1.5		-58.36	153.20	-3.23	-0.01	
		2.3		-76.37	102.68	-3.23	-2.43	
		3.0		-91.41	39.43	-3.23	-4.36	
3	106.27							.82
		.0		20.83	214.31	2.62	-5.02	
		.8		5.74	224.79	2.62	-3.36	
		1.5		-12.28	222.35	2.62	-1.09	
		2.3		-30.30	206.38	2.62	.88	
		3.0		-45.34	177.89	2.62	2.55	
4	12.41							-2.62
		.0		-26.09	211.33	-2.76	4.33	
		.8		-41.18	186.43	-2.76	2.76	
		1.5		-59.19	148.80	-2.76	.59	
		2.3		-77.21	97.64	-2.76	-1.36	
		3.0		-92.25	33.76	-2.76	-3.46	
5	104.34							.47
		.0		19.99	211.36	3.10	-5.04	
		.8		4.90	221.02	3.10	-2.71	
		1.5		-13.12	217.94	3.10	-0.39	
		2.3		-31.13	201.34	3.10	1.93	
		3.0		-46.18	172.03	3.10	4.25	
6	14.34							-2.17
		.0		-25.25	214.48	-3.23	4.84	
		.8		-40.34	190.21	-3.23	2.42	
		1.5		-58.36	153.20	-3.23	-0.01	
		2.3		-76.37	102.68	-3.23	-2.43	
		3.0		-91.41	39.43	-3.23	-4.36	
7	106.27							.82
		.0		20.83	214.31	2.62	-5.02	
		.8		5.74	224.79	2.62	-3.36	
		1.5		-12.28	222.35	2.62	-1.09	
		2.3		-30.30	206.38	2.62	.88	
		3.0		-45.34	177.89	2.62	2.55	
8	12.41							-2.62
		.0		-26.09	211.33	-2.76	4.33	
		.8		-41.18	186.43	-2.76	2.76	
		1.5		-59.19	148.80	-2.76	.59	
		2.3		-77.21	97.64	-2.76	-1.36	
		3.0		-92.25	33.76	-2.76	-3.46	
9	104.34							.47
		.0		19.99	211.36	3.10	-5.04	
		.8		4.90	221.02	3.10	-2.71	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD DOMB	AXIAL FORCE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
10	4.50	1.5	-13.12	217.94	3.10	-1.39	-1.86
		2.3	-31.13	201.34	3.10	1.93	
		3.0	-44.18	172.03	3.10	4.25	
		4.0	-22.25	153.91	-2.87	4.17	
		4.8	-32.98	135.42	-2.87	2.01	
		1.5	-45.62	105.94	-2.87	-1.14	
		2.3	-58.27	66.98	-2.87	-2.29	
11	83.35	3.0	-68.97	19.05	-2.87	-4.44	.77
		4.0	17.28	155.94	2.16	-4.30	
		4.8	6.54	145.08	2.16	-2.68	
		1.5	-5.10	135.25	2.16	-1.06	
		2.3	-18.74	155.93	2.16	.56	
		3.0	-39.45	137.65	2.16	2.18	
		4.0	-23.37	151.70	-2.23	4.14	
12	1.91	4.8	-34.10	130.36	-2.23	2.47	-2.27
		1.5	-44.75	100.05	-2.23	.79	
		2.3	-59.39	60.24	-2.23	-1.88	
		3.0	-70.09	11.47	-2.23	-2.56	
		4.0	-23.37	151.70	-2.23	4.14	
		4.8	-34.10	130.36	-2.23	2.47	
		1.5	-44.75	100.05	-2.23	.79	
13	80.77	2.3	-59.39	60.24	-2.23	-1.88	.30
		3.0	-70.09	11.47	-2.23	-2.56	
		4.0	-23.37	151.70	-2.23	4.14	
		4.8	-34.10	130.36	-2.23	2.47	
		1.5	-44.75	100.05	-2.23	.79	
		2.3	-59.39	60.24	-2.23	-1.88	
		3.0	-70.09	11.47	-2.23	-2.56	
14	35.10	3.0	-39.45	137.65	2.16	2.18	-1.36
		4.0	17.28	155.94	2.16	-4.30	
		4.8	6.54	145.08	2.16	-2.68	
		1.5	-5.10	135.25	2.16	-1.06	
		2.3	-18.74	155.93	2.16	.56	
		3.0	-39.45	137.65	2.16	2.18	
		4.0	-23.37	151.70	-2.23	4.14	
15	58.77	4.8	-34.10	130.36	-2.23	2.47	.41
		1.5	-44.75	100.05	-2.23	.79	
		2.3	-59.39	60.24	-2.23	-1.88	
		3.0	-70.09	11.47	-2.23	-2.56	
		4.0	-23.37	151.70	-2.23	4.14	
		4.8	-34.10	130.36	-2.23	2.47	
		1.5	-44.75	100.05	-2.23	.79	
16	26.50	2.3	-59.39	60.24	-2.23	-1.88	-1.92
		3.0	-70.09	11.47	-2.23	-2.56	
		4.0	-23.37	151.70	-2.23	4.14	
		4.8	-34.10	130.36	-2.23	2.47	
		1.5	-44.75	100.05	-2.23	.79	
		2.3	-59.39	60.24	-2.23	-1.88	
		3.0	-70.09	11.47	-2.23	-2.56	
17	50.16	3.0	-44.18	172.03	3.10	4.25	-1.15
		4.0	-22.25	153.91	-2.87	4.17	
		4.8	-32.98	135.42	-2.87	2.01	
		1.5	-45.62	105.94	-2.87	-1.14	
		2.3	-58.27	66.98	-2.87	-2.29	
		3.0	-68.97	19.05	-2.87	-4.44	
		4.0	-23.37	151.70	-2.23	4.14	

PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
654						
1	42.02					10.18
	.0	182.78	-368.67	-1.63	.93	
	.8	172.04	-334.86	-1.63	.46	
	1.5	152.00	-311.10	-1.63	-.01	
	2.3	131.97	-28.86	-1.63	-.49	
	3.0	121.51	87.36	-1.63	-.95	
2	-9.44					5.19
	.0	96.14	-88.79	-3.56	6.15	
	.8	88.12	-19.20	-3.56	3.48	
	1.5	74.05	41.77	-3.56	.81	
	2.3	59.98	91.87	-3.56	-1.86	
	3.0	52.00	133.37	-3.56	-4.53	
3	64.42					10.20
	.0	163.91	-436.12	3.10	-5.96	
	.8	155.89	-315.71	3.10	-3.63	
	1.5	141.81	-203.91	3.10	-1.31	
	2.3	127.75	-102.98	3.10	1.02	
	3.0	119.77	-10.66	3.10	3.34	
4	-7.16					3.24
	.0	96.07	-87.34	-3.90	7.13	
	.8	88.05	-18.30	-3.90	4.20	
	1.5	73.98	42.62	-3.90	1.27	
	2.3	59.91	92.67	-3.90	-1.66	
	3.0	51.93	134.11	-3.90	-4.58	
5	66.70					8.25
	.0	163.84	-435.17	2.75	-4.97	
	.8	155.82	-314.80	2.75	-2.91	
	1.5	141.74	-203.06	2.75	-.84	
	2.3	127.68	-102.19	2.75	1.23	
	3.0	119.70	-9.92	2.75	3.29	
6	-9.44					5.19
	.0	96.14	-88.79	-3.56	6.15	
	.8	88.12	-19.20	-3.56	3.48	
	1.5	74.05	41.77	-3.56	.81	
	2.3	59.98	91.87	-3.56	-1.86	
	3.0	52.00	133.37	-3.56	-4.53	
7	64.42					10.20
	.0	163.91	-436.12	3.10	-5.96	
	.8	155.89	-315.71	3.10	-3.63	
	1.5	141.81	-203.91	3.10	-1.31	
	2.3	127.75	-102.98	3.10	1.02	
	3.0	119.77	-10.66	3.10	3.34	
8	-7.16					3.24
	.0	96.07	-87.34	-3.90	7.13	
	.8	88.05	-18.30	-3.90	4.20	
	1.5	73.98	42.62	-3.90	1.27	
	2.3	59.91	92.67	-3.90	-1.66	
	3.0	51.93	134.11	-3.90	-4.58	
9	66.70					8.25
	.0	163.84	-435.17	2.75	-4.97	
	.8	155.82	-314.80	2.75	-2.91	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10	-12.83		1.5	141.74	-203.06	2.75	-1.84	3.80
			2.3	127.68	-102.19	2.75	1.23	
			3.0	119.70	-9.92	2.75	3.29	
			.0	67.07	-45.19	-2.89	4.92	
			.8	60.90	3.13	-2.89	2.75	
			1.5	50.61	45.05	-2.89	.38	
11	50.52		2.3	40.33	79.04	-2.89	-1.59	8.10
			3.0	34.17	106.64	-2.89	-3.76	
			.0	125.20	-343.11	2.82	-5.46	
			.8	119.02	-251.19	2.82	-3.35	
			1.5	108.73	-165.68	2.82	-1.24	
			2.3	98.45	-88.09	2.82	.88	
12	-9.78		3.0	92.30	-15.90	2.82	2.82	1.18
			.0	86.98	-43.92	-3.36	6.24	
			.8	80.50	4.34	-3.36	3.72	
			1.5	50.51	46.19	-3.36	1.20	
			2.3	40.23	60.11	-3.36	-1.31	
			3.0	34.08	107.64	-3.36	-3.83	
13	53.58		3.0	34.08	107.64	-3.36	-3.83	3.49
			.0	125.11	-341.84	2.36	-4.14	
			.8	118.93	-249.99	2.36	-2.38	
			1.5	108.64	-164.54	2.36	-.61	
			2.3	98.36	-87.02	2.36	1.16	
			3.0	92.20	-15.90	2.36	2.82	
14	5.78		3.0	92.20	-15.90	2.36	2.82	8.34
			.0	87.53	-150.94	-.36	-1.25	
			.8	81.35	-87.28	-.36	-.52	
			1.5	71.06	-30.02	-.36	-.79	
			2.3	60.78	19.31	-.36	-1.05	
			3.0	54.62	62.25	-.36	-1.32	
15	24.79		3.0	54.62	62.25	-.36	-1.32	9.64
			.0	104.97	-240.35	1.36	-3.27	
			.8	98.79	-163.60	1.36	-2.35	
			1.5	88.50	-93.26	1.36	-1.33	
			2.3	78.22	-30.85	1.36	-.31	
			3.0	72.07	25.17	1.36	.71	
16	15.95		3.0	72.07	25.17	1.36	.71	-1.35
			.0	87.21	-146.68	-1.90	4.14	
			.8	81.03	-93.25	-1.90	2.72	
			1.5	70.75	-26.23	-1.90	1.30	
			2.3	60.47	22.87	-1.90	-.13	
			3.0	54.31	65.37	-1.90	-1.35	
17	34.97		3.0	54.31	65.37	-1.90	-1.35	.54
			.0	104.66	-236.09	-.18	1.03	
			.8	98.48	-159.58	-.18	.89	
			1.5	88.19	-89.47	-.18	.75	
			2.3	77.91	-27.29	-.18	.62	
			3.0	71.75	28.50	-.18	.48	



PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COND	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORG
				SHEAR	MOMENT	SHEAR	MOMENT	
789								
1		53.05						14.05
			.0	134.46	-249.54	.57	-1.01	
			.8	126.52	-151.16	.57	-1.55	
			1.5	110.36	-51.32	.57	-1.16	
			2.3	94.26	14.40	.57	.27	
			3.0	86.40	81.63	.57	.69	
2		43.65						-2.93
			.0	106.12	-175.25	-9.73	16.93	
			.8	100.09	-97.58	-9.73	9.63	
			1.5	88.73	-26.44	-9.73	2.34	
			2.3	77.40	35.52	-9.73	-4.96	
			3.0	71.43	91.00	-9.73	-12.25	
3		35.76						24.44
			.0	78.05	-136.16	10.27	-18.06	
			.8	72.02	-79.55	10.27	-10.36	
			1.5	60.66	-29.46	10.27	-2.65	
			2.3	49.33	11.45	10.27	5.05	
			3.0	43.36	45.87	10.27	12.76	
4		39.36						-4.20
			.0	116.01	-221.57	-9.47	16.64	
			.8	109.98	-136.49	-9.47	9.54	
			1.5	98.62	-37.93	-9.47	2.44	
			2.3	87.29	11.45	-9.47	-4.66	
			3.0	81.32	74.34	-9.47	-11.76	
5		31.47						23.18
			.0	97.94	-182.49	10.53	-18.35	
			.8	81.91	-118.46	10.53	-10.45	
			1.5	70.35	-50.96	10.53	-2.55	
			2.3	59.22	-12.63	10.53	5.35	
			3.0	53.25	29.21	10.53	13.25	
6		43.65						-2.93
			.0	106.12	-175.25	-9.73	16.93	
			.8	100.09	-97.58	-9.73	9.63	
			1.5	88.73	-26.44	-9.73	2.34	
			2.3	77.40	35.52	-9.73	-4.96	
			3.0	71.43	91.00	-9.73	-12.25	
7		35.76						24.44
			.0	78.05	-136.16	10.27	-18.06	
			.8	72.02	-79.55	10.27	-10.36	
			1.5	60.66	-29.46	10.27	-2.65	
			2.3	49.33	11.45	10.27	5.05	
			3.0	43.36	45.87	10.27	12.76	
8		39.36						-4.20
			.0	116.01	-221.57	-9.47	16.64	
			.8	109.98	-136.49	-9.47	9.54	
			1.5	98.62	-37.93	-9.47	2.44	
			2.3	87.29	11.45	-9.47	-4.66	
			3.0	81.32	74.34	-9.47	-11.76	
9		31.47						23.18
			.0	97.94	-182.49	10.53	-18.35	
			.8	81.91	-118.46	10.53	-10.45	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10	33.92		1.5	70.55	-60.96	10.53	-2.55	-3.33
			2.3	59.22	-12.63	10.53	5.35	
			3.0	53.25	29.21	10.53	13.25	
			.0	73.03	-119.84	-8.45	14.59	
			.8	73.33	-61.85	-8.45	8.34	
			1.5	65.02	-9.75	-8.45	2.00	
			2.3	56.72	35.68	-8.45	-4.34	
11	27.14		3.0	52.06	76.24	-8.45	-10.68	20.15
			.0	53.25	-35.32	8.70	-15.33	
			.8	49.26	-46.39	8.70	-8.81	
			1.5	40.94	-12.34	8.70	-3.28	
			2.3	32.64	15.02	8.70	4.24	
			3.0	27.98	37.33	8.70	10.77	
		12	28.17		.0	27.98	37.33	
	.8			21.27	-160.88	-8.10	14.39	
	1.5			86.58	-113.96	-8.10	8.21	
	2.3			78.26	-31.22	-8.10	2.13	
	3.0			69.96	3.43	-8.10	-3.95	
	3.0			65.30	33.93	-8.10	-10.02	
13	21.40				.0	65.30	33.93	-8.10
			.8	67.19	-147.36	9.05	-15.72	
			1.5	62.30	-98.50	9.05	-8.94	
			2.3	54.18	-54.32	9.05	-3.15	
			3.0	45.88	-17.22	9.05	4.64	
			3.0	41.33	15.22	9.05	11.43	
		14	38.25		.0	41.33	15.22	9.05
	.8			54.16	-34.76	-2.86	4.53	
	1.5			49.46	4.32	-2.86	2.46	
	2.3			41.15	38.53	-2.86	.34	
	3.0			32.85	66.05	-2.86	-1.80	
	3.0			28.19	88.71	-2.86	-3.94	
15	35.22				.0	28.19	88.71	-2.86
			.8	46.93	-24.70	2.29	-4.39	
			1.5	42.24	8.97	2.29	-2.66	
			2.3	33.92	37.75	2.29	-1.94	
			3.0	25.62	59.65	2.29	.77	
			3.0	20.95	77.09	2.29	2.49	
		16	19.09		.0	20.95	77.09	2.29
	.8			98.29	-241.50	-1.69	3.33	
	1.5			93.59	-169.32	-1.69	2.06	
	2.3			85.28	-102.01	-1.69	.79	
	3.0			76.98	-41.40	-1.69	-1.48	
	3.0			72.32	14.66	-1.69	-1.75	
17	17.06				.0	72.32	14.66	-1.69
			.8	91.06	-231.44	3.45	-3.67	
			1.5	86.37	-164.68	3.45	-3.08	
			2.3	78.05	-102.79	3.45	-1.49	
			3.0	69.75	-47.59	3.45	2.10	
			3.0	65.10	2.75	3.45	4.69	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
797								
1	35.95							-1.37
		.0		124.04	-193.46	.26	-4.72	
		1.5		98.45	-22.52	.26	-4.33	
		3.0		48.71	88.82	.26	.06	
		4.5		-9.90	123.65	.26	.45	
		6.0		-25.51	89.26	.26	.34	
2	27.24							-9.10
		.0		75.19	-85.87	-4.78	14.14	
		1.5		56.99	15.52	-4.78	6.96	
		3.0		24.81	77.43	-4.78	-1.21	
		4.5		-7.30	89.95	-4.78	-7.38	
		6.0		-25.34	63.23	-4.78	-14.55	
3	37.29							8.45
		.0		73.13	-75.95	6.96	-20.99	
		1.5		54.93	22.36	6.96	-10.54	
		3.0		32.75	81.18	6.96	-1.10	
		4.5		-9.36	90.62	6.96	10.34	
		6.0		-27.40	60.81	6.96	20.78	
4	11.83							-8.99
		.0		91.99	-178.73	-6.63	20.08	
		1.5		73.79	-82.14	-6.63	10.13	
		3.0		41.81	34.97	-6.63	.17	
		4.5		9.50	72.70	-6.63	-9.78	
		6.0		-8.34	71.17	-6.63	-19.73	
5	21.89							8.56
		.0		89.94	-168.81	5.11	-15.04	
		1.5		71.73	-45.30	5.11	-7.38	
		3.0		39.35	58.73	5.11	.28	
		4.5		7.44	73.37	5.11	7.94	
		6.0		-10.50	68.76	5.11	15.80	
6	27.24							-9.10
		.0		75.19	-85.87	-4.78	14.14	
		1.5		56.99	15.52	-4.78	6.96	
		3.0		24.81	77.43	-4.78	-1.21	
		4.5		-7.30	89.95	-4.78	-7.38	
		6.0		-25.34	63.23	-4.78	-14.55	
7	37.29							8.45
		.0		73.13	-75.95	6.96	-20.99	
		1.5		54.93	22.36	6.96	-10.54	
		3.0		32.75	81.18	6.96	-1.10	
		4.5		-9.36	90.62	6.96	10.34	
		6.0		-27.40	60.81	6.96	20.78	
8	11.83							-8.99
		.0		91.99	-178.73	-6.63	20.08	
		1.5		73.79	-82.14	-6.63	10.13	
		3.0		41.81	34.97	-6.63	.17	
		4.5		9.50	72.70	-6.63	-9.78	
		6.0		-8.34	71.17	-6.63	-19.73	
9	21.89							8.56
		.0		89.94	-168.81	5.11	-15.04	
		1.5		71.73	-45.30	5.11	-7.38	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORS
				SHEAR	MOMENT	SHEAR	MOMENT	
10	23.52		3.0	39.55	38.73	5.11	.28	-7.80
			4.5	7.44	73.37	5.11	7.94	
			6.0	-10.60	68.76	5.11	15.60	
			1.0	46.94	-29.71	-3.69	10.79	
			1.5	33.62	32.07	-3.69	5.26	
			3.0	11.35	66.52	-3.69	-.28	
11	32.15		4.5	-9.86	67.65	-3.69	-5.81	7.25
			6.0	-23.09	41.38	-3.69	-11.34	
			1.0	45.18	-21.20	6.38	-19.33	
			1.5	31.86	37.94	6.38	-9.76	
			3.0	10.09	69.74	6.38	-.19	
			4.5	-11.63	66.22	6.38	9.39	
12	2.89		6.0	-24.35	39.51	6.38	19.96	-7.65
			1.0	69.44	-134.07	-6.17	18.74	
			1.5	56.12	-58.53	-6.17	9.49	
			3.0	24.35	9.66	-6.17	-.23	
			4.5	12.64	44.54	-6.17	-9.02	
			6.0	-.39	62.22	-6.17	-13.28	
13	11.52		6.0	-3.39	62.22	-6.17	-13.28	7.40
			1.0	67.68	-145.36	3.90	-11.38	
			1.5	54.36	-52.67	3.90	-5.53	
			3.0	32.09	12.89	3.90	.32	
			4.5	10.87	45.12	3.90	6.17	
			6.0	-2.35	50.15	3.90	12.02	
14	50.60		6.0	-2.35	50.15	3.90	12.02	-2.70
			1.0	20.09	118.28	2.73	-9.03	
			1.5	6.76	139.79	2.73	-4.94	
			3.0	-13.00	133.95	2.73	-.34	
			4.5	-36.72	94.60	2.73	3.26	
			6.0	-49.94	28.44	2.73	7.35	
15	53.19		6.0	-49.94	28.44	2.73	7.35	1.81
			1.0	19.36	120.84	5.75	-18.07	
			1.5	6.23	141.55	5.75	-9.44	
			3.0	-13.03	134.92	5.75	-.81	
			4.5	-37.25	94.97	5.75	7.82	
			6.0	-50.47	27.82	5.75	15.45	
16	-16.15		6.0	-50.47	27.82	5.75	15.45	-2.31
			1.0	85.06	-296.10	-5.54	17.48	
			1.5	81.74	-162.14	-5.54	9.17	
			3.0	39.98	-55.71	-5.54	.26	
			4.5	38.26	17.80	-5.54	-7.45	
			6.0	35.03	61.91	-5.54	-15.76	
17	-15.56		6.0	35.03	61.91	-5.54	-15.76	2.31
			1.0	94.53	-297.33	-2.52	3.44	
			1.5	81.21	-160.38	-2.52	4.66	
			3.0	39.45	-54.64	-2.52	.89	
			4.5	37.73	17.97	-2.52	-2.89	
			6.0	24.50	63.29	-2.52	-5.67	

PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
799							
1	68.51						-6.89
		.0	169.51	-271.78	9.15	-8.34	
		.5	155.33	-190.46	9.15	-3.76	
		1.0	139.42	-116.90	9.15	.81	
		1.5	121.51	-52.04	9.15	5.39	
		2.0	107.33	5.06	9.15	9.97	
2	29.41						-42.33
		.0	74.11	-107.61	4.33	-2.19	
		.5	64.64	-72.84	4.33	-1.03	
		1.0	53.39	-43.26	4.33	2.14	
		1.5	42.13	-19.46	4.33	4.30	
		2.0	32.66	-6.83	4.33	6.47	
3	56.70						30.50
		.0	139.06	-182.38	9.90	-13.76	
		.5	129.59	-115.14	9.90	-8.42	
		1.0	119.34	-33.09	9.90	-3.47	
		1.5	107.09	3.20	9.90	1.48	
		2.0	97.61	34.30	9.90	6.43	
4	36.66						-39.54
		.0	91.13	-187.04	2.37	2.24	
		.5	81.66	-143.77	2.37	3.42	
		1.0	70.41	-105.68	2.37	4.61	
		1.5	59.15	-73.36	2.37	5.79	
		2.0	49.68	-46.23	2.37	6.98	
5	63.97						33.29
		.0	156.09	-261.81	7.93	-8.33	
		.5	146.61	-186.06	7.93	-4.97	
		1.0	135.36	-115.50	7.93	-1.00	
		1.5	124.11	-50.71	7.93	2.97	
		2.0	114.63	8.90	7.93	6.93	
6	29.41						-42.33
		.0	74.11	-107.61	4.33	-2.19	
		.5	64.64	-72.84	4.33	-1.03	
		1.0	53.39	-43.26	4.33	2.14	
		1.5	42.13	-19.46	4.33	4.30	
		2.0	32.66	-6.83	4.33	6.47	
7	56.70						30.50
		.0	139.06	-182.38	9.90	-13.76	
		.5	129.59	-115.14	9.90	-8.42	
		1.0	119.34	-33.09	9.90	-3.47	
		1.5	107.09	3.20	9.90	1.48	
		2.0	97.61	34.30	9.90	6.43	
8	36.66						-39.54
		.0	91.13	-187.04	2.37	2.24	
		.5	81.66	-143.77	2.37	3.42	
		1.0	70.41	-105.68	2.37	4.61	
		1.5	59.15	-73.36	2.37	5.79	
		2.0	49.68	-46.23	2.37	6.98	
9	63.97						33.29
		.0	156.09	-261.81	7.93	-8.33	
		.5	146.61	-186.06	7.93	-4.97	

PORTAL TIGA DINESNSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ		
				SHEAR	MOMENT	SHEAR	MOMENT			
10	16.66		1.0	135.36	-115.50	7.93	-1.00	-36.21		
			1.5	124.11	-50.71	7.93	2.97			
			2.0	114.63	3.90	7.93	6.93			
			1.0	42.37	-43.24	3.22	-2.06			
			1.5	35.75	-26.26	3.22	-1.45			
			2.0	27.92	-10.29	3.22	1.17			
11	40.06		1.0	20.10	1.67	3.22	2.76	26.27		
			1.5	20.10	1.67	3.22	4.39			
			2.0	13.48	10.01	3.22	4.39			
			1.0	59.08	-109.97	3.00	-11.34			
			1.5	91.46	-62.54	3.00	-7.64			
			2.0	83.84	-18.71	3.00	-3.84			
12	26.38		1.0	75.81	21.10	3.00	1.36	-32.47		
			1.5	59.19	37.30	3.00	4.33			
			2.0	63.16	-132.22	1.60	3.38			
			1.0	58.54	-121.24	1.60	4.17			
			1.5	50.72	-93.87	1.60	4.47			
			2.0	42.89	-70.52	1.60	4.77			
13	49.79		1.0	76.28	-30.78	1.60	5.07	36.01		
			1.5	120.37	-216.35	3.37	-7.70			
			2.0	114.26	-137.52	3.37	-3.02			
			1.0	106.43	-102.30	3.37	-1.74			
			1.5	98.61	-51.09	3.37	2.38			
			2.0	91.79	-3.48	3.37	3.03			
14	13.50		1.0	81.99	-3.48	3.37	3.03	-18.70		
			1.5	35.78	35.77	7.96	-12.33			
			2.0	28.87	71.80	7.96	-8.38			
			1.0	20.84	84.23	7.96	-4.37			
			1.5	13.02	92.65	7.96	-1.39			
			2.0	6.40	97.15	7.96	1.39			
15	20.37		1.0	37.15	97.15	7.96	1.39	1.05		
			1.5	32.00	55.57	9.39	-10.21			
			2.0	45.39	60.82	9.39	-10.51			
			1.0	37.36	31.70	9.39	-5.21			
			1.5	29.73	98.48	9.39	-1.12			
			2.0	23.12	111.24	9.39	3.38			
16	45.92		1.0	111.24	-228.71	-1.80	7.44	-5.25		
			1.5	104.62	-244.70	-1.80	7.84			
			2.0	96.90	-194.29	-1.80	6.64			
			1.0	82.35	-105.12	-1.80	3.84			
			1.5	82.35	-105.12	88.97	-147.70		-1.80	6.24
			2.0	82.35	-105.12	-1.80	3.84			
17	52.94		1.0	111.24	-228.71	-1.80	7.44	12.50		
			1.5	104.62	-244.70	-1.80	7.84			
			2.0	96.90	-194.29	-1.80	6.64			
			1.0	82.35	-105.12	-1.80	3.84			
			1.5	82.35	-105.12	88.97	-147.70		-1.80	6.24
			2.0	82.35	-105.12	-1.80	3.84			

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
821								
1	46.18							-3.52
		.0		-56.97	106.30	.79	-1.17	
		.8		-62.87	62.32	.79	-1.57	
		1.6		-77.06	10.42	.79	.02	
		2.4		-91.50	-53.35	.79	.61	
		3.0		-97.01	-124.20	.79	1.21	
2	46.13							3.14
		.0		-46.53	91.87	-0.09	-1.44	
		.8		-50.32	45.90	-0.09	-1.51	
		1.6		-59.68	5.00	-0.09	-1.58	
		2.4		-69.00	-43.60	-0.09	-1.65	
		3.0		-72.73	-97.10	-0.09	-1.72	
3	47.09							-3.66
		.0		-45.45	90.28	-0.67	1.64	
		.8		-49.24	45.11	-0.67	1.14	
		1.6		-58.60	5.02	-0.67	.64	
		2.4		-67.92	-42.77	-0.67	.14	
		3.0		-71.50	-95.46	-0.67	-0.36	
4	17.09							2.98
		.0		-35.74	69.26	1.82	-3.35	
		.8		-39.53	41.38	1.82	-1.98	
		1.6		-48.89	9.07	1.82	-1.01	
		2.4		-58.21	-31.94	1.82	.75	
		3.0		-61.84	-77.34	1.82	2.12	
5	18.04							-3.85
		.0		-34.66	67.66	1.25	-1.27	
		.8		-38.45	40.59	1.25	-1.33	
		1.6		-47.81	9.39	1.25	.60	
		2.4		-57.13	-31.11	1.25	1.54	
		3.0		-60.86	-75.70	1.25	2.48	
6	46.13							3.14
		.0		-46.53	91.87	-0.09	-1.44	
		.8		-50.32	45.90	-0.09	-1.51	
		1.6		-59.68	5.00	-0.09	-1.58	
		2.4		-69.00	-43.60	-0.09	-1.65	
		3.0		-72.73	-97.10	-0.09	-1.72	
7	47.09							-3.66
		.0		-45.45	90.28	-0.67	1.64	
		.8		-49.24	45.11	-0.67	1.14	
		1.6		-58.60	5.02	-0.67	.64	
		2.4		-67.92	-42.77	-0.67	.14	
		3.0		-71.50	-95.46	-0.67	-0.36	
8	17.09							2.98
		.0		-35.74	69.26	1.82	-3.35	
		.8		-39.53	41.38	1.82	-1.98	
		1.6		-48.89	9.07	1.82	-1.01	
		2.4		-58.21	-31.94	1.82	.75	
		3.0		-61.84	-77.34	1.82	2.12	
9	18.04							-3.85
		.0		-34.66	67.66	1.25	-1.27	
		.8		-38.45	40.59	1.25	-1.33	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10	42.27		1.5	-47.81	8.59	1.25	.60	2.79
			2.3	-37.13	-31.11	1.25	1.54	
			3.0	-40.86	-75.70	1.25	2.48	
			.0	-37.74	63.78	-1.50	.41	
			.8	-40.43	34.69	-1.50	-1.04	
			1.5	-46.74	2.33	-1.50	-1.49	
11	43.10		2.3	-53.03	-35.41	-1.60	-1.94	-3.07
			3.0	-55.67	-76.40	-1.60	-1.39	
			.0	-55.82	62.41	-1.09	2.20	
			.8	-39.50	34.02	-1.09	1.38	
			1.5	-45.81	2.25	-1.09	.36	
			2.3	-52.10	-34.70	-1.09	-1.27	
12	3.38		3.0	-54.75	-74.99	-1.09	-1.09	2.57
			.0	-33.29	46.89	1.97	-3.49	
			.8	-25.98	25.64	1.97	-2.01	
			1.5	-33.29	7.02	1.97	-1.04	
			2.3	-38.88	-19.79	1.97	.94	
			3.0	-41.22	-49.94	1.97	2.42	
13	4.20		.0	-22.37	45.32	1.48	-1.71	-5.29
			.8	-25.05	27.97	1.48	-1.50	
			1.5	-31.36	7.04	1.48	.31	
			2.3	-37.65	-19.07	1.48	1.51	
			3.0	-40.30	-48.33	1.48	2.72	
			.0	-34.27	85.00	-3.77	3.39	
14	87.92		.8	-56.96	41.61	-3.77	2.76	1.00
			1.5	-63.37	-3.34	-3.77	-1.07	
			2.3	-69.55	-53.38	-3.77	-2.89	
			3.0	-77.70	-106.76	-3.77	-5.72	
			.0	-33.99	82.59	-3.92	6.13	
			.8	-56.66	41.31	-3.92	3.19	
15	88.16		1.5	-62.99	-3.34	-3.92	.25	-1.76
			2.3	-69.28	-53.16	-3.92	-2.69	
			3.0	-71.92	-106.34	-3.92	-5.63	
			.0	-35.12	76.72	4.79	-7.42	
			.8	-6.60	21.35	4.79	-3.83	
			1.5	-15.11	12.61	4.79	-1.33	
16	-41.87		2.3	3.3	3.3	-31.40	-1.32	.26
			3.0	-24.05	-18.39	4.79	6.96	
			.0	-5.84	26.31	4.65	-6.98	
			.8	-8.53	21.15	4.65	-3.40	
			1.5	-14.84	12.31	4.65	.08	
			2.3	-21.13	-1.10	4.65	3.57	
17	-41.44		3.0	-23.77	-18.17	4.65	7.05	-1.50
			.0	-3.84	26.31	4.65	-6.98	
			.8	-8.53	21.15	4.65	-3.40	
			1.5	-14.84	12.31	4.65	.08	
			2.3	-21.13	-1.10	4.65	3.57	
			3.0	-23.77	-18.17	4.65	7.05	

4.79 3.36



PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
836								
1	-1.12							1.87
		.0		43.14	-73.74	-1.08	.24	
		1.5		32.43	-15.55	-1.08	.12	
		3.0		11.34	17.96	-1.08	.01	
		4.5		-9.74	18.47	-1.08	-1.10	
		6.0		-20.40	-3.85	-1.08	-1.32	
2	9.28							1.20
		.0		30.16	-49.29	-1.49	1.82	
		1.5		22.30	-8.00	-1.49	.88	
		3.0		7.94	13.10	-1.49	.14	
		4.5		-6.40	15.83	-1.49	-1.89	
		6.0		-14.23	-1.80	-1.49	-1.33	
3	-9.86							1.39
		.0		27.86	-44.25	.43	-1.14	
		1.5		20.00	-7.41	.43	-1.79	
		3.0		5.65	12.25	.43	-1.14	
		4.5		-8.70	9.53	.43	.81	
		6.0		-16.53	-10.34	.43	1.16	
4	9.66							1.16
		.0		31.59	-35.58	-1.34	1.77	
		1.5		23.73	-13.14	-1.34	.86	
		3.0		9.38	12.12	-1.34	.15	
		4.5		-4.97	15.00	-1.34	-1.88	
		6.0		-12.79	.73	-1.34	-1.47	
5	-9.48							1.35
		.0		29.20	-51.34	.38	-1.08	
		1.5		21.44	-12.54	.38	-1.71	
		3.0		7.09	9.27	.38	-1.08	
		4.5		-7.27	8.70	.38	.44	
		6.0		-10.09	-9.01	.38	1.12	
6	9.28							1.20
		.0		30.16	-49.29	-1.49	1.82	
		1.5		22.30	-8.00	-1.49	.88	
		3.0		7.94	13.10	-1.49	.14	
		4.5		-6.40	15.83	-1.49	-1.89	
		6.0		-14.23	-1.80	-1.49	-1.33	
7	-9.86							1.39
		.0		27.86	-44.25	.43	-1.14	
		1.5		20.00	-7.41	.43	-1.79	
		3.0		5.65	12.25	.43	-1.14	
		4.5		-8.70	9.53	.43	.81	
		6.0		-16.53	-10.34	.43	1.16	
8	9.66							1.16
		.0		31.59	-35.58	-1.34	1.77	
		1.5		23.73	-13.14	-1.34	.86	
		3.0		9.38	12.12	-1.34	.15	
		4.5		-4.97	15.00	-1.34	-1.88	
		6.0		-12.79	.73	-1.34	-1.47	
9	-9.48							1.35
		.0		29.20	-51.34	.38	-1.08	
		1.5		21.44	-12.54	.38	-1.71	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID DMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
10	3.0	7.08	9.27	.38	-1.13	.85
	4.5	-7.27	8.70	.38	.44	
	6.0	-15.09	-2.01	.38	1.02	
	7.87					
	1.0	21.39	-32.14	-.40	1.33	
	1.5	15.43	-3.91	-.40	.72	
11	3.0	3.23	11.85	-.40	.12	1.01
	4.5	-4.97	11.77	-.40	-.48	
	6.0	-10.91	-.76	-.40	-1.08	
	-8.55					
	1.0	17.42	-28.68	.39	-1.29	
	1.5	13.46	-3.40	.39	-.70	
12	3.0	3.26	9.41	.39	-.12	.80
	4.5	-6.94	6.36	.39	.46	
	6.0	-12.88	-2.12	.39	1.05	
	8.38					
	1.0	25.31	-41.90	-.47	1.33	
	1.5	17.35	-10.79	-.47	.53	
13	3.0	7.10	7.86	-.47	.15	.76
	4.5	-3.05	10.56	-.47	-.37	
	6.0	-8.99	1.02	-.47	-1.27	
	-8.04					
	1.0	21.54	-38.14	.52	-1.02	
	1.5	15.38	-10.28	.52	-.50	
14	3.0	3.18	5.42	.52	-.11	.76
	4.5	-5.02	5.26	.52	.37	
	6.0	-10.95	-2.34	.52	.86	
	1.32					
	1.0	13.46	-19.55	-.05	.17	
	1.5	12.49	4.28	-.05	.10	
15	3.0	2.29	15.80	-.05	.03	1.01
	4.5	-7.90	11.16	-.05	-.04	
	6.0	-13.84	-5.76	-.05	-.12	
	-3.40					
	1.0	17.87	-18.31	.19	-.31	
	1.5	11.90	4.43	.19	-.33	
16	3.0	1.70	14.92	.19	-.04	.80
	4.5	-5.49	9.34	.19	.24	
	6.0	-11.43	-3.27	.19	.82	
	3.23					
	1.0	24.87	-32.07	-.27	.35	
	1.5	15.90	-18.63	-.27	.45	
17	3.0	3.70	7.33	-.27	.05	.85
	4.5	-1.49	7.48	-.27	-.35	
	6.0	-7.43	1.17	-.27	-.73	
	-1.70					
	1.0	24.28	-31.03	-.03	.07	
	1.5	18.31	-18.47	-.03	.02	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL DIST FORCE ENDI	1-3 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
839							
1	3.57						-3.73
		0	80.98	-23.42	-2.22	1.39	
		1.0	56.58	-23.88	-2.22	1.16	
		2.0	24.23	16.90	-2.22	-1.06	
		3.0	-4.65	25.93	-2.22	-1.28	
		4.0	-25.63	10.41	-2.22	-1.51	
2	.80						-3.28
		0	46.89	-48.88	-2.31	4.36	
		1.0	31.19	-9.40	-2.31	2.25	
		2.0	10.82	11.83	-2.31	-1.07	
		3.0	-7.51	13.05	-2.31	-2.38	
		4.0	-21.21	-1.53	-2.31	-4.70	
3	15.48						.05
		0	53.19	-10.35	1.86	-3.79	
		1.0	37.48	-14.57	1.86	-1.93	
		2.0	17.11	12.95	1.86	-1.06	
		3.0	-1.22	20.45	1.86	1.78	
		4.0	-14.92	12.17	1.86	3.24	
4	-11.09						-3.21
		0	51.88	-63.12	-2.15	4.70	
		1.0	35.97	-18.85	-2.15	2.19	
		2.0	15.61	7.12	-2.15	-1.00	
		3.0	-2.73	13.16	-2.15	-2.16	
		4.0	-16.42	3.37	-2.15	-4.31	
5	3.59						.12
		0	57.97	-74.59	2.02	-4.06	
		1.0	42.27	-24.03	2.02	-2.93	
		2.0	21.90	8.28	2.02	-1.01	
		3.0	3.57	20.57	2.02	2.01	
		4.0	-10.13	17.07	2.02	4.07	
6	.80						-3.28
		0	46.89	-48.88	-2.31	4.36	
		1.0	31.19	-9.40	-2.31	2.25	
		2.0	10.82	11.83	-2.31	-1.07	
		3.0	-7.51	13.05	-2.31	-2.38	
		4.0	-21.21	-1.53	-2.31	-4.70	
7	15.48						.05
		0	53.19	-10.35	1.86	-3.79	
		1.0	37.48	-14.57	1.86	-1.93	
		2.0	17.11	12.95	1.86	-1.06	
		3.0	-1.22	20.45	1.86	1.78	
		4.0	-14.92	12.17	1.86	3.24	
8	-11.09						-3.21
		0	51.88	-63.12	-2.15	4.70	
		1.0	35.97	-18.85	-2.15	2.19	
		2.0	15.61	7.12	-2.15	-1.00	
		3.0	-2.73	13.16	-2.15	-2.16	
		4.0	-16.42	3.37	-2.15	-4.31	
9	3.59						.12
		0	57.97	-74.59	2.02	-4.06	
		1.0	42.27	-24.03	2.02	-2.93	

PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10	3.08		2.0	21.90	8.28	2.02	-1.01	
			3.0	3.57	20.57	2.02	3.01	
			4.0	-10.13	17.07	2.02	4.03	
			.0	29.59	-28.20	-2.00	3.93	-4.19
			1.0	19.02	-3.63	-2.00	1.93	
			2.0	3.84	8.84	-2.00	-1.07	
			3.0	-6.51	8.13	-2.00	-2.07	
11	15.67		4.0	-15.86	-3.18	-2.00	-4.07	
			.0	34.98	-38.04	1.58	-3.23	.38
			1.0	24.42	-8.07	1.58	-1.65	
			2.0	11.04	9.79	1.58	-1.07	
			3.0	-1.11	14.49	1.58	1.51	
			4.0	-10.47	8.37	1.58	3.09	
			.0	35.99	-47.27	-1.78	3.38	-4.10
12	-12.85		1.0	25.43	-16.29	-1.78	1.80	
			2.0	12.05	2.88	-1.78	.82	
			3.0	-.10	8.29	-1.78	-1.78	
			4.0	-9.46	3.37	-1.78	-3.74	
			.0	41.39	-57.10	1.80	-3.59	.47
			1.0	30.82	-20.73	1.80	-1.79	
			2.0	17.44	3.84	1.80	.01	
13	-2.26		3.0	5.29	14.64	1.80	1.81	
			4.0	-4.06	19.13	1.80	3.81	
			.0	24.00	-9.41	-1.00	1.34	-2.69
			1.0	13.44	9.58	-1.00	.83	
			2.0	.06	16.48	-1.00	-1.17	
			3.0	-12.09	10.16	-1.00	-1.17	
			4.0	-31.44	-5.72	-1.00	-2.17	
14	26.06		.0	25.82	-12.55	.07	-1.32	-1.32
			1.0	15.06	8.25	.07	-1.24	
			2.0	1.88	16.75	.07	-1.17	
			3.0	-10.47	12.09	.07	-1.10	
			4.0	-19.82	-3.19	.07	-1.02	
			.0	45.35	-77.94	-.27	.86	-2.40
			1.0	34.79	-32.60	-.27	.39	
15	-27.02		2.0	21.41	-4.38	-.27	.11	
			3.0	9.26	10.69	-.27	-1.16	
			4.0	-.10	15.13	-.27	-1.43	
			.0	46.97	-75.89	.80	-1.19	-1.03
			1.0	36.41	-33.94	.80	-1.69	
			2.0	23.02	-4.09	.80	.11	
			3.0	10.87	12.60	.80	.91	
16	-23.24		4.0	1.52	18.66	.80	1.71	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			END1	SHEAR	MOMENT	SHEAR	
842							
1	46.87						7.95
		.0	81.37	-51.98	-4.23	3.40	
		.5	65.84	-15.03	-4.23	1.29	
		1.0	49.70	13.85	-4.23	-1.82	
		1.5	33.57	34.67	-4.23	-2.94	
		2.0	17.43	47.42	-4.23	-3.05	
2	62.55						-7.32
		.0	23.27	7.65	-12.35	18.46	
		.5	12.59	16.61	-12.35	12.39	
		1.0	1.92	20.24	-12.35	6.11	
		1.5	-8.75	18.54	-12.35	-1.06	
		2.0	-19.42	11.49	-12.35	-6.24	
3	11.48						18.50
		.0	49.09	-23.32	3.07	-12.25	
		.5	38.41	-1.45	3.07	-10.72	
		1.0	27.74	15.09	3.07	-9.18	
		1.5	17.07	26.29	3.07	-7.65	
		2.0	6.40	32.16	3.07	-6.12	
4	51.12						-7.16
		.0	60.25	-46.42	-8.88	16.91	
		.5	49.57	-18.97	-8.88	12.47	
		1.0	38.90	3.15	-8.88	8.03	
		1.5	28.23	19.93	-8.88	3.50	
		2.0	17.56	31.38	-8.88	-1.04	
5	.05						18.64
		.0	86.07	-77.39	6.54	-13.20	
		.5	75.39	-37.03	6.54	-10.53	
		1.0	64.72	-2.00	6.54	-7.86	
		1.5	54.05	27.69	6.54	-3.39	
		2.0	43.38	32.06	6.54	-1.73	
6	62.55						-7.32
		.0	23.27	7.65	-12.35	18.46	
		.5	12.59	16.61	-12.35	12.39	
		1.0	1.92	20.24	-12.35	6.11	
		1.5	-8.75	18.54	-12.35	-1.06	
		2.0	-19.42	11.49	-12.35	-6.24	
7	11.48						18.50
		.0	49.09	-23.32	3.07	-12.25	
		.5	38.41	-1.45	3.07	-10.72	
		1.0	27.74	15.09	3.07	-9.18	
		1.5	17.07	26.29	3.07	-7.65	
		2.0	6.40	32.16	3.07	-6.12	
8	51.12						-7.16
		.0	60.25	-46.42	-8.88	16.91	
		.5	49.57	-18.97	-8.88	12.47	
		1.0	38.90	3.15	-8.88	8.03	
		1.5	28.23	19.93	-8.88	3.50	
		2.0	17.56	31.38	-8.88	-1.04	
9	.05						18.64
		.0	86.07	-77.39	6.54	-13.20	
		.5	75.39	-37.03	6.54	-10.53	

PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORD
		SHEAR	MOMENT	SHEAR	MOMENT	
10	1.0	64.72	-2.00	6.54	-7.26	
	1.5	54.05	27.69	6.54	-3.99	
	2.0	43.38	52.05	6.54	-0.73	
	51.40					-5.98
	1.0	2.27	35.04	-11.02	15.88	
	1.5	-5.11	24.33	-11.02	10.37	
	1.0	-12.49	19.93	-11.02	4.86	
11	1.5	-19.87	11.84	-11.02	-0.65	
	2.0	-27.25	0.06	-11.02	-6.16	
	7.60					15.16
	1.0	24.42	-1.52	2.20	-10.47	
	1.5	17.04	8.84	2.20	-9.37	
	1.0	9.66	15.32	2.20	-8.27	
	1.5	2.28	18.50	2.20	-7.17	
12	2.0	-5.10	17.79	2.20	-6.06	
	36.10					-6.79
	1.0	31.80	-17.37	-6.37	13.80	
	1.5	44.42	-23.32	-6.37	10.61	
	1.0	37.04	-2.96	-6.37	7.43	
	1.5	29.65	13.72	-6.37	4.24	
	2.0	22.27	26.70	-6.37	1.06	
13	-7.71					15.36
	1.0	73.94	-73.94	6.85	-12.35	
	1.5	66.56	-38.81	6.85	-9.12	
	1.0	39.18	-7.37	6.85	-5.89	
	1.5	31.80	20.37	6.85	-2.67	
	2.0	44.42	44.43	6.85	0.56	
	55.92					0.54
14	1.0	-47.73	100.19	-11.82	9.08	
	1.5	-55.11	74.48	-11.82	3.17	
	1.0	-62.49	45.98	-11.82	-2.74	
	1.5	-69.87	11.99	-11.82	-6.94	
	2.0	-77.25	-24.79	-11.82	-14.35	
	40.77					7.19
	1.0	-41.08	32.22	-7.85	1.19	
15	1.5	-48.46	69.83	-7.85	-2.75	
	1.0	-55.84	43.75	-7.85	-6.57	
	1.5	-63.22	13.99	-7.85	-10.50	
	2.0	-70.61	-19.47	-7.85	-14.32	
	2.92					1.17
	1.0	117.30	-141.11	3.68	2.15	
	1.5	109.92	-54.31	3.68	3.99	
16	1.0	102.54	-31.20	3.68	5.24	
	1.5	95.16	18.23	3.68	7.66	
	2.0	87.78	67.96	3.68	9.92	
	-10.22					7.83
	1.0	123.94	-149.08	7.65	-3.75	
	1.5	116.56	-88.96	7.65	-1.93	
	1.0	109.18	-32.62	7.65	1.90	
17	1.5	101.80	20.23	7.65	5.72	
	2.0	94.42	69.29	7.65	9.55	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD DOMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
265								
1	34.04							46.30
		.0		-69.68	126.41	-2.93	4.08	
		.6		-85.82	79.92	-2.93	2.32	
		1.2		-105.24	22.77	-2.93	.36	
		1.8		-124.65	-46.36	-2.93	-1.20	
		2.4		-140.78	-126.15	-2.93	-2.95	
2	-7.49							51.88
		.0		-108.48	109.61	18.07	-17.61	
		.6		-119.87	41.21	18.07	-6.67	
		1.2		-133.39	-34.66	18.07	4.17	
		1.8		-146.91	-118.86	18.07	18.01	
		2.4		-158.29	-210.32	18.07	28.85	
3	3.41							12.49
		.0		-75.62	98.42	-27.12	27.76	
		.6		-87.01	49.73	-27.12	11.39	
		1.2		-100.53	-8.42	-27.12	-3.18	
		1.8		-114.05	-70.90	-27.12	-21.45	
		2.4		-125.43	-142.36	-27.12	-37.77	
4	43.44							51.95
		.0		-18.78	78.81	23.00	-21.67	
		.6		-30.16	64.23	23.00	-7.87	
		1.2		-43.69	42.18	23.00	3.93	
		1.8		-57.20	11.81	23.00	19.73	
		2.4		-68.59	-26.03	23.00	33.33	
5	47.34							12.54
		.0		14.08	67.62	-22.19	23.20	
		.6		2.69	72.76	-22.19	9.39	
		1.2		-10.83	70.42	-22.19	-3.42	
		1.8		-24.35	69.76	-22.19	-15.73	
		2.4		-35.73	41.83	-22.19	-30.06	
6	-7.49							51.88
		.0		-108.48	109.61	18.07	-17.61	
		.6		-119.87	41.21	18.07	-6.67	
		1.2		-133.39	-34.66	18.07	4.17	
		1.8		-146.91	-118.86	18.07	18.01	
		2.4		-158.29	-210.32	18.07	28.85	
7	3.41							12.49
		.0		-75.62	98.42	-27.12	27.76	
		.6		-87.01	49.73	-27.12	11.39	
		1.2		-100.53	-8.42	-27.12	-3.18	
		1.8		-114.05	-70.90	-27.12	-21.45	
		2.4		-125.43	-142.36	-27.12	-37.77	
8	43.44							51.95
		.0		-18.78	78.81	23.00	-21.67	
		.6		-30.16	64.23	23.00	-7.87	
		1.2		-43.69	42.18	23.00	3.93	
		1.8		-57.20	11.81	23.00	19.73	
		2.4		-68.59	-26.03	23.00	33.33	
9	47.34							12.54
		.0		14.08	67.62	-22.19	23.20	
		.6		2.69	72.76	-22.19	9.39	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ				
				SHEAR	MOMENT	SHEAR	MOMENT					
10	-14.27		1.2	-10.83	70.42	-22.19	-3.42	40.22				
			1.8	-24.35	59.76	-22.19	-16.73					
			2.4	-35.73	41.63	-22.19	-30.05					
			.0	-107.55	90.11	14.57	-14.59					
			.6	-115.92	23.14	14.57	-5.65					
			1.2	-125.73	-49.28	14.57	3.09					
			1.8	-135.53	-127.73	14.57	11.94					
11	-10.92		2.4	-143.89	-211.53	14.57	20.38	6.43				
			.0	-79.37	30.51	-24.19	24.10					
			.6	-87.73	30.45	-24.19	7.59					
			1.2	-97.54	-25.06	-24.19	-4.93					
			1.8	-107.35	-86.60	-24.19	-19.44					
			2.4	-115.71	-155.59	-24.19	-37.95					
			12	44.56		.0	12.58		48.86	21.18	-19.95	40.28
.6	4.22	53.97				21.18	-7.26					
1.2	-5.59	53.63				21.18	3.45					
1.8	-15.40	47.26				21.18	16.16					
2.4	-23.76	35.44				21.18	30.86					
13	47.91					.0	40.76	39.26	-17.38	18.53	5.49	
						.6	32.40	61.28	-17.38	7.98		
			1.2	22.39	77.85	-17.38	-2.07					
			1.8	12.78	88.39	-17.38	-13.12					
			2.4	4.42	93.48	-17.38	-27.57					
			14	-81.71		.0	-237.39	174.34	-6.70	3.39		13.33
						.6	-246.14	-10.26	-6.70	1.56		
1.2	-255.95	-160.81				-6.70	-20.46					
1.8	-265.76	-317.40				-6.70	-55.46					
2.4	-274.12	-479.44				-6.70	-100.50					
15	-80.70					.0	-129.32	131.96	-18.33	17.13	18.19	
						.6	-237.68	-8.06	-18.33	5.13		
			1.2	-247.50	-157.55	-18.33	-4.97					
			1.8	-257.30	-305.06	-18.33	-15.87					
			2.4	-265.66	-462.02	-18.33	-36.87					
			16	114.34		.0	152.53	-2.50	15.32	-12.99		26.51
						.6	154.17	92.49	15.32	-3.60		
1.2	144.76	182.12				15.32	7.37					
1.8	134.55	265.72				15.32	14.56					
2.4	126.19	343.87				15.32	23.77					
17	115.35					.0	170.99	-3.48	3.69	-1.44	18.38	
						.6	162.63	94.68	3.69	.77		
			1.2	152.82	199.39	3.69	2.98					
			1.8	143.01	278.06	3.69	5.20					
			2.4	134.65	361.29	3.69	7.41					



PORTAL TIGA DIMENSI - UNITS: KN - M

## FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
870								
1		72.97						3.85
			1.0	-12.43	104.67	1.86	-1.80	
			1.5	-37.41	169.00	1.86	-5.81	
			3.0	-88.46	73.96	1.86	1.17	
			4.5	-133.69	-90.04	1.86	1.16	
			6.0	-160.10	-310.72	1.86	0.14	
2		46.32						21.05
			1.0	-9.45	100.40	4.83	-14.18	
			1.5	-29.43	78.34	4.83	-6.93	
			3.0	-82.55	10.03	4.83	1.32	
			4.5	-95.21	-108.70	4.83	7.57	
			6.0	-110.03	-288.46	4.83	14.82	
3		60.98						-8.79
			1.0	-33.35	184.32	-1.37	3.35	
			1.5	-83.31	121.43	-1.37	3.30	
			3.0	-88.24	17.29	-1.37	1.24	
			4.5	-119.09	-137.26	-1.37	-1.71	
			6.0	-138.92	-332.85	-1.37	-4.06	
4		42.13						21.05
			1.0	17.19	104.74	2.42	-7.25	
			1.5	-2.50	117.82	2.42	-4.92	
			3.0	-35.72	29.26	2.42	-1.79	
			4.5	-88.57	10.48	2.42	3.24	
			6.0	-88.40	-109.33	2.42	6.27	
5		56.79						-5.61
			1.0	-6.70	183.66	-3.98	11.37	
			1.5	-36.68	160.71	-3.98	3.70	
6		46.32						21.05
			1.0	-9.45	100.40	4.83	-14.18	
			1.5	-29.43	78.34	4.83	-6.93	
			3.0	-82.55	10.03	4.83	1.32	
			4.5	-95.21	-108.70	4.83	7.57	
			6.0	-110.03	-288.46	4.83	14.82	
7		60.98						-8.79
			1.0	-33.35	184.32	-1.37	3.35	
			1.5	-83.31	121.43	-1.37	3.30	
			3.0	-88.24	17.29	-1.37	1.24	
			4.5	-119.09	-137.26	-1.37	-1.71	
			6.0	-138.92	-332.85	-1.37	-4.06	
8		42.13						21.05
			1.0	17.19	104.74	2.42	-7.25	
			1.5	-2.50	117.82	2.42	-4.92	
			3.0	-35.72	29.26	2.42	-1.79	
			4.5	-88.57	10.48	2.42	3.24	
			6.0	-88.40	-109.33	2.42	6.27	
9		56.79						-5.61
			1.0	-6.70	183.66	-3.98	11.37	
			1.5	-36.68	160.71	-3.98	3.70	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL FORCE	DIST ENDT	1-2 PLANE		1-3 PLANE		AXIAL TORQ		
			SHEAR	MOMENT	SHEAR	MOMENT			
10	34.42	3.0	-59.60	96.52	-3.98	-1.07	17.22		
		4.5	-92.46	-18.08	-3.98	-6.04			
		6.0	-112.29	-173.72	-3.98	-12.01			
		7.5	-13.96	72.79	4.65	-13.52			
		1.5	-38.48	43.01	4.65	-6.54			
		3.0	-92.04	-17.26	4.65	1.43			
11	46.99	4.5	-76.16	-114.00	4.65	7.41	-8.37		
		6.0	-91.47	-241.08	4.65	14.39			
		7.5	-33.55	140.49	-1.84	3.33			
		1.5	-48.96	79.97	-1.84	1.97			
		3.0	-72.35	-11.03	-1.84	1.71			
		4.5	-96.64	-138.50	-1.84	-1.55			
12	28.80	6.0	-111.96	-396.31	-1.84	-1.81	17.47		
		7.5	32.81	71.91	1.42	-4.77			
		1.5	7.19	35.82	1.42	-2.88			
		3.0	-16.97	38.84	1.42	-1.07			
		4.5	-46.49	45.60	1.42	1.61			
		6.0	-75.81	-37.89	1.42	3.74			
13	41.38	7.5	2.12	139.50	-4.07	11.97	-6.13		
		1.5	-13.50	132.88	-4.07	5.87			
		3.0	-37.16	35.08	-4.07	-1.21			
		4.5	-60.98	21.11	-4.07	-6.38			
		6.0	-76.30	-97.21	-4.07	-12.16			
		7.5	-81.82	97.32	6.50	-17.85			
14	45.36	1.5	-77.24	-5.40	6.50	-8.10	7.98		
		3.0	-101.10	-138.81	6.50	1.84			
		4.5	-124.91	-308.89	6.50	11.39			
		6.0	-140.23	-508.90	6.50	21.13			
		7.5	-67.97	117.84	4.35	-12.22			
		1.5	-83.38	5.89	4.35	-3.83			
15	49.13	3.0	-107.25	-136.94	4.35	1.73	1.29		
		4.5	-131.06	-316.04	4.35	9.00			
		6.0	-146.38	-525.48	4.35	16.27			
		7.5	37.03	84.36	-4.27	11.28			
		1.5	41.81	169.90	-4.27	4.87			
		3.0	17.75	214.75	-4.27	-1.53			
16	26.66	4.5	4.5	4.5	-6.07	323.13	8.50	-4.27	-7.94
		6.0	-21.39	201.19	-4.27	-14.35			
		7.5	56.88	114.87	-5.92	16.31			
		1.5	35.46	180.99	-5.92	7.43			
		3.0	11.60	216.63	-5.92	-1.45			
		4.5	-12.22	215.79	-5.92	-10.33			
17	30.43	6.0	-27.54	184.62	-5.92	-19.21	1.12		

PORTAL TIGA DIMENSI - UNITS: KN - M

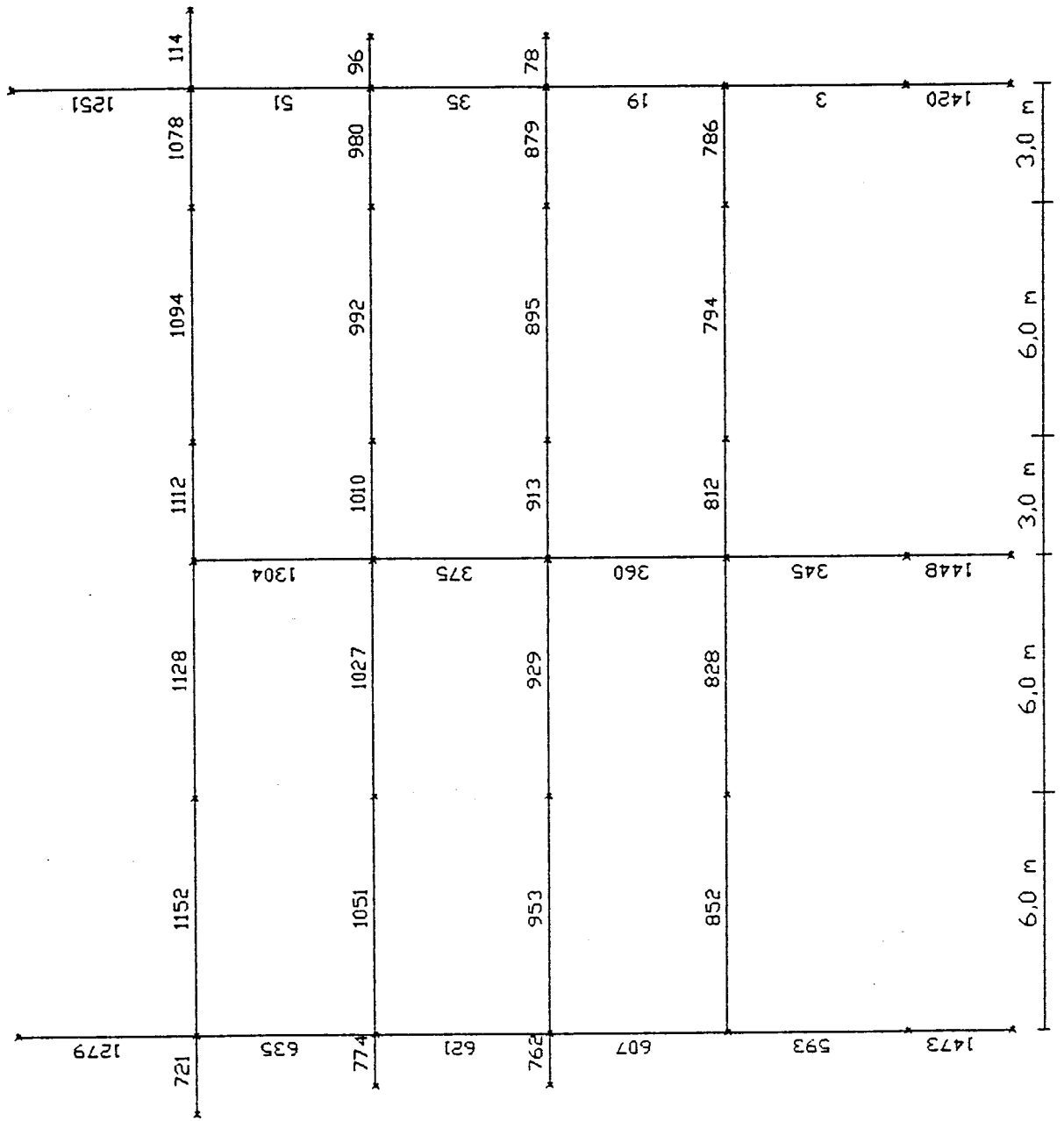
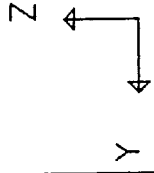
FRAME ELEMENT FORCES

ELT ID	LOAD COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
971								
1	-2.97							1.36
			0.0	53.70	-53.81	.39	-1.12	
			1.5	39.13	17.13	.39	-1.53	
			3.0	-3.92	44.06	.39	.05	
			4.5	-46.90	5.38	.39	1.04	
			6.0	-71.27	-87.06	.39	1.22	
2	-13.41							1.10
			0.0	28.36	-1.43	2.86	-3.36	
			1.5	10.95	30.97	2.86	-4.07	
			3.0	-18.48	25.82	2.86	.22	
			4.5	-47.88	-24.46	2.86	4.52	
			6.0	-55.76	-112.17	2.86	3.81	
3	-23.80							2.26
			0.0	26.46	6.34	-1.86	3.00	
			1.5	8.45	34.99	-1.86	1.71	
			3.0	-20.98	25.39	-1.86	.42	
			4.5	-50.38	-27.94	-1.86	-1.57	
			6.0	-58.26	-119.40	-1.86	-7.16	
4	18.35							1.21
			0.0	52.85	-94.60	1.36	-4.42	
			1.5	44.84	-11.36	1.36	-2.38	
			3.0	15.41	24.13	1.36	-1.34	
			4.5	-13.99	34.87	1.36	1.70	
			6.0	-31.87	-2.00	1.36	3.73	
5	7.97							2.37
			0.0	60.35	-56.83	-2.36	6.35	
			1.5	42.34	-7.34	-2.36	3.40	
			3.0	12.91	54.40	-2.36	-1.14	
			4.5	-16.49	31.39	-2.36	-3.67	
			6.0	-34.37	-9.23	-2.36	-7.24	
6	-13.41							1.10
			0.0	28.36	-1.43	2.86	-3.36	
			1.5	10.95	30.97	2.86	-4.07	
			3.0	-18.48	25.82	2.86	.22	
			4.5	-47.88	-24.46	2.86	4.52	
			6.0	-55.76	-112.17	2.86	3.81	
7	-23.80							2.26
			0.0	26.46	6.34	-1.86	3.00	
			1.5	8.45	34.99	-1.86	1.71	
			3.0	-20.98	25.39	-1.86	.42	
			4.5	-50.38	-27.94	-1.86	-1.57	
			6.0	-58.26	-119.40	-1.86	-7.16	
8	18.35							1.21
			0.0	52.85	-94.60	1.36	-4.42	
			1.5	44.84	-11.36	1.36	-2.38	
			3.0	15.41	24.13	1.36	-1.34	
			4.5	-13.99	34.87	1.36	1.70	
			6.0	-31.87	-2.00	1.36	3.73	
9	7.97							2.37
			0.0	60.35	-56.83	-2.36	6.35	
			1.5	42.34	-7.34	-2.36	3.40	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

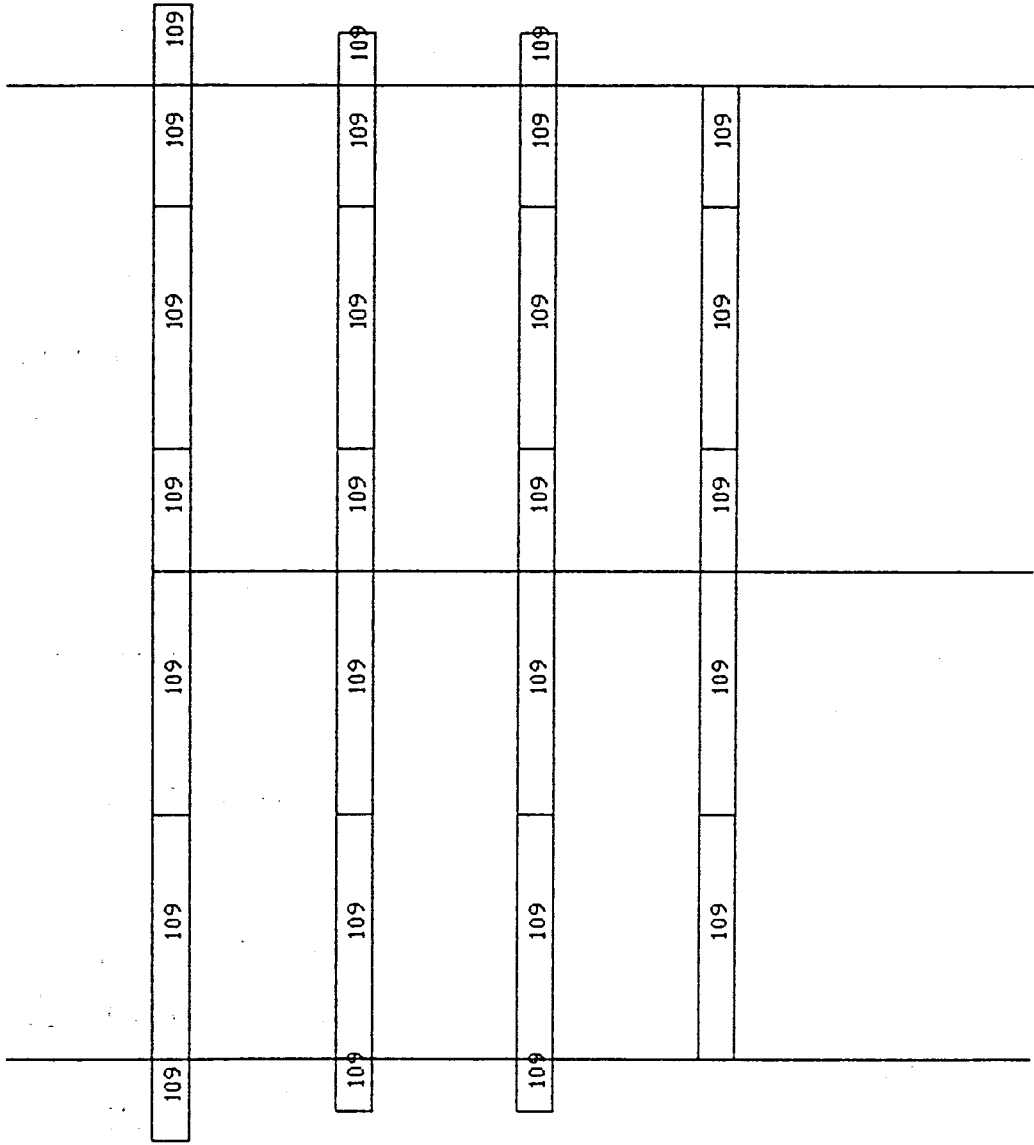
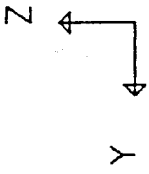
ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ		
		SHEAR	MOMENT	SHEAR	MOMENT			
10	3.0	12.91	34.40	-2.36	-1.14			
	4.5	-16.49	31.39	-2.36	-3.69			
	6.0	-34.37	-9.32	-2.36	-7.24			
	-15.21							
	7.0	10.98	27.22	2.77	-7.98			-1.11
	1.5	-2.67	35.13	2.77	-3.53			
11	3.0	-23.98	13.33	2.77	.73			
	4.5	-44.67	-35.91	2.77	4.48			
	6.0	-98.24	-114.77	2.77	8.84			
	-28.12							
	7.0	8.93	33.89	-1.42	1.77			1.74
	1.5	-4.82	38.57	-1.42	1.13			
12	3.0	-25.93	15.76	-1.42	.30			
	4.5	-46.61	-38.90	-1.42	-1.14			
	6.0	-90.99	-120.97	-1.42	-3.77			
	33.33							
	7.0	36.56	-97.00	.76	-2.70			.63
	1.5	42.71	-21.86	.76	-1.57			
13	3.0	21.70	28.82	.76	-1.43			
	4.5	.72	43.66	.76	.70			
	6.0	-12.83	72.78	.76	1.84			
	14.42							
	7.0	34.33	-90.98	-2.44	7.03			1.88
	1.5	40.57	-18.12	-2.44	3.39			
14	3.0	19.56	27.13	-2.44	-1.26			
	4.5	-11.42	40.57	-2.44	-3.82			
	6.0	-45.00	76.37	-2.44	-7.37			
	-71.94							
	7.0	-42.70	175.05	4.00	-10.75			.37
	1.5	-58.35	102.44	4.00	-4.73			
15	3.0	-77.56	2.33	4.00	1.27			
	4.5	-98.34	-129.62	4.00	7.27			
	6.0	-111.91	-789.99	4.00	13.27			
	-74.81							
	7.0	-43.74	177.05	3.04	-7.31			.92
	1.5	-58.97	103.48	3.04	-3.74			
16	3.0	-78.00	2.46	3.04	1.32			
	4.5	-98.98	-130.52	3.04	5.88			
	6.0	-112.56	-790.95	3.04	10.49			
	59.82							
	7.0	108.34	-240.71	-2.71	6.37			.85
	1.5	94.89	-86.47	-2.71	2.81			
17	3.0	73.88	40.28	-2.71	-11.25			
	4.5		4.0	32.89	130.18		-2.71	-5.32
	6.0	39.32	202.66	-2.71	-9.38			
	67.15							
	7.0	107.69	-238.71	-3.67	9.80			1.40
	1.5	94.24	-85.43	-3.67	4.30			
18	3.0	73.24	40.35	-3.67	-11.20			
	4.5	52.25	174.28	-3.67	-9.71			
	6.0	38.68	200.80	-3.67	-12.21			



trata  
UNDEFORMED  
SHAPE

OPTIONS  
ALL JOINTS  
ELEMENT IDS  
WIRE FRAME

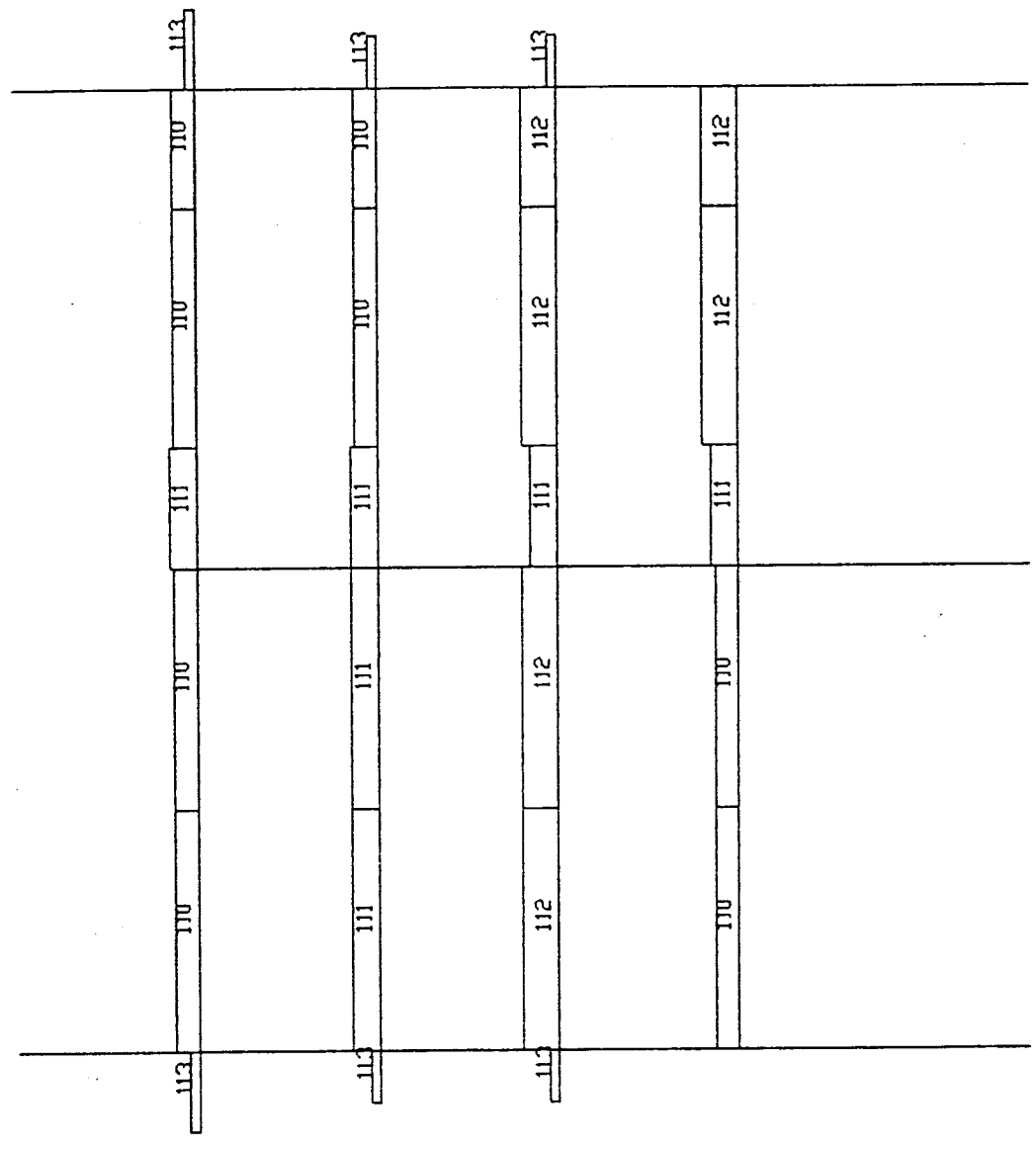
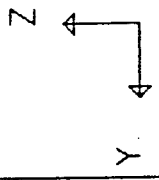
SAP90



trata  
 FRAME  
 LOADS  
 LOAD 1

MINIMA  
 W -.9532E+01  
 P .0000E+00  
 MAXIMA  
 W -.9532E+01  
 P .0000E+00

GAMBAR PEMBEANAN PORTAL AKIBAT BEBAN MATI

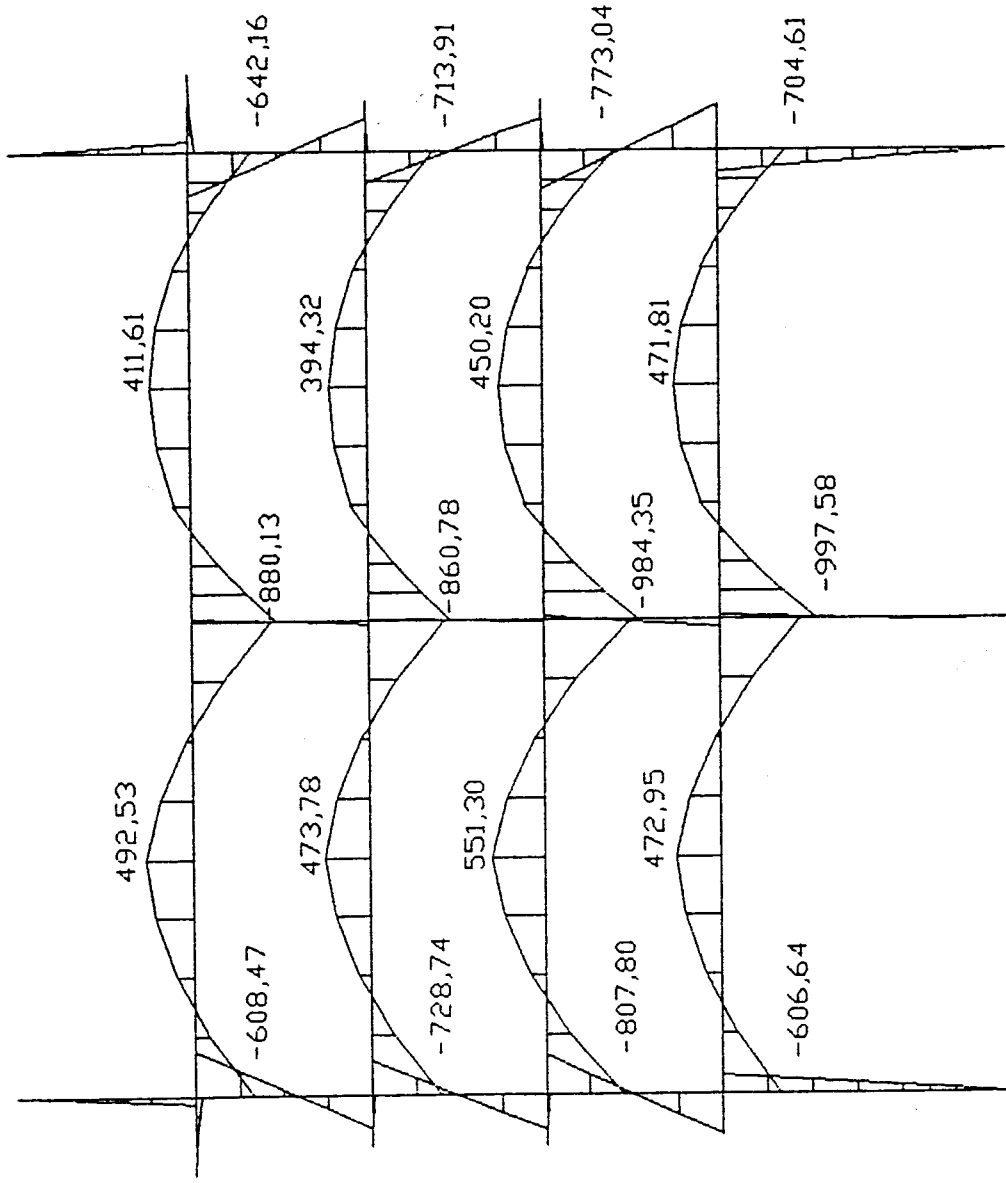
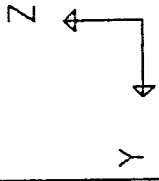


trata  
 FRAME  
 LOADS  
 LOAD 3

MINIMA  
 w -.8000E+01  
 P .0000E+00  
 MAXIMA  
 w -.2000E+01  
 P .0000E+00  
 110 = 5 KN/m  
 111 = 6 KN/m  
 112 = 8 KN/m  
 113 = 2 KN/m

SAP90

GAMBAR PEMBEANAN PORTAL AKIBAT BEBAN HIDUP



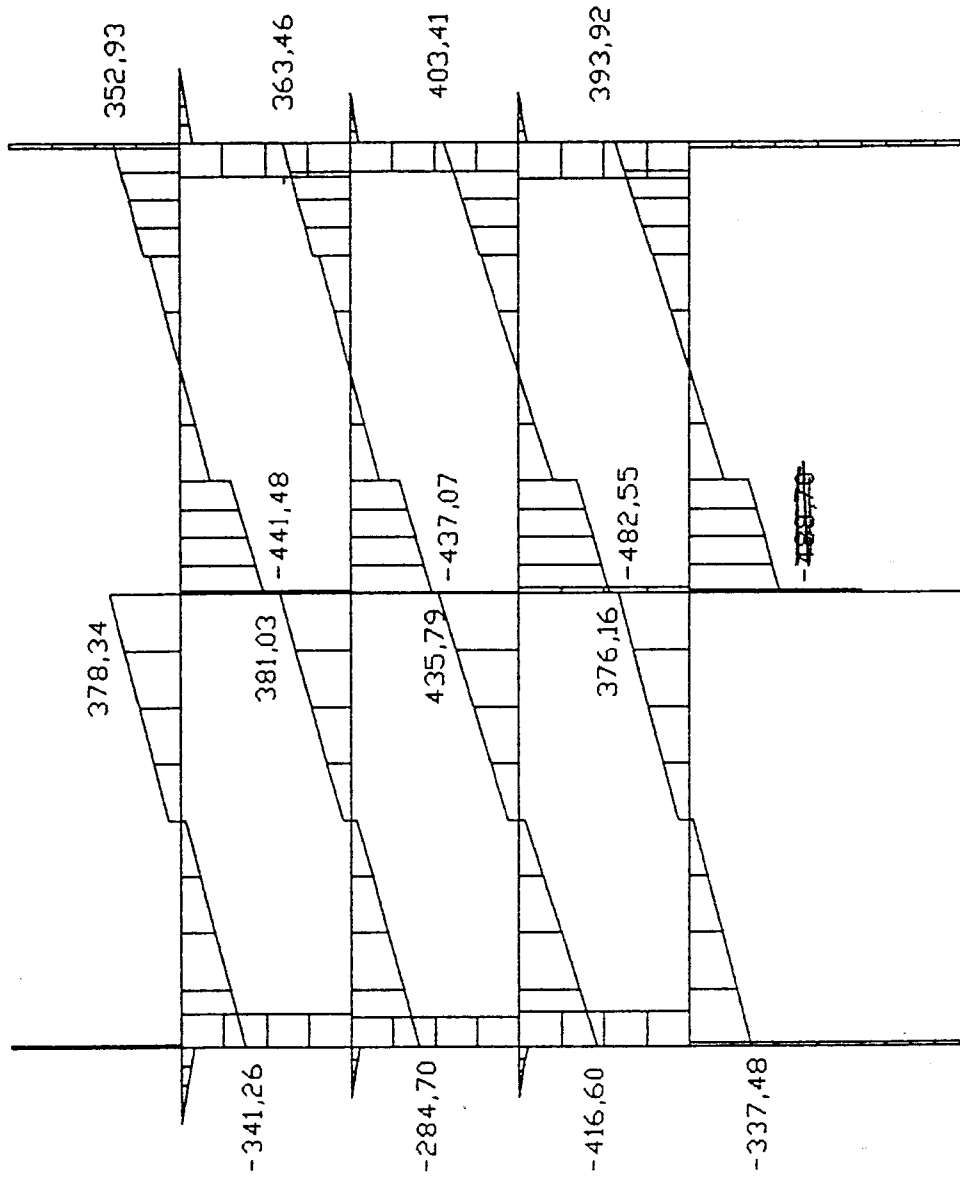
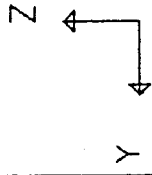
trata  
 FRAME  
 OUTPUT M33  
 LOAD 1

ENVELOPES  
 MIN < 812>  
 -9976E+03  
 AT 3.00  
 MAX < 929>  
 .5513E+03  
 AT 6.00

SAP90

GAMBAR MOMEN AKIBAT BEBAN TETAP





TRATA  
 FRAME  
 OUTPUT V22  
 LOAD 1

ENVELOPES  
 MIN < 812>  
 -.4887E+03  
 AT 3.00  
 MAX < 929>  
 .4358E+03  
 AT .00

SAP90

GAMBAR GAYA GESER AKIBAT BEBAN TETAP

ORTAL TIGA DIMENSI - UNITS: KN - M

## R A M E E L E M E N T F O R C E S

ELT ID	LOAD COMB	AXIAL FORCE	DIST ENDI	1-3 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
-----								
788	1	164.11						-54.52
			.0	393.92	-704.61	-7.65	12.18	
			.8	345.97	-427.31	-7.65	5.44	
			1.5	297.21	-186.27	-7.65	.71	
			2.3	249.86	18.51	-7.65	-5.03	
			3.0	200.50	187.02	-7.65	-10.77	
-----								
794	1	165.61						-3.71
			.0	196.17	173.42	1.13	-2.28	
			1.5	99.46	395.15	1.13	-.59	
			3.0	2.76	471.81	1.13	1.10	
			4.5	-93.95	403.41	1.13	2.79	
			6.0	-190.66	189.95	1.13	4.49	
-----								
812	1	167.44						65.49
			.0	-316.68	210.79	-12.96	17.23	
			.8	-359.84	-42.98	-12.96	7.51	
			1.5	-402.79	-328.96	-12.96	-2.22	
			2.3	-445.75	-647.17	-12.96	-11.94	
			3.0	-488.70	-997.58	-12.96	-21.66	
-----								
828	1	156.63						-.53
			.0	376.15	-837.45	-.09	.18	
			1.5	297.28	-332.37	-.09	.04	
			3.0	218.40	54.39	-.09	-.11	
			4.5	139.52	322.83	-.09	-.25	
			6.0	60.64	472.95	-.09	-.39	
-----								
852	1	158.69						.69
			.0	-21.97	471.71	-1.89	5.50	
			1.5	-100.85	379.59	-1.89	2.77	
			3.0	-179.73	169.17	-1.89	.13	
			4.5	-258.60	-159.58	-1.89	-2.71	
			6.0	-337.48	-606.64	-1.89	-6.55	
-----								
879	1	-43.96						-51.67
			.0	403.41	-773.04	1.06	-1.55	
			.8	355.06	-488.61	1.06	-.85	
			1.5	306.71	-240.45	1.06	-.05	
			2.3	258.35	-28.55	1.06	.75	
			3.0	210.00	147.08	1.06	1.55	
-----								
893	1	-43.64						-3.17
			.0	202.26	133.54	.10	-.30	
			1.5	105.55	384.41	.10	-.15	
			3.0	8.84	450.20	.10	-.01	
			4.5	-87.86	390.94	.10	.14	
			6.0	-184.57	186.61	.10	.25	
-----								
913	1	-43.41						60.91
			.0	-310.74	205.59	.62	-1.35	

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	.8	-353.69	-43.57	.62		-.89
	1.5	-395.65	-324.95	.62		-.42
	2.3	-439.60	-638.54	.62		.04
	3.0	-482.55	-784.33	.62		.51
-----						
929	1	-24.88				-.22
	.0	435.79	-922.50	.05		-.12
	1.5	340.71	-340.13	.05		-.04
	3.0	245.63	99.63	.05		.04
	4.5	150.95	396.77	.05		.12
	6.0	55.48	551.30	.05		.20
-----						
953	1	-24.60				.68
	.0	-36.28	550.84	.48		-1.32
	1.5	-131.36	425.11	.48		-.60
	3.0	-226.44	156.76	.48		.12
	4.5	-321.52	-254.21	.48		.84
	6.0	-416.60	-807.80	.48		1.56
-----						
980	1	27.98				-38.02
	.0	363.46	-713.91	3.51		-8.56
	.8	323.21	-456.41	3.51		-3.93
	1.5	282.95	-229.10	3.51		-3.29
	2.3	242.70	-31.98	3.51		-.66
	3.0	202.44	134.95	3.51		1.98
-----						
992	1	27.84				-7.78
	.0	168.03	131.76	.41		-.47
	1.5	87.52	323.42	.41		.15
	3.0	7.01	394.32	.41		.77
	4.5	-73.50	344.45	.41		1.36
	6.0	-154.01	173.82	.41		2.00
-----						
1010	1	27.52				36.60
	.0	-265.25	172.69	3.16		-5.31
	.8	-308.20	-22.36	3.16		-2.94
	1.5	-351.16	-269.61	3.16		-.57
	2.3	-394.11	-549.09	3.16		1.81
	3.0	-437.07	-860.78	3.16		4.18
-----						
1027	1	20.09				.04
	.0	381.03	-801.10	.79		-2.19
	1.5	296.75	-292.76	.79		-1.00
	3.0	212.47	89.16	.79		.19
	4.5	128.20	344.67	.79		1.37
	6.0	43.92	473.75	.79		2.56
-----						
1051	1	19.36				.24
	.0	-31.86	473.78	1.94		-5.33
	1.5	-116.14	362.78	1.94		-2.42
	3.0	-200.42	125.36	1.94		.49
	4.5	-284.70	-238.48	1.94		3.41

PORTAL TIGA DIMENSI - UNITS: KN - M

FRAME ELEMENT FORCES

ELT LOAD ID COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	6.0	-368.98	-726.74	1.94	6.32	
1078	1 -161.50					-40.47
	.0	352.93	-642.16	12.65	-28.05	
	.8	312.68	-392.56	12.65	-18.56	
	1.5	272.42	-173.15	12.65	-9.07	
	2.3	232.17	16.08	12.65	.42	
	3.0	191.91	175.11	12.65	9.91	
1094	1 -162.11					-6.55
	.0	160.49	171.65	-93	4.10	
	1.5	79.99	352.01	-93	2.70	
	3.0	-.52	411.61	-93	1.30	
	4.5	-81.03	350.45	-93	-.10	
	6.0	-161.54	168.52	-93	-1.50	
1112	1 -165.83					55.17
	.0	-269.67	186.60	13.60	-22.04	
	.8	-312.62	-31.76	13.60	-11.84	
	1.5	-355.58	-282.34	13.60	-1.65	
	2.3	-398.53	-565.12	13.60	8.55	
	3.0	-441.48	-880.13	13.60	18.75	
1128	1 -153.23					-9.02
	.0	378.34	-829.77	1.46	-3.38	
	1.5	299.46	-321.42	1.46	-1.20	
	3.0	220.58	68.61	1.46	.99	
	4.5	141.70	340.32	1.46	3.18	
	6.0	62.82	493.71	1.46	5.37	
1152	1 -157.29					1.02
	.0	-25.74	492.53	3.71	-10.30	
	1.5	-104.62	394.76	3.71	-4.74	
	3.0	-193.50	178.67	3.71	.82	
	4.5	-262.38	-155.74	3.71	6.38	
	6.0	-341.26	-608.47	3.71	11.94	

HITUNGAN SAPCON STRUKTUR TIGA DIMENSI

FLEXURAL AND SHEAR DESIGN OF BEAM-TYPE ELEMENTS

ELEM	SECTION SIZE		STATN	-----REQUIRED REINFORCING-----			(-DESIGN FORCES-)		
	1D	DEPTH X WIDTH		LOC	TOP (LC)	BOT (LC)	SHR (LC)	-M33	+M33
78	.70	X .10	.0	2.20 <15>	2.20 <16>	10.49 <1>	7	4	102
			.3	2.20 <1>	2.20 <17>	10.99 <1>	7	4	107
			.6	2.20 <1>	2.20 <17>	11.49 <1>	7	4	112
			1.0	2.20 <1>	2.20 <17>	12.00 <1>	17	4	117
			1.3	2.20 <1>	2.20 <17>	12.50 <1>	30	15	122
96	.70	X .10	.0	2.20 <1>	2.20 <16>	10.49 <1>	7	4	102
			.3	2.20 <1>	2.20 <17>	10.99 <1>	7	4	107
			.6	2.20 <1>	2.20 <17>	11.49 <1>	7	4	112
			1.0	2.20 <1>	2.20 <17>	12.00 <1>	17	4	117
			1.3	2.20 <1>	2.20 <17>	12.50 <1>	30	15	122
114	.70	X .10	.0	2.20 <14>	2.20 <17>	8.33 <1>	18	9	81
			.5	2.20 <1>	2.20 <17>	9.10 <1>	18	9	89
			1.0	2.20 <1>	2.20 <17>	9.87 <1>	18	9	96
			1.5	2.20 <1>	2.20 <17>	10.64 <1>	39	9	104
			2.0	3.25 <1>	2.20 <17>	11.41 <1>	70	35	111
762	.70	X .10	.0	2.20 <1>	2.20 <16>	10.49 <1>	7	4	102
			.3	2.20 <1>	2.20 <17>	10.99 <1>	7	4	107
			.6	2.20 <1>	2.20 <17>	11.49 <1>	7	4	112
			1.0	2.20 <1>	2.20 <17>	12.00 <1>	17	4	117
			1.3	2.20 <1>	2.20 <17>	12.50 <1>	30	15	122
774	.70	X .10	.0	2.20 <1>	2.20 <15>	10.49 <1>	7	4	102
			.3	2.20 <1>	2.20 <17>	10.99 <1>	7	4	107
			.6	2.20 <1>	2.20 <17>	11.49 <1>	7	4	112
			1.0	2.20 <1>	2.20 <17>	12.00 <1>	17	4	117
			1.3	2.20 <1>	2.20 <17>	12.50 <1>	30	15	122
721	.70	X .10	.0	2.20 <17>	2.20 <1>	8.33 <1>	18	9	81
			.5	2.20 <1>	2.20 <17>	9.10 <1>	18	9	89
			1.0	2.20 <1>	2.20 <17>	9.87 <1>	18	9	96
			1.5	2.20 <1>	2.20 <17>	10.64 <1>	39	9	104
			2.0	3.25 <1>	2.20 <17>	11.41 <1>	70	35	111
786	.80	X .40	.0	30.17 <1>	14.06 <14>	51.19 <1>	705	332	578
			.8	17.29 <1>	10.17 <14>	49.68 <1>	427	88	551
			1.5	10.17 <16>	10.17 <15>	48.16 <1>	259	101	543
			2.3	10.17 <16>	10.17 <15>	46.65 <1>	176	160	526
			3.0	10.17 <16>	10.17 <15>	45.13 <1>	176	203	509
794	.80	X .40	.0	10.17 <16>	10.17 <15>	16.70 <1>	53	205	138
			1.5	10.17 <15>	15.85 <1>	13.67 <1>	13	395	154
			3.0	10.17 <16>	19.25 <1>	10.84 <1>	13	472	120
			4.5	10.17 <15>	16.25 <1>	13.43 <1>	13	4	

			6.0	10.17 <15>	10.17 <1>	16.46 <1>	15	190	166
812	.80 X	.40	.0	10.17 <15>	10.17 <1>	59.95 <1>	249	311	449
			.8	10.17 <15>	10.17 <16>	61.46 <1>	249	125	467
			1.5	13.07 <1>	10.17 <16>	62.98 <1>	329	125	484
			2.3	27.36 <1>	10.17 <16>	64.49 <1>	647	125	501
			3.0	46.02 <1>	20.46 <16>	66.01 <1>	998	499	516
828	.95 X	.30	.0	30.26 <1>	13.77 <15>	33.28 <1>	637	419	452
			1.5	11.47 <16>	9.18 <15>	30.88 <1>	348	118	419
			3.0	9.18 <16>	9.18 <15>	28.48 <1>	209	157	387
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852	.95 X	.30	.0	9.18 <14>	13.88 <1>	18.83 <1>	152	472	256
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			6.0	20.92 <1>	9.73 <16>	28.43 <1>	607	303	786
879	.80 X	.40	.0	33.63 <1>	15.52 <14>	54.12 <1>	773	387	611
			.8	20.00 <1>	10.17 <14>	52.60 <1>	489	97	594
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			3.0	10.17 <16>	10.17 <1>	46.06 <1>	193	147	542
895	.80 X	.40	.0	10.17 <16>	10.17 <15>	17.02 <1>	34	145	192
			1.5	10.17 <16>	14.57 <1>	13.98 <1>	8	364	158
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			3.0	45.24 <1>	20.16 <16>	65.12 <1>	984	492	735
929	.95 X	.30	.0	33.97 <1>	13.50 <15>	35.48 <1>	723	461	482
			1.5	11.20 <1>	9.18 <15>	33.08 <1>	340	138	449
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953	.95 X	.30	.0	9.18 <15>	18.80 <1>	23.57 <1>	202	351	320
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980	.80 X	.40	.0	30.63 <1>	14.25 <14>	52.28 <1>	714	357	590
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			2.3	10.17 <16>	10.17 <15>	47.73 <1>	178	89	539
			3.0	10.17 <16>	10.17 <1>	46.22 <1>	178	135	521
992	.80 X	.40	.0	10.17 <16>	10.17 <1>	16.88 <1>	0	132	190
			1.5	10.17 <16>	12.84 <1>	13.85 <1>	0	323	158
			3.0	10.17 <16>	15.85 <1>	10.81 <1>	0	394	122
			4.5	10.17 <14>	13.72 <1>	13.26 <1>	0	344	150
			6.0	10.17 <15>	10.17 <1>	16.29 <1>	0	174	184

1010	.80 X	.40	.0	10.17 <15>	10.17 <1>	54.68 <1>	215	193	517
			.8	10.17 <15>	10.17 <16>	56.20 <1>	215	108	534
			1.5	10.60 <1>	10.17 <16>	57.71 <1>	270	108	551
			2.3	22.74 <1>	10.17 <16>	59.23 <1>	549	108	568
			3.0	35.26 <1>	17.42 <16>	60.74 <1>	861	430	655
			1027	.95 X	.30	.0	28.72 <1>	13.32 <15>	32.32 <1>
1.5	9.57 <1>	9.18 <15>	29.92 <1>	293	118	406			
3.0	9.18 <16>	9.18 <15>	27.53 <1>	200	118	374			
4.5	9.18 <16>	11.36 <1>	25.13 <1>	200	345	341			
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			3.0	9.18 <15>	9.18 <16>	25.84 <1>	182	136	351
			4.5	9.18 <1>	9.18 <16>	28.23 <1>	238	118	383
			6.0	25.74 <1>	12.05 <16>	30.63 <1>	729	364	416
			1078	.80 X	.40	.0	27.12 <1>	12.74 <15>	49.35 <1>
.8	15.78 <1>	10.17 <15>				47.83 <1>	393	80	540
1.5	10.17 <1>	10.17 <15>				46.32 <1>	173	80	523
2.3	10.17 <16>	10.17 <15>				44.80 <1>	161	80	506
3.0	10.17 <16>	10.17 <1>				43.29 <1>	161	175	498
1094	.80 X	.40				.0	10.17 <16>	10.17 <1>	16.71 <1>
			1.5	10.17 <16>	14.05 <1>	13.68 <1>	0	352	154
			3.0	10.17 <17>	16.60 <1>	10.65 <1>	0	412	120
			4.5	10.17 <15>	13.98 <1>	13.42 <1>	0	350	151
			6.0	10.17 <15>	10.17 <1>	16.45 <1>	0	169	186
			1112	.80 X	.40	.0	10.17 <15>	10.17 <1>	55.33 <1>
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1128	.95 X	.30				.0	29.93 <1>	13.83 <15>	33.48 <1>
			1.5	10.55 <1>	9.18 <15>	31.08 <1>	321	123	422
			3.0	9.18 <16>	9.18 <1>	28.68 <1>	207	123	389
			4.5	9.18 <16>	11.21 <1>	26.28 <1>	207	340	357
			6.0	9.18 <14>	16.68 <1>	23.88 <1>	207	494	324
			1152	.95 X	.30	.0	9.18 <14>	16.64 <1>	19.35 <1>
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3.0	9.18 <14>	9.18 <1>				24.15 <1>	152	179	328
4.5	9.18 <1>	9.18 <17>				26.55 <1>	156	123	360
6.0	20.99 <1>	9.95 <17>				28.94 <1>	608	304	393

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**LAMPIRAN 4. GAMBAR BANGUNAN**

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