

TUGAS AKHIR

STUDI "SOFT STOREY EFFECT" PADA GEDUNG BERTINGKAT AKIBAT BEBAN GEMPA



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**JURUSAN TEKNIK SIPIL
FAKULTAS TEKNIK SIPIL DAN PERENCANAAN
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YOGYAKARTA
1996**

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**Diajukan kepada Universitas Islam Indonesia
untuk memenuhi sebagian persyaratan memperoleh
derajat Sarjana Teknik Sipil**

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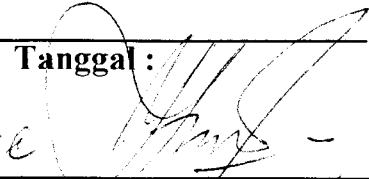
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KATA PENGANTAR

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Yazhidatul Asror

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ABSTRAK

Secara geografi Indonesia termasuk daerah potensial gempa yaitu terletak pada pertemuan Sirkum Pasifik dan Sirkum Mediterania. Mengingat besarnya-kerugian-kerugian yang ditimbulkan akibat gempa, maka sebaiknya benar-benar diperhatikan dalam perencanaan struktur. Dinding/*infill wall* biasanya kurang diperhatikan dalam pelaksanaan konstruksi karena hanya dipakai untuk pembatas ruangan atau sebagai partisi pengisi yang menahan gangguan dari angin dan cuaca. *Infill wall* merupakan non-struktural yang pada saat terjadi gempa akan sangat bervariasi perilakunya. Pada beberapa paper, gempa yang terjadi di Peru dan Mexico menunjukkan bahwa *infill wall* mempengaruhi kekakuan struktur lebih baik dan sekaligus lebih buruk/*soft storey effect*. Oleh karena itu dengan menganalisa retak-retak yang terjadi maka beberapa teori mengasumsikan *infill wall* sebagai *diagonal strut* dengan lebar efektif tergantung dari luasan dinding. Dengan menggunakan *Strukture Analysis Program 90* dianalisa kombinasi penempatan *infill wall* dari 3 model portal yang bervariasi baik tinggi tingkat maupun zone yang berbeda menurut Peraturan Gempa di Indonesia 1987. *Out program* berupa momen, gaya geser, dan defleksi baik pada kolom maupun balok ditabelkan dan dibuat grafik. Analisa secara umum menghasilkan beberapa kesimpulan salah satunya yaitu *infill wall* ternyata dapat berfungsi sebagai pengaku portal sehingga struktur akan dapat menahan gaya horisontal lebih baik karena sebagian gaya tersebut ditahan oleh *diagonal strut*. *Infill wall* akan berperilaku cukup baik apabila penempatannya menerus. Oleh karena itu ini merupakan salah satu alternatif yang baik dalam menerapkan konsep bangunan tahan gempa. Mengingat harga batu bata yang /batako yang relatif murah maka *infill wall* harus mulai diperhatikan dalam pelaksanaannya.

BAB I

PENDAHULUAN

1.1 Umum

Secara geografis Indonesia termasuk daerah potensial gempa yaitu terletak pada pertemuan Sirkum Pasifik dan Sirkum Mediterania. Mengingat besarnya kerugian-kerugian baik harta maupun jiwa yang terjadi akibat gempa, maka pengaruh beban gempa harus benar-benar diperhatikan dalam perencanaan struktur. Saat terjadi gempa pada struktur bangunan, getaran gempa dari lapisan tanah di bawah bangunan akan menggetarkan bangunan di atasnya dalam berbagai arah. Dua hal penting yang perlu dibahas dalam perencanaan struktur tahan gempa yaitu perilaku material dan struktur bangunan maupun komponen dari struktur bangunan itu sendiri.

Berbagai peraturan perencanaan bangunan terhadap beban gempa, termasuk pedoman perencanaan yang berlaku di Indonesia telah menetapkan suatu taraf pembeban gempa. Yaitu rencana pembebanan yang menjamin suatu struktur agar struktur tidak rusak apabila terjadi gempa kecil dan sedang, tetapi disaat terjadi gempa besar struktur tersebut masih mampu berperilaku daktil dengan memancarkan energi gempa sekaligus membatasi

beban gempa yang masuk dalam struktur. Dalam perencanaan bangunan tahan gempa, terbentuknya sendi-sendi plastis yang mampu memancarkan energi gempa dan membatasi beban gempa yang masuk dalam struktur, harus dikendalikan sedemikian rupa agar struktur berperilaku baik dan tidak runtuh saat terjadi gempa kuat. Pengendalian terbentuknya sendi-sendi plastis pada lokasi-lokasi tertentu harus direncanakan terlebih dahulu secara pasti terlepas dari kekuatan dan karakteristik gempa. Filosofi perencanaan seperti ini dikenal sebagai konsep Desain Kapasitas.

Dengan konsep desain kapasitas, untuk menghadapi gempa kuat yang mungkin terjadi pada periode tertentu, maka mekanisme keruntuhan suatu portal bangunan tingkat tinggi dapat dipilih sedemikian rupa, sehingga pemancaran energi gempa yang terjadi dapat berperilaku baik dan keruntuhan dapat dihindarkan.

1.2 LATAR BELAKANG MASALAH

Dalam perencanaan bangunan gedung tahan gempa, pada dasarnya akan dipengaruhi hal-hal sebagai berikut ini.

1. Tingkat kesulitan dalam perencanaan atau analisa.

Semakin tidak teraturnya bentuk dari suatu bangunan maka semakin sulit juga perencanaannya karena:

- a. tidak teraturnya bentuk sehingga diperlukan analisa dinamis,

- b. timbul kesulitan-kesulitan dalam perilakunya atau menentukan anggapan-anggapan yang diperlukan dalam analisa dinamis,
- c. keterbatasan kita dalam melakukan perhitungan sehingga memerlukan bantuan komputer untuk menyelesaikannya,
- d. pada pendetailan dari elemen-elemen strukturnya harus dibutuhkan perhatian yang khusus dalam perencanaan.

Tetapi kesemuanya itu belum bisa menjamin bahwa strukturnya akan berperilaku baik sewaktu terjadi gempa.

2. Perilaku bangunan sewaktu terjadi gempa, bentuk simetris dan sederhana cenderung mempunyai ketahanan lebih baik daripada struktur yang tidak simetris. Gaya puntir pada bentuk simetris relatif lebih kecil dibandingkan dengan bentuk yang tidak teratur, selain itu juga perlu diperhatikan elemen-elemen struktur dan non struktur.

Akibat beban gempa, *Soft storey* bisa terjadi karena kekakuan tingkat portal tidak seragam. Hal ini mungkin diakibatkan oleh perbedaan dimensi kolom atau akibat penempatan elemen-elemen non struktur seperti tembok pengisi yang penempatannya pada tingkat tertentu sehingga menimbulkan perbedaan kekakuan struktur. Pola keruntuhan struktur pada kolom dapat mengakibatkan *coloum sway mechanism* yang sangat membahayakan dan tidak diharapkan terjadi pada struktur. Tingkat dimana kekakuan relatif kecil

tersebut biasa disebut *soft storey*. Sendi plastis yang mungkin terjadi pada kolom akan mengakibatkan kegagalan pada struktur.

1.3 POKOK MASALAH

Bangunan yang baik adalah bangunan yang kekakuannya relatif merata mulai dari tingkat bawah sampai tingkat atas. Apabila ada salah satu tingkat yang kekakuannya lemah maka kemungkinan pada tingkat itulah yang akan mengalami keruntuhan akibat gempa. *soft storey* dapat disebabkan oleh:

1. ukuran kolom dari lantai bawah sampai lantai atas berubah secara drastis,
2. tinggi tingkat yang berlebihan terhadap tinggi tingkat yang relatif sama,
3. dinding tidak menerus dari tingkat atas sampai tingkat bawah,
4. dinding kantilever yang terputus dalam satu tingkat,
5. adanya balok yang tidak menerus/terpotong.

Banyak desain bangunan bertingkat seringkali mengabaikan faktor-faktor yang mempengaruhi *soft storey effect* karena hanya mengutamakan keindahan dan seni. Sebenarnya apabila faktor tersebut diabaikan maka bangunan tersebut akan mengalami keruntuhan/rusak pada waktu terjadi gempa.

1.4 RUMUSAN MASALAH

Bagaimana pengaruh *infill wall* pada gedung bertingkat terhadap struktur akibat beban gempa ?

1.5 BATASAN MASALAH

Mengingat banyaknya masalah yang ditimbulkan akibat beban gempa yang terjadi, maka tugas akhir ini membatasi permasalahan *soft storey efect* hanya pada permasalahan ke 3 yaitu dinding yang tidak menerus/mengalami pemutusan dari lantai atas sampai bawah.

Perhitungan Mekanika menggunakan *Structure Analisis Program 90* dua dimensi. Pada pembahasan diberikan gambaran seberapa jauh perilaku portal akibat penempatan *infill frame* yang dikombinasi. Hasil program berupa momen, gaya geser dan defleksi yang sedemikian banyak akan disederhanakan dalam bentuk tabel dan grafik.

Tinjauan Portal diambil dari proyek berikut ini.

1. Laboratorium Teknologi X ITB, Bandung (4 lantai).
2. Gedung Kampus Universitas Islam As-Syafi'iyah, Jakarta (8 lantai).
3. Graha Pangeran Building (BNI) , Surabaya (13 lantai).

1.6 TUJUAN DAN MANFAAT

Penulisan tugas akhir ini bertujuan untuk menerapkan konsep dasar perencanaan struktur tahan gempa pada gedung bertingkat yang aman bagi bangunan dan isinya. Dengan demikian hasil dari penulisan Tugas Akhir ini dapat diambil manfaatnya diantaranya:

1. bahan masukan dan dapat digunakan untuk merencanakan bangunan bertingkat,
2. memberikan gambaran perencanaan bagi pembaca yang sedang atau akan mempelajari ilmu yang berhubungan dengan gempa khususnya *soft storey efect*.

1.7 LINGKUP BAHASAN

Lingkup Pembahasan Tugas Akhir ini mengenai perilaku Portal akibat beban gempa dan non struktur yang mempengaruhinya (bata/batako) yang mana bata/batako diasumsikan sebagai *diagonal strut* yang lebar efektifnya tergantung dari perbandingan tinggi tingkat dengan bentang panel. Perhitungan mekanika digunakan *Structure Analisis Program 90* dua dimensi dengan beban-beban yang bekerja meliputi beban mati, beban hidup dan beban gempa. Hasil/kesimpulan dari penyusunan mengacu pada *output* program dan pembahasan.

BAB II

TINJAUAN PUSTAKA

2.1 Umum

Suatu struktur harus menjamin keamanan penghuninya, termasuk keamanan terhadap gempa. Tetapi perlu disadari juga bahwa tidaklah ekonomis jika suatu struktur dirancang untuk tahan terhadap gempa tanpa sedikitpun mengalami kerusakan. Oleh karena itu *Aplied Technology Council (ATC)* di Amerika Serikat pada tahun 1978 menyarankan agar struktur harus dapat:

1. menahan gempa kecil tanpa mengalami kerusakan,
2. menahan gempa sedang tanpa mengalami kerusakan konstruksi, walaupun kerusakan pada bagian non-struktur masih diperbolehkan,
3. menahan gempa besar tanpa runtuh, walaupun kerusakan konstruksi tidak dapat dihindari. Yang terpenting adalah tidak ada korban jiwa.

Gempa bumi mengguncang struktur dalam segala arah, tetapi dalam analisis biasanya diasumsikan menjadi gaya arah horisontal dan vertikal¹.

¹ K.Muto, Analisis Perencanaan Gedung Tahan Gempa, 1997

Akibat gempa terjadi gaya inersia (internal) pada titik-titik massa struktur. Struktur jarang sekali mengalami keruntuhan akibat gaya gempa pada arah vertikal. Hal ini disebabkan karena gaya gempa pada arah vertikal hanya memberikan pengaruh kecil terhadap gaya gravitasi yang bekerja pada struktur, dan pada umumnya struktur dirancang sudah memiliki faktor keamanan yang memadai terhadap gaya vertikal. Yang perlu mendapat perhatian disini adalah gaya gempa pada arah horisontal, karena pada titik-titik lemah struktur yang kekakuannya tidak memadai kemungkinan besar akan mengalami keruntuhan pada bangunan.

Untuk meningkatkan daya tahan struktur tingkat banyak terhadap bahaya gempa (terutama daya tahan horisontal), Ada tiga sistem struktur yang dapat digunakan yaitu:

1. portal terbuka (*open frames*),
2. portal dinding (*walled frames*),
3. dinding geser (*shear walls*) dan portal dengan penyokong diagonal (*diagonally braced frames*)

Portal terbuka segi empat yang terdiri dari kolom dan balok dengan hubungan monolit membentuk ruangan yang besar dan memberikan daya tahan horisontal pada kerangka keseluruhan. Oleh karena itu sistem seperti ini diperlukan pada konstruksi gedung yang bertingkat banyak. Pada struktur beton bertulang dan yang sejenis, kekuatan batang tidak begitu besar sehingga

daya tahan terbatas. Pada gedung bertingkat banyak pemakaian gabungan portal terbuka dan dinding geser umumnya banyak digunakan untuk mengatasi deformasi yang besar pada tingkat-tingkat bawah tetapi dinding geser sendiri biasanya akan menimbulkan masalah pada tingkat-tingkat atas. Namun kekuatan dapat ditingkatkan dengan menggunakan portal terbuka konstruksi baja struktural murni kuat. Dengan adanya baja yang berkekuatan tinggi, perencanaan gedung bertingkat banyak terutama gedung pencakar langit dapat hanya dengan portal terbuka.

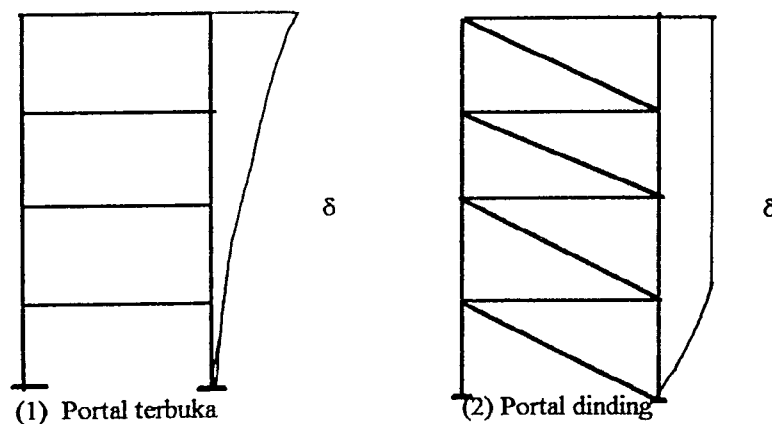
Portal dinding, balok tinggi (biasanya bagian bawah jendela dianggap sebagai balok). Struktur seperti ini akan menunjukkan daya tahan dan ketegaran yang beberapa kali lebih tinggi daripada portal terbuka biasa. Portal dinding merupakan sistem penahan gempa yang rasional tetapi perencana tidak menyukai karena struktur ini dapat menimbulkan *short coloum dan beam effect*². Hal ini juga tidak disukai para arsitek karena bidang kolom dan balok yang besar membatasi tampak gedung sehingga segi artistiknya kurang baik.

Portal dengan penyokong diagonal dan dinding geser, dinding geser dengan lebar yang besar akan menghasilkan daya tahan lentur dan geser yang sangat tinggi dan merupakan sistem struktur yang paling rasional dengan memanfaatkan sifat-sifat beton bertulang. Pada konstruksi baja struktural,

² Eigt WCEE, Non struktural elements, San Fransisco journal 1984

portal-portal dengan penyokong (*bracing*) merupakan sistem struktur yang efektif dan kuat.

Portal dengan dinding penuh akan menambah kekakuan struktur cukup besar dan relatif mampu menahan gaya horisontal. Kemampuan menahan gaya horisontal tergantung pada luasan dinding dan Modulus Elastis dinding/batako yang mendukungnya. Terhadap beban horisontal dinding dapat menahan gaya sampai pada tahap tertentu. Dinding dimodel sebagai *diagonal strut* yang dapat berfungsi sebagai *brace* pada portal³. Deformasi antara portal terbuka dan *infill wall* akan berlainan karena *brace frame* mampu menahan gaya horisontal (gambar 2.1). Terlihat bahwa *deflected shape* pada *infill wall* lebih kecil dibanding portal terbuka.



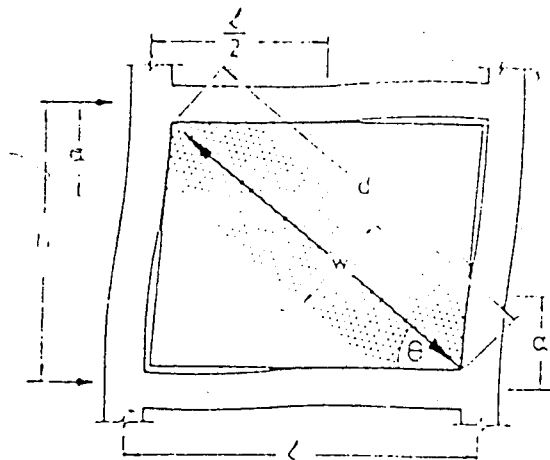
Gambar 2.1 Deformasi Portal Terbuka dan Portal Dinding¹

³ T.Paulay & MJN Priestley, Seismic Assessment and Retrofit of Concrete Structure

2.2 Tinjauan Analisa Portal Dinding

Dinding merupakan non struktur yang biasanya dikategorikan sebagai beban sama seperti partisi-partisi yang lain. Dinding sebenarnya mampu sebagai penambah kekuatan apabila penempatannya secara penuh pada seluruh portal dan dipasang menempel dengan balok maupun kolom. Balok pada dinding yang biasanya di bawah jendela sebenarnya menambah kekakuan, tetapi untuk mempermudah analisa diasumsikan sebagai dinding kosong.

Infill wall diasumsikan sebagai *diagonal strut* yang mampu menahan gaya horisontal, dengan lebar efektif (w) dan panjang (d) yaitu sepanjang ujung portal dengan ujung diagonal portal. Tebal *diagonal strut* tergantung ukuran bata/batako yang dipergunakan⁴. Untuk lebih jelasnya dapat dilihat pada gambar 2.2.



Gambar 2.2 *Diagonal Strut Pada Infill Wall*⁴

⁴ M.J. Nigel Priestley, Towards a Capacity-Design Assessment Prosedure for Reinforced Concrete Frames, Seminar May 1992, NZCC.

Lebih lanjut M.J. Nigel Priestley mengulas pengaruh-pengaruh kemungkinan struktur akan mengalami perubahan *displacement* pada struktur portal dengan dinding penuh. *Diagonal strut* akan diselidiki kemampuannya terhadap respon gaya gempa. Perubahan momen, gaya geser, defleksi dapat dilihat akibat adanya *diagonal strut* dan perilaku yang ditimbulkan terhadap kekakuan kolom. *Soft storey* ditinjau akibat kombinasi *infill wall* diubah penempatannya. Untuk lebih jelasnya masalah ini bisa dilihat pada Tinjauan Teoritis dan Pembahasan.

BAB III
TINJAUAN TEORITIS STRUKTUR TAHAN GEMPA DI
INDONESIA

3.1 Umum

Gempa merupakan gejala alam yang tidak mungkin kita hindari, tetapi tetap harus diusahakan sedemikian rupa agar gempa yang terjadi tidak diterjemahkan sebagai suatu bencana yang tidak akan ditanggulangi. Oleh karena itu dalam disain bangunan, diharapkan bangunan yang bersangkutan tidak mengalami kerusakan apabila terjadi gempa kecil. Apabila terjadi gempa sedang maka bangunan tersebut diharapkan hanya terjadi kerusakan non struktur dan apabila terjadi gempa besar maka bangunan yang direncanakan boleh terjadi kerusakan struktur tetapi tidak terjadi keruntuhan.

Indonesia telah menetapkan suatu peraturan yang dapat dipakai pegangan dalam perencanaan bangunan tahan gempa yaitu Peraturan Perencanaan Tahan Gempa Indonesia Untuk Gedung 1987 (PPTGIUG 87). Pada perencanaan portal, beban-beban yang bekerja meliputi beban mati, beban hidup, beban gempa. Beban-beban tersebut harus benar-benar

diperhatikan sehingga struktur yang direncanakan aman. Dinding merupakan non-struktur yang biasanya dianggap beban pada portal. Dinding sebenarnya mampu memberikan tambahan kekakuan pada portal apabila penempatannya diperhatikan, tetapi perilaku portal akan berubah apabila tidak benar dalam penempatannya. Batu bata/Batako dengan lebar efektif (w), seperti disebut di depan pada kenyataannya mampu memberikan tambahan kekuatan dalam menahan gaya lateral. Kekakuan portal harus lebih besar daripada kekakuan dinding. Apabila kekakuan portal lebih kecil daripada kekakuan dinding pengisi/*infill frame* maka pola keruntuhan dapat membahayakan struktur.

3.2 Tinjauan Teoritis Struktur Tahan Gempa

3.2.1 Prinsip Dasar Gaya Gempa

Dalam perencanaan struktur beton di daerah gempa, dikenal suatu konsep pembebanan yakni struktur beton selama masa layanannya akan dibebani berkali-kali oleh gempa kecil sampai sedang, yang mempunyai waktu ulang 20 tahun sampai 50 tahun. Struktur beton selama masa layanannya harus dapat menahan beban gempa yang besar, yang pada waktu ulangnya dapat terjadi sekali dalam 200 tahun. Gempa kecil atau sedang adalah beban gempa yang tercantum dalam Peraturan Perencanaan Tahan Gempa Indonesia Untuk Gedung 1987 (PPTGIUG), yang mana Indonesia

dibagi dalam enam zona. Besarnya beban gempa ini tergantung dari waktu getar struktur beban tersebut. Besarnya gaya gempa dinyatakan dalam:

$$V = C * I * K * W_t \quad (3.1)$$

Yangmana:

V = beban geser dasar akibat gempa

C = koefisien gempa dasar

I = faktor keutamaan

K = faktor jenis struktur

W_t = kombinasi beban dalam arah vertikal

Beban gempa besar adalah gempa dengan waktu ulang 200 tahunan. Sehingga disyaratkan agar struktur beton selain mempunyai kekuatan dan kekakuan yang cukup, juga mempunyai daktilitas yang besar, yang dinyatakan dalam nilai m (tingkat daktilitas struktur = m).

3.2.2 Bentuk Dan Konfigurasi

Suatu konstruksi yang ideal sulit untuk direncanakan apabila memenuhi semua syarat-syarat yang diijinkan. Tetapi beberapa pedoman dasar dapat dipakai dalam merencanakan konfigurasi, yaitu:

1. bangunan sebaiknya mempunyai bentuk yang simetris,
2. bangunan sebaiknya mempunyai bentuk yang sederhana,

3. bangunan tidak terlalu langsing, baik pada denah maupun potongannya,
4. distribusi kekuatan sepanjang tinggi bangunan seragam dan menerus,
5. kekakuan yang cukup,
6. terbentuknya sendi plastis harus terjadi pada elemen-elemen horisontal lebih dahulu dibanding dengan elemen vertikal,
7. perlu detail yang daktil pada sendi-sendi plastis.

Dari pengalaman kerusakan struktur akibat gempa, telah terbukti bahwa struktur yang mempunyai bentuk sederhana dan simetris lebih tahan terhadap gempa, hal ini disebabkan karena:

1. apabila terjadi ketidaksimetrisan struktur pada kedua arah x dan y, akan dapat mengakibatkan ketidaksamaan respon struktur dalam arah-arrah tersebut. Keadaan yang demikian dapat mengakibatkan torsi yang sulit diperkirakan sehingga dapat merusak struktur bila terjadi gempa,
2. struktur yang sederhana dan simetris lebih mudah diketahui perilakunya daripada yang rumit.

3.2.3 Distribusi Kekuatan Sepanjang Tinggi Bangunan

Struktur bangunan akan mempunyai ketahanan terhadap gempa bila dipengaruhi syarat-syarat dibawah ini:

1. distribusi kekuatan secara seragam,

2. semua kolom dan dinding menerus tanpa adanya pemutusan mulai dari atap sampai pondasi.

Disamping itu, sebaiknya harus dipertimbangkan pula beberapa hal agar bangunan tahan terhadap gempa, yaitu:

1. semua balok berhubungan secara menerus,
2. penampang-penampang penahan beban gempa tidak boleh berubah secara tiba-tiba,
3. struktur beton harus menerus baik pada kolom maupun balok dan harus sedapat-dapatnya monolit pada sambungan.

Bila syarat-syarat tersebut diikuti maka struktur akan lebih baik perilakunya. Konsentrasi tegangan-tegangan yang tidak dikehendaki dapat dihindari, dan momen torsi yang besar dapat pula dihindari.

Masalah yang amat penting untuk diselesaikan dalam perencanaan struktur adalah terputusnya elemen vertikal penahan beban gempa. Bila ini terjadi maka bagian struktur yang kekakuannya relatif tidak sama pada tingkat tertentu dapat mengakibatkan keruntuhan yang disebut *soft storey*. Dengan mengikuti syarat-syarat ini maka pendetailannya akan lebih mudah dan penerusan gaya-gaya dari balok ke kolom lewat pertemuannya lebih sederhana .

3.3 Taraf Pembebanan Gempa dan Peta Wilayah Gempa di Indonesia

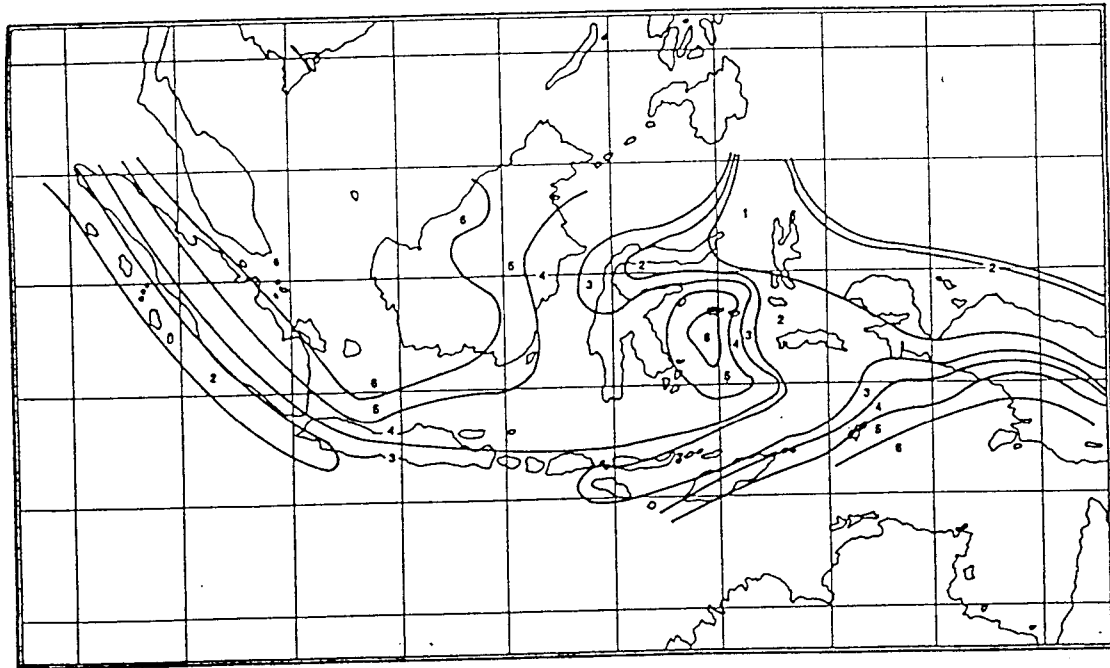
3.3.1 Taraf Pembebanan Gempa

Dengan berpedoman pada konsep perencanaan yang dapat menjamin struktur tidak runtuh walaupun mengalami deformasi in-elastis yang cukup besar pada saat terjadi gempa, maka struktur tidak lagi perlu direncanakan agar tetap dalam batas elastis. Suatu taraf pembebanan gempa yang sekian kali lebih kecil dari beban gempa maksimum dinyatakan sebagai beban gempa rencana. Dengan beban rencana seperti itu struktur dapat didisain lebih ekonomis. Besarnya taraf pembebanan tidak sama di wilayah Indonesia, melainkan bervariasi dari satu wilayah ke wilayah yang lain. Perbedaan ini dikarenakan kondisi seismotektonik, geografi, dan geologis setempat. Dalam wilayah Indonesia terdapat beberapa daerah dengan perbedaan tingkat resiko gempa yang cukup berarti. Tentu saja hal ini menyebabkan perlunya penentuan taraf pembebanan gempa yang berbeda-beda pula.

3.3.2 Peta Wilayah Gempa di Indonesia

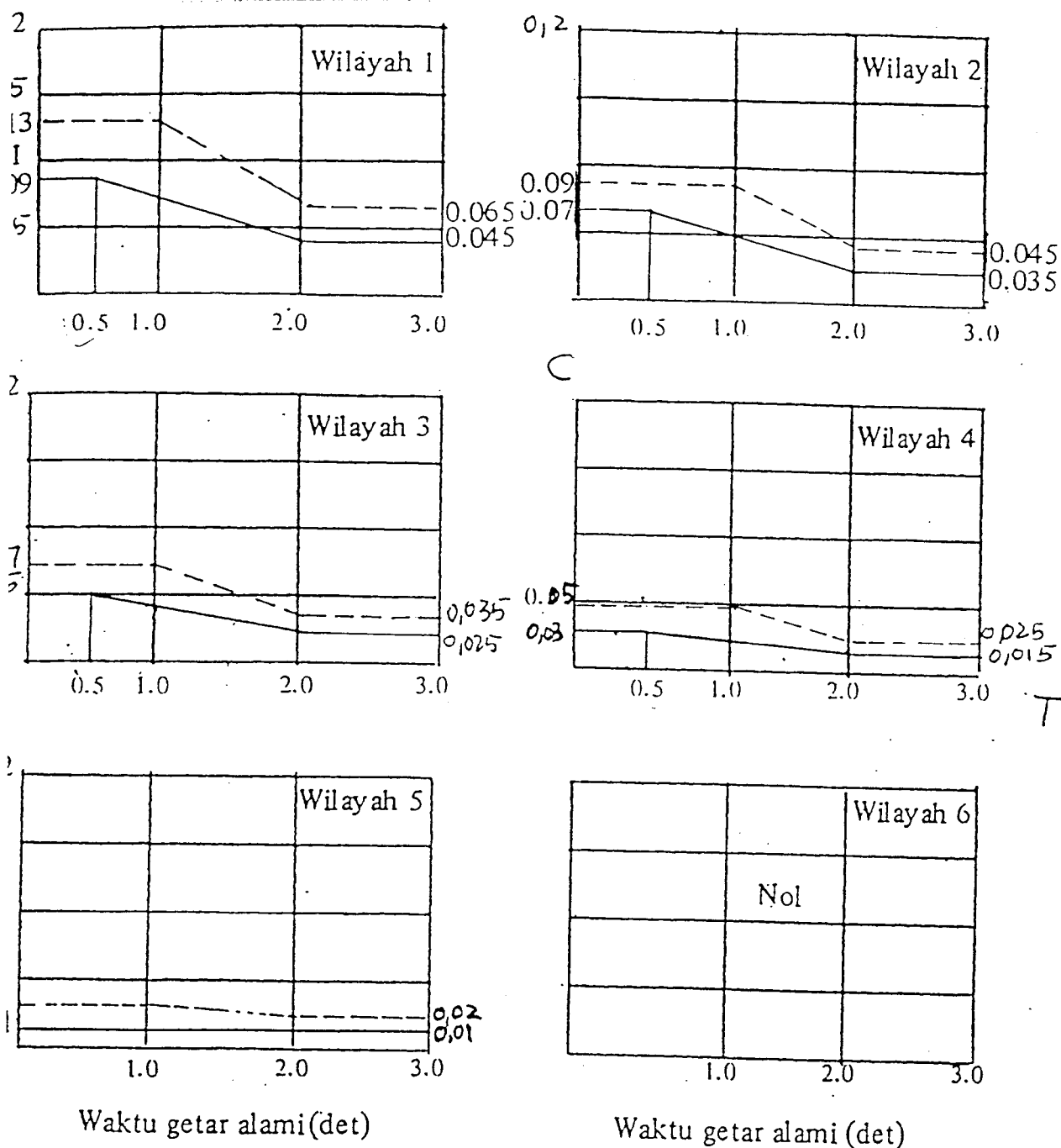
Untuk keperluan perencanaan, dirasakan lebih praktis untuk menggunakan suatu peta wilayah gempa di Indonesia. Gempa dibagi menjadi 6 wilayah seperti pada gambar 3.1. Suatu lokasi gedung terletak dekat pada batas wilayah sehingga kepastian wilayahnya tidak jelas, maka gedung tersebut harus dianggap terletak di dalam yang mensyaratkan nilai koefisien

gempa dasar yang lebih besar. Koefisien gempa dasar (C) ditentukan pada gambar 3.2 dengan waktu getar (T). Dua jenis tanah bawah harus dibedakan dalam memilih nilai C, yaitu tanah keras dan tanah lunak.



Gambar 3.1 Peta Wilayah Gempa Di Indonesia⁵

⁵ Peraturan Perencanaan Tahan Gempa Indonesia Untuk Gedung 1997, PU.



keterangan

..... Struktur diatas tanah lunak

_____ Struktur diatas tanah keras

Gambar 3.2 Grafik Koefisien Gempa Dasar⁵

⁵ PPTUGUG 1987,PU.

3.4 Perencanaan Struktur dengan Daktilitas Penuh

3.4.1 Konsep Daktilitas Struktur

Daktilitas adalah kemampuan suatu struktur gedung atau unsur struktur gedung untuk mengalami simpangan-simpangan plastis secara berulang-ulang dan bolak-balik diatas titik leleh pertama sambil mempertahankan kemampuan awalnya dalam memikul beban. Dewasa ini telah disepakati secara umum, bahwa tidaklah ekonomis dalam merencanakan struktur bangunan untuk berperilaku elastis bila mengalami gempa yang sangat kuat (gempa maksimum) yang diharapkan akan terjadi di lokasi struktur tersebut.

Pada umumnya dewasa ini dianut prinsip perencanaan bangunan tahan gempa, bertujuan menghasilkan kekuatan struktur yang memenuhi dua kriteria sebagai berikut ini.

1. Struktur bangunan harus didisain dengan kekuatan dan kekakuan cukup sedemikian rupa, sehingga akibat gempa dengan kekuatan sedang hanya akan mengalami kerusakan ringan yang masih dapat diperbaiki.
2. Kekuatan dan kekakuan struktur yang direncanakan untuk memenuhi kondisi diatas adalah juga cukup untuk memberikan kemampuan kepada struktur bangunan untuk melakukan deformasi (simpangan) yang bersifat in-elastik tanpa runtuh, bila mengalami gempa yang

kuat (gempa maksimum) yang dapat diharapkan akan terjadi di lokasi struktur tersebut.

Struktur bangunan yang memiliki kemampuan untuk melakukan simpangan yang bersifat in-elastik tanpa runtuh, deformasi secara berkelanjutan tanpa mengalami penurunan kekuatan saat simpangan awal dikatakan memiliki daktilitas yang tinggi.

Agar struktur-struktur gedung tinggi memiliki daktilitas yang tinggi sendi-sendi plastis yang terjadi akibat beban gempa yang sangat kuat, harus terjadi di dalam balok dan tidak terjadi pada kolom. Hal ini hanya tercapai bila kapasitas (momen leleh) kolom adalah lebih tinggi dari kapasitas (momen leleh) balok. Konsep struktur yang memiliki karakteristik kapasitas unsur-unsurnya seperti ini disebut konsep kolom kuat-balok lemah.

3.4.2 Tingkat Daktilitas Struktur

Ketentuan untuk merencanakan tahanan gempa diklasifikasikan sebagai berikut ini.

1. Tingkat Daktilitas 1

Struktur beton diproporsikan sedemikian hingga ketentuan tambahan atas penyelesaian detail struktur sangat sedikit. Struktur sepenuhnya berperilaku elastis, maka daktilitas simpangan (μ) = 1. Beban gempa rencana harus dihitung berdasarkan faktor $K = 4$

2. Tingkat Daktilitas 2

Struktur beton diproporsikan berdasarkan suatu ketentuan penyelesaian detail khusus yang memungkinkan struktur memberikan respon in-elastis terhadap beban siklis yang bekerja tanpa mengalami keruntuhan getas, maka daktilitas simpangan (μ) = 2. Kondisi ini dinamakan juga kondisi daktilitas terbatas. Dalam hal ini beban gempa rencana harus diperhitungkan dengan menggunakan nilai K minimum = 2.

3. Tingkat Daktilitas 3

Struktur beton diporposikan berdasarkan suatu ketentuan penyelesaian detail khusus yang memungkinkan struktur memberikan respon in-elastik terhadap beban siklis yang bekerja tanpa mengalami keruntuhan getas, maka daktilitas simpangan (μ) = 3 Kondisi ini dinamakan juga kondisi daktilitas penuh dalam hal ini beban gempa rencana harus diperhitungkan dengan menggunakan nilai faktor K minimum =1

3.5 Kekakuan Struktur Rangka Terbuka

Untuk menyatakan kekakuan suatu portal secara sederhana dapat diperoleh dengan menyederhanakan persoalan yaitu portal mempunyai balok

kaku tak terhingga, sehingga kekakuan portal dinyatakan sebagai jumlah dari kekakuan tiap-tiap kolomnya.

$$\text{Kekakuan kolom} = \frac{12 \cdot E_c \cdot I}{H^3} \quad (3.2)$$

yangmana: E_c = Modulus elastisitas dari beton

I = Momen inerti dari penampang kolom

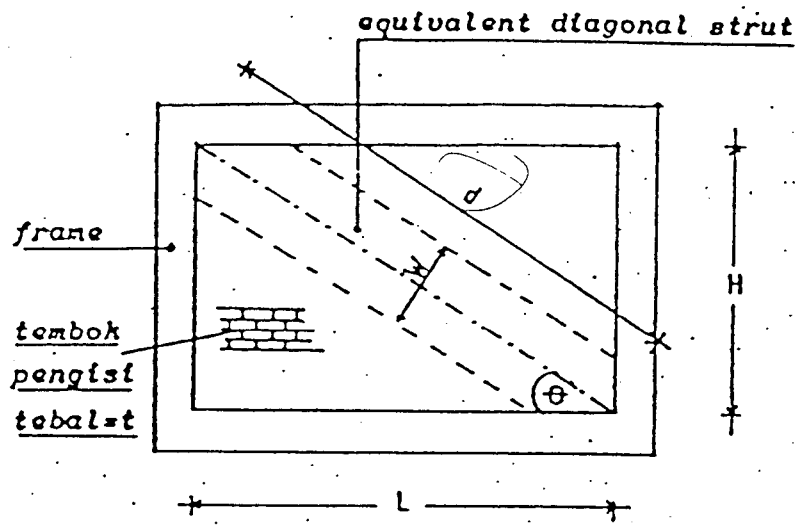
H = Tinggi bersih dari kolom

Akibat pengaruh retak pada beton kekakuan dari kolom dapat berkurang, menurut penyelidikan akibat retak pada beton kekakuan kolom dapat berkurang menjadi 33% sampai 75%. Karena dengan menganggap balok beton kaku tak terhitung, maka anggapan kekakuan struktur agak *overestimate*, oleh karena itu kekakuan kolom direduksi menjadi 33%⁶.

3.5.1 Kekakuan dinding pengisi bata merah

Dinding pengisi untuk suatu struktur secara teoritis dapat dilihat pada gambar 3.3. Gaya horisontal (F_j) yang bekerja pada dinding pengisi akan mengakibatkan simpangan (Δ). Pada rumus 3.3 diuraikan besar simpangan yang terjadi⁶.

⁶ Beca Carter Hollings and Ferner Ltd. (INDONESIAN EARTQUAKE STUDY) vol.7



Gambar 3.3 Gambar *Equivalent Diagonal Strut*

$$\Delta = \frac{F_j}{\cos \theta} + \frac{d}{w \cdot t} + \frac{1}{E_{bata}} + \frac{1}{\cos \theta} \quad (3.3)$$

Lebar *Diagonal Strut* (w)

Menurut penyelidikan lebar *diagonal strut* tergantung pada besarnya perbandingan tinggi portal terhadap bentang panel atau perbandingan kekakuan rangka/kekakuan dinding. Tetapi untuk dinding yang dibuat dari batu-bata di Indonesia perbandingan beban terhadap beban runtuh dinding adalah sangat rendah, jadi harga λH (perbandingan kekakuan rangka/kekakuan dinding) dapat diambil sebesar 2, sehingga harga w dapat ditentukan dari tabel dibawah ini:

Tabel 3.1 Perbandingan nilai pada *infill wall*⁷

L/H	1,0	1,5	2,0	2,5
w/d	0,45	0,40	0,34	0,30

catatan : - nilai w/d dalam tabel tersebut ditentukan dari grafik

- w/d dinyatakan dengan β

dan pada saat terjadi sendi Plastis w bisa diambil $0.25 d^8$

Parameter lainnya yang diperlukan dalam perhitungan kekakuan dinding adalah “tebal efektif dinding”. Karena plesteran dan batu-bata mempunyai modulus elastisitas yang tidak sama, maka tebal efektif dari kedua bahan ini harus dihitung.

Dalam praktek tebal plesteran diambil sebesar 15% dari tebal tembok, sehingga tebal dinding efektif terhadap tebal dinding dapat dinyatakan sebagai berikut⁹ :

$$t + 0,15t * \frac{E \text{ plesteran}}{E \text{ bata}} = 3,25 t \approx 3t \quad (3.4)$$

⁷ Stafford Smith, B and Carter, “A Method of Analysis for Infilled Frame” Inst. of Civil Eng., 1969.

⁸ Paulay T, 1988, “Seismic Design of Reinforced Concrete and Masonry Building”, The State of the Art in New Zealand.

⁹ Gideon Kusuma dkk, “Perencanaan Struktur Tahan Gempa Jenis A,B2,D”, 1984

Perubahan bentuk horisontal dari dinding akibat beban horisontal F_j selanjutnya dapat ditulis sebagai berikut:

$$\Delta = \frac{F_j}{\cos \theta} * \frac{d}{\beta d \times 3t} * \frac{1}{E \text{ bata}} * \frac{1}{\cos \theta} \quad (3.5)$$

karena $\cos \theta = L/D$ dan $d = \sqrt{(H^2 + L^2)}$

$$\text{jadi kekakuan dinding bata merah} = E \text{ bata} * \beta * 3t * \frac{L^2}{(H^2 + L^2)} \quad (3.6)$$

3.5.2 Kekakuan dinding Pengisi Batako

Dengan cara yang sama seperti digunakan pada penurunan rumus kekakuan untuk dinding pengisi bata merah, kecuali tebal efektif dinding diambil = t (karena tidak ada plesteran) maka kekakuan dinding pengisi batako adalah sebagai berikut:

$$E \text{ batako} * \beta * t * \frac{L^2}{(H^2 + L^2)} \quad (4.7)$$

3.5.3 Pola Keruntuhan /*Knee Braced Frame*

Bila suatu *infilled frame* dibebani dengan beban horisontal maka pada bagian dari dinding tersebut terlihat terpisah dari rangkanya (lihat gambar 3.6(a)). Bagian-bagian yang masih mempunyai bidang kontak dengan rangkanya terlihat dengan ujung-ujung *diagonal compression strut* (lihat

gambar 3.6(a)). Dengan perilaku seperti itu maka *infilled frame* (struktur rangka dengan dinding pengisi) dapat diidealisasikan sebagai rangka kosong dengan diagonal desak/*compression strut* (lihat gambar 3.6(b)). Apabila beban horisontal diperbesar maka beberapa *mode of failure* (pola keruntuhan) yang mungkin terjadi sebagai berikut ini.

1. Jika kekuatan rangka (*frame*) > kekuatan dinding (*infill*).

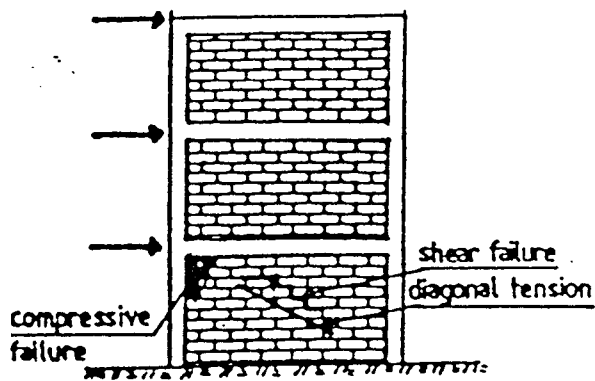
Pada keadaan ini pola keruntuhan yang terjadi adalah sebagai berikut:

- a. *diagonal tensile Cracking* (retak tarik diagonal) lihat gambar 3.4,
- b. *diagonal Shear Cracking* (retak geser diagonal) lihat gambar 3.4,
- c. *compressive Colapse* pada ujung-ujung sudut yang dibebani ter-
lihat pada gambar 3.4.

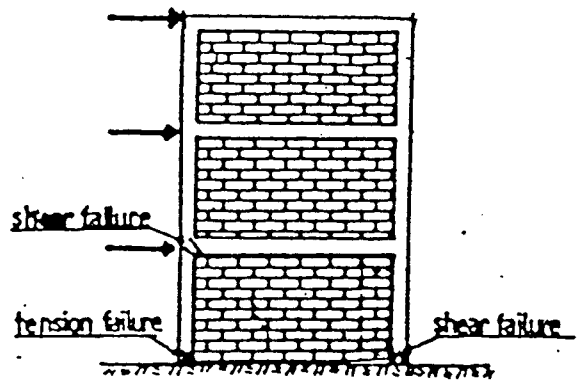
2. Jika kekuatan dinding (*infill*) > Kekuatan rangka (*frame*)

Untuk keadaan ini pola keruntuhan yang terjadi adalah sebagai berikut:

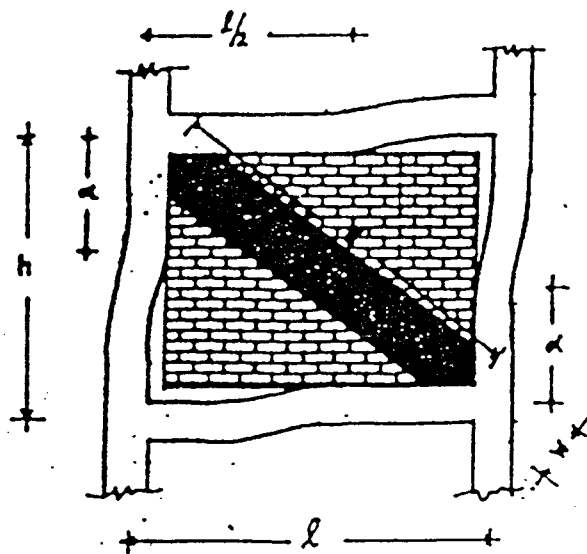
- a. keruntuhan tarik pada ujung bawah kolom tingkat dasar (lihat gambar 3.5),
- b. keruntuhan geser pada kolom atau pada balok dengan sambungannya terhadap kolom (lihat gambar 3.5),
- c. keruntuhan lentur atau geser masing-masing pada tengah kolom dan 1/4 tinggi kolom, akibat *knee braced frame concept for sliding shear failure* lihat gambar 3.7.



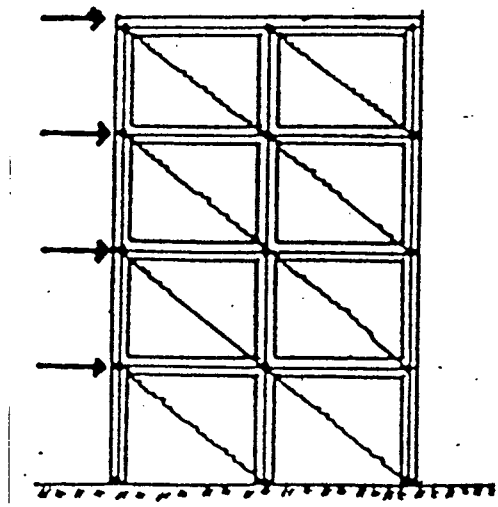
Gb. 3.4 Modes of Infill Frame



Gb. 3.5 Modes of frame failure

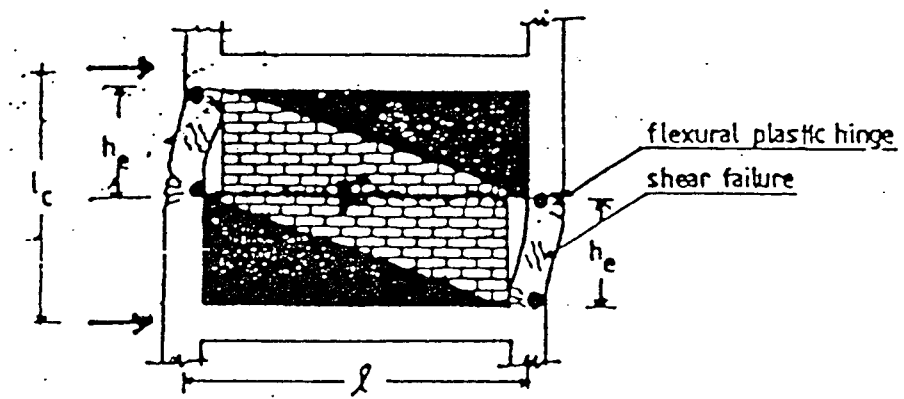


a. Deformation under shear load



b. Equivalent braced frame

Gb. 3.6 Infill Wall Concept



Gb. 3.7 Knee Braced Frame Concept For
Sliding Shear Failure⁹

BAB IV

MODEL KAJIAN

4.1 Umum

Pada perhitungan mekanika, untuk memperoleh data yang akurat diambil contoh 3 portal 2 dimensi. Perencanaan pembebanan meliputi beban mati, beban hidup dan beban gempa. Berat sendiri meliputi berat balok, berat pelat, tembok, spesi, plafon dan yang melengkapinya dikategorikan sebagai beban mati. Beban berguna dan air hujan dikategorikan sebagai beban hidup. Seluruh pembebanan diambil dari “Tata Cara Perencanaan Pembebanan untuk rumah dan Gedung 1987”. Gaya geser horisontal akibat beban gempa dipengaruhi oleh berat total portal yang juga meliputi pengaruh waktu getar bangunan (T), koefisien gempa dasar, faktor keutamaan (I), faktor jenis struktur (K), gaya geser horisontal total akibat gempa dan distribusi gaya geser pada portal. Peraturan yang digunakan “Peraturan Perencanaan Tahan Gempa Indonesia untuk Gedung 1987 (PPTGIUG 87).

Perhitungan mekanika digunakan *Structure Analisis Program 90* dengan 2 dimensi. *Input program* meliputi koordinat join, penempatan frame, dimensi balok/kolom, jumlah pembebanan beban hidup dan mati, beban gempa, *type restraint*, modulus Elastis Beton, kombinasi pembebanan dan instrumen lain yang melengkapinya. *Output* program berupa gambar diambil dari Saplot dan hasil perhitungan berupa momen, gaya geser, defleksi diambil dari file.f3f dan file.sol.

Infill frame dibuat bervariasi penempatannya mulai dari portal penuh sampai portal terbuka. Tembok dimodel sebagai *diagonal strut* dengan dimensi menurut Tabel 3.1.

Karena perhitungan pembebanan relatif sama maka dalam bab ini hanya diambil contoh pada model kajian 2 sedang perhitungan model kajian lain dapat dilihat pada lampiran.

Model Kajian gedung yang kami ambil sebagai berikut ini.

1. Gedung Laboratorium Teknologi ITB, bertingkat 4.
2. Gedung Kampus As-syafiiyah Jakarta, bertingkat 8.
3. Gedung BNI Surabaya, bertingkat 13.

Ketiga gedung ini diambil portal yang simetris untuk mempermudah dalam menganalisa hasil yang diperolehnya.

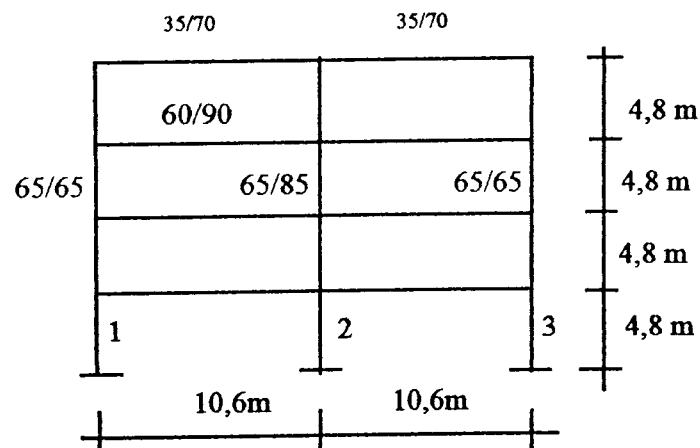
4.2 Perhitungan Pembebanan

4.2.1 Gedung Laboratorium Teknologi X ITB

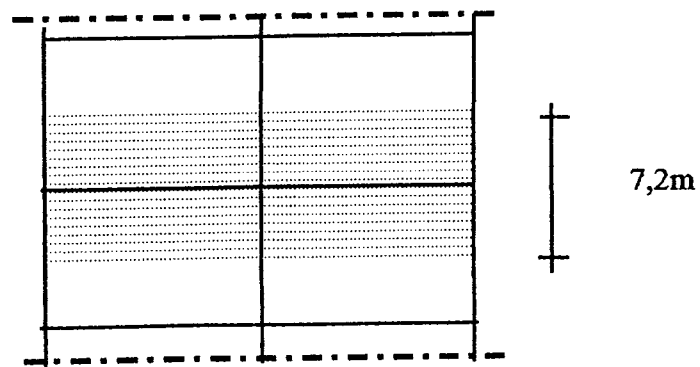
Gedung bertingkat 4 di daerah bandung dengan wilayah gempa 3

Tingkat	1	2	3	4
Kolom 1&3	65/65	65/65	65/65	65/65
Kolom 2	65/85	65/85	65/85	65/85
Balok Atap	35/70	35/70	35/70	35/70
Balok Lantai	60/90	60/90	60/90	60/90

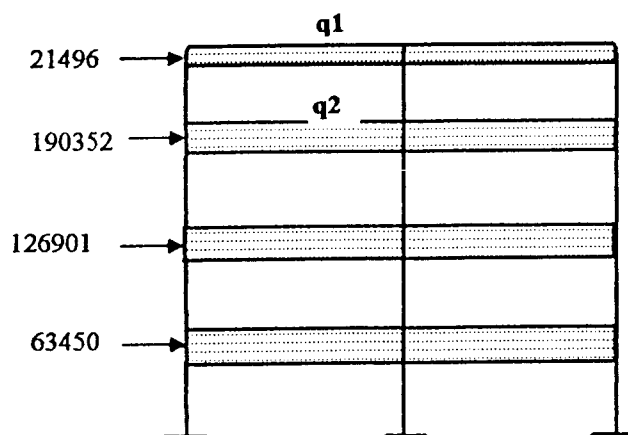
Model Portal



Denah Portal



Pembebanan pada portal penuh (I-P)



Gambar 4.1 Model Kajian 1

Mutu balok : k 250 ; Mutu kolom : k 250

$$q_{ekiv} = (q \text{ mati} + q \text{ hidup})$$

$$q_{ekiv.1} = 1550 \text{ kg/m} + 490 \text{ kg/m} = 2040 \text{ kg/m}$$

$$q_{ekiv.2} = 3646 \text{ kg/m} + 900 \text{ kg/m} = 4546 \text{ kg/m}$$

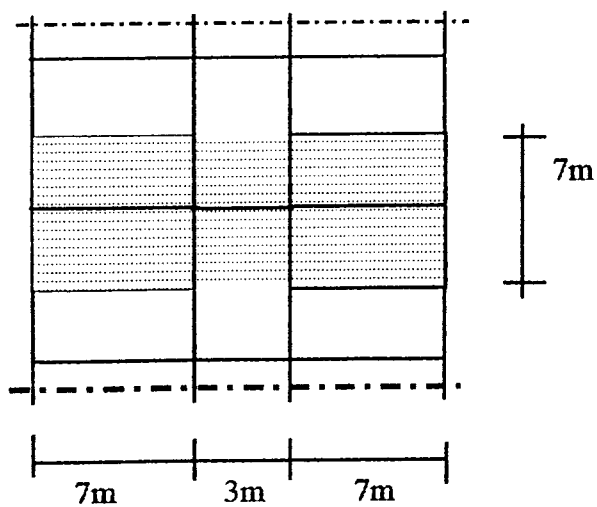
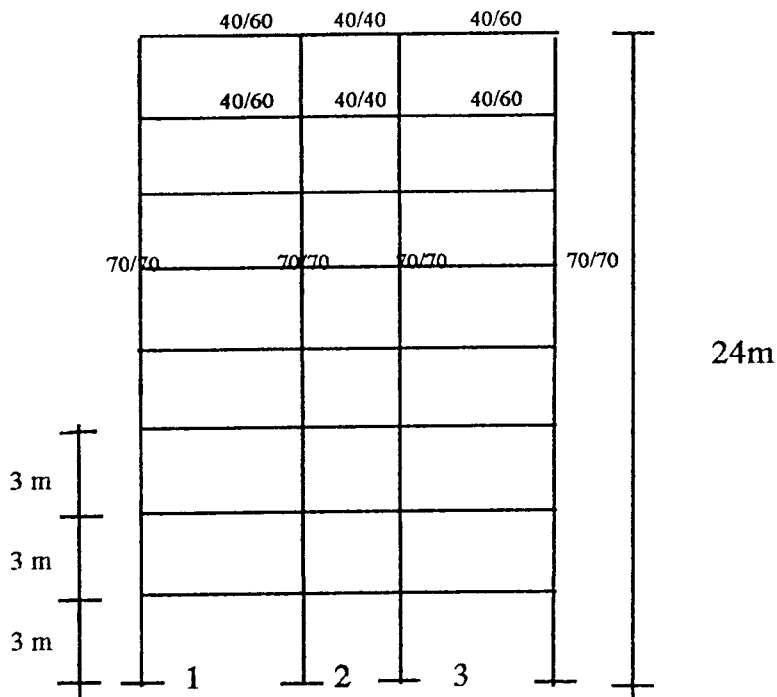
$$E \text{ balok} = 6400 \sqrt{250} = 1,01 \cdot 10^5 \text{ kg/cm}^2 = 1,01 \cdot 10^9 \text{ kg/m}^2$$

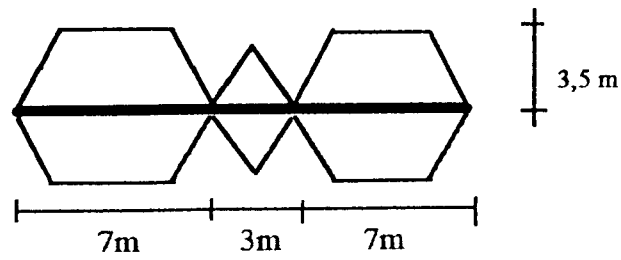
$$E \text{ kolom} = 6400 \sqrt{250} = 1,01 \cdot 10^5 \text{ kg/cm}^2 = 1,01 \cdot 10^9 \text{ kg/m}^2$$

4.2.2 Gedung Kampus As-syafi'iyah Jakarta

Gedung bertingkat 8 di daerah Jakarta dengan wilayah gempa 4

Tingkat	1	2	3	4	5	6	7	8
Kolom	70/70	70/70	70/70	70/70	70/70	70/70	70/70	70/70
Balok 1&3	40/60	40/60	40/60	40/60	40/60	40/60	40/60	40/60
Balok 2	40/40	40/40	40/40	40/40	40/40	40/40	40/40	40/40





Gambar 4.2 Model Kajian 2

Pada Pelat Atap**Beban mati***

Berat sendiri	:	$0,1 * 2400$	=	240 kg/m^2
Spesi	:	$2 * 21$	=	42 kg/m^2
Plafon & penggantung	:	$11 + 7$	=	18 kg/m^2
				<hr/>
				$= 300 \text{ kg/m}^2$

Beban hidup

Beban berguna			=	100 kg/m^2
Air hujan			=	40 kg/m^2
				<hr/>
			=	140 kg/m^2

Pelat Lantai**Beban mati***

Berat sendiri	:	$0,12 * 2400$	=	288 kg/m^2
Spesi	:	$2 * 21$	=	42 kg/m^2
Plafon & penggantung	:	$11 + 7$	=	18 kg/m^2

$$\begin{array}{rcl} \text{Tegel} & : 3 * 24 & = 72 \text{ kg/m}^2 \\ & & \hline & & 420 \text{ kg/m}^2 \end{array}$$

Beban hidup

$$\text{Beban berguna} = 250 \text{ kg/m}^2$$

***Berat sendiri balok**

$$\text{a. Berat balok atap (1\&3)} : 0,4 * (0,6 - 0,10) * 2400 = 480 \text{ kg/m'}$$

$$\text{b. Berat balok atap (2)} : 0,4 * (0,4 - 0,10) * 2400 = 288 \text{ kg/m'}$$

$$\text{a. Berat balok lantai (1\&3)} : 0,4 * (0,6 - 0,12) * 2400 = 460,8 \text{ kg/m'}$$

$$\begin{array}{rcl} \text{Tembok} & : 250 * 2,4 & = 600 \text{ kg/m'} \\ & & \hline & & 1060,8 \text{ kg/m'} \end{array}$$

$$\text{b. Berat balok lantai (2)} : 0,4 * (0,4 - 0,12) * 2400 = 268,8 \text{ kg/m'}$$

$$\begin{array}{rcl} \text{Tembok} & : 250 * 2,6 & = 650 \text{ kg/m'} \\ & & \hline & & 918,8 \text{ kg/m'} \end{array}$$

***Berat sendiri kolom**

$$0,7 * 0,7 * 2400 = 1176 \text{ kg/m'}$$

***Beban Ekivalen**

Beban mati

Balok atap

$$M = 0,0208 * q \text{ pelat} * l_x * (3 * l_y^2 - l_x^2) \dots\dots\dots(\text{balok 1\&3})$$

$$M = 0,0208 * 300 * 3,5 * (3 * 7^2 - 3,5^2) = 2942,94 \text{ kgm}$$

$$M = 1/8 * q * l^2$$

$$q = 480,48 * 2 = 960,96 \text{ kg/m'}$$

$$q \text{ balok} = 480 \text{ kg/m'}$$

$$q_{\text{equiv. (1\&3)}} = \frac{960,96}{2} \text{ kg/m'} \approx 480,48 \text{ kg/m'}$$

$$M = 0,0417 * q \text{ pelat} * l^3 \dots\dots\dots(\text{balok 2})$$

$$M = 0,0417 * 300 * 3^3 = 337,77 \text{ kg.m}$$

$$q = 300,24 * 2 = 600,48 \text{ kg/m'}$$

$$q \text{ balok} = 288 \text{ kg/m'}$$

$$q_{\text{ektiv. 2}} = \frac{600,48}{2} \text{ kg/m'} \approx 300,24 \text{ kg/m'}$$

Balok lantai

$$M = 0,0208 * 420 * 3,5 * (3 * 7^2 - 3,5^2) = 4120,116 \text{ kg.m (balok 1\&3)}$$

$$q = 672,672 * 2 = 1345,344 \text{ kg/m'}$$

$$q \text{ balok} = 1060,8 \text{ kg/m'}$$

$$q_{\text{ektiv. 1\&3}} = \frac{1345,344}{2} \text{ kg/m'} \approx 672,672 \text{ kg/m'}$$

$$M = 0,0417 * 420 * 3^3 = 472,878 \text{ kg.m (balok 2)}$$

$$q = 420,336 * 2 = 840,672$$

$$q \text{ balok} = 1018,8$$

$$q_{\text{ektiv. 2}} = \frac{840,672}{2} \text{ kg/m'} \approx 420,336 \text{ kg/m'}$$

Beban hidup

Balok atap

$$M = 0,0208 * 140 * 3,5 * (3 * 7^2 - 3,5^2) = 1373,372 \text{ kg.m (balok 1\&3)}$$

$$q_{\text{ekiv.1\&3}} = 224,224 * 2 = 488,488 \text{ kg/m}' \approx 490 \text{ kg/m}$$

$$M = 0,0417 * 140 * 3^3 = 157,626 \text{ kg.m (balok 2)}$$

$$q_{\text{ekiv. 2}} = 140,112 * 2 = 280,224 \text{ kg/m}' \approx 300 \text{ kg/m}$$

Balok lantai

$$M = 0,0208 * 250 * 3,5 (3 * 7^2 - 3,5^2) = 2452,45 \text{ kg.m (balok 1\&3)}$$

$$q_{\text{ekiv. 1\&3}} = 400,4 * 2 = 800,8 \text{ kg/m}' \approx 810 \text{ kg/m}$$

$$M = 0,0417 * 250 * 3^3 = 281,475 \text{ kg.m (balok2)}$$

$$q_{\text{ekiv. 2}} = 250,2 * 2 = 500,4 \text{ kg/m}' \approx 510 \text{ kg/m}$$

***Berat Atap**

Beban mati

$$\text{Pelat} \quad : \quad 300 * 17 * 7 \quad = \quad 35700 \text{ Kg}$$

$$\text{Balok Bujur} \quad : \quad 0,4 * 0,6 * 2 * 7 * 2400 \quad = \quad 8064 \text{ Kg}$$

$$0,4 * 0,3 * 1 * 3 * 2400 \quad = \quad 1152 \text{ Kg}$$

$$\text{Balok Anak} \quad : \quad 0,3 * 0,4 * 4 * 7 * 2400 \quad = \quad 8064 \text{ Kg}$$

$$\text{Balok melintang} \quad : \quad 0,4 * 0,5 * 4 * 7 * 2400 \quad = \quad 13440 \text{ Kg}$$

$$\text{Kolom} \quad : \quad 0,7 * 0,7 * 4 * 1,5 * 2400 \quad = \quad 7056 \text{ Kg}$$

$$\text{Tembok} \quad : \quad 4 * 7 * 1,5 * 250 \quad = \quad 10500 \text{ Kg}$$

$$17 * 1,5 * 250 \quad = \quad 6375 \text{ Kg}$$

$$\hline 90351 \text{ Kg}$$

Beban hidup

$$q = 140 \text{ kg/m}^2$$

$$\text{Faktor reduksi} = 0,3$$

$$W \text{ atap} = 17 * 7 * 140 * 0,3 = 4998 \text{ Kg}$$

$$W \text{ total atap} = 90351 + 4998 = 95349 \text{ Kg}$$

Berat lantai***Beban Mati***

$$\text{Pelat} : 670 * 17 * 7 = 79730 \text{ Kg}$$

$$\text{Balok bujur} : 0,4 * 0,60 * 2 * 7 * 2400 = 8064 \text{ Kg}$$

$$0,4 * 0,60 * 3 * 2400 = 1728 \text{ Kg}$$

$$\text{Balok anak} : 0,3 * 0,50 * 4 * 7 * 2400 = 10080 \text{ Kg}$$

$$\text{Balok melintang} : 0,4 * 0,60 * 4 * 7 * 2400 = 16128 \text{ Kg}$$

$$\text{Kolom} : 0,7 * 0,7 * 4 * 3 * 2400 = 14112 \text{ Kg}$$

$$\text{Tembok} : 4 * 7 * 3 * 250 = 31000 \text{ Kg}$$

$$17 * 3 * 250 = 12750 \text{ Kg}$$

$$\hline 163592 \text{ Kg}$$

Beban hidup

$$q = 250 \text{ kg/m}^2$$

$$\text{Faktor reduksi} = 0,3$$

$$W \text{ lantai (h)} = 17 * 7 * 250 * 0,3 = 8925 \text{ Kg}$$

$$W \text{ total lantai} = 163592 + 8925 = 172517 \text{ Kg}$$

$$W \text{ total atap \& lantai} = 95349 + 7 * (172517) = 1302968 \text{ Kg}$$

***Waktu getar bangunan (T)**

dengan rumus empiris

$$T = 0,06 H^{3/4}$$

$$H = 8 * 3,0 = 24 \text{ m}$$

$$T = 0,06 * (24)^{3/4}$$

$$= 0,6506 \text{ detik}$$

***Koefisien gempa dasar (C)**

C diperoleh dari gambar (struktur diatas tanah lunak)

$$C = 0,05$$

***Faktor Keutamaan (I) dan Faktor jenis struktur (K)**

I = 1,5 K = 1,0 untuk bangunan kampus yang menggunakan

struktur rangka beton bertulang dengan daktilitas penuh

***Gaya geser horisontal total akibat gempa**

$$V = C * I * K * W1$$

$$= 0,05 * 1,5 * 1,0 * 1302968 \text{ Kg}$$

$$= 97722,6$$

***Distribusi gaya geser horisontal total akibat gempa sepanjang tinggi gedung**

$$H/A = 24/17 = 1,4117 < 3$$

hi (m)	wi (t)	wi hi (tm)	Fi (t)
24	90351	2168424	12719
21	172517	3622857	21251
18	172517	3105306	18215
15	172517	2587755	15179
12	172517	2070204	12143
9	172517	1552653	9107
6	172517	1035102	6071
3	172517	517551	3036
16659812			

Data dimensi balok : 40/60 (balok 1&3)

40/40 (balok2)

kolom : 70/70

Mutu bahan

kolom = k 300 ; balok = k 250

Beban Merata Atap = (Beban mati + hidup)

$$q_{\text{ekiv. 1\&3}} = 1450 \text{ kg/m} + 490 \text{ kg/m} = 1940 \text{ kg/m}$$

$$q_{\text{ekiv. 2}} = 890 \text{ kg/m} + 300 \text{ kg/m} = 1190 \text{ kg/m}$$

Beban Merata Lantai = (Beban mati +Beban Hidup)

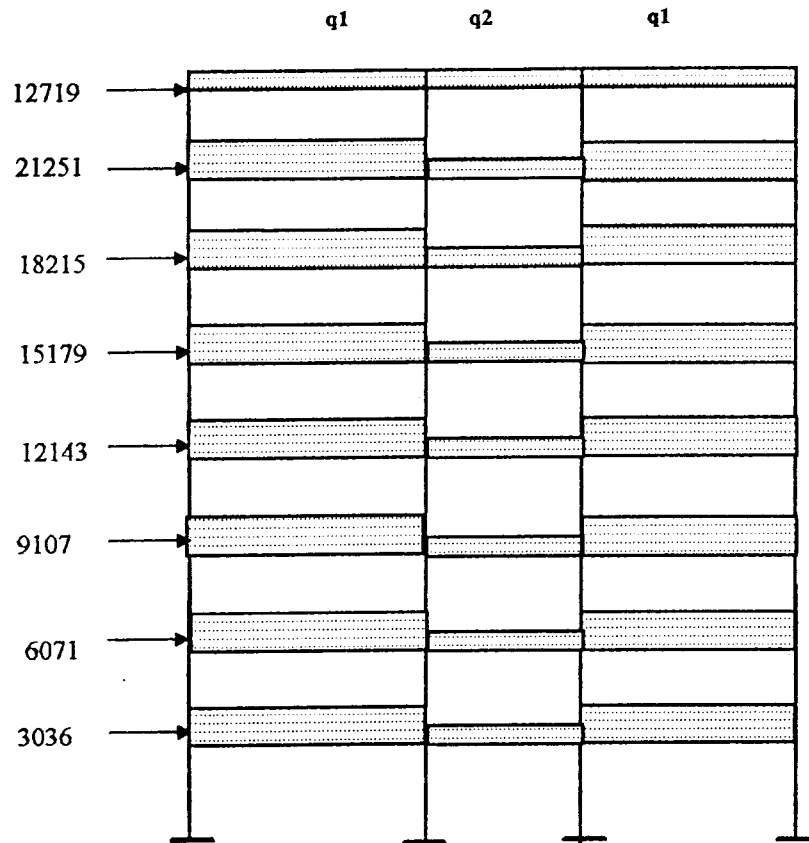
$$q_{\text{ekiv. 1\&3}} = 2410 \text{ kg/m} + 810 \text{ kg/m} = 3220 \text{ kg/m}$$

$$q_{\text{ekiv.2}} = 1760 \text{ kg/m} + 510 \text{ kg/m} = 2270 \text{ kg/m}$$

$$E \text{ kolom} = 6400 \sqrt{300} = 1,1 \cdot 10^5 \text{ kg/cm}^2 = 1,1 \cdot 10^9 \text{ kg/m}^2$$

$$E \text{ balok} = 6400 \sqrt{250} = 1,01 \cdot 10^5 \text{ kg/cm}^2 = 1,01 \cdot 10^9 \text{ kg/m}^2$$

Pembebanan pada portal penuh (A-P)



Pengurangan *Infill Wall*

$$\begin{aligned} q_{\text{ekiv.1\&3}} \text{ (akibat pengurangan)} &= q_{\text{ekiv. 1\&3}} - \text{berat tembok} \\ &= 3220 - 600 = 3214 \text{ kg/m} \end{aligned}$$

$$\begin{aligned} q_{\text{ekiv. 2}} \text{ (akibat pengurangan)} &= q_{\text{ekiv. 2}} - \text{berat tembok} \\ &= 2270 - 650 = 1620 \text{ kg/m} \end{aligned}$$

Tabel 4.1. Distribusi Gaya Horizontal Akibat Pengulangan IIIIII VV GIII

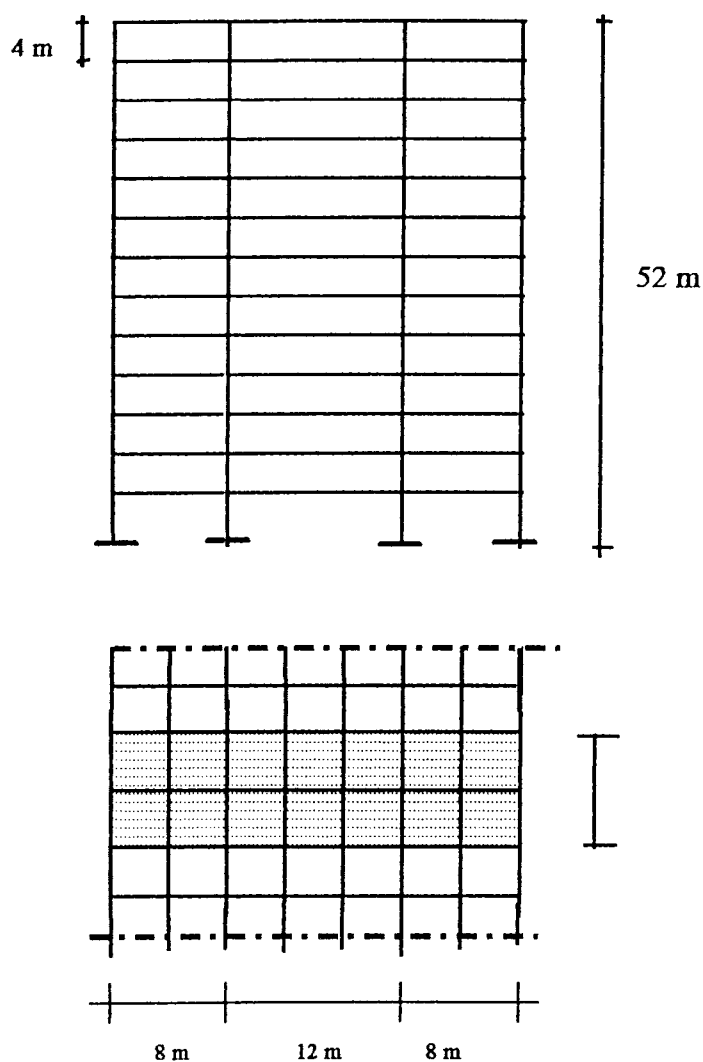
hi	wi (A-1)	wi (A-2)	wi (A-3)	wi (A-4)	wi (A-5)	wi (A-6)	wi (A-7)	wi (A-K)
wi	90351	90351	90351	90351	90351	90351	90351	83976
24	172517	172517	172517	172517	172517	172517	159767	159767
21	172517	172517	172517	172517	172517	159767	159767	159767
18	172517	172517	172517	172517	159767	159767	159767	159767
15	172517	172517	172517	159767	159767	159767	159767	159767
12	172517	172517	159767	159767	159767	159767	159767	159767
9	172517	159767	159767	159767	159767	159767	159767	159767
6	172517	159767	159767	159767	159767	159767	159767	159767
3	159767	159767	159767	159767	159767	159767	159767	159767
Wi.hi	2168424	2168424	2168424	2168424	2168424	2168424	2168424	2015424
24	3622857	3622857	3622857	3622857	3622857	3622857	3355107	3355107
21	3105306	3105306	3105306	3105306	3105306	2875806	2875806	2875806
18	2587755	2587755	2587755	2587755	2396505	2396505	2396505	2396505
15	2070204	2070204	2070204	1917204	1917204	1917204	1917204	1917204
12	1552653	1552653	1437903	1437903	1437903	1437903	1437903	1437903
9	1035102	958602	958602	958602	958602	958602	958602	958602
6	479301	479301	479301	479301	479301	479301	479301	479301
3	16621602	16545102	16430352	16277352	16086102	15856602	15588852	15435852
V (Kg)	96766.35	95810.1	94853.85	93897.6	92941.35	91985.1	91028.85	90072.6
Fi(Kg)	12623.962	12557.005	12518.5	12508.78	12528.595	12579.158	12662.199	11760.574
24	21091.267	20979.399	20915.068	20898.828	20931.934	21016.411	19591.663	19578.006
21	18078.229	17982.342	17927.201	17913.281	17941.657	16682.723	16792.854	16781.148
18	15065.191	14985.285	14939.335	14927.734	13846.388	13902.269	13994.045	13984.29
15	12052.153	11988.228	11951.468	11059.591	11077.11	11121.815	11195.236	11187.432
12	9039.1145	8991.1709	8301.139	8294.6932	8307.8328	8341.3616	8396.4269	8390.5742
9	6026.0763	5551.1144	5534.0927	5529.7955	5538.5552	5560.9077	5597.6179	5593.7161
6	2790.3573	2775.5572	2767.0463	2764.8977	2769.2776	2780.4539	2798.809	2796.8581

4.2.3 Gedung BNI

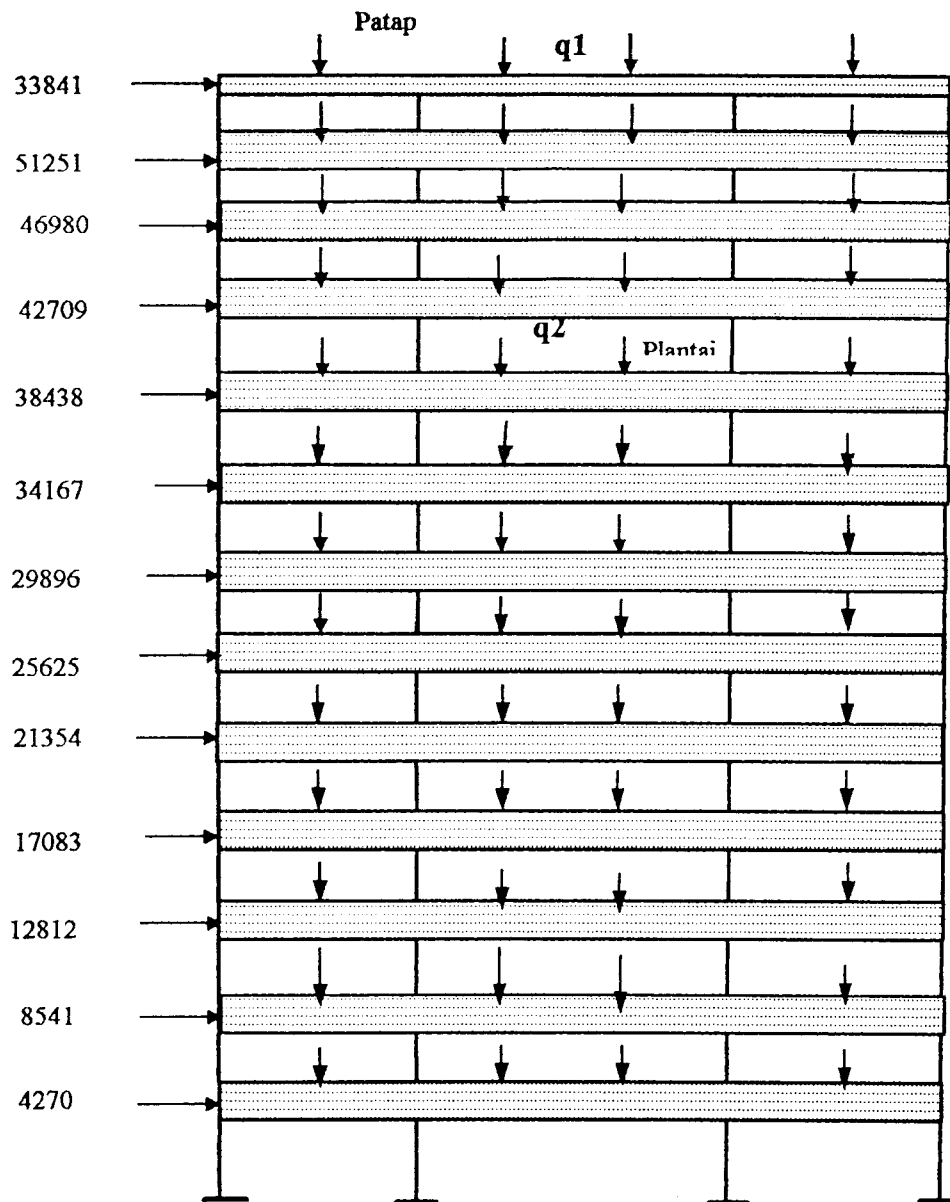
Gedung bertingkat 13 terletak di Surabaya dengan wilayah gempa 4

Tingkat	1	2	3	4	5	6	7	8	9	10	11	12	13
Kolom	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90	90/90
Balok	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80	60/80
Balok Anak	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30	60/30

Model Portal



Pembebanan pada portal penuh (B-P)



Gambar 4.3 Model Kajian 3

P atap : 3520 kg

P lantai : 5360 kg

$q_{ekiv.1} : 1900 + 400 = 2300 \text{ kg/m}$

$$q_{\text{ekiv.2}} : 3100 + 700 = 3800 \text{ kg/m}$$

$$E \text{ balok} : 6400\sqrt{300} = 1,1 \cdot 10^5 \text{ kg/cm}^2 = 1,1 \cdot 10^9 \text{ kg/m}^2$$

$$E \text{ kolom} : 6400\sqrt{350} = 1,2 \cdot 10^5 \text{ kg/cm}^2 = 1,2 \cdot 10^9 \text{ kg/m}^2$$

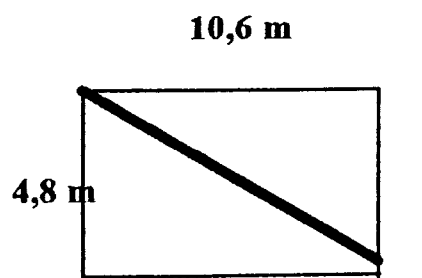
4.3 Infill frame

Batu bata dikonversikan sebagai *diagonal strut* yaitu batang diagonal yang diasumsikan mampu mendukung gaya horisontal yang disebabkan akibat beban gempa. Modulus Elastis bata + plesteran diambil sebagai beton non struktural yaitu B_0 dengan kesalahan pembuatan campuran untuk plesteran 0,5 maka diambil k50 baik pada batu bata maupun batako.

Beberapa macam variasi yang digunakan untuk mendapatkan hasil yaitu sebagai berikut ini.

1. Portal Terbuka.
2. *Infill Frame* penuh.
3. Pengurangan tembok secara bertahap

4.3.1 *Infill frame* pada kampus ITB Bandung



$$L/H = 10,6/4,8 = 2,208 \text{ m}$$

$$D = \sqrt{(10,6^2 + 4,8^2)} = 11,636 \text{ m}$$

Interpolasi diperoleh W/D (Tabel 3.1)

$$\frac{0,3 + (0,34 - 0,3) (2,208 - 2)}{(2,5 - 2)}$$

$$W = 0,3166 D$$

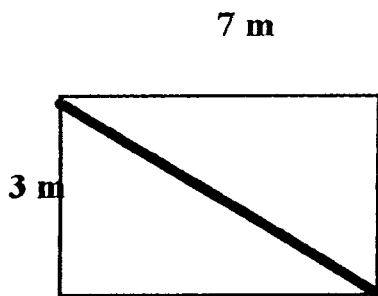
$$= 0,3166 * 11,636 = 3,6844\text{m}$$

Dimensi tembok $\frac{1}{2}$ bata

diambil tebal bata & plesteran = 12 cm

dimensi lebar efektif bata = $368,4 * 12$ cm

4.3.2 *Infill frame* gedung kampus As-syafi'iah Jakarta



$$L/H = 7/3 = 2,33$$

$$D = 7,616 \text{ m}$$

Interpolasi diperoleh W/D (Tabel 3.1)

$$\frac{0,3 + (0,34 - 0,3) (2,33 - 2)}{(2,5 - 2)}$$

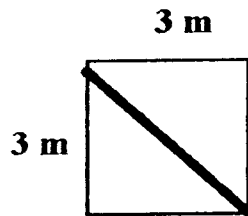
$$= 0,326$$

$$W = 0,326 * 7,616 = 2,483$$

dimensi tembok $\frac{1}{2}$ bata

diambil tebal bata & plesteran = 12 cm

dimensi lebar efektif bata = $248,3 * 12$ cm



$$L/H = 1$$

$$D = 4,243$$

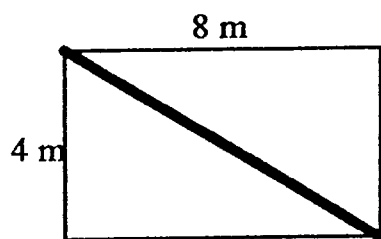
$$W/D = 0,45$$

$$W = 0,45 * 4,243 = 1,909 \text{ m}$$

diambil tebal bata & plesteran = 12 cm

dimensi lebar efektif bata = $190,9 * 12$ cm

4.3.3 *Infill frame* pada gedung BNI Surabaya



$$L/H = 2$$

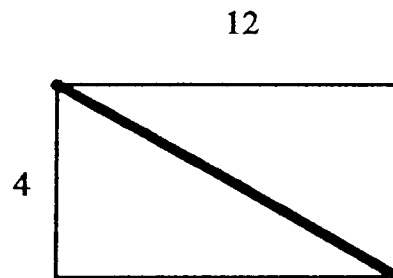
$$D = \sqrt{8^2 + 4^2} = 8,944 \text{ m}$$

$$W/D = 0,34$$

$$W = 0,34 * 8,994 = 3,041 \text{ m}$$

diambil tebal bata & plesteran = 12 cm

dimensi lebar efektif bata = $304,1 * 12 \text{ cm}$



$$L/H = 3$$

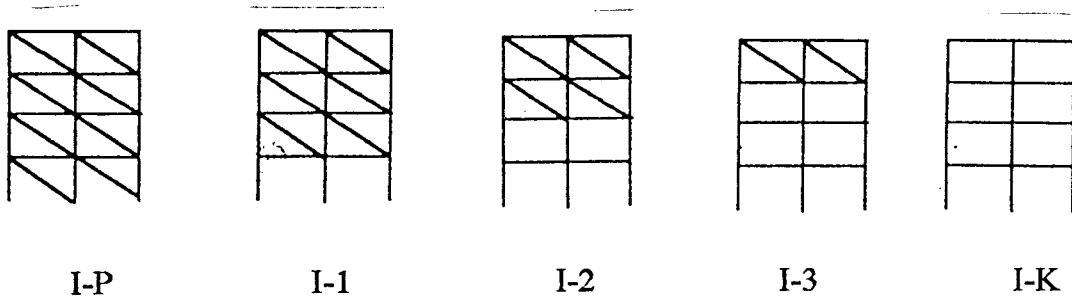
$$D = \sqrt{12^2 + 4^2} = 12,649$$

$$W/D = 0,3$$

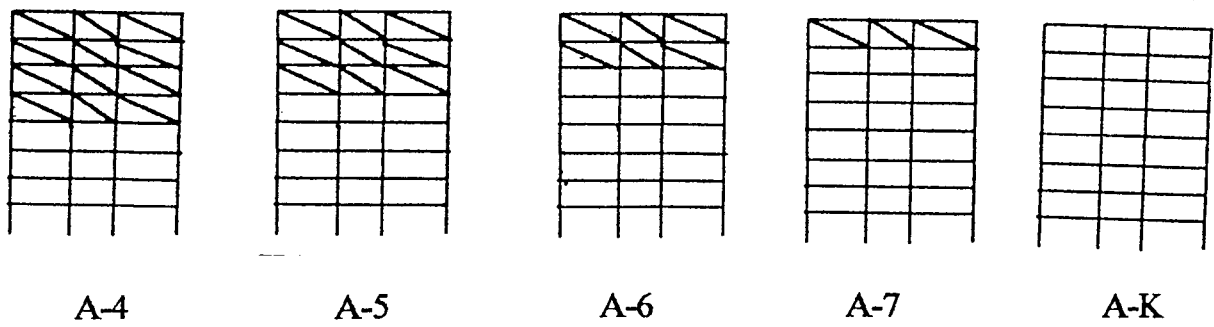
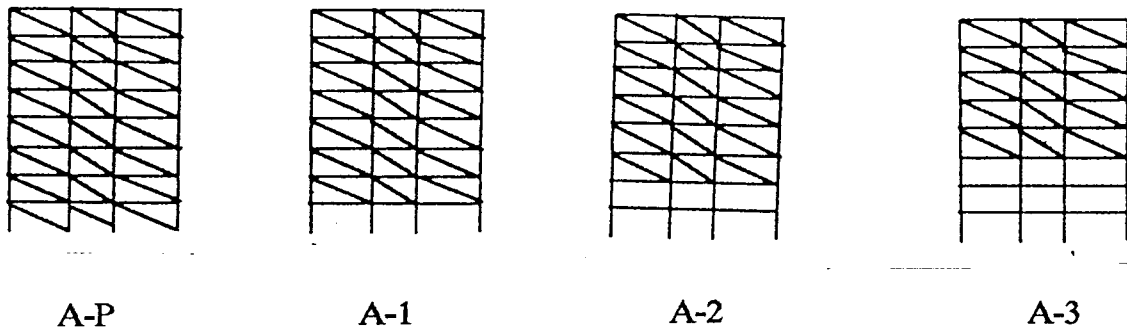
$$W = 0,3 * 12,649 = 3,795 \text{ m}$$

diambil tebal bata & plesteran = 12 cm

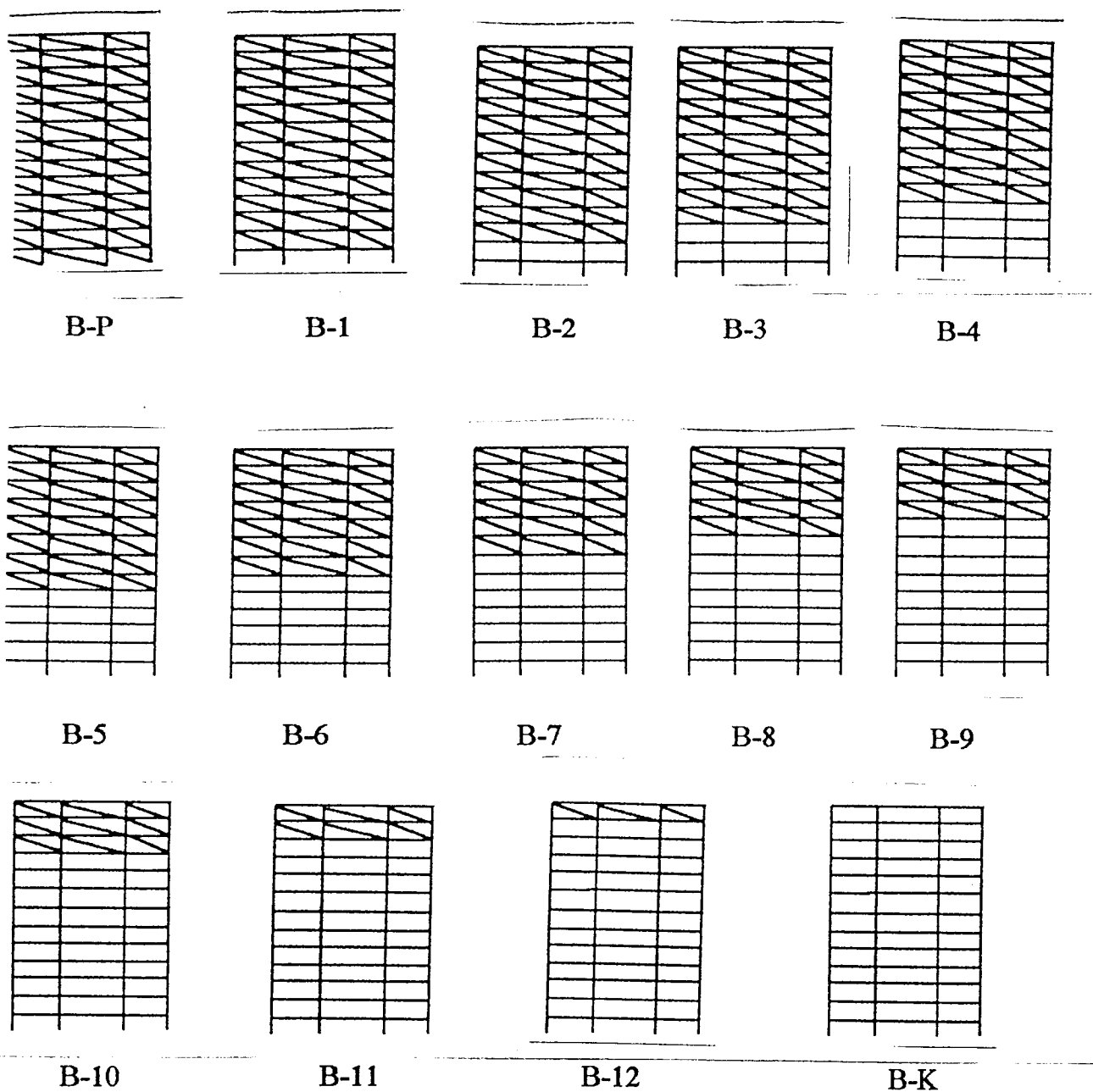
dimensi lebar efektif bata = $379,5 * 12 \text{ cm}$



Gambar 5.1a Kombinasi Model 1



Gambar 5.1b Kombinasi Model 2



Gambar 5.1c Kombinasi Model 3

5.3 Hasil /Out put Program dan Pembahasan

Tabel dan grafik disajikan menurut penempatan *infill wall* tiap tingkat dalam berbagai kombinasi.

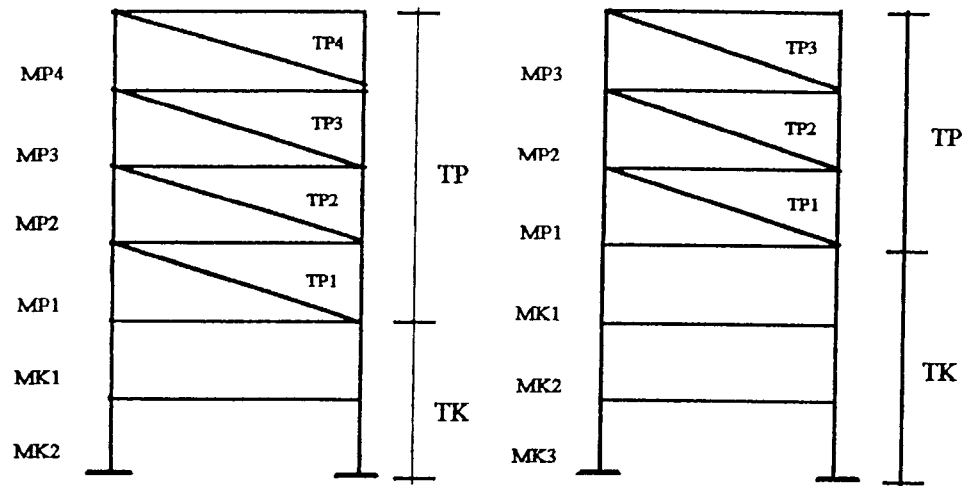
5.3.1 Kolom

5.3.1.a Momen

Momen kolom pada tiap tingkat pada berbagai kombinasi disajikan pada gambar 5.4, 5.5, 5.6. Pengaruh *infil wall* terhadap struktur secara umum adalah sebagai berikut ini.

1. Momen ujung bawah kolom pada tingkat ke-1 pada setiap kombinasi akan bertambah besar secara bertahap dari kondisi portal penuh (I-P) ke portal terbuka (I-K) atau tanpa dinding, seperti dapat dilihat pada gambar 5.4, 5.5, 5.6. Hal ini disebabkan karena adanya simpangan horisontal pada kondisi yang sama juga bertambah besar, ini juga berlaku pada model 2 dan 3 (lampiran *output program SAP 90*)
2. Pada kondisi kombinasi, struktur dapat dibedakan menjadi dua bagian yaitu tingkat-tingkat yang kosong (tidak ada dinding) dinotasikan TK dan tingkat-tingkat yang penuh (ada dinding) dinotasikan TP. Momen kolom pada tingkat yang penuh didefinisikan sebagai momen bawah (MP), sedangkan momen-momen kolom pada tingkat yang kosong didefinisikan sebagai momen kosong (MK), untuk lebih jelasnya bisa dilihat pada gambar 5.2 . Jika n adalah bilangan cacah, MK yang terdiri atas tingkat-tingkat dapat didefinisikan MK $n+1$ (dalam arah

ke bawah) dan pada MP yang terdiri dari tingkat-tingkat pada satu portal dapat didefinisikan MP $n+1$ (arah ke atas).

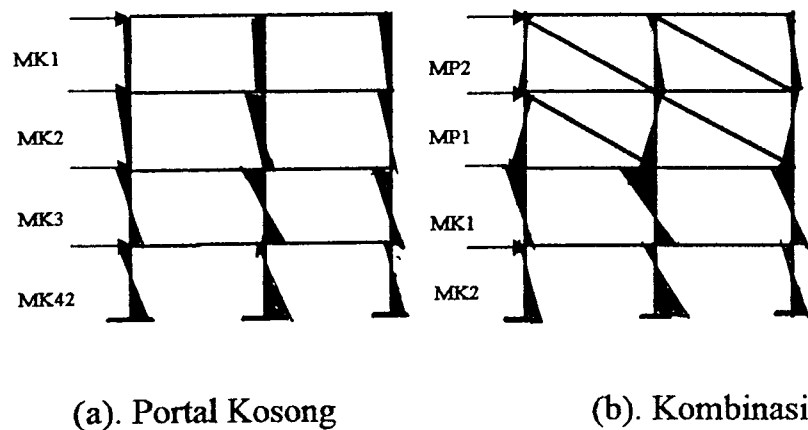


Gambar. 5.2 Pembagian Kombinasi Portal

3. Dari gambar 5.4, 5.5, 5.6. Dapat dilihat bahwa MK1 akan lebih besar daripada momen-momen yang lain kecuali pada momen kolom tingkat 1. Perubahan MK1 yang lebih besar terhadap momen-momen lain ini disebabkan karena *diagonal strut* pada tingkat di atasnya (MP1) memberikan kontribusi kekuatan ke arah yang berlawanan terhadap gaya gempa. Sebagian gaya tersebut dilimpahkan ke bawahnya karena kekakuan (MK1) lebih kecil maka momen yang terjadi akan semakin besar, lihat gambar 5.3. Gaya horisontal yang

harus ditahan oleh *infill wall* dan mengakibatkan perubahan pola momen kolom pada arah gempa datang serta tingkat dibawahnya.

4. Kekakuan portal pada TP1 lebih besar dari kekakuan pada tingkat yang kosong. MK2 pada kenyataannya dipengaruhi oleh MK1, hal ini dikarenakan reduksi dan induksi pada portal sangat berpengaruh pada tingkat tersebut dan selanjutnya MK3, MK4 dan seterusnya akan relatif sama pada kondisi portal kosong. Dengan demikian maka *diagonal strut* akan mempengaruhi momen pada tingkat yang bersangkutan dan 2 tingkat dibawahnya.



Gambar 5.3 Analisa Perilaku Momen Kolom Akibat *Diagonal Strut*

5. Diantara semua kombinasi, nilai MK1 yang paling besar adalah pada tingkat-tingkat bawah dimana pada kondisi portal penuh/portal

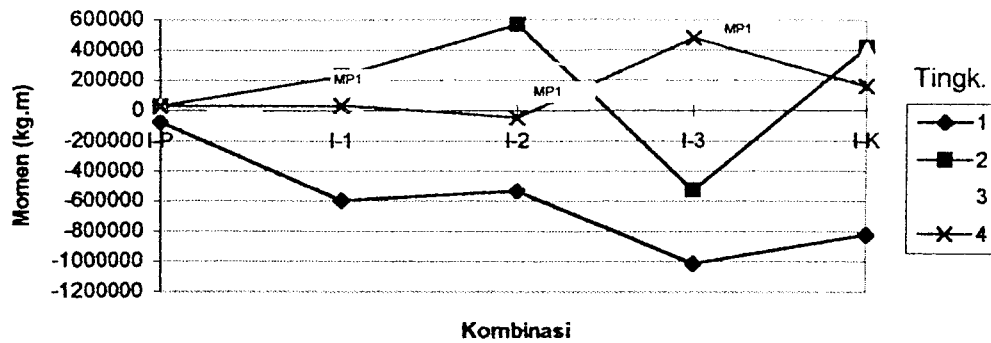
kosong defleksi pada tingkat tersebut mulai linear sampai tingkat paling tinggi , (lampiran *output program SAP 90*)

6. Secara keseluruhan perilaku statik portal penuh relatif lebih baik dari pada portal kosong karena *infill wall* ternyata mampu memberikan tambahan kekuatan terhadap gaya horisontal. Momen pada portal penuh mempunyai kecenderungan yang sama pada setiap tingkat sehingga memudahkan dalam perencanaan struktur.

Kombinasi dan perilaku tingkat lainnya disajikan dalam gambar-gambar sebagai berikut ini, (5.4, 5.5, 5.6).

Tabel 5.1 Momen Kolom (Model 1)

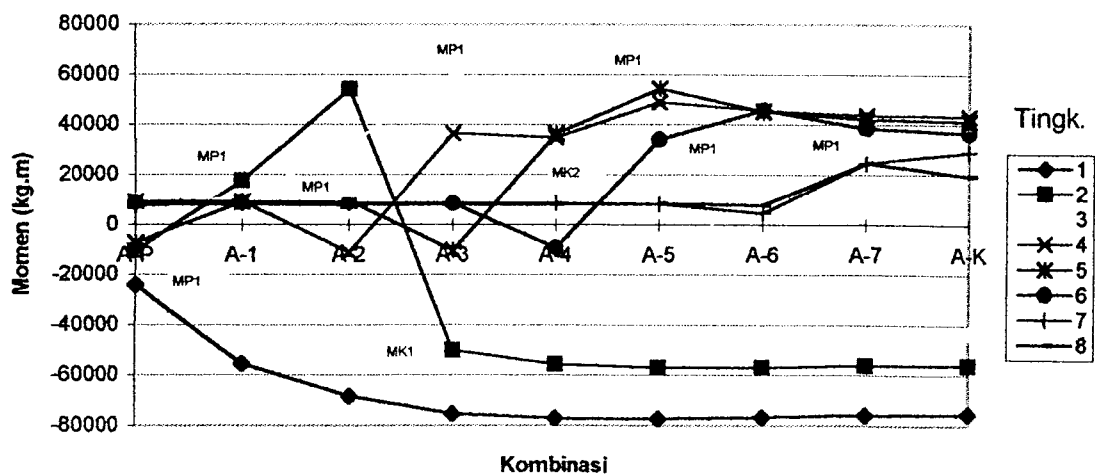
Tingk.	I-P	I-1	I-2	I-3	I-K
1	-74632	-596737	-533348	-1014260	-820929
2	28674	228642	568199	-526530	417595
3	26425	-37024	355951	157245	397132
4	30149	34341	-47238	478531	163534



Gambar 5.4 Grafik Momen Kolom (Model 1)

Tabel 5.2 Momen Kolom (Model 2)

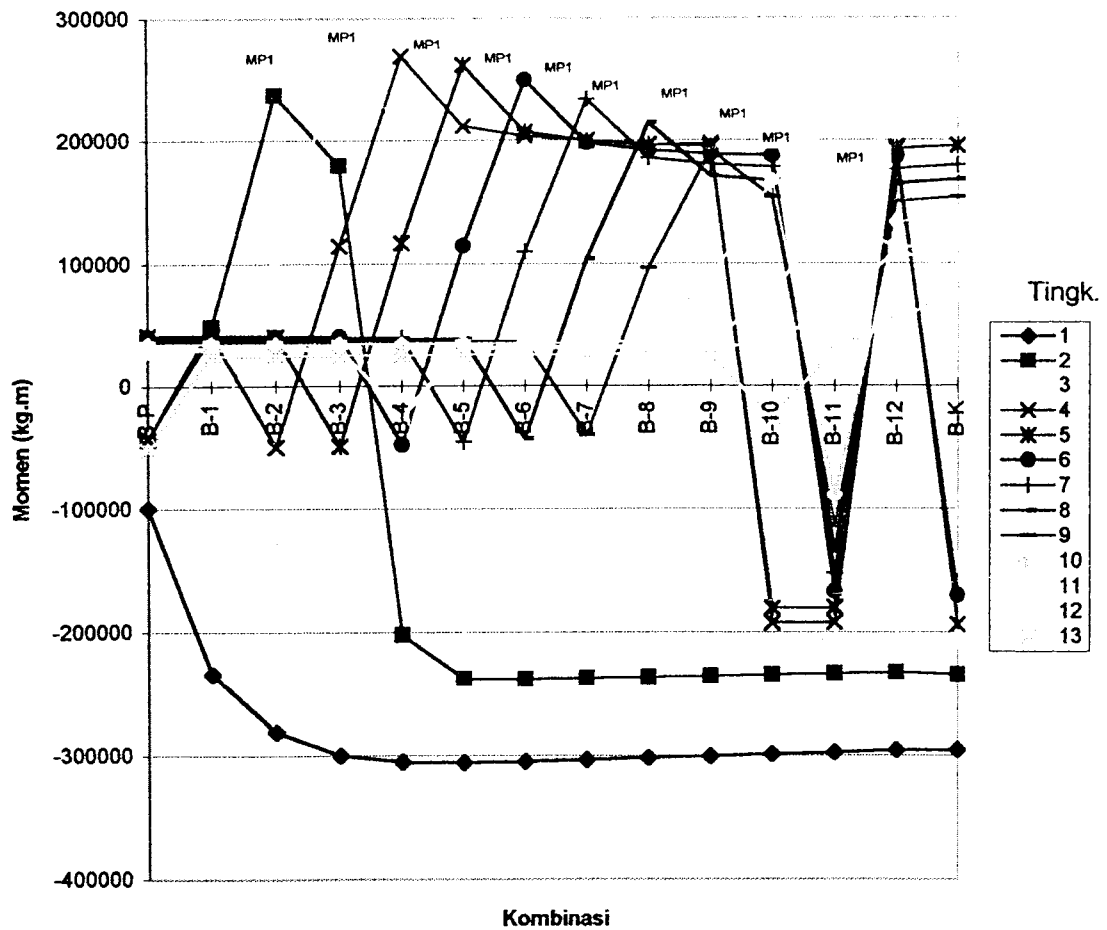
Tingk	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
1	-23961	-55364	-68330	-75266	-77065	-77149	-76615	-75782	-75312
2	-10650	17570	54059	-50075	-55557	-56879	-56876	-56170	-56011
3	9254	10513	31836	61025	44090	44862	-43618	-43247	-43142
4	9260	9288	-10897	36722	34884	49086	45562	43847	43442
5	-7403	9157	9226	-10347	36327	54263	45154	42116	41240
6	8778	8773	8766	8361	-9297	34107	45720	38550	36599
7	7652	8621	8631	8635	8722	8225	4649	24657	29108
8	8045	8046	8049	8077	8132	8505	7836	24680	19623



Gambar 5.5 Grafik Momen Kolom (Model 2)

Tabel 5.3 Momen Kolom (Model 3)

Tingk.	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
1	-99476	-234283	-281039	-300066	-305405	-305749	-304751	-303385	-301916	-300417	-298906	-297402	-295918	-295551
2	-43786	46642	237164	179749	-201883	-237491	-237902	-237244	-236310	-235277	-234232	-233196	-232193	-234036
3	-42800	-48022	99374	265870	207947	-203919	-208018	-208495	-207489	-207225	-206395	-205568	-204351	-207039
4	-42105	41786	-49512	114380	268562	211999	203804	200142	198381	197333	-192177	-191475	195010	-193439
5	41675	41922	40642	-49190	116949	261497	207183	198878	198551	194995	-180282	-179702	192648	195398
6	41082	41312	40784	40870	-47684	114650	249323	198051	191742	188803	187424	-186931	188052	-189733
7	40061	40294	39731	40329	39941	-45581	110009	233490	185939	180738	178234	-152148	178518	179797
8	38621	38861	38308	38646	38976	38972	-43033	103789	214438	171224	167266	-134865	164477	167939
9	36796	37014	36482	36999	37074	37287	37080	-40100	96211	192302	154004	-114283	150100	153607
10	34531	34790	34285	34780	34848	34964	35243	35145	37820	87336	167195	-88670	133589	136739
11	31698	31968	31487	31967	-28186	32152	32346	32967	32682	35038	77194	138810	111905	117113
12	30084	30423	29928	30465	-22833	30720	30968	31325	31851	32185	34642	86424	110498	92927
13	-51643	25618	25207	25683	25773	25917	26133	26438	26859	24715	-28007	29792	53284	77080



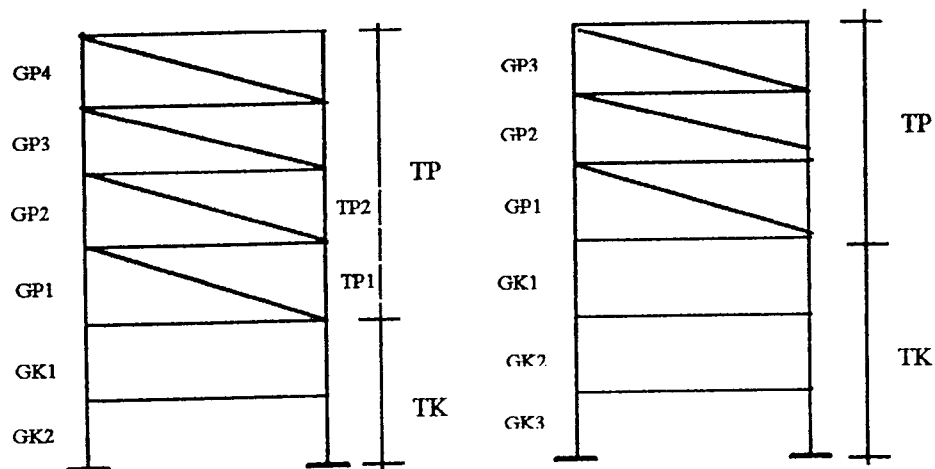
Gambar 5.6 Grafik Momen Kolom (Model 3)

5.3.1.b Gaya Geser

Melihat perubahan perilaku momen kolom akibat *diagonal strut* maka perubahan ini juga akan mempengaruhi gaya geser kolom. Gambar 5.9, 5.10, 5.11, menunjukkan gaya geser kolom pada tiap tingkat dan berbagai kombinasi. Secara umum pengaruh *infill wall* terhadap struktur adalah sebagai berikut ini.

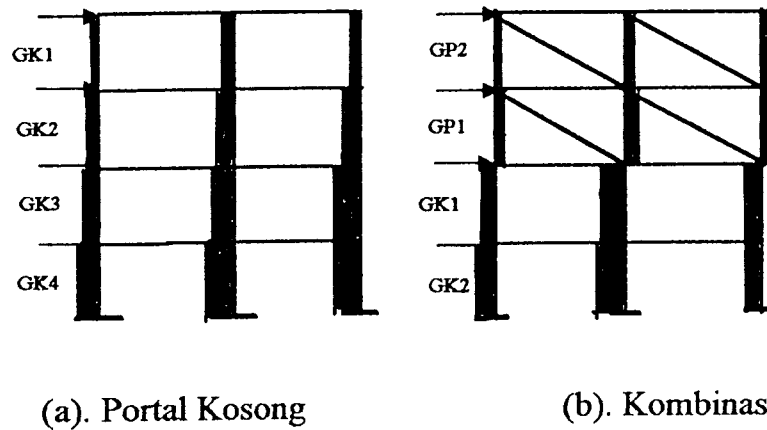
1. Gaya geser kolom pada tingkat yang penuh didefinisikan sebagai geser penuh (GP), sedangkan gaya geser kolom pada tingkat yang kosong didefinisikan sebagai geser kosong (GK), gambar 5.7. Jika n adalah bilangan cacah, GK yang terdiri atas tingkat-tingkat dapat didefinisikan GK $n+1$ (arah bawah) dan pada GP yang terdiri dari tingkat-tingkat pada satu portal dapat didefinisikan GP $n+1$ (arah atas).
2. Karena $MK1$ besar maka dapat dilihat juga bahwa $GK1$ akan besar. $GK1$ akan lebih besar dari tingkat yang lain seperti terlihat pada gambar 5.9, 5.10, 5.11. Hal ini disebabkan karena gaya geser merupakan fungsi dari momen, yaitu jumlah kedua momen ujung pada $MBK1$ (bawah) dan $MAK1$ (atas) dibagi tinggi tingkat. Perubahan $GK1$ yang lebih besar ini disebabkan juga karena *diagonal strut* pada tingkat di atasnya memberikan kontribusi kekuatan kearah yang berlawanan terhadap gaya gempa lihat gambar 5.8. Sehingga

diasumsikan gaya gempa yang bekerja pada struktur di tingkat tersebut menjadi kecil (sebagian gaya horisontal ditahan oleh “strut”). Gaya horisontal yang harus ditahan oleh *infill wall* mengakibatkan perubahan pola gaya geser kolom pada arah gempa datang.



Gambar. 5.7 Pembagian Kombinasi Portal

- TP1 mempunyai kekakuan tingkat yang lebih besar dari kekakuan pada tingkat kosong yang lain. Seperti halnya pada momen kolom GP1 akan mempengaruhi GK1 dan GK2 tetapi tidak mempengaruhi GK3, GK4 dan seterusnya. GK3, GK4 dan seterusnya berperilaku sama pada kondisi portal kosong. Dengan demikian gaya geser pada tingkat bersangkutan dan 2 tingkat dibawahnya akan dipengaruhi oleh *diagonal strut* tersebut.



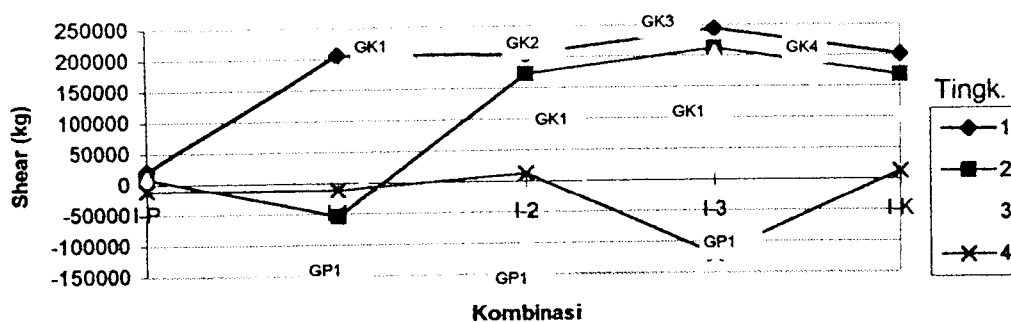
Gambar 5.8 Analisa Perilaku Gaya Geser Kolom Akibat *Diagonal Strut*

4. Nilai GK1 yang paling besar pada semua kombinasi yaitu dimana pada kondisi portal kosong gaya geser akan mengalami perubahan tanda (dari nilai positif (+) ke negatif (-) atau sebaliknya), lihat lampiran SAP 90.
5. Gaya geser pada portal penuh mempunyai kecenderungan yang sama pada setiap tingkat sehingga memudahkan dalam perencanaan struktur. Maka secara analisa mekanika pada kondisi portal penuh boleh dianggap struktur relatif mampudalam menahan gaya gempa.

Kombinasi dan perilaku tingkat lainnya disajikan dalam gambar-gambar sebagai berikut ini.

Tabel 5.4 Shear Kolom-1 (Model 1)

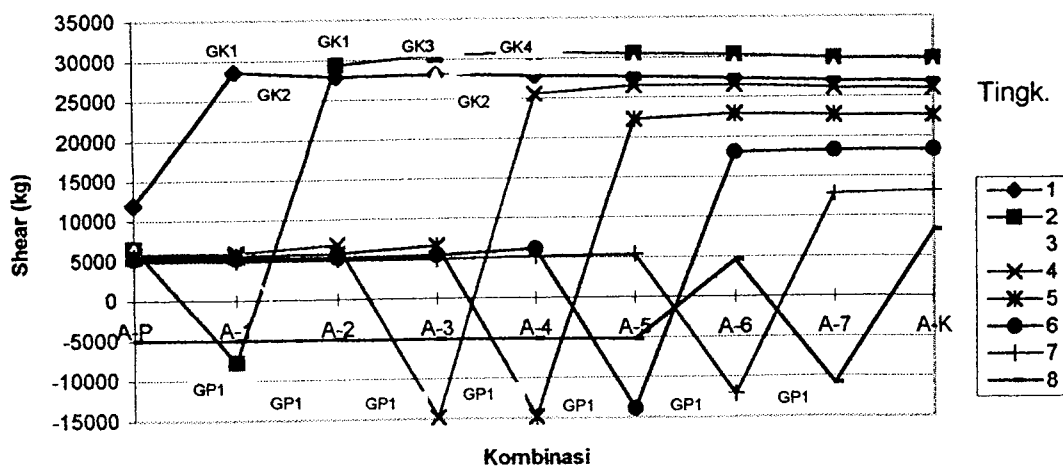
Tingk.	I-P	I-1	I-2	I-3	I-K
1	21531	206086	204625	244303	201948
2	9194	-54460	172209	212148	168481
3	9590	13419	-88421	203442	103424
4	-10110	-11250	12934	-120636	13351



Gambar 5.9 Grafik Shear Kolom (Model 1)

Tabel 5.5 Shear Kolom (Model 2)

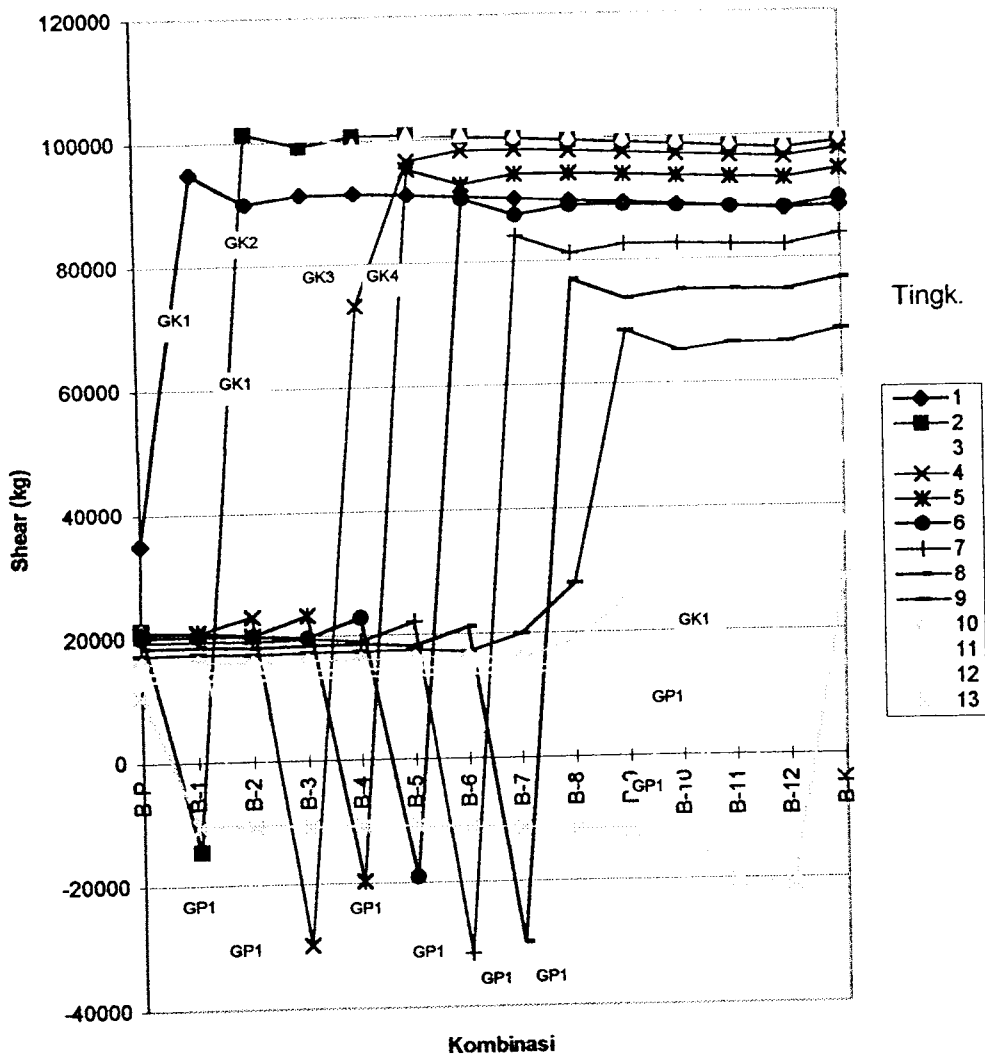
Tingk.	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
1	11964	28591	27897	28075	27845	27628	27380	27022	26858
2	6487	-7794	29377	30499	30722	30517	30277	29832	29711
3	6125	6660	-13077	27835	28965	29155	28926	28491	28365
4	5776	5883	6849	-14824	25520	26485	26586	26143	26036
5	5523	5511	5729	6605	-14812	22241	22950	22743	22654
6	5163	5166	5143	5402	6112	-13860	18114	18320	18401
7	4931	4939	4948	4925	5180	5375	-12145	12845	13237
8	-5019	-5029	-5040	-5059	-5083	-5200	4557	-10862	8229



Gambar 5.10 Grafik Shear Kolom (Model 2)

Tabel 5.6 Shear Kolom (Model 3)

Tingkt	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
1	34953	94907	89097	91381	91450	91082	90635	90200	89761	89323	88884	88446	88020	88420
2	21288	-14579	101330	99236	100792	100820	100443	100013	99572	99126	98684	98247	97824	98771
3	21047	22785	-25551	101363	99101	100724	100786	100412	100006	99594	99178	98773	98389	99405
4	20673	20786	23238	-29623	73214	98588	98202	98245	97918	97545	97166	96796	96457	97727
5	20658	20816	20112	23301	-19856	95258	92610	94178	94216	93915	93576	93246	92949	94371
6	20066	20209	20085	19711	22804	-19062	90255	87437	88928	86654	86661	86390	86135	80703
7	19337	19458	19235	19543	18999	22050	-31429	84124	81147	82536	82545	82314	82098	83805
8	18384	18509	18284	18497	18634	18138	21066	-29633	76901	73763	75032	75034	74878	76713
9	17241	17370	17151	17367	17396	17561	17129	16632	27904	68604	65290	66442	66486	66446
10	15922	18057	15843	16060	16093	16151	16344	15966	18613	25665	56277	55780	56836	59014
11	14370	14511	14302	14521	14554	14614	14701	14621	14646	17069	-23258	48849	45041	48472
12	13116	13280	13071	13305	13348	13419	13526	13674	13661	13657	16096	-21423	36293	36706
13	10501	-10527	-10341	-10578	-10645	-10754	-10914	-11137	-11453	-11785	-12470	-20313	-20635	26355



Gambar 5.11 Grafik Shear Kolom (Model 3)

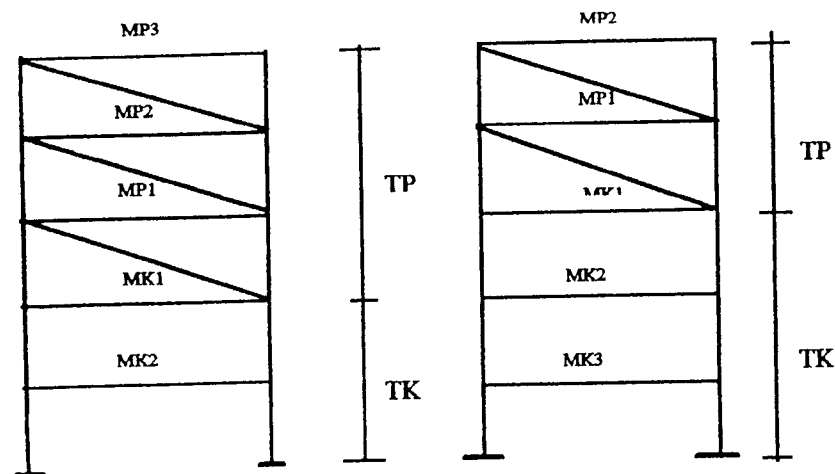
5.3.2 Balok

5.3.2a Momen

Setelah melihat *output* program ternyata *infill wall* juga mempengaruhi balok. Gambar 5.14, 5.15, 5.16 menunjukkan momen balok pada tiap tingkat untuk berbagai kombinasi. Pengaruh *Infill Wall* pada balok secara umum diuraikan sebagai berikut ini.

1. Momen balok pada tingkat paling atas akan relatif berperilaku sama pada setiap kombinasi dari kondisi portal penuh ke portal terbuka, kecuali pada kombinasi (portal kosong) I-K/A-K/B-K. Pada keadaan ini momen balok mengalami penurunan, hal ini disebabkan karena pengaruh beban merata yang ditahan oleh tingkat tersebut (balok atap) semakin kecil dan *diagonal strut* tidak secara langsung mempengaruhi tingkat atas.
2. Momen balok pada tingkat yang penuh didefinisikan sebagai (MP), sedangkan momen-momen balok pada tingkat yang kosong didefinisikan sebagai (MK), seperti dapat dilihat pada gambar 5.7. Jika n adalah bilangan cacah, MK yang terdiri dari atas tingkat-tingkat dapat didefinisikan MK $n+1$ (arah bawah) dan pada MP yang

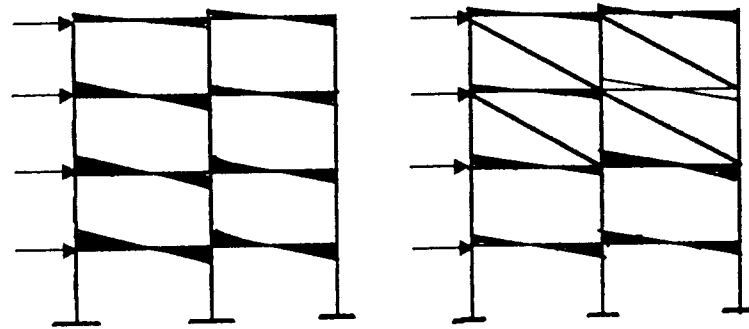
terdiri atas tingkat-tingkat pada satu portal dapat didefinisikan MP $n+1$ (arah atas).



Gambar 5.12 Pembagian Kombinasi Portal

3. Dari gambar 5.14, 5.15, 5.16 dapat dilihat bahwa MP1 relatif konstan seperti pada kondisi portal penuh. Momen akan mulai naik pada kondisi pengurangan *infill wall*. MK1 naik dan MK2, MK3 juga akan dipengaruhi besarnya (mengalami kenaikan), hal ini disebabkan karena faktor distribusi momen. MK4, MK5 dan seterusnya akan relatif konstan sama seperti pada kondisi portal kosong, hal ini dikarenakan simpangan pada kondisi tersebut juga berperilaku sama.
4. Pada TP *diagonal strut* memberikan tambahan kekuatan dalam menahan gaya gempa. Gambar 5.13, menunjukkan arah *diagonal strut*

yang berlawanan terhadap gaya gempa. MK1 pada balok berubah lebih besar daripada MP1 sehingga pada kondisi tingkat dimana *infill* dikurangi akan dapat mengakibatkan *beam sway mechanism*.



(a). Portal Kosong

(b). Kombinasi

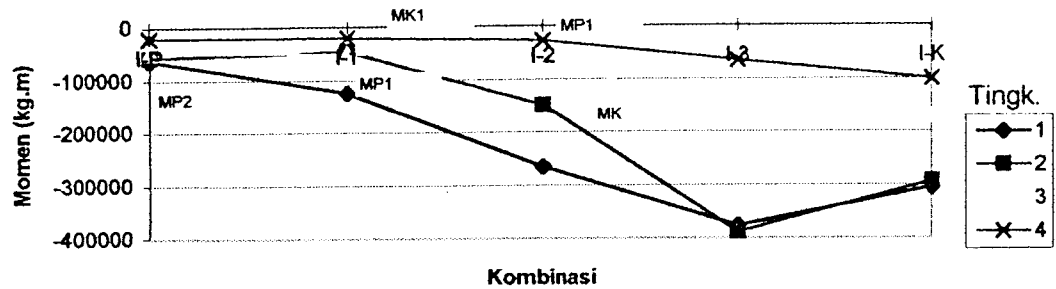
Gambar 5.13 Analisa Perilaku Momen Balok Akibat *Diagonal Strut*

- Perilaku portal penuh secara keseluruhan relatif lebih baik daripada portal terbuka. *Infill Wall* ternyata memberikan tambahan kekuatan terhadap gaya horisontal sehingga balok relatif aman terhadap gaya horisontal. Portal penuh mempunyai kecenderungan yang sama pada setiap tingkat sehingga memudahkan dalam perencanaan struktur.

Kombinasi dan perilaku tingkat lainnya disajikan dalam gambar-gambar 5.14, 5.15, 5.16 berikut ini.

Tabel 5.7 Momen Balok (Model 1)

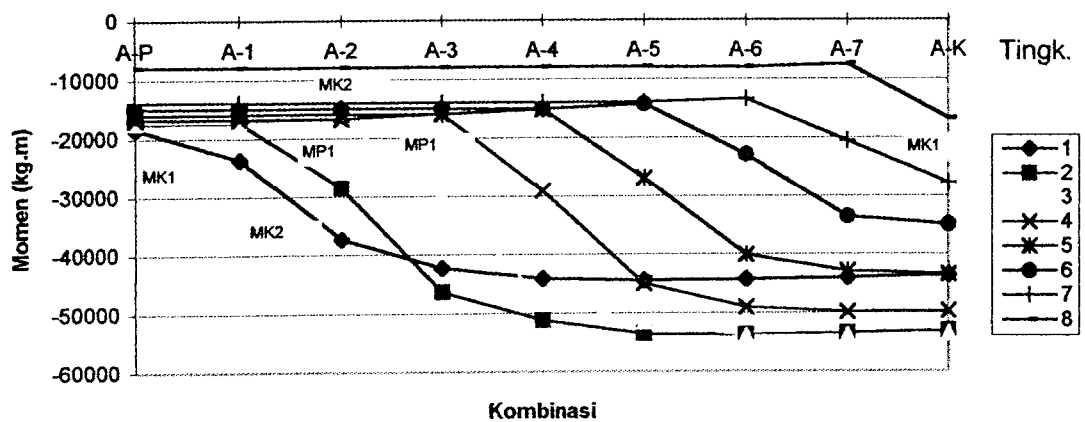
Tingk.	I-P	I-1	I-2	I-3	I-K
1	-62320	-124915	-265044	-377205	-304974
2	-57153	-46671	-149034	-388798	-293185
3	-48198	-50078	-55165	-182663	-171687
4	-19716	-20222	-27601	-65034	-100141



Gambar 5.14 Grafik Momen Balok (Model 1)

Tabel 5.8 Momen Balok (Model 2)

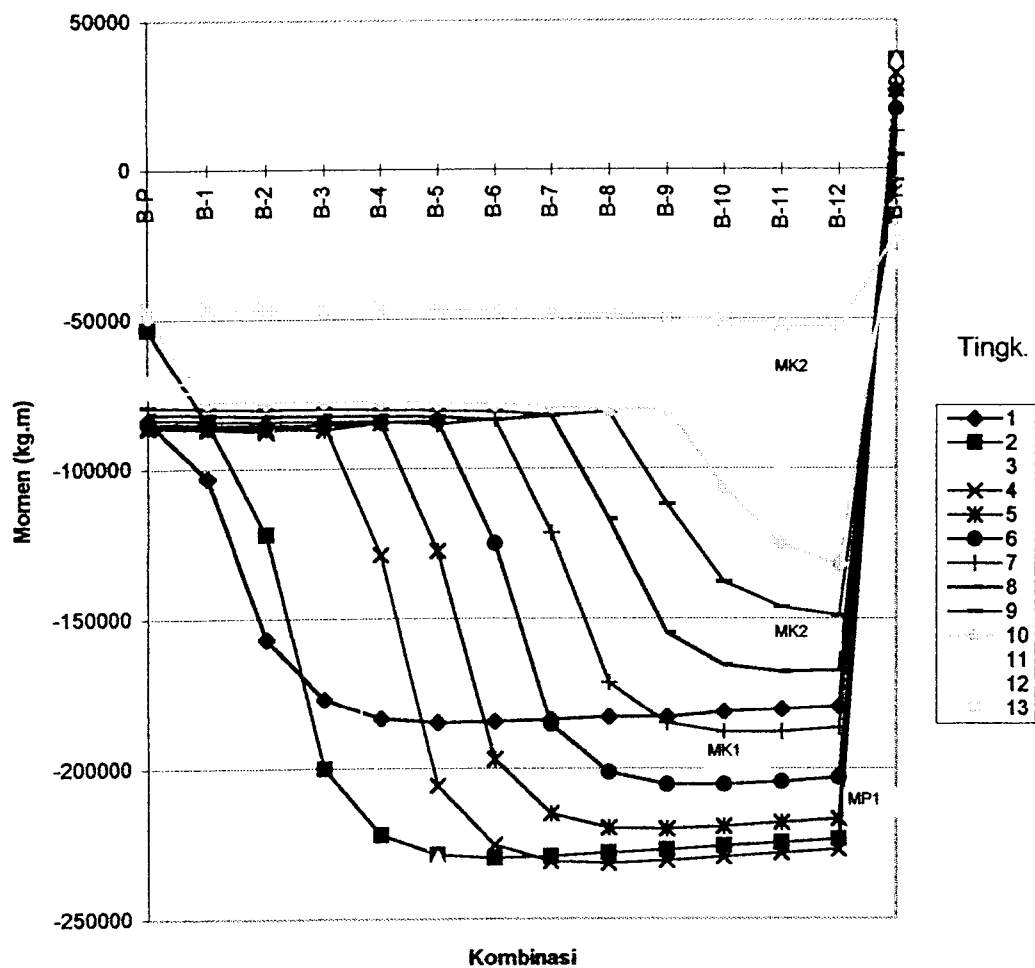
Tingk.	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
1	-18332	-23612	-37332	-42306	-44090	-44455	-44309	-44010	-43598
2	-17488	-17268	-28593	-46433	-51265	-53652	-53830	-53522	-53027
3	-17267	-17254	-16579	-29902	-47679	-51643	-53941	-53900	-53474
4	-16712	-16647	-16749	-15899	-29052	-45031	-49054	-49824	-49656
5	-15953	-15894	-15853	-16004	-15151	-26973	-40122	-42912	-43425
6	-15021	-14970	-14952	-14966	-15065	-14340	-22886	-33591	-34943
7	-13913	-13926	-13936	-13944	-13954	-13941	-13442	-20592	-27806
8	-7928	-7910	-7917	-7922	-7924	-7936	-8056	-7468	-16802



Gambar 5.15 Grafik Momen Balok (Model 2)

Tabel 5.9 Momen Balok (Model 3)

Tngk.	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
1	-83806	-103315	-156797	-176980	-183505	-184871	-184566	-183708	-182928	-182928	-181133	-180243	-178358	26178
2	-53396	-84788	-122098	-200225	-222140	-228716	-229861	-228266	-228227	-227081	-225915	-224760	-223607	36803
3	-86358	-87064	-84970	-128001	-209070	-230201	-236289	-237190	-236452	-235309	-234075	-232839	-231600	36290
4	-86401	-86851	-87534	-84844	-128896	-205863	-225383	-230883	-231536	-230699	-229503	-228248	-226987	32158
5	-85915	-86369	-86639	-87004	-84883	-127579	-197085	-215044	-219857	-220253	-219320	-218094	-216786	28503
6	-84920	-85408	-85749	-85474	-84533	-84225	-124995	-185423	-201321	-205348	-205453	-204425	-203120	19978
7	-83475	-83989	-84338	-84074	-84284	-85259	-83467	-121454	-171278	-184814	-187942	-187735	-186541	12637
8	-81622	-82125	-82524	-82208	-82454	-82804	-83908	-82443	-117091	-154800	-165695	-167795	-167183	4683
9	-79410	-79923	-80348	-79997	-80211	-80588	-81081	-82305	-81213	-112008	-137973	-146263	-149072	-3712
10	-48834	-77409	-77855	-77483	-77880	-78016	-78527	-79171	-80525	-78885	-106323	-125511	-131989	-12646
11	-74142	-74880	-75148	-74782	-74949	-75285	-75734	-76394	-77212	-78729	-78563	-100264	-112187	-21998
12	-70983	-71505	-72004	-71598	-71774	-72070	-72508	-73114	-73928	-74935	-76642	-77104	-93805	-30509
13	-48089	-48042	-48559	-48129	-48292	-48590	-48954	-47498	-48221	-49171	-50323	-51984	-52940	-21435

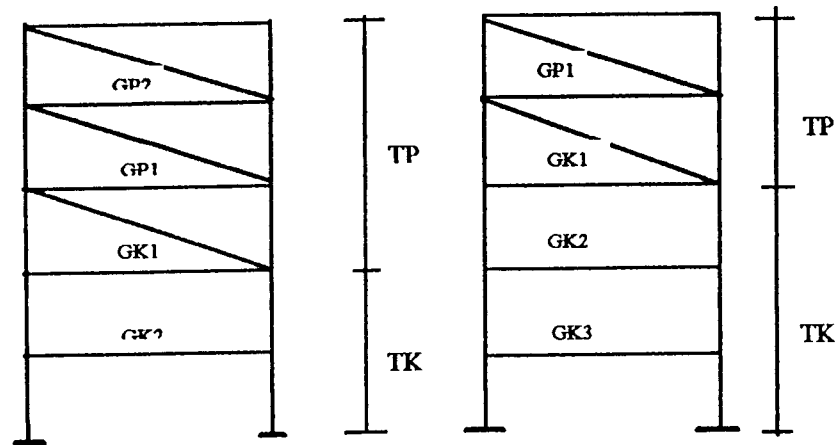


Gambar 5.16 Grafik Momen Balok (Model 3)

5.3.2b Gaya Geser

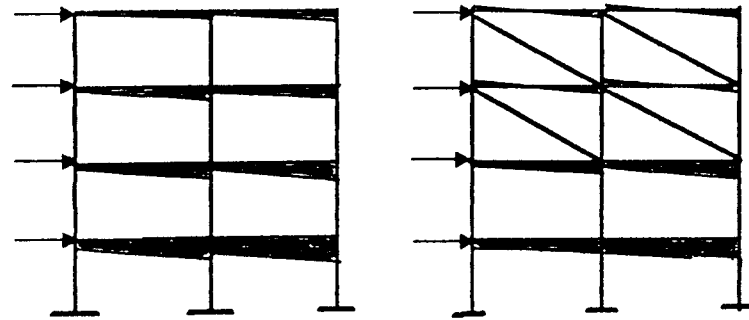
Dari hasil momen balok maka dapat juga disajikan hasil gaya geser, karena momen mempengaruhi besar kecilnya gaya geser. Pada gambar 5.19, 5.20, 5.21 menunjukkan gaya geser balok pada tiap tingkat untuk berbagai kombinasi. Berikut ini diuraikan pengaruh *infill wall* secara umum.

1. Dari kondisi portal penuh (I-P) Ke portal kosong (I-K), pada tingkat paling atas gaya geser akan relatif berperilaku sama kecuali pada portal kosong, kombinasi I-3/A-7/B-12. Pada kombinasi tersebut gaya geser balok akan mengalami kenaikan karena adanya pengaruh *diagonal strut* pada tingkat atas ikut menahan gaya horisontal.
2. Seperti halnya pada momen balok maka gaya geser balok dapat juga didefinisikan pada tiap-tiap tingkat. Gaya geser balok pada tingkat-tingkat yang penuh didefinisikan sebagai gaya geser penuh (GP), sedangkan gaya geser balok pada tingkat yang kosong didefinisikan sebagai geser kosong (GK). Hal ini dapat dilihat pada gambar 5.17. Jika n adalah bilangan cacah, GK yang terdiri atas tingkat-tingkat dapat didefinisikan GK $n+1$ dan pada GP yang terdiri atas tingkat-tingkat pada satu portal dapat definisikan GP $n+1$.



Gambar 5.17 Pembagian Kombinasi Portal

3. Pada GP2, GP3 dan seterusnya gaya geser relatif konstan seperti pada gaya geser pada portal penuh. Pada GK1, GK2 dan seterusnya gaya geser akan berubah relatif besar bertahap hingga sama dengan gaya geser pada portal terbuka, lihat pada gambar 5.19, 5.20, 5.21. Hal ini disebabkan karena simpangan yang terjadi sama seperti keadaan diatas.
4. Pada gambar 5.18, menunjukkan arah *diagonal strut* yang berlawanan terhadap gaya gempa. Perubahan GK1, GK2, dan seterusnya pada model 1 dan 2 relatif kecil. Sedangkan pada model 3 perubahan gaya geser balok akan besar karena gaya horisontal yang ditahan oleh struktur juga besar.



(a). Portal Kosong

(b). Kombinasi

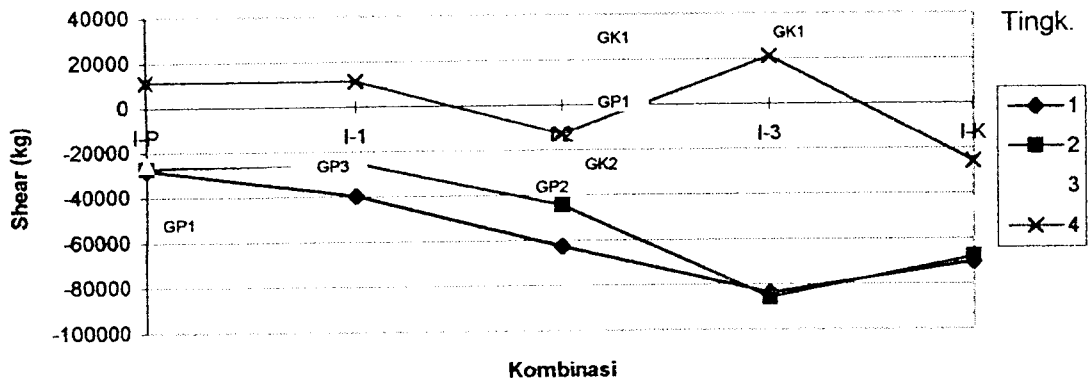
Gambar 5.18 Analisa Perilaku Gaya geser Balok Akibat *Diagonal Strut*

5. Seperti halnya pada gaya geser kolom ternyata gaya geser balok pada portal penuh juga memberikan perilaku yang lebih baik daripada portal kosong. Ternyata *Infill Wall* mampu memberikan tambahan kekuatan terhadap gaya horisontal. Gaya geser balok relatif sama pada setiap tingkat sehingga memudahkan dalam perencanaan.

Kombinasi dan perilaku tingkat lainnya disajikan dalam gambar-gambar berikut ini.

Tabel 5.10 Shear Balok (Model 1)

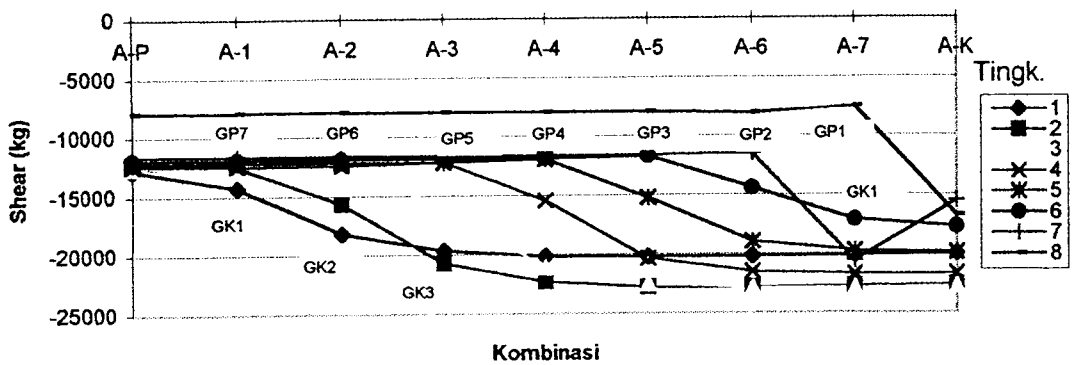
Tingk.	I-P	I-1	I-2	I-3	I-K
1	-27972	-39775	-62580	-83717	-70176
2	-27177	-25162	-44335	-85805	-67912
3	-25420	-25793	26749	-50728	-44881
4	11380	11455	-12662	21077	-25796



Gambar 5.19 Grafik Shear Balok (Model 1)

Tabel 5.11 Shear Balok (Model 2)

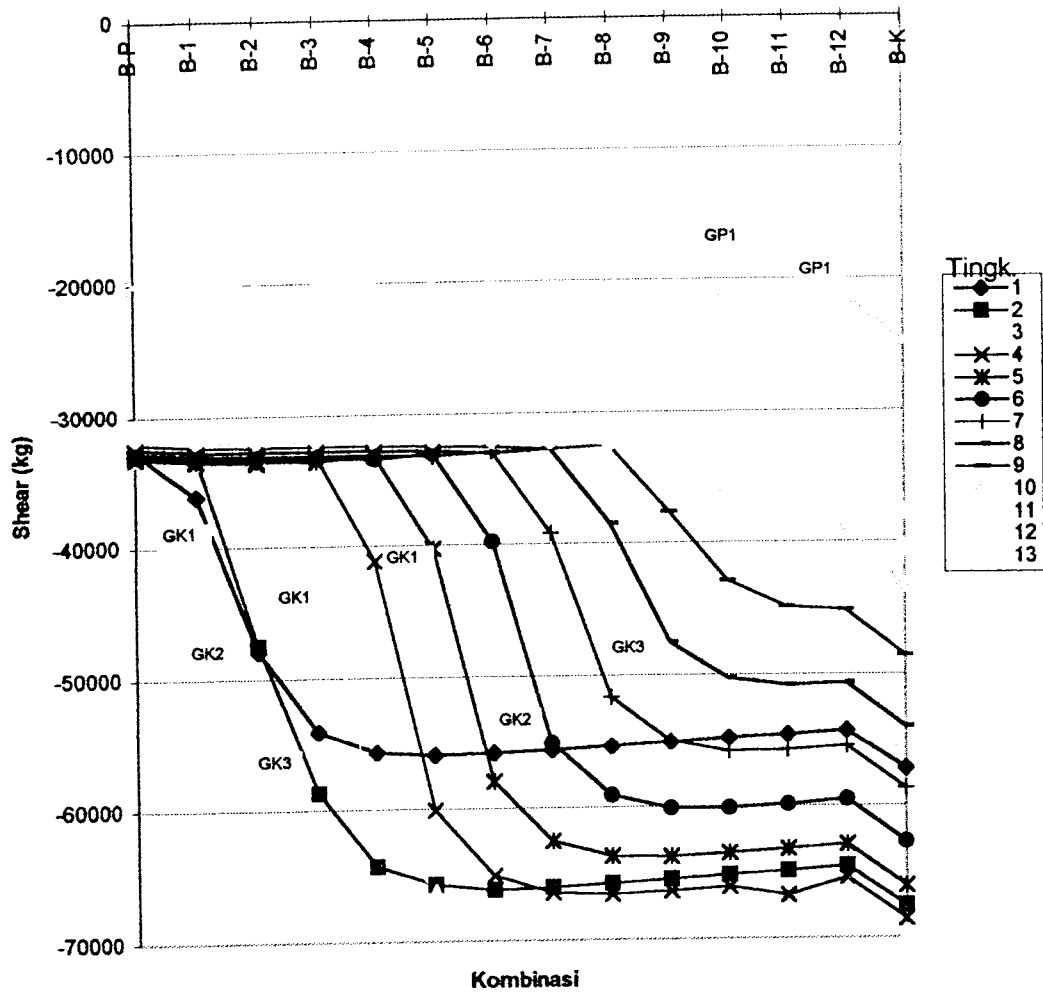
Tingk.	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
1	-12792	-14281	-18185	-19645	-20153	-20250	-20213	-20131	-20019
2	-12600	-12520	-15684	-20745	-22339	-22819	-22869	-22780	-22642
3	-12558	-12554	-12339	-16063	-21107	-22543	-22906	-22893	-22774
4	-12403	-12384	-12415	-12153	-15430	-20363	-21523	-21740	-21695
5	-12187	-12170	-12159	-12204	-11946	-15251	-18979	-19783	-19931
6	-11922	-11908	-11895	-11892	-11893	-11717	-14444	-17131	-17758
7	-11597	-11601	-11603	-11606	-11606	-11606	-11463	-20592	-15510
8	-7928	-7910	-7917	-7922	-7924	-7936	-8056	-7468	-16802



Gambar 5.20 Grafik Shear Balok (Model 2)

Tabel 5.12 Shear Balok (Model 3)

Tingk.	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
1	-32677	-36132	-47957	-54059	-55675	-55920	-55809	-55590	-55339	-55077	-54810	-54530	-54251	-57168
2	-33039	-33108	-47549	-58725	-64267	-65691	-68148	-65998	-65740	-65456	-65167	-64880	-64564	-67565
3	-33195	-33518	-33153	-41273	-60916	-66226	-67740	-67956	-67772	-67487	-67190	-66873	-66564	-69606
4	-33211	-33492	-33605	-33146	-41151	-60090	-65035	-66400	-66556	-66346	-66048	-66734	-65415	-68523
5	-32978	-33421	-33461	-33530	-33129	-40253	-57967	-62478	-63673	-63765	-63531	-63224	-62897	-66063
6	-32832	-33262	-33315	-33278	-33422	-33061	-39770	-55094	-59087	-60087	-60106	-59847	-59510	-62735
7	-32743	-33028	-33086	-33046	-33086	-33254	-32943	-39189	-51609	-55010	-55787	-55727	-55425	-58686
8	-32439	-32726	-32788	-32740	-32782	-32845	-33035	-32780	-38472	-47551	-50280	-50809	-50642	-53961
9	-32074	-32363	-32430	-32376	-32412	-32476	-32563	-32773	-32583	-37635	-42925	-44901	-45130	-48578
10	-31657	-31947	-32018	-31960	-31994	-32051	-32137	-32249	-32481	-32366	-36698	-37725	-38826	-43362
11	-31200	-31492	-31566	-31506	-31538	-31591	-31671	-31782	-31924	-32183	-32158	-35699	-34445	-40515
12	-30690	-30985	-31062	-31000	-31031	-31082	-31157	-31261	-31400	-31576	-31872	-31956	-34674	-37753
13	-20044	-20040	-20115	-20056	-20085	-20133	-20203	-20299	-20427	-20596	-20807	-21174	-21288	-24778



Gambar 5.21 Grafik Shear Balok (Model 3)

5.3.3 Defleksi pada Join

Defleksi join pada tiap tingkat untuk berbagai kombinasi disajikan pada gambar 5.22, 5.23, 5.24, 5.25, 5.26, 5.27. Pengaruh *Infill wall* secara umum diuraikan sebagai berikut ini.

1. Arah X (Horisontal)

Defleksi join akan mulai berubah apabila pada tingkat tersebut tidak terisi tembok. Defleksi akan bertambah secara bertahap pada kombinasi mulai dari portal penuh ke portal terbuka (Gambar 5.22.1). Secara keseluruhan defleksi pada portal penuh sangat kecil dibanding dengan portal terbuka.

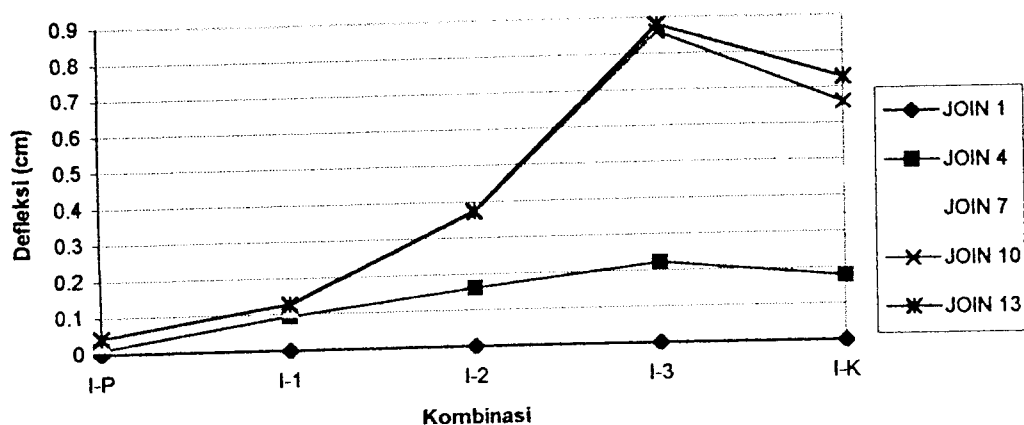
2. Arah Y (Vertikal)

Infill wall akan mempengaruhi pada arah vertikal tetapi perubahan ini relatif dan perubahan ini tidak secara langsung akibat pengurangan *infill wall*.

Kombinasi dan perilaku tingkat lainnya disajikan dalam gambar 5.22, 5.23, 5.24, 5.25, 5.26, 5.27 berikut ini.

Tabel 5.13 Defleksi Arah X (Model 1)

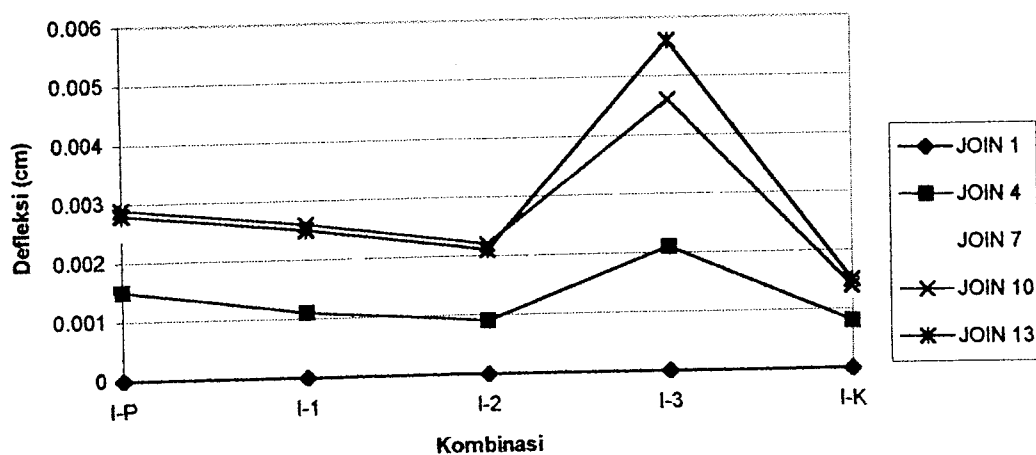
JOIN	I-P	I-1	I-2	I-3	I-K
JOIN 1	0	0	0	0	0
JOIN 4	0.0109	0.0944	0.1619	0.222	0.178
JOIN 7	0.0287	0.1145	0.3566	0.584	0.457
JOIN 10	0.0422	0.1279	0.3729	0.8594	0.6603
JOIN 13	0.0428	0.1287	0.374	0.8786	0.7282



Gambar 5.22 Grafik Defleksi Arah X (Model 1)

Tabel 5.14 Defleksi Arah Y (Model 1)

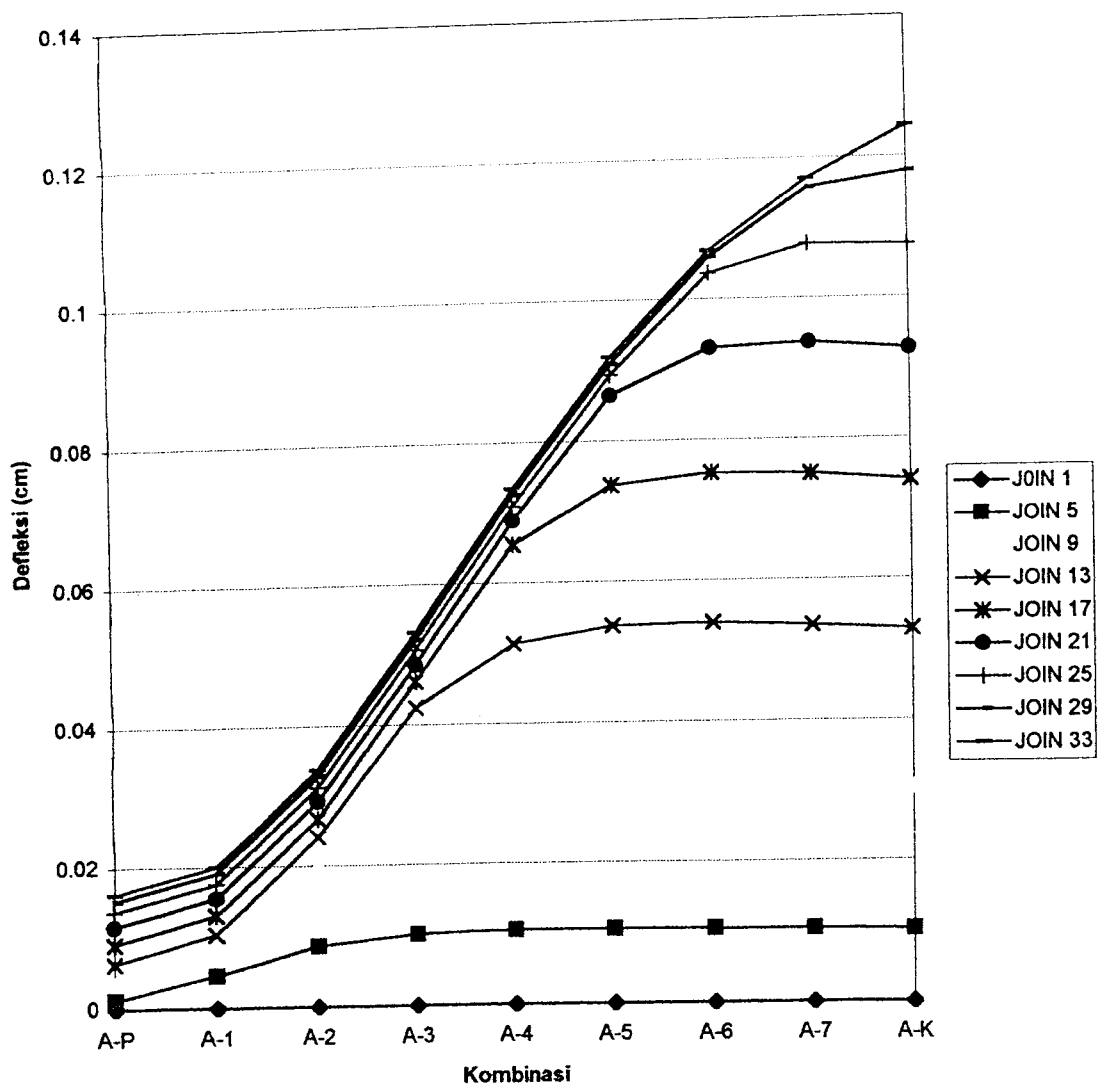
JOIN	I-P	I-1	I-2	I-3	I-K
JOIN 1	0	0	0	0	0
JOIN 4	0.0015	0.0011	0.0009	0.0021	0.0008
JOIN 7	0.0025	0.0022	0.0015	0.0036	0.0013
JOIN 10	0.0029	0.0026	0.0022	0.0046	0.0014
JOIN 13	0.0028	0.0025	0.0021	0.0056	0.0015



Gambar 5.23 Grafik Defleksi Arah Y (Model 1)

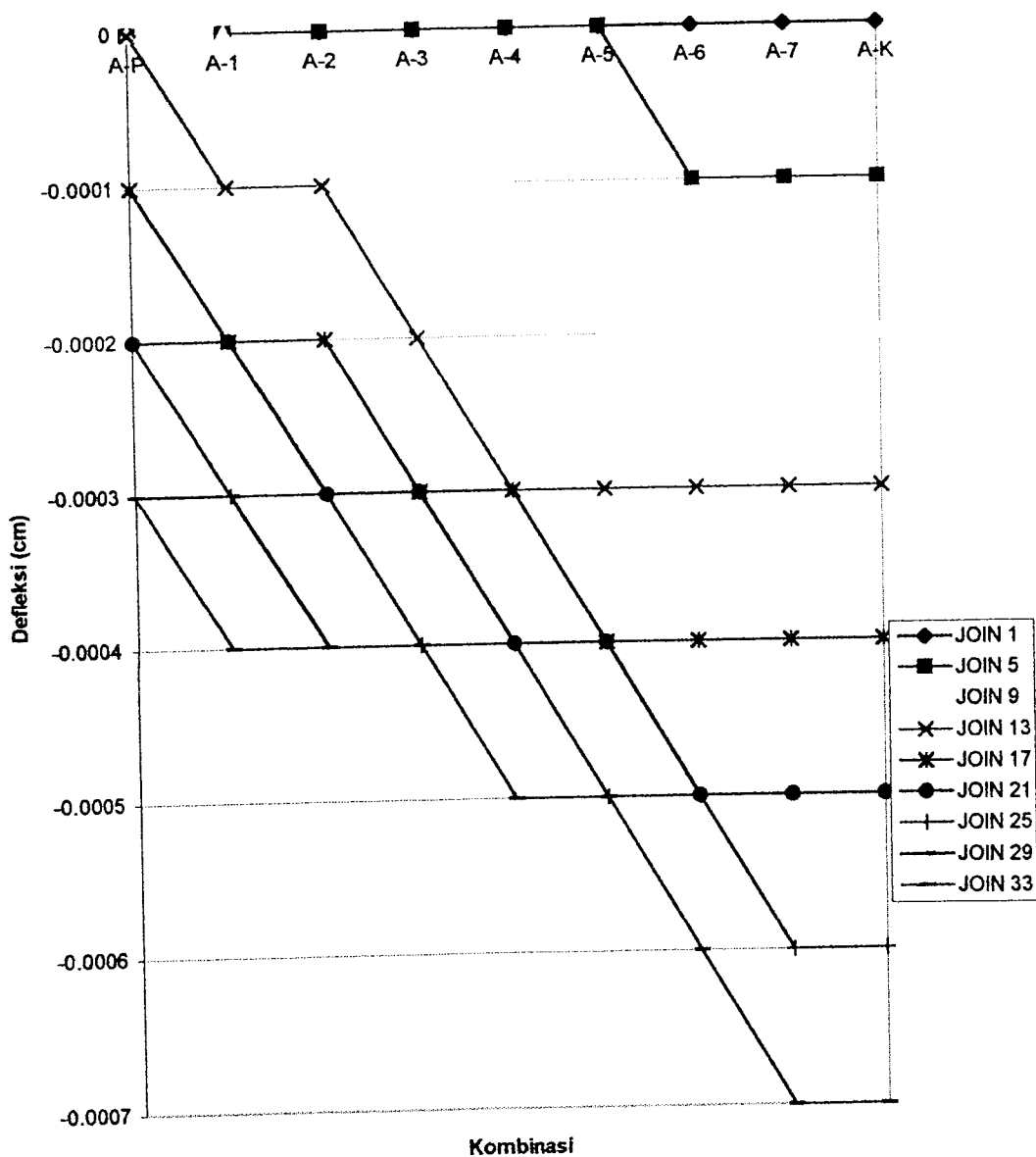
Tabel 5.15 Defleksi Arah X (Model 2)

JOIN	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
JOIN 1	0	0	0	0	0	0	0	0	0
JOIN 5	0.0012	0.0046	0.0086	0.0101	0.0105	0.0105	0.0105	0.0104	0.0102
JOIN 9	0.0036	0.0076	0.0204	0.0282	0.0307	0.0312	0.0311	0.0309	0.0304
JOIN 13	0.0064	0.0104	0.0241	0.0424	0.0513	0.0537	0.054	0.0535	0.0528
JOIN 17	0.0091	0.0131	0.0267	0.0461	0.0654	0.0737	0.0753	0.0751	0.0741
JOIN 21	0.0116	0.0155	0.0292	0.0485	0.0688	0.0865	0.093	0.0937	0.0927
JOIN 25	0.0137	0.0176	0.0312	0.0506	0.0709	0.0894	0.1037	0.1077	0.1075
JOIN 29	0.0152	0.0191	0.0328	0.0521	0.0725	0.0909	0.106	0.1157	0.1179
JOIN 33	0.0162	0.02	0.0337	0.0531	0.0734	0.0919	0.1069	0.117	0.1244



Tabel 5.16 Defleksi Arah Y (Model 2)

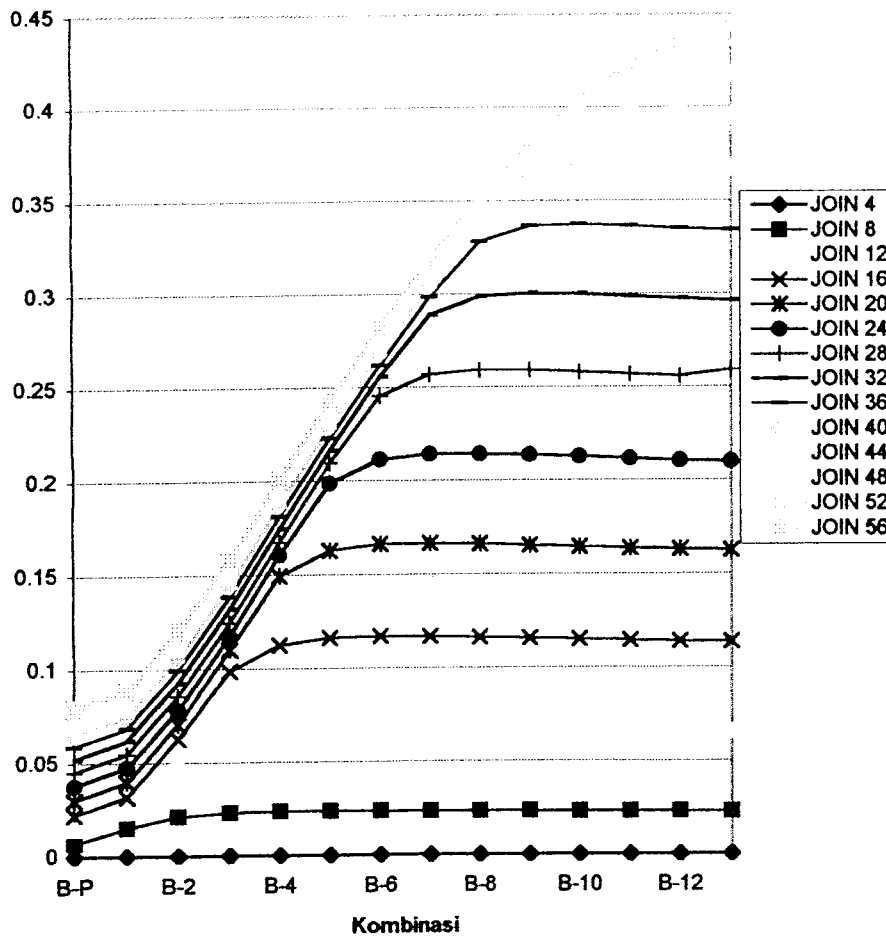
JOIN	A-P	A-1	A-2	A-3	A-4	A-5	A-6	A-7	A-K
JOIN 1	0	0	0	0	0	0	0	0	0
JOIN 5	0	0	0	0	0	0	-0.0001	-0.0001	-0.0001
JOIN 9	0	0	-0.0001	-0.0001	-0.0001	-0.0002	-0.0002	-0.0002	-0.0002
JOIN 13	0	-0.0001	-0.0001	-0.0002	-0.0003	-0.0003	-0.0003	-0.0003	-0.0003
JOIN 17	-0.0001	-0.0002	-0.0002	-0.0003	-0.0003	-0.0004	-0.0004	-0.0005	-0.0005
JOIN 21	-0.0002	-0.0002	-0.0003	-0.0003	-0.0004	-0.0004	-0.0005	-0.0005	-0.0006
JOIN 25	-0.0002	-0.0003	-0.0003	-0.0004	-0.0004	-0.0005	-0.0005	-0.0006	-0.0007
JOIN 29	-0.0003	-0.0003	-0.0004	-0.0004	-0.0005	-0.0005	-0.0006	-0.0007	-0.0007
JOIN 33	-0.0003	-0.0004	-0.0004	-0.0004	-0.0005	-0.0005	-0.0006	-0.0007	-0.0007



Gambar 5.25 Grafik Defleksi Arah Y (Model 2)

Tabel 5.17 Defleksi Arah X (Model 3)

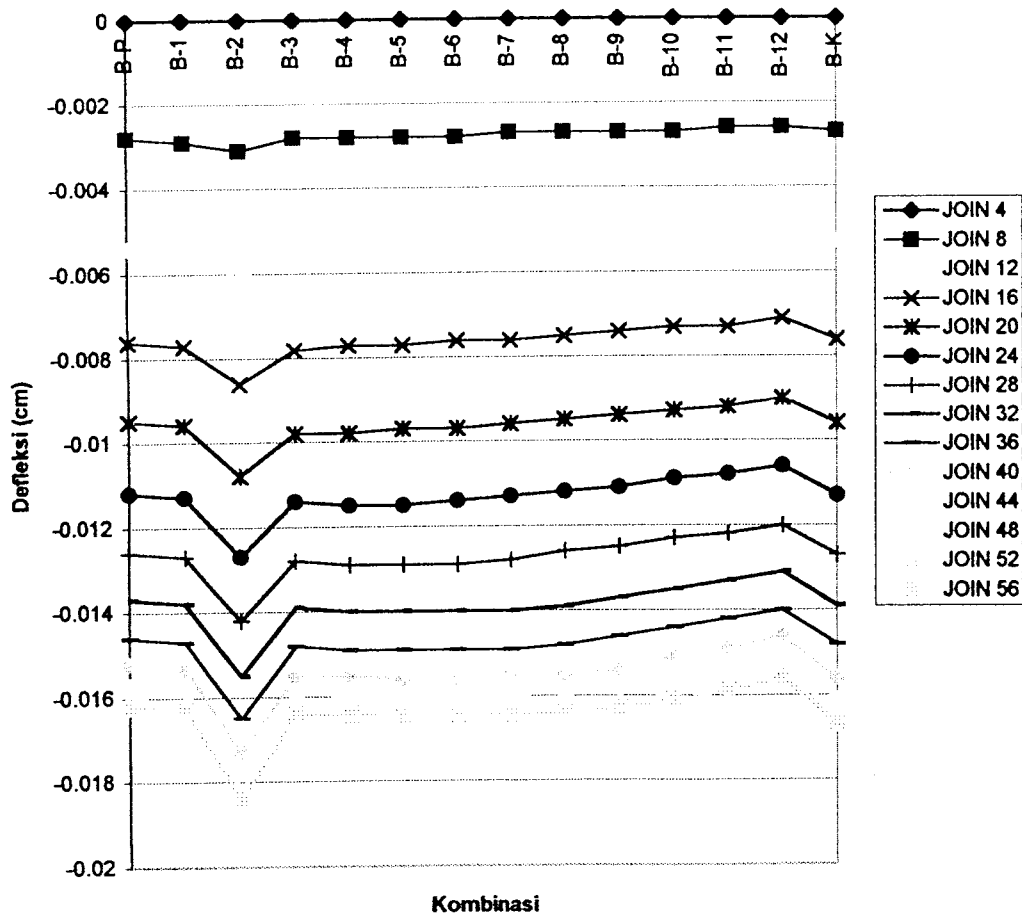
JOIN	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
JOIN 4	0	0	0	0	0	0	0	0	0	0	0	0	0	0
JOIN 8	0.0068	0.0149	0.0208	0.0229	0.0234	0.0235	0.0234	0.0233	0.0233	0.0232	0.023	0.0229	0.0228	0.0227
JOIN 12	0.0146	0.0245	0.0515	0.0633	0.0667	0.0676	0.0676	0.0673	0.0671	0.0667	0.0664	0.0661	0.0658	0.0654
JOIN 16	0.0222	0.0319	0.0625	0.0989	0.1126	0.1163	0.1171	0.1168	0.1164	0.1158	0.1153	0.1147	0.1142	0.1137
JOIN 20	0.0299	0.0396	0.0701	0.1103	0.1492	0.1627	0.1661	0.1665	0.1661	0.1654	0.1646	0.1638	0.1631	0.1624
JOIN 24	0.0376	0.0472	0.0777	0.1175	0.1605	0.1985	0.2112	0.2141	0.2142	0.2135	0.2126	0.2116	0.2105	0.2098
JOIN 28	0.045	0.0546	0.0853	0.1248	0.1675	0.2095	0.2454	0.2568	0.2592	0.259	0.258	0.2568	0.2556	0.259
JOIN 32	0.052	0.0616	0.0926	0.1319	0.1745	0.2162	0.2558	0.2888	0.2989	0.3005	0.3	0.2987	0.2974	0.2965
JOIN 36	0.0586	0.0682	0.0994	0.1384	0.1811	0.2227	0.2621	0.2986	0.3283	0.3366	0.3375	0.3366	0.3352	0.3343
JOIN 40	0.0645	0.0741	0.1056	0.1443	0.1869	0.2286	0.2681	0.3044	0.3372	0.3629	0.3694	0.3695	0.3683	0.3675
JOIN 44	0.0697	0.0793	0.1111	0.1494	0.1921	0.2338	0.2733	0.3096	0.3423	0.3708	0.392	0.3965	0.396	0.3955
JOIN 48	0.074	0.0835	0.1157	0.1537	0.1964	0.2382	0.2777	0.3141	0.3468	0.3752	0.3989	0.4151	0.4175	0.4178
JOIN 52	0.0773	0.0868	0.1193	0.1569	0.1996	0.2415	0.2811	0.3175	0.3503	0.3787	0.4023	0.4206	0.4316	0.4341
JOIN 56	0.0794	0.0888	0.1218	0.1589	0.2016	0.2435	0.2831	0.3196	0.3524	0.3809	0.4046	0.4228	0.4352	0.4448



Gambar 5.26 Grafik Defleksi Arah X (Model 3)

Tabel 5.18 Defleksi Arah Y (Model 3)

JOIN	B-P	B-1	B-2	B-3	B-4	B-5	B-6	B-7	B-8	B-9	B-10	B-11	B-12	B-K
JOIN 4	0	0	0	0	0	0	0	0	0	0	0	0	0	0
JOIN 8	-0.0028	-0.0029	-0.0031	-0.0028	-0.0028	-0.0028	-0.0028	-0.0027	-0.0027	-0.0027	-0.0027	-0.0028	-0.0026	-0.0027
JOIN 12	-0.0054	-0.0054	-0.0061	-0.0054	-0.0054	-0.0054	-0.0053	-0.0053	-0.0052	-0.0052	-0.0051	-0.0051	-0.005	-0.0053
JOIN 16	-0.0076	-0.0077	-0.0086	-0.0078	-0.0077	-0.0077	-0.0076	-0.0076	-0.0075	-0.0074	-0.0073	-0.0073	-0.0071	-0.0076
JOIN 20	-0.0095	-0.0096	-0.0108	-0.0098	-0.0098	-0.0097	-0.0097	-0.0096	-0.0095	-0.0094	-0.0093	-0.0092	-0.009	-0.0096
JOIN 24	-0.0112	-0.0113	-0.0127	-0.0114	-0.0115	-0.0115	-0.0114	-0.0113	-0.0112	-0.0111	-0.0109	-0.0108	-0.0106	-0.0113
JOIN 28	-0.0126	-0.0127	-0.0142	-0.0128	-0.0129	-0.0129	-0.0129	-0.0128	-0.0126	-0.0125	-0.0123	-0.0122	-0.012	-0.0127
JOIN 32	-0.0137	-0.0138	-0.0155	-0.0139	-0.014	-0.014	-0.014	-0.014	-0.0139	-0.0137	-0.0135	-0.0133	-0.0131	-0.0139
JOIN 36	-0.0146	-0.0147	-0.0165	-0.0148	-0.0149	-0.0149	-0.0149	-0.0149	-0.0148	-0.0146	-0.0144	-0.0142	-0.014	-0.0148
JOIN 40	-0.0152	-0.0153	-0.0173	-0.0155	-0.0155	-0.0156	-0.0156	-0.0155	-0.0155	-0.0154	-0.0151	-0.0149	-0.0146	-0.0156
JOIN 44	-0.0157	-0.0158	-0.0178	-0.0159	-0.016	-0.016	-0.016	-0.016	-0.0159	-0.0158	-0.0157	-0.0154	-0.0151	-0.0161
JOIN 48	-0.016	-0.0161	-0.0182	-0.0162	-0.0163	-0.0163	-0.0163	-0.0163	-0.0162	-0.0161	-0.016	-0.0157	-0.0154	-0.0164
JOIN 52	-0.0162	-0.0163	-0.0184	-0.0164	-0.0164	-0.0165	-0.0165	-0.0164	-0.0164	-0.0163	-0.0161	-0.0159	-0.0156	-0.0166
JOIN 56	-0.0162	-0.0163	-0.0184	-0.0164	-0.0165	-0.0165	-0.0165	-0.0165	-0.0164	-0.0163	-0.0161	-0.0159	-0.0156	-0.0167



Gambar 5.27 Grafik Defleksi Arah Y (Model 3)

BAB VI

KESIMPULAN DAN SARAN

6.1 Kesimpulan

Berdasarkan uraian dari bab-bab terdahulu, maka dapat diambil beberapa kesimpulan seperti diuraikan berikut ini.

1. Dinding batu-bata/batako ternyata dapat berfungsi sebagai pengaku portal sehingga struktur akan dapat menahan gaya gempa lebih baik, karena dinding mampu menahan gaya horisontal. Gaya horisontal yang bekerja pada struktur juga ikut ditahan oleh dinding dalam bentuk *diagonal bracing* yang berlawanan arah terhadap gaya gempa.
2. Dinding akan berperilaku baik, apabila penempatannya menerus. Menerus adalah dinding/*infill wall* tidak mengalami pemutusan dari lantai bawah sampai lantai atas.
3. Dinding yang tidak menerus atau mengalami pemutusan akan sangat berpengaruh pada kolom sehingga akan mengakibatkan *soft storey efect*. Apabila pada perencanaan tanpa memperhitungkan dinding, momen, gaya geser pada kolom maupun balok akan berubah drastis

(berbanding terbalik) dengan perencanaan semula, sehingga akan berakibat fatal jika terjadi gempa.

4. Modulus elastis, ketinggian bangunan berpengaruh terhadap kemampuan *infill wall* menahan gaya horisontal.

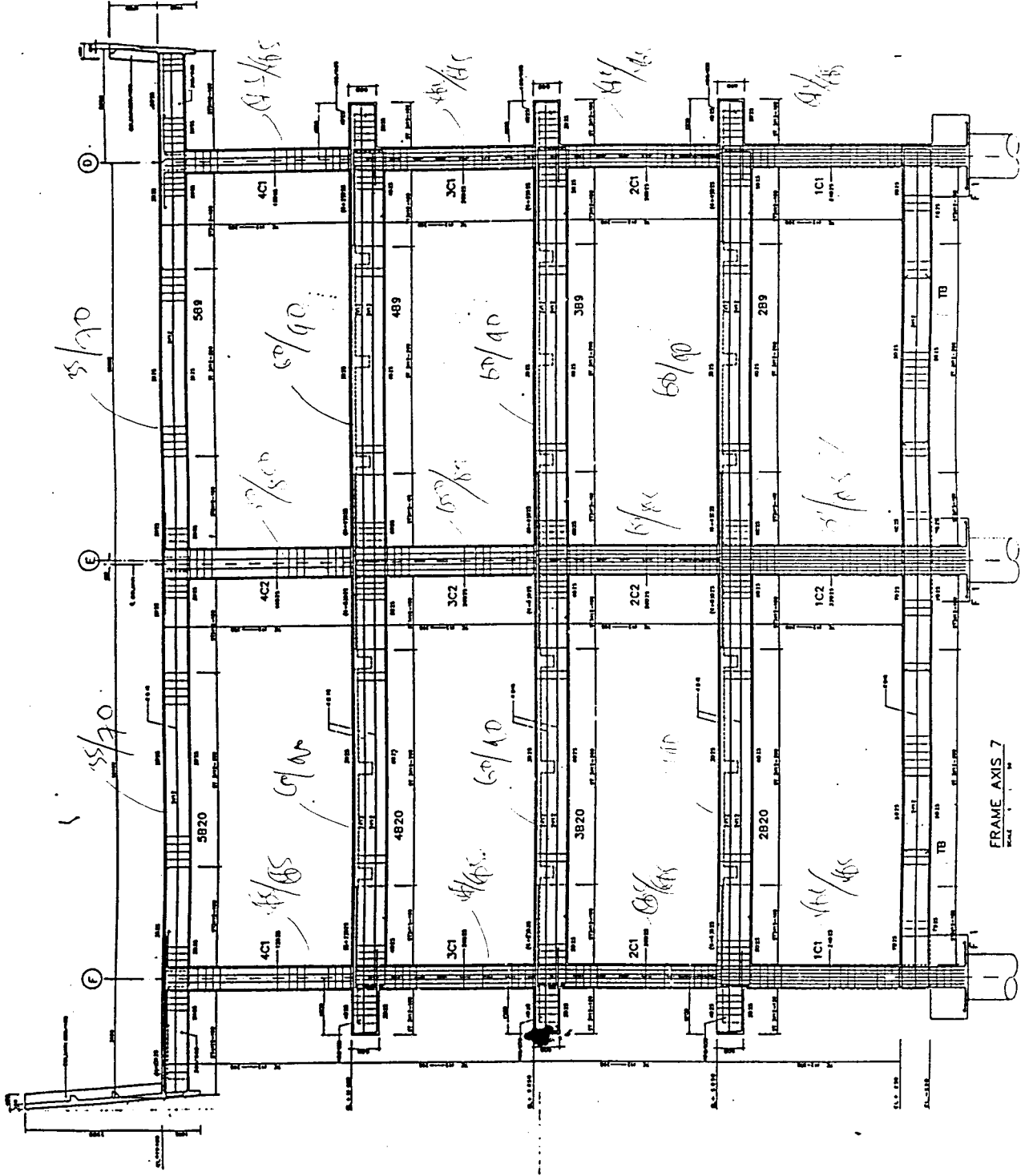
6.2. Saran

1. Karena banyaknya manfaat dari *infill wall* sebaiknya mulai diperhatikan dalam perencanaan struktur. Hal ini perlu dipertimbangkan sebagai suatu alternatif struktur tahan gempa dengan biaya relatif murah.
2. Pelaksanaan pada struktur *infill wall* sebaiknya dilakukan menempel dengan kolom dan balok, karena akan sangat berpengaruh terhadap kekuatan dinding dalam menahan gaya gempa.
3. Untuk portal tidak menerus, sebaiknya *infill wall* benar-benar dipisahkan dari portal sehingga apabila terjadi gempa, struktur tidak akan mengalami perubahan momen, gaya geser, defleksi yang berlebihan dan struktur akan relatif aman sesuai perencanaan.
4. Dalam pelaksanaan sebaiknya mulai diperhitungkan mutu dari campuran untuk dinding batu-bata/batako karena akan mempengaruhi kekakuan struktur.


DAFTAR PUSTAKA

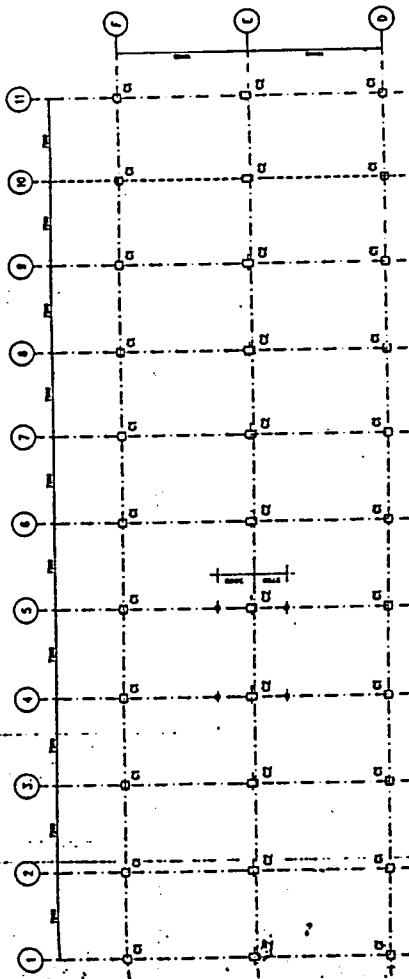
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LAMPIRAN



FRAME AXIS 7
SCALE 1/8\"/>

			
PROJECT: <u>INDUSTRIAL REFINERY PROJECT</u>			
SHEET: <u>LAB-TEK X</u>			
DRAWING: <u>FRAME AXIS 7</u>			
DESIGNER: <u>...</u>	CHECKED: <u>...</u>	DATE: <u>...</u>	SCALE: <u>...</u>
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CLIENT: <u>...</u>	ARCHITECT: <u>...</u>	ENGINEER: <u>...</u>	CONSULTANT: <u>...</u>
ADDRESS: <u>...</u>	CITY: <u>...</u>	STATE: <u>...</u>	ZIP: <u>...</u>
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KEY PLAN COLUMN LABTEK X
SCALE 1:200

COLUMN SCHEDULE	
MARKS	DETAILS
DL + 11.00	
DL + 11.00	
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DL + 11.00	
DL + 11.00	
DL + 11.00	

NO.	REVISION	DATE	BY

PROF. PERMULIAN PERMULIAN, M.Eng.
 INSTITUTE TEKNOLOGI SURABAYA

NO.	REVISION	DATE	BY

FIELD NO.
 PROJECT NO.
 DRAWING NO.

LABTEK X

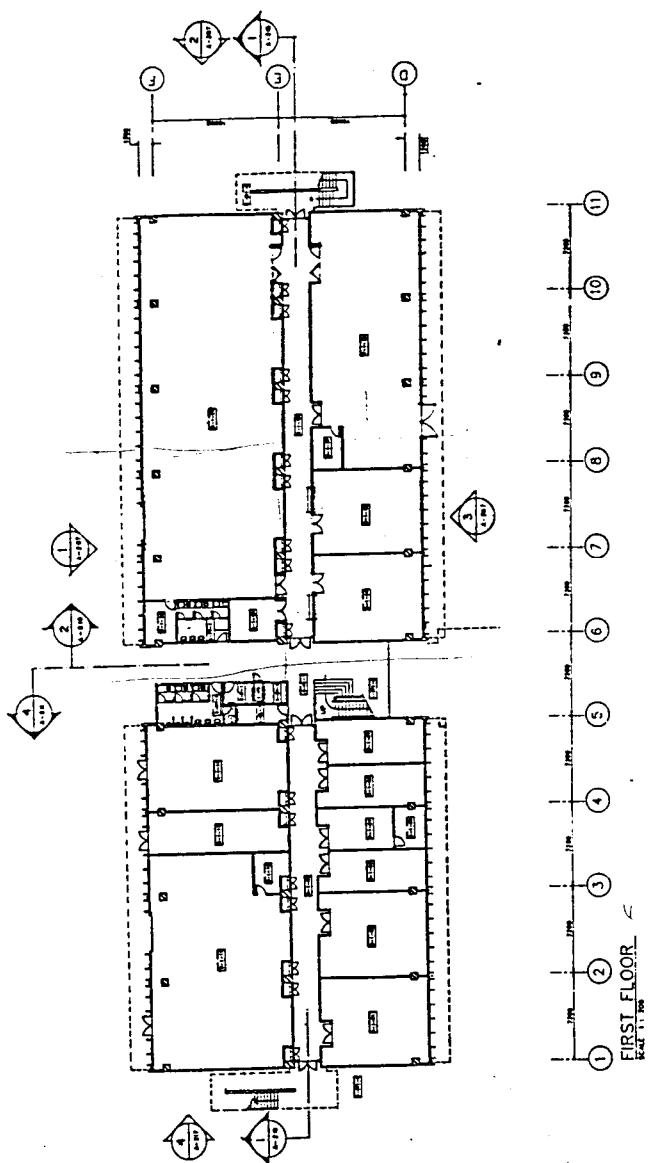
COLUMN SCHEDULE

DESIGNED BY: PERMULIAN PERMULIAN
 CHECKED BY: PERMULIAN PERMULIAN
 APPROVED BY: PERMULIAN PERMULIAN

NO.	REVISION	DATE	BY

LEGEND

NUMBER	ROOM NAME
2-1-1	MECHANICAL WORKSHOP
2-1-2	MECHANICAL WORKSHOP
2-1-3	MECHANICAL WORKSHOP
2-1-4	MECHANICAL WORKSHOP
2-1-5	MECHANICAL WORKSHOP
2-1-6	MECHANICAL WORKSHOP
2-1-7	MECHANICAL WORKSHOP
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2-1-97	MECHANICAL WORKSHOP
2-1-98	MECHANICAL WORKSHOP
2-1-99	MECHANICAL WORKSHOP
2-1-100	MECHANICAL WORKSHOP



FIRST FLOOR
SCALE: 1/8" = 1'-0"

1. Mechanical Room
2. 2-1-100
3. 2-1-101
4. 2-1-102
5. 2-1-103
6. 2-1-104
7. 2-1-105
8. 2-1-106
9. 2-1-107
10. 2-1-108



NO.	REVISION	DATE	BY	APP'D

PROJECT: INDUSTRIAL FLOORING PROJECT
 CLIENT: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

PROJECT NO.: LABTEK X
 PROJECT NAME: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

DATE: 10/1/78

SCALE: 1:200

PROJECT NO.: A-207

NO.	REVISION	DATE	BY	APP'D

PROJECT: INDUSTRIAL FLOORING PROJECT
 CLIENT: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

PROJECT NO.: LABTEK X
 PROJECT NAME: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

DATE: 10/1/78

SCALE: 1:200

PROJECT NO.: A-207

NO.	REVISION	DATE	BY	APP'D

PROJECT: INDUSTRIAL FLOORING PROJECT
 CLIENT: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

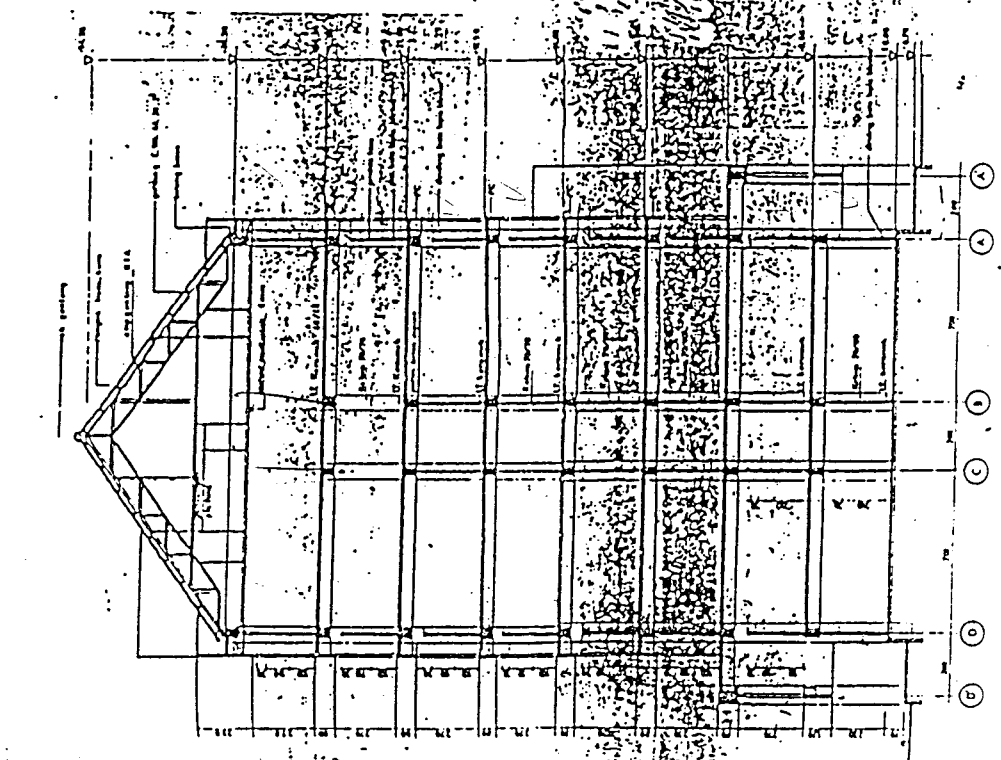
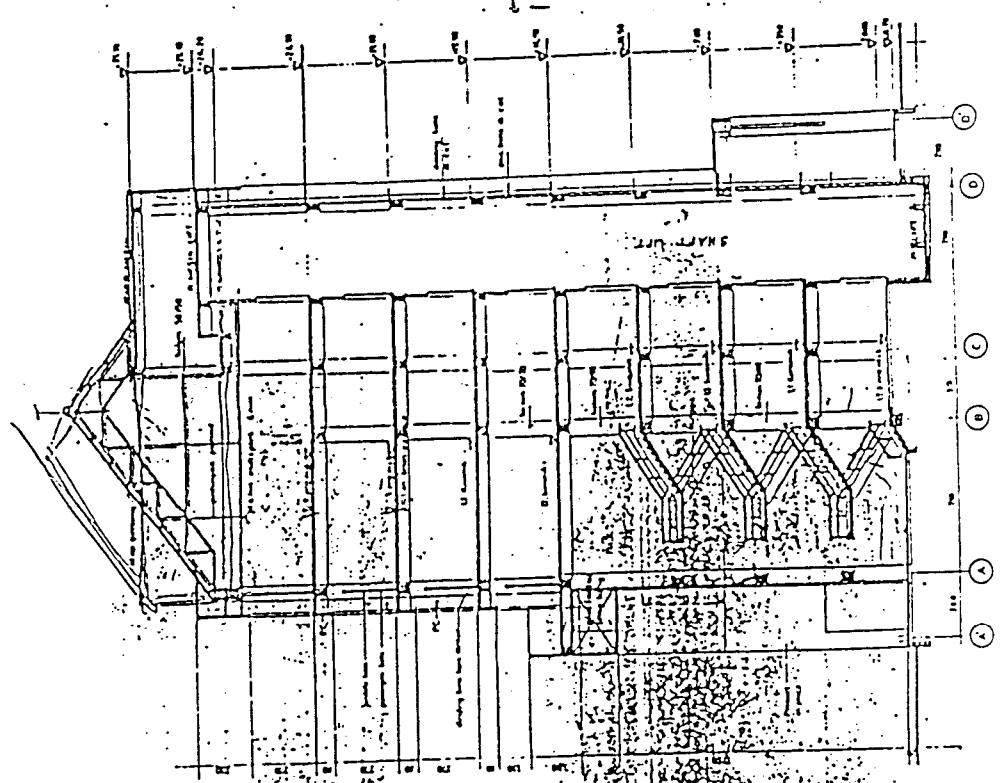
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 PROJECT NAME: INDUSTRIAL BUILDING DEVELOPMENT PROJECT

DATE: 10/1/78

SCALE: 1:200

PROJECT NO.: A-207

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PT. ARSERVO

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GEDUNG UNIVERSITAS
ISLAM AS-SYAFI'AH
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BEKASI

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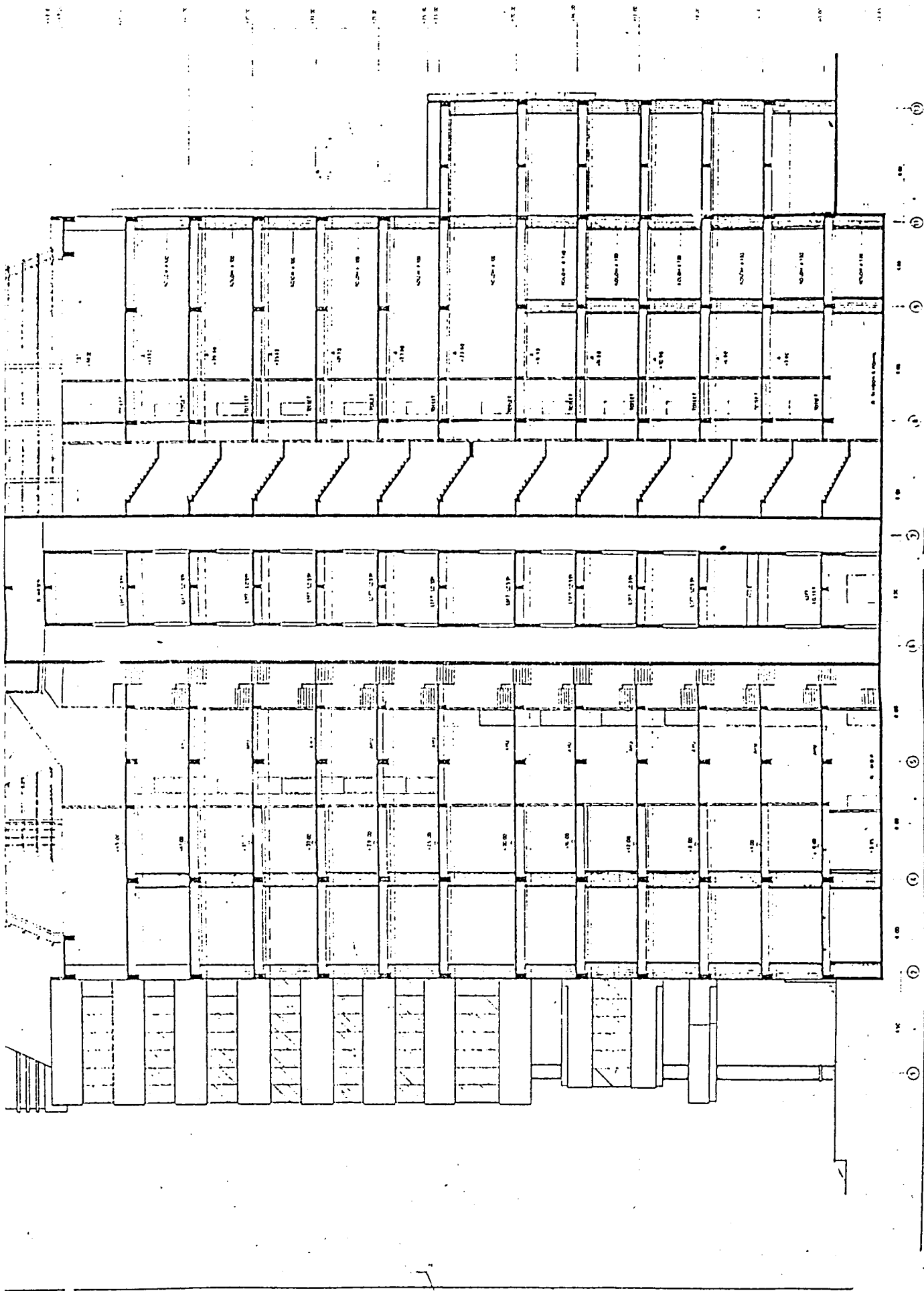
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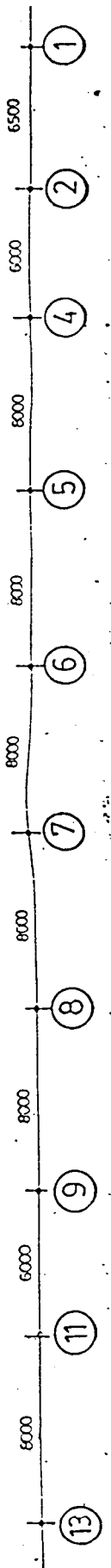
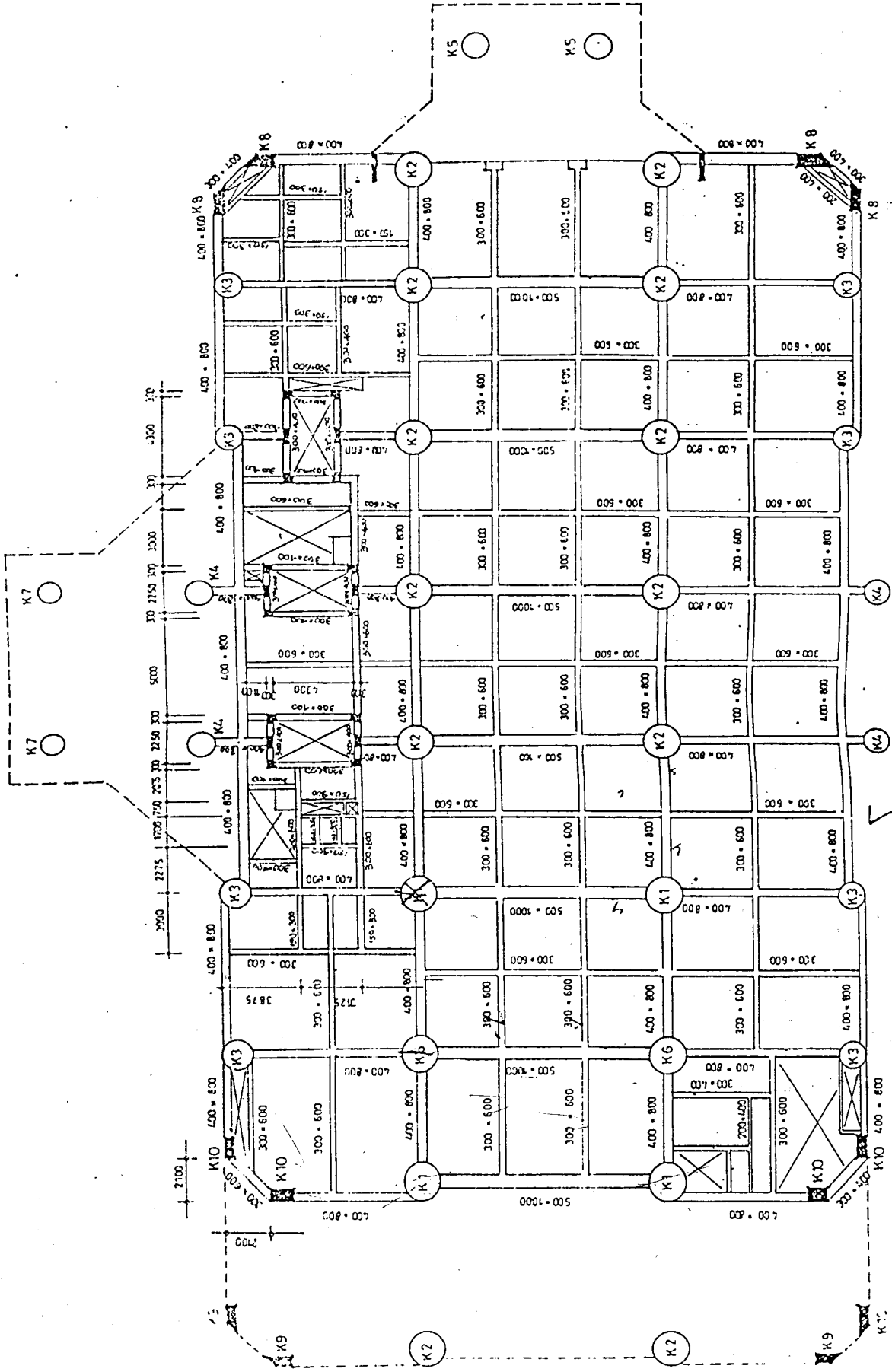
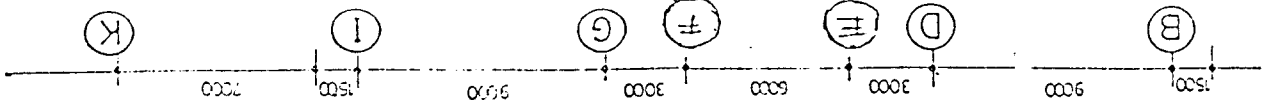
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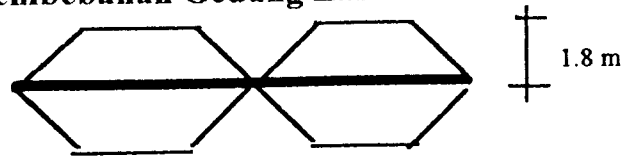
TABEL KOLOM I

LOKASI	TYPE K1	TYPE K2	TYPE K3	TYPE K4	TYPE KX-1	TYPE KLI	SKALA
LOKASI 1							1:20
LOKASI 2							1:20
LOKASI 3							1:20
LOKASI 4							1:20
LOKASI 5							1:20
LOKASI 6							1:20
LOKASI 7							1:20
LOKASI 8							1:20
LOKASI 9							1:20
LOKASI 10							1:20





Perhitungan Pembebanan Gedung Laboratorium Teknologi X ITB



* Pada Pelat Atap

Beban mati

Berat sendiri	$0,1 * 2400$	$= 240 \text{ kg/m}^2$
Spesi	$2 * 21$	$= 42 \text{ kg/m}^2$
Plafon & penggantung	$11 + 7$	$= 18 \text{ kg/m}^2$
		<u>300 kg/m^2</u>

Beban hidup

Beban berguna	$= 100 \text{ kg/m}^2$
Air hujan	$= 40 \text{ kg/m}^2$
	<u>140 kg/m^2</u>

*Pelat Lantai

Beban mati

Berat sendiri	$0,12 * 2400$	$= 288 \text{ kg/m}^2$
Spesi	$2 * 21$	$= 42 \text{ kg/m}^2$
Plafon & penggantung	$11 + 7$	$= 18 \text{ kg/m}^2$
Tegel	$3 * 24$	$= 72 \text{ kg/m}^2$
		<u>420 kg/m^2</u>

Beban hidup

$$= 250 \text{ kg/m}^2$$

* Berat sendiri balok

a. Berat balok atap $0,35 * (0,7 - 0,1) * 2400 = 504 \text{ kg/m}'$

b. Berat balok lantai dan tembok

$$0,6 * (0,9 - 0,12) * 2400 + 250 * 4,1 = 2148,2 \text{ kg/m}'$$

*Berat sendiri kolom

$$0,65 * 0,65 * 2400 = 1014 \text{ kg/m}' \text{(kolom 1&3)}$$

$$0,65 * 0,85 * 2400 = 1326 \text{ kg/m}' \text{(kolom 2)}$$

*Beban ekuivalen

Beban mati

Balok atap

$$M = 0,0208 * q * l_x * (3l_y - l_x)$$

$$M = 0,0208 * 300 * 3,6 * (3 * 10,6^2 - 3,6^2) = 7281,0317 \text{ kg.m}$$

$$q = 8 \frac{M}{l_y^2} = 518,4074$$

$$518,4074 * 2 = 1036,814$$

$$q \text{ balok} = \frac{504}{}$$

$$q \text{ ekiv.} = 1540,814 \text{ kg/m} \approx 1550 \text{ kg/m}$$

Balok lantai

$$M = 0,0208 * 420 * 3,6 * (3 * 10,6^2 - 3,6^2) = 10487,81 \text{ kg m}$$

$$q = 746,729 * 2 = 1497,45$$

$$q \text{ balok} = \frac{2148,2}{2}$$

$$q \text{ ekiv. lantai} = 3645,65 \text{ kg/m} \approx 3646 \text{ kg/m}$$

Beban hidup**Balok atap**

$$M = 0,0208 * 140 * 3,6 * (3 * 10,6^2 - 3,6^2) = 3397,81 \text{ kgm}$$

$$q \text{ ekiv. atap} = 241,923 * 2 = 483,846 \text{ kg/m} \approx 490 \text{ kg/m}$$

Balok lantai

$$M = 0,0208 * 250 * 3,6 * (3 * 10,6^2 - 3,6^2) = 6067,526 \text{ kgm}$$

$$q \text{ ekiv.} = 432,006 * 2 = 864,0123 \text{ kg/m} \approx 900 \text{ kg/m}$$

*** Pembebanan Gempa*****Berat atap****Beban mati**

Pelat	: 21,2 * 7,2 * 300	= 45792 kg
Balok bujur	: 0,35 * 0,7 * 21,2 * 1 * 2400	= 12465 kg
Balok melintang	: 0,35 * 0,7 * 7,2 * 3 * 2400	= 12700 kg
Kolom	: 0,65 * 0,65 * 2 * 2,4 * 2400	= 4867 kg
	: 0,65 * 0,85 * 2,4 * 1 * 2400	= 3182 kg
Tembok	: 21,2 * 2,4 * 250	= 12720 kg
	: 3 * 7,2 * 2,4 * 250	= 12960 kg
		<u>104686 kg</u>

Beban hidup

$$q = 140 \text{ kg/m}^2$$

$$\text{Faktor Reduksi} = 0,3$$

$$W \text{ atap(h)} = 21,2 * 7,2 * 140 * 0,3 = 6410,8 \text{ kg}$$

$$W \text{ total atap} = 104686 + 6410,8 = 111096,8 \text{ kg}$$

Berat lantai*Beban mati**

Pelat	: 670 * 21,2 * 7,2	= 1022688 kg
Balok Bujur	: 0,6 * 0,9 * 21,2 * 1 * 2400	= 127475,2 kg
Balok melintang	: 0,6 * 0,9 * 7,2 * 3 * 2400	= 27993,6 kg
Kolom	: 0,65 * 0,65 * 2 * 4,8 * 2400	= 9734,4 kg
	: 0,65 * 0,85 * 1 * 4,8 * 2400	= 6364,8 kg
Tembok	: 21,2 * 250 * 3,9	= 20670 kg
	: 7,2 * 3 * 250 * 3,9	= 21060 kg
		<u>1235986 kg</u>

Beban hidup

$q = 250 \text{ kg/m}^2$

Faktor reduksi = 0,3

$W \text{ lantai} = 21,2 * 2,4 * 250 * 0,3 = 3816 \text{ kg}$

$W \text{ lantai} = 1235986 + 3816 = 1239802 \text{ kg}$

$W \text{ atap \& lantai total} = 3(1239802) + 111096,8 = 3830502 \text{ kg}$

***Waktu getar bangunan (T)**

Dengan rumus Empiris

$T = 0,06 * H^{0,75}$

$H = 4 * 4,8 = 19,2$

$T = 0,06 * (19,2)^{0,75} = 0,550335 \text{ detik}$

***Koefisien gempa dasar (C)**

C diperoleh dari gambar (struktur diatas tanah lunak)

$C = 0,07$

***Faktor keutamaan I dan faktor jenis struktur K**

$I = 1,5$ $K = 1,0$ untuk bangunan kampus yang menggunakan struktur rangka beton bertulang dengan daktilitas penuh.

***Gaya geser horisontal total akibat gempa**

$v_i = C * I * K * W$

$= 0,07 * 1,5 * 1,0 * 3830502 = 402202$

Gaya geser horisontal total akibat gempa sepanjang tinggi gedung

$H/A = 19,2/21,2 = 0,9056 < 3$

hi (m)	wi (kg)	wi hi (kgm)	fi xi
19,2	104686	2009971	21496
14,4	1235986	17798198	190352
9,6	1235986	11865465	126901
4,8	1235986	5932732	63450

37606366

Pengurangan akibat Infill Wall

$q_{\text{ekiv. lantai (akibat pengurangan)}}$

$= q_{\text{ekiv. lantai}} - q_{\text{tembok}}$

$= 20670 - 1035 = 3521 \text{ kg/m}$

Perhitungan Pembebanan Gedung BNI Surabaya

*Pelat Atap

$$\text{Beban Mati} = 300 \text{ kg/m}$$

$$\text{Beban hidup} = 140 \text{ kg/m}^2$$

*Pelat lantai

$$\text{Beban mati} = 420 \text{ kg/m}^2$$

Beban hidup

$$\text{Beban berguna} = 250 \text{ kg/m}^2$$

*Berat sendiri balok

$$\text{Berat balok atap} = 0,6 * (0,8 - 0,1) * 2400 = 1008 \text{ kg/m'}$$

$$\text{Berat balok lantai} = 0,6 * (0,8 - 0,12) * 2400 = 979,2 \text{ kg/m'}$$

$$\text{Tembok} = 250 * 3,2 = 800 \text{ kg/m'}$$

$$\underline{1779,2 \text{ kg/m'}}$$

$$\text{Balok anak atap} = 0,6 * (0,3 - 0,1) * 2400 = 288 \text{ kg/m}$$

$$\text{Balok anak lantai} = 0,6 * (0,3 - 0,12) * 2400 = 259,2 \text{ kg/m}$$

*Berat sendiri kolom

$$0,9 * 0,9 * 2400 = 1944 \text{ kg/m'}$$

*Beban Ekvivalen

Beban akibat gravitasi

Beban Mati

Balok Atap

$$M = 0,0417 * 300 * 4^3 = 400,32 \text{ kg m}$$

$$q = 400,32 * 2 = 800,64 \text{ kg/m}$$

$$q \text{ balok} + \text{tembok} = \underline{1008} \text{ kg/m}$$

$$q_{\text{ekiv.}} = 1808,64 \text{ kg/m} \approx 1810 \text{ kg/m}$$

Balok lantai

$$M = 0,0417 * 420 * 4^3 = 1120,896 \text{ kg.m}$$

$$q = 560,448 * 2 = 1120,896$$

$$q \text{ balok} = \underline{1779,2}$$

$$q_{\text{ekiv.}} = \underline{2900,1 \text{ kg/m}}$$

Beban hidup

Balok atap

$$M = 0,0417 * 140 * 4^3 = 373,632 \text{ kg.m}$$

$$q_{\text{ekiv.}} = 186,816 * 2 = 373,632 \approx 380 \text{ kg/m}$$

Balok lantai

$$M = 0,0417 * 250 * 4^3 = 667,2 \text{ kgm}$$

$$q_{\text{ekiv.}} = 333,6 * 2 = 667,2 \approx 670 \text{ kg/m}$$

Distribusi Gaya Horizontal Akibat Pengurangan Infill Wall

hi	wl (l-1)	wl (l-2)	wl (l-3)	wl (l-k)
----	----------	----------	----------	----------

wi	19.2	104686	104686	104686	91966
	14.4	1235986	1235986	1215316	1215316
	9.6	1235986	1215316	1215316	1215316
	4.8	1215316	1215316	1215316	1215316

Wi.hi	19.2	2014158.6	2014158.6	2014158.6	1769425.8
	14.4	17798198	17798198	17500550	17500550
	9.6	11865466	11667034	11667034	11667034
	4.8	5833516.8	5833516.8	5833516.8	5833516.8
Jumlah		37511339	37312907	37015259	36770527

V (Kg)		400032	397862	395691	393521
--------	--	--------	--------	--------	--------

Fi(Kg)	19.2	21479.582	21476.675	21531.24	18936.531
	14.4	189805.24	189779.55	187079.88	187292.23
	9.6	126536.83	124403.85	124719.92	124861.49
	4.8	62210.346	62201.925	62359.96	62430.745

***Beban titik**

Atap

$$q = q \text{ pelat(mati +hidup) + berat balok anak} \\ = (800,64 + 380) + 288 = 1468,64 \text{ kg/m}$$

$$P = q * L = 1468,64 * 4 = 5880 \text{ kg}$$

Lantai

$$q = (1120,896 + 670) + 259,2 = 2050,096 \text{ kg/m}$$

$$P = q * L = 2050,096 * 4 = 8200 \text{ kg}$$

***Pembebanan Gempa**

Berat atap

Beban mati

Pelat	: 300 * 28 * 8	= 67200 kg
Balok bujur	: 0,6 * 0,8 * 28 * 2400	= 32256 kg
Balok anak	: 0,6 * 0,3 * 8 * 4 * 2400	= 13824 kg
	: 0,6 * 0,3 * 2 * 12 * 2400	= 10368 kg
Balok lintang	: 0,6 * 0,8 * 4 * 8 * 2400	= 36864 kg
Kolom	: 0,9 * 0,9 * 4 * 2 * 2400	= 15552 kg
Tembok	: 28 * 2 * 250	= 14000 kg
	: 8 * 4 * 250 * 2	= 16000 kg
		<hr/>
		206064 kg

Beban hidup

$$q = 140 \text{ kg/m}^2$$

$$\text{Faktor reduksi} = 0,3$$

$$W \text{ atap} = 28 * 8 * 140 * 0,3 = 9408 \text{ kg}$$

$$W \text{ total atap} = 206064 + 9408 = 215472 \text{ kg}$$

Berat lantai

Beban mati

Pelat	: 670 * 28 * 8	= 150080 kg
Balok bujur	: 0,6 * 0,8 * 1 * 28 * 2400	= 32256 kg
Balok anak	: 0,3 * 0,6 * 8 * 4 * 2400	= 13824 kg
	: 0,3 * 0,6 * 2 * 12 * 2400	= 10368 kg
Balok melintang	: 0,6 * 0,8 * 4 * 8 * 2400	= 36864 kg
Kolom	: 0,9 * 0,9 * 4 * 4 * 2400	= 31104 kg
Tembok	: 28 * 3,2 * 250	= 22400 kg
	: 8 * 4 * 3,2 * 250	= 25600 kg
		<hr/>
		322496 kg

Beban hidup

$$q = 250 \text{ kg/m}^2$$

$$\text{Faktor reduksi} = 0,3$$

$$W \text{ lantai} = 28 * 8 * 250 * 0,3 = 16800 \text{ kg}$$

$$W \text{ total lantai} = 322496 + 16800 = 339296 \text{ kg}$$

$$W \text{ total lantai + atap} = 12(339296) + 215472 = 4287024 \text{ kg}$$

***Waktu getar bangunan(T)**

$$T = 0,06 H^{3/4}$$

$$H = 4 * 13 = 52 \text{ m}$$

$$T = 0,06 (52)^{3/4} = 1,162 \text{ detik}$$

***Koefisien gempa dasar (C)**

$$C = 0,05 \text{ (struktur diatas tanah lunak)}$$

***Faktor keutamaan (I) dan Faktor jenis struktur (K)**

$I = 1,5$ $K = 1,0$ untuk bangunan gedung perkantoran yang menggunakan struktur rangka beton bertulang dengan daktilitas penuh

***Gaya horisontal total akibat gempa**

$$V = CIK * W_i$$

$$= 0,05 * 1,5 * 1,0 * 4287024$$

$$= 321526,8$$

***Distribusi Gaya geser horisontal total akibat gempa kesepanjang tinggi gedung**

$$H/A = 52/28$$

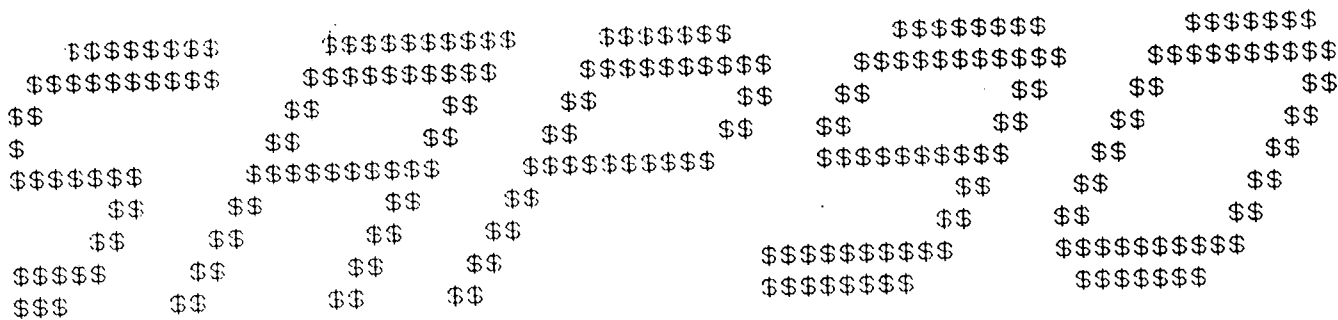
$$= 1,8571 < 3$$

hi (m)	wi (kg)	wi hi (kgm)	Fixi y (kg)
52	206064	10725728	29548
48	339296	16286208	44911
44	339296	14929024	41168
40	339296	13571840	37425
36	339296	12214656	33683
32	339296	10867472	29940
28	339296	9500288	26198
24	339296	8143104	22455
20	339296	6785920	18712
16	339296	5428736	14970
12	339296	4071552	11227
8	339296	2714368	7485
4	339296	1357184	3742
		116596080	

$$q_{\text{ekiv.}} \text{ (akibat pengurangan)} = 3570 - 800 = 2770 \text{ kg/m}$$

Distribusi Gaya Horizontal Akibat Pengurangan Infill Wall

hi	wi (B-1)	wi (B-2)	wi (B-3)	wi (B-4)	wi (B-5)	wi (B-6)	wi (B-7)	wi (B-8)	wi (B-9)	wi (B-9)	wi (B-10)	wi (B-11)	wi (B-12)	wi (B-K)
62	206064	206064	206064	206064	206064	206064	206064	206064	206064	206064	206064	206064	206064	192264
48	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	316896	316896
44	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	316896	316896	316896
40	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	316896	316896	316896	316896
36	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	316896	316896	316896	316896
32	339296	339296	339296	339296	339296	339296	339296	339296	339296	339296	316896	316896	316896	316896
28	339296	339296	339296	339296	339296	339296	316896	316896	316896	316896	316896	316896	316896	316896
24	339296	339296	339296	339296	339296	316896	316896	316896	316896	316896	316896	316896	316896	316896
20	339296	339296	339296	339296	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896
16	339296	339296	339296	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896
12	339296	339296	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896
8	339296	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896
4	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896	316896
62	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	10715328	8997728
48	16286208	16286208	16286208	16286208	16286208	16286208	16286208	16286208	16286208	16286208	16286208	16286208	15211008	15211008
44	14929024	14929024	14929024	14929024	14929024	14929024	14929024	14929024	14929024	14929024	14929024	13943424	13943424	13943424
40	13571840	13571840	13571840	13571840	13571840	13571840	13571840	13571840	13571840	13571840	12675840	12675840	12675840	12675840
36	12214656	12214656	12214656	12214656	12214656	12214656	12214656	12214656	12214656	12214656	11408256	11408256	11408256	11408256
32	10857472	10857472	10857472	10857472	10857472	10857472	10857472	10857472	10857472	10857472	10140672	10140672	10140672	10140672
28	9500288	9500288	9500288	9500288	9500288	9500288	8873088	8873088	8873088	8873088	8873088	8873088	8873088	8873088
24	8143104	8143104	8143104	8143104	8143104	8143104	7605504	7605504	7605504	7605504	7605504	7605504	7605504	7605504
20	6785920	6785920	6785920	6785920	6785920	6785920	6337920	6337920	6337920	6337920	6337920	6337920	6337920	6337920
16	5428736	5428736	5428736	5428736	5428736	5428736	5070336	5070336	5070336	5070336	5070336	5070336	5070336	5070336
12	4071552	4071552	4071552	4071552	4071552	4071552	3802752	3802752	3802752	3802752	3802752	3802752	3802752	3802752
8	2714368	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168	2535168
4	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584	1267584
	116486080	116306880	116038080	115679680	115231680	114694080	114066880	113350080	112543680	111647680	110662080	109586880	108689280	
	319846.8	318166	316486.8	314806	313126.8	311446.8	309766.8	308086.8	306406.8	304726.8	303046.8	301366.8	299686.8	
52	29422.085	29312.566	29225.405	29160.26	29117.482	29097.008	29099.182	29124.383	29173.112	29245.996	29343.799	29467.434	27520.96	
48	44718.576	44552.116	44419.641	44320.627	44255.609	44224.491	44227.795	44266.098	44340.161	44450.938	44595.588	44830.672	41871.668	
44	40992.027	40839.44	40718.004	40627.242	40567.642	40539.117	40542.145	40577.256	40645.147	40746.693	40883.902	41133.782	38382.362	
40	37265.479	37126.764	37016.367	36933.856	36879.674	36853.743	36856.495	36888.415	36950.134	37058.841	37212.538	37428.893	34893.056	
36	33538.931	33414.087	33314.731	33240.471	33191.707	33168.369	33170.846	33199.573	33259.649	33337.247	33424.374	33520.004	31403.751	
32	29872.383	29701.411	29613.094	29547.085	29503.739	29482.994	29485.196	29524.461	29587.577	29662.653	29749.711	29847.751	27914.445	
28	26085.835	25988.735	25911.457	25853.698	25815.772	25797.62	25806.285	25824.117	25857.505	25902.847	25950.145	26009.398	24425.139	
24	22359.288	22276.058	22208.92	22160.314	22127.806	22105.418	22103.32	22122.457	22162.632	22209.653	22263.519	22323.247	20935.834	
20	18632.74	18563.382	18508.184	18466.928	18436.712	18413.524	18411.362	18430.136	18468.861	18517.538	18576.174	18644.771	17446.528	
16	14906.192	14850.705	14806.547	14773.207	14749.278	14733.278	14733.207	14756.278	14794.353	14843.433	14898.519	14964.614	13957.223	
12	11179.644	11138.029	11101.775	11073.655	11053.474	11043.208	11043.208	11053.923	11065.216	11078.082	11091.431	11105.368	10467.917	
8	7453.0958	7435.138	7418.1036	7403.1036	7389.1036	7376.1036	7376.1036	7383.1036	7391.1036	7400.1036	7410.1036	7421.1036	6978.6113	
4	3480.5248	3467.569	3457.582	3449.5518	3444.4913	3442.0694	3442.0694	3445.3077	3451.0721	3458.6941	3471.2638	3485.8893	3489.3056	



STRUCTURAL ANALYSIS PROGRAMS

VERSION 5.20

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a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE ENDI	DIST	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1135851.88	.0		931.94	-15689.92			
	1.2		931.94	-14571.59			
	2.4		931.94	-13453.26			
	3.6		931.94	-12334.93			
	4.8		931.94	-11216.60			

1 90328.04	.0		-3930.65	9700.73			
	1.2		-3930.65	4983.95			
	2.4		-3930.65	267.17			
	3.6		-3930.65	-4449.62			
	4.8		-3930.65	-9166.40			

1 30422.92	.0		-3892.51	14904.18			
	1.2		-3892.51	10233.17			
	2.4		-3892.51	5562.16			
	3.6		-3892.51	891.15			
	4.8		-3892.51	-3779.86			

1 -5795.70	.0		-10110.76	30149.55			
	1.2		-10110.76	18016.64			
	2.4		-10110.76	5883.73			
	3.6		-10110.76	-6249.18			
	4.8		-10110.76	-18382.09			

1*****	.0		21014.28	-74632.20			
	1.2		21014.28	-49415.07			
	2.4		21014.28	-24197.94			
	3.6		21014.28	1019.19			
	4.8		21014.28	26236.32			

5 1-78248.41	.0		7969.68	-11507.85			
	1.2		7969.68	-1944.23			
	2.4		7969.68	7619.39			
	3.6		7969.68	17183.01			
	4.8		7969.68	26746.62			

3 1-35993.24	.0		3001.91	2601.64			
	1.2		3001.91	6203.93			
	2.4		3001.91	9806.22			
	3.6		3001.91	13408.50			
	4.8		3001.91	17010.79			

1 1-14578.94	.0		-2249.89	9575.36			

a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.2	-2249.89	6875.48			
		2.4	-2249.89	4175.61			
		3.6	-2249.89	1475.74			
		4.8	-2249.89	-1224.13			

1*****		.0	21531.18	-60060.99			
		1.2	21531.18	-34223.58			
		2.4	21531.18	-8386.17			
		3.6	21531.18	17451.24			
		4.8	21531.18	43288.65			

1*****		.0	9194.45	-15458.94			
		1.2	9194.45	-4425.59			
		2.4	9194.45	6607.75			
		3.6	9194.45	17641.09			
		4.8	9194.45	28674.43			

1-42012.47		.0	9590.69	-19610.17			
		1.2	9590.69	-8101.35			
		2.4	9590.69	3407.48			
		3.6	9590.69	14916.31			
		4.8	9590.69	26425.13			

1-10243.31		.0	5965.55	-14945.98			
		1.2	5965.55	-7787.32			
		2.4	5965.55	-628.67			
		3.6	5965.55	6529.99			
		4.8	5965.55	13688.64			

1 86502.16		.0	20215.23	-21207.60			
		2.7	8168.33	16400.61			
		5.3	-3878.57	22084.53			
		7.9	-15925.47	-4155.84			
		10.6	-27972.37	-62320.49			

1 51591.11		.0	21009.77	-24462.42			
		2.7	8962.87	15251.34			
		5.3	-3084.03	23040.81			
		7.9	-15130.93	-1094.01			
		10.6	-27177.83	-57153.10			

1-53958.58		.0	22766.95	-34133.91			
		2.7	10720.05	10236.37			
		5.3	-1326.85	22682.36			
		7.9	-13373.75	3204.07			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE	AXIAL
		SHEAR	MOMENT	SHEAR	TORQ
	10.6	-25420.65	-48198.51		

1134648.54	.0	20950.77	-25166.21		
	2.7	8903.87	14391.20		
	5.3	-3143.03	22024.32		
	7.9	-15189.93	-2266.85		
	10.6	-27236.83	-58482.30		

1 73368.99	.0	22699.99	-33462.41		
	2.7	10653.09	10730.44		
	5.3	-1393.81	22999.00		
	7.9	-13440.71	3343.27		
	10.6	-25487.61	-48236.74		

1 13223.76	.0	24041.63	-40870.83		
	2.7	11994.73	6877.35		
	5.3	-52.17	22701.26		
	7.9	-12099.07	6600.87		
	10.6	-24145.97	-41423.80		

1-20564.55	.0	10802.56	-18408.71		
	2.7	5396.56	3055.12		
	5.3	-9.44	10193.05		
	7.9	-5415.44	3005.08		
	10.6	-10821.44	-18508.79		

1 -5965.55	.0	11380.69	-19716.81		
	2.7	5974.69	3279.08		
	5.3	568.69	11949.07		
	7.9	-4837.31	6293.16		
	10.6	-10243.31	-13688.64		

a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ

1	97301.62					
	.0	88712.58	-261095.01			
	1.2	88712.58	-154639.92			
	2.4	88712.58	-48184.82			
	3.6	88712.58	58270.28			
	4.8	88712.58	164725.38			

	1105713.29					
	.0	-34257.06	123409.97			
	1.2	-34257.06	82301.50			
	2.4	-34257.06	41193.03			
	3.6	-34257.06	84.55			
	4.8	-34257.06	-41023.92			

1	27795.26					
	.0	1832.57	-6348.19			
	1.2	1832.57	-4149.10			
	2.4	1832.57	-1950.02			
	3.6	1832.57	249.07			
	4.8	1832.57	2448.15			

1	-5839.12					
	.0	-11250.77	34341.70			
	1.2	-11250.77	20840.77			
	2.4	-11250.77	7339.85			
	3.6	-11250.77	-6161.08			
	4.8	-11250.77	-19662.01			

	1*****					
	.0	206086.73	-596737.03			
	1.2	206086.73	-349432.96			
	2.4	206086.73	-102128.88			
	3.6	206086.73	145175.20			
	4.8	206086.73	392479.28			

1	-46470.48					
	.0	-54460.36	228642.28			
	1.2	-54460.36	163289.85			
	2.4	-54460.36	97937.41			
	3.6	-54460.36	32584.98			
	4.8	-54460.36	-32767.46			

3	1-41031.65					
	.0	13419.98	-37024.83			
	1.2	13419.98	-20920.85			
	2.4	13419.98	-4816.87			
	3.6	13419.98	11287.11			
	4.8	13419.98	27391.10			

1	1-13106.74					
	.0	-4061.57	16475.82			

3a Statis Portal (4 lantai) Kg/m

M E M B E R E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	1.2	-4061.57	11601.94			
	2.4	-4061.57	6728.05			
	3.6	-4061.57	1854.17			
	4.8	-4061.57	-3019.71			

1*****						
	.0	105230.69	-295680.71			
	1.2	105230.69	-169403.88			
	2.4	105230.69	-43127.06			
	3.6	105230.69	83149.77			
	4.8	105230.69	209426.59			

1-94076.98						
	.0	-16979.00	87015.60			
	1.2	-16979.00	66640.79			
	2.4	-16979.00	46265.99			
	3.6	-16979.00	25891.18			
	4.8	-16979.00	5516.37			

1-43520.46						
	.0	13078.69	-33471.27			
	1.2	13078.69	-17776.84			
	2.4	13078.69	-2082.42			
	3.6	13078.69	13612.00			
	4.8	13078.69	29306.43			

1-10168.54						
	.0	5589.58	-13428.18			
	1.2	5589.58	-6720.69			
	2.4	5589.58	-13.20			
	3.6	5589.58	6694.30			
	4.8	5589.58	13401.79			

3 1 60759.64						
	.0	8411.67	41315.41			
	2.7	-3635.23	47644.19			
	5.3	-15682.13	22048.69			
	7.9	-27729.03	-35471.09			
	10.6	-39775.93	-124915.16			

5 1 59906.74						
	.0	23025.04	-35342.82			
	2.7	10978.14	9711.39			
	5.3	-1068.76	22841.32			
	7.9	-13115.66	4046.97			
	10.6	-25162.56	-46671.67			

7 1-53042.56						
	.0	22393.93	-32060.07			
	2.7	10347.03	11321.71			
	5.3	-1699.87	22779.20			
	7.9	-13746.77	2312.41			

a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
		10.6	-25793.67	-50078.67			

1	98774.37	.0	9086.45	37747.71			
		2.7	-2960.45	45864.65			
		5.3	-15007.35	22057.31			
		7.9	-27054.25	-33674.32			
		10.6	-39101.15	-121330.23			

1	89331.35	.0	24469.12	-43028.16			
		2.7	12422.22	5852.86			
		5.3	375.32	22809.60			
		7.9	-11671.58	7842.05			
		10.6	-23718.48	-39049.79			

1	12234.63	.0	23762.36	-39260.54			
		2.7	11715.46	7747.57			
		5.3	-331.44	22831.40			
		7.9	-12378.34	5990.94			
		10.6	-24425.24	-42773.80			

1	-21251.75	.0	11046.04	-19695.53			
		2.7	5640.04	2413.52			
		5.3	234.04	10196.68			
		7.9	-5171.96	3653.93			
		10.6	-10577.96	-17214.71			

1	-5589.58	.0	11455.46	-20222.45			
		2.7	6049.46	2971.57			
		5.3	643.46	11839.68			
		7.9	-4762.54	6381.90			
		10.6	-10168.54	-13401.79			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
1	80761.55					
	.0	94914.21	-358771.65			
	1.2	94914.21	-244874.60			
	2.4	94914.21	-130977.55			
	3.6	94914.21	-17080.50			
	4.8	94914.21	96816.55			
1	55503.93					
	.0	71903.58	-103679.61			
	1.2	71903.58	-17395.32			
	2.4	71903.58	68888.97			
	3.6	71903.58	155173.26			
	4.8	71903.58	241457.55			
1	59356.51					
	.0	-47019.09	175935.42			
	1.2	-47019.09	119512.51			
	2.4	-47019.09	63089.60			
	3.6	-47019.09	6666.69			
	4.8	-47019.09	-49756.22			
1	-7498.49					
	.0	-1434.59	-1130.30			
	1.2	-1434.59	-2851.82			
	2.4	-1434.59	-4573.33			
	3.6	-1434.59	-6294.85			
	4.8	-1434.59	-8016.36			
1	*****					
	.0	204625.30	-778899.17			
	1.2	204625.30	-533348.81			
	2.4	204625.30	-287798.45			
	3.6	204625.30	-42248.09			
	4.8	204625.30	203302.26			
1	*****					
	.0	172209.38	-258405.17			
	1.2	172209.38	-51753.91			
	2.4	172209.38	154897.35			
	3.6	172209.38	361548.60			
	4.8	172209.38	568199.86			
1	3409.90					
	.0	-88421.87	355951.34			
	1.2	-88421.87	249845.10			
	2.4	-88421.87	143738.86			
	3.6	-88421.87	37632.62			
	4.8	-88421.87	-68473.62			
1	-25268.27					
	.0	12934.88	-47238.59			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	1.2	12934.88	-31716.73			
	2.4	12934.88	-16194.88			
	3.6	12934.88	-673.02			
	4.8	12934.88	14848.83			

1	*****					
	.0	98318.49	-361494.75			
	1.2	98318.49	-243512.55			
	2.4	98318.49	-125530.36			
	3.6	98318.49	-7548.17			
	4.8	98318.49	110434.02			

1	*****					
	.0	91544.04	-150236.35			
	1.2	91544.04	-40383.50			
	2.4	91544.04	69469.36			
	3.6	91544.04	179322.21			
	4.8	91544.04	289175.06			

1	-30455.80					
	.0	-32173.61	142972.59			
	1.2	-32173.61	104364.25			
	2.4	-32173.61	65755.92			
	3.6	-32173.61	27147.58			
	4.8	-32173.61	-11460.75			

2	1-11455.68					
	.0	12026.74	-38234.24			
	1.2	12026.74	-23802.15			
	2.4	12026.74	-9370.06			
	3.6	12026.74	5062.03			
	4.8	12026.74	19494.12			

3	1-39190.37					
	.0	-25257.61	200496.16			
	2.7	-34588.26	121200.38			
	5.3	-43918.91	17178.37			
	7.9	-53249.56	-111569.87			
	10.6	-62580.21	-265044.32			

4	1 -6774.45					
	.0	-24483.37	196663.11			
	2.7	-33814.02	119419.07			
	5.3	-43144.67	17448.82			
	7.9	-52475.32	-109247.67			
	10.6	-61805.97	-260670.37			

5	1 -5480.33					
	.0	3852.58	65522.13			
	2.7	-8194.32	59769.33			
	5.3	-20241.22	22092.24			
	7.9	-32288.12	-47509.13			

a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
	10.6	-44335.02	-149034.78			

	1-31669.22					
	.0	25577.61	-49258.81			
	2.7	13530.71	2559.71			
	5.3	1483.81	22453.94			
	7.9	-10563.09	10423.88			
	10.6	-22609.99	-33530.45			

	1 51457.64					
	.0	4597.93	61788.66			
	2.7	-7448.97	58011.03			
	5.3	-19495.87	22309.11			
	7.9	-31542.77	-45317.10			
	10.6	-43589.67	-144867.59			

	1 38883.66					
	.0	26749.65	-55165.14			
	2.7	14702.75	-240.72			
	5.3	2655.85	22759.41			
	7.9	-9391.05	13835.26			
	10.6	-21437.95	-27013.18			

	1-19644.93					
	.0	8961.88	-7989.91			
	2.7	3555.88	8596.12			
	5.3	-1850.12	10856.26			
	7.9	-7256.12	-1209.51			
	10.6	-12662.12	-27601.17			

	1-12026.74					
	.0	10168.32	-12671.13			
	2.7	4762.32	7111.97			
	5.3	-643.68	12569.17			
	7.9	-6049.68	3700.48			
	10.6	-11455.68	-19494.12			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE ENDI	DIST	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1184738.99							
	.0		114069.09	-466968.85			
	1.2		114069.09	-330085.94			
	2.4		114069.09	-193203.03			
	3.6		114069.09	-56320.13			
	4.8		114069.09	80562.78			

1138344.13							
	.0		94078.68	-231826.84			
	1.2		94078.68	-118932.43			
	2.4		94078.68	-6038.01			
	3.6		94078.68	106856.40			
	4.8		94078.68	219750.81			

1 89861.48							
	.0		91696.63	-103176.98			
	1.2		91696.63	6858.97			
	2.4		91696.63	116894.93			
	3.6		91696.63	226930.88			
	4.8		91696.63	336966.83			

1 87320.73							
	.0		-62830.88	237304.08			
	1.2		-62830.88	161907.03			
	2.4		-62830.88	86509.97			
	3.6		-62830.88	11112.92			
	4.8		-62830.88	-64284.13			

1*****							
	.0		244303.16	-1014260.02			
	1.2		244303.16	-721096.22			
	2.4		244303.16	-427932.43			
	3.6		244303.16	-134768.63			
	4.8		244303.16	158395.17			

1*****							
	.0		212148.97	-526530.60			
	1.2		212148.97	-271951.84			
	2.4		212148.97	-17373.08			
	3.6		212148.97	237205.68			
	4.8		212148.97	491784.44			

1-82783.65							
	.0		203442.74	-219279.39			
	1.2		203442.74	24851.90			
	2.4		203442.74	268983.19			
	3.6		203442.74	513114.48			
	4.8		203442.74	757245.77			

1 73621.77							
	.0		-120636.48	478531.11			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.2	-120636.48	333767.34			
		2.4	-120636.48	189003.57			
		3.6	-120636.48	44239.80			
		4.8	-120636.48	-100523.97			

1*****							
		.0	118963.75	-471638.59			
		1.2	118963.75	-328882.09			
		2.4	118963.75	-186125.60			
		3.6	118963.75	-43369.10			
		4.8	118963.75	99387.40			

1*****							
		.0	108749.36	-271079.21			
		1.2	108749.36	-140579.98			
		2.4	108749.36	-10080.75			
		3.6	108749.36	120418.48			
		4.8	108749.36	250917.70			

1*****							
		.0	107358.63	-135502.63			
		1.2	107358.63	-6672.27			
		2.4	107358.63	122158.08			
		3.6	107358.63	250988.44			
		4.8	107358.63	379818.79			

1	-546.32						
		.0	-49881.69	201658.35			
		1.2	-49881.69	141800.32			
		2.4	-49881.69	81942.29			
		3.6	-49881.69	22084.26			
		4.8	-49881.69	-37773.77			

1-42368.59							
		.0	-46394.85	312389.62			
		2.7	-55725.50	177080.14			
		5.3	-65056.15	17044.44			
		7.9	-74386.80	-167717.48			
		10.6	-83717.45	-377205.62			

1-10096.95							
		.0	-48482.65	322927.80			
		2.7	-57813.30	182085.67			
		5.3	-67143.95	16517.32			
		7.9	-76474.60	-173777.26			
		10.6	-85805.25	-388798.06			

1-10214.39							
		.0	-45318.58	307720.15			
		2.7	-54649.23	175262.79			
		5.3	-63979.88	18079.22			
		7.9	-73310.53	-163830.58			

Statis Portal (4 lantai) Kg/m

M E M B E R E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		10.6	-82641.18	-370466.60		

1	-1390.73					
		.0	-48195.88	322265.77		
		2.7	-57526.53	182183.58		
		5.3	-66857.18	17375.17		
		7.9	-76187.83	-172159.47		
		10.6	-85518.48	-386420.33		

1	-32651.49					
		.0	-2540.76	99662.76		
		2.7	-14587.66	76967.61		
		5.3	-26634.56	22348.18		
		7.9	-38681.46	-64195.53		
		10.6	-50728.36	-182663.53		

1	55421.73					
		.0	-1437.72	94213.06		
		2.7	-13484.62	74440.95		
		5.3	-25531.52	22744.56		
		7.9	-37578.42	-60876.12		
		10.6	-49625.32	-176421.08		

1	-42143.88					
		.0	19794.06	-65034.01		
		2.7	14388.06	-19742.70		
		5.3	8982.06	11222.71		
		7.9	3576.06	27862.22		
		10.6	-1829.94	30175.83		

1	49881.69					
		.0	21077.68	-71042.49		
		2.7	15671.68	-22349.58		
		5.3	10265.68	12017.44		
		7.9	4859.68	32058.55		
		10.6	-546.32	37773.77		

a Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	75131.81					
	.0	92713.25	-376251.70			
	1.2	92713.25	-264995.79			
	2.4	92713.25	-153739.89			
	3.6	92713.25	-42483.99			
	4.8	92713.25	68771.91			

1	42277.42					
	.0	74557.07	-172320.39			
	1.2	74557.07	-82851.91			
	2.4	74557.07	6616.57			
	3.6	74557.07	96085.05			
	4.8	74557.07	185553.54			

1	11687.36					
	.0	46574.10	-43325.72			
	1.2	46574.10	12563.20			
	2.4	46574.10	68452.12			
	3.6	46574.10	124341.03			
	4.8	46574.10	180229.95			

1	4128.28					
	.0	-3282.45	73981.42			
	1.2	-3282.45	70042.48			
	2.4	-3282.45	66103.54			
	3.6	-3282.45	62164.60			
	4.8	-3282.45	58225.66			

1	*****					
	.0	201948.41	-820929.69			
	1.2	201948.41	-578591.60			
	2.4	201948.41	-336253.50			
	3.6	201948.41	-93915.40			
	4.8	201948.41	148422.70			

1	-97767.70					
	.0	168481.76	-391117.03			
	1.2	168481.76	-188938.92			
	2.4	168481.76	13239.19			
	3.6	168481.76	215417.30			
	4.8	168481.76	417595.41			

1	-59361.73					
	.0	103424.38	-99304.51			
	1.2	103424.38	24804.75			
	2.4	103424.38	148914.01			
	3.6	103424.38	273023.26			
	4.8	103424.38	397132.52			

1	-21579.36					
	.0	8866.87	120973.86			

la Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.2	8866.87	131614.11			
		2.4	8866.87	142254.36			
		3.6	8866.87	152894.61			
		4.8	8866.87	163534.86			

	1*****	.0	98857.33	-382723.77			
		1.2	98857.33	-264094.97			
		2.4	98857.33	-145466.17			
		3.6	98857.33	-26837.37			
		4.8	98857.33	91791.43			

	1*****	.0	88050.17	-205726.38			
		1.2	88050.17	-100066.17			
		2.4	88050.17	5594.04			
		3.6	88050.17	111254.25			
		4.8	88050.17	216914.45			

	1-70218.83	.0	56229.52	-69951.25			
		1.2	56229.52	-2475.83			
		2.4	56229.52	64999.60			
		3.6	56229.52	132475.02			
		4.8	56229.52	199950.44			

	1-25796.91	.0	13351.58	31358.93			
		1.2	13351.58	47380.82			
		2.4	13351.58	63402.71			
		3.6	13351.58	79424.60			
		4.8	13351.58	95446.50			

	1-44273.82	.0	-32854.39	241092.30			
		2.7	-42185.04	141665.05			
		5.3	-51515.69	17511.58			
		7.9	-60846.34	-131368.11			
		10.6	-70176.99	-304974.02			

	1-96878.03	.0	-30590.05	228879.26			
		2.7	-39920.70	135452.51			
		5.3	-49251.35	17299.55			
		7.9	-58582.00	-125579.64			
		10.6	-67912.65	-293185.06			

	1*****	.0	-7559.09	106248.53			
		2.7	-16889.74	73853.84			
		5.3	-26220.39	16732.92			
		7.9	-35551.04	-65114.23			

sa Statis Portal (4 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		10.6	-44881.69	-171687.59			

1-10807.16		.0	-31535.26	234565.70			
		2.7	-40865.91	138634.16			
		5.3	-50196.56	17976.39			
		7.9	-59527.21	-127407.60			
		10.6	-68857.86	-297517.81			

1-31820.65		.0	-29506.68	223714.87			
		2.7	-38837.33	133159.06			
		5.3	-48167.98	17877.03			
		7.9	-57498.63	-122131.23			
		10.6	-66829.28	-286865.70			

1-42877.94		.0	-7099.32	104471.07			
		2.7	-16429.97	73294.76			
		5.3	-25760.62	17392.23			
		7.9	-35091.27	-63236.53			
		10.6	-44421.92	-168591.51			

1-22218.45		.0	-4128.28	58225.66			
		2.7	-9534.28	40122.78			
		5.3	-14940.28	7694.00			
		7.9	-20346.28	-39060.68			
		10.6	-25752.28	-100141.26			

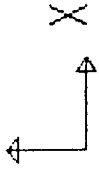
1-13351.58		.0	-4172.91	63393.60			
		2.7	-9578.91	45172.42			
		5.3	-14984.91	12625.35			
		7.9	-20390.91	-34247.62			
		10.6	-25796.91	-95446.50			

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STRUCTURAL ANALYSIS PROGRAMS

VERSION 5.20

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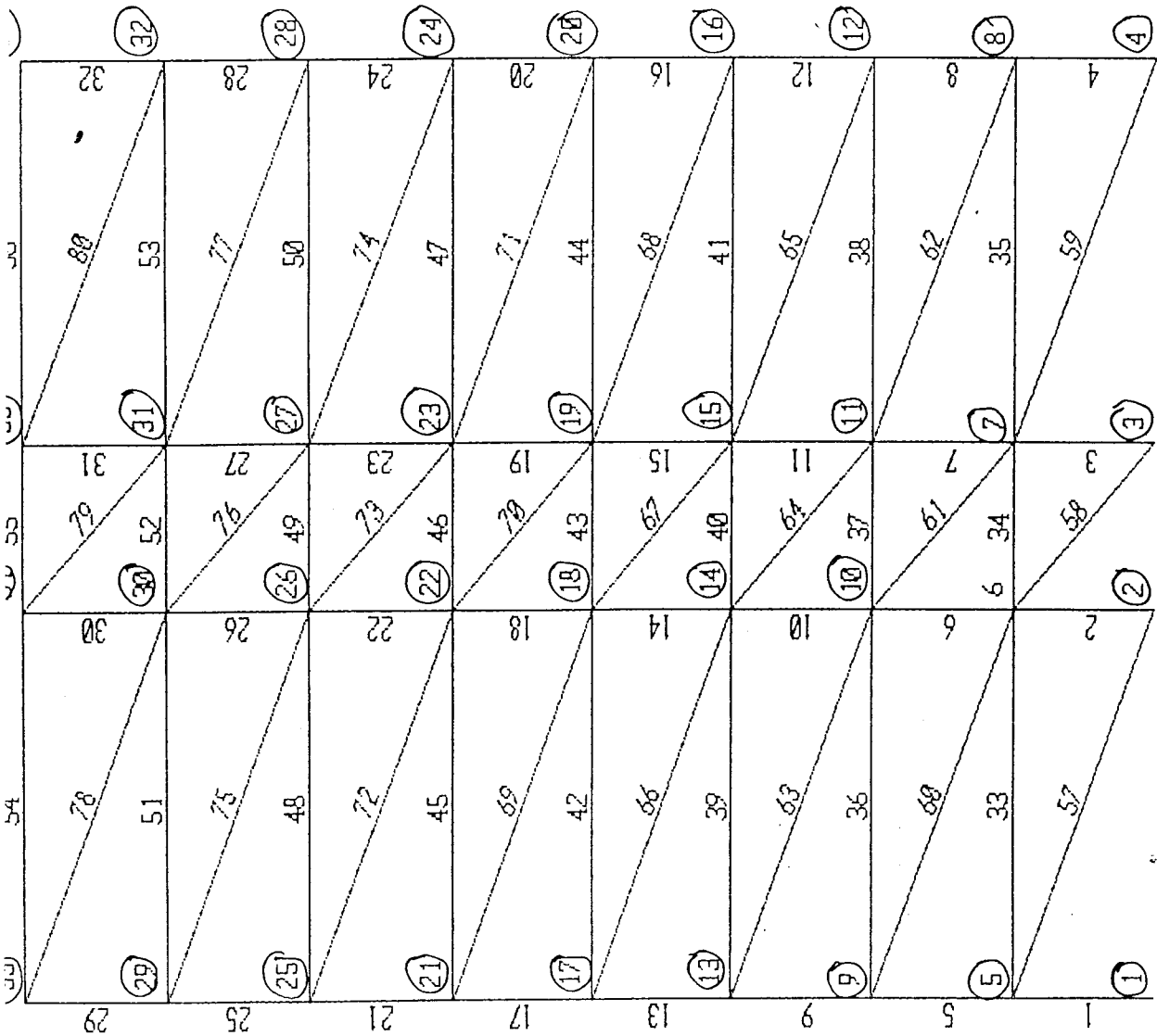


AS-2

UNDEFORMED
SHAPE

OPTIONS
JOINT IDS
ELEMENT IDS
WIRE FRAME

SAP90



a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
1	-6568.34	.0	-405.19	-5427.48		
		.8	-405.19	-5731.37		
		1.5	-405.19	-6035.26		
		2.3	-405.19	-6339.15		
		3.0	-405.19	-6643.04		
1	-4092.36	.0	-2345.70	2212.45		
		.8	-2345.70	453.18		
		1.5	-2345.70	-1306.10		
		2.3	-2345.70	-3065.37		
		3.0	-2345.70	-4824.64		
1	-6086.59	.0	-2342.34	3248.48		
		.8	-2342.34	1491.72		
		1.5	-2342.34	-265.03		
		2.3	-2342.34	-2021.79		
		3.0	-2342.34	-3778.55		
1	-8505.37	.0	-2656.73	4375.04		
		.8	-2656.73	2382.49		
		1.5	-2656.73	389.94		
		2.3	-2656.73	-1602.60		
		3.0	-2656.73	-3595.15		
1	-10034.84	.0	-2923.04	5097.90		
		.8	-2923.04	2905.61		
		1.5	-2923.04	713.33		
		2.3	-2923.04	-1478.95		
		3.0	-2923.04	-3671.23		
1	-10032.56	.0	-3210.11	5784.29		
		.8	-3210.11	3376.71		
		1.5	-3210.11	969.13		
		2.3	-3210.11	-1438.45		
		3.0	-3210.11	-3846.03		
1	-8005.07	.0	-3377.55	6548.92		
		.8	-3377.55	4015.76		
		1.5	-3377.55	1482.60		
		2.3	-3377.55	-1050.56		
		3.0	-3377.55	-3583.72		
1	-3280.91	.0	-5019.61	8045.82		

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		.8	-5019.61	4281.11			
		1.5	-5019.61	516.41			
		2.3	-5019.61	-3248.30			
		3.0	-5019.61	-7013.00			

1-52809.32		.0	7129.79	-14690.53			
		.8	7129.79	-9343.13			
		1.5	7129.79	-3995.84			
		2.3	7129.79	1351.51			
		3.0	7129.79	6698.85			

1-50563.96		.0	6487.81	-10650.40			
		.8	6487.81	-5784.54			
		1.5	6487.81	-918.69			
		2.3	6487.81	3947.17			
		3.0	6487.81	8813.03			

1-50101.75		.0	6125.19	-9121.35			
		.8	6125.19	-4527.46			
		1.5	6125.19	66.43			
		2.3	6125.19	4660.33			
		3.0	6125.19	9254.22			

1-46501.64		.0	5762.13	-8168.55			
		.8	5762.13	-3846.95			
		1.5	5762.13	474.65			
		2.3	5762.13	4796.25			
		3.0	5762.13	9117.85			

3	1-40515.89		.0	5386.52	-7333.42		
			.8	5386.52	-3293.53		
			1.5	5386.52	746.36		
			2.3	5386.52	4786.26		
			3.0	5386.52	8826.15		

2	1-32343.69		.0	4933.04	-6395.06		
			.8	4933.04	-2695.28		
			1.5	4933.04	1004.50		
			2.3	4933.04	4704.28		
			3.0	4933.04	8404.06		

5	1-21976.03		.0	4326.10	-5325.87		
			.8	4326.10	-2081.30		
			1.5	4326.10	1163.27		
			2.3	4326.10	4407.84		

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 3.0	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
			4326.10	7652.41		

1	-9102.75					
		.0	3577.48	-4333.15		
		.8	3577.48	-1650.04		
		1.5	3577.48	1033.07		
		2.3	3577.48	3716.18		
		3.0	3577.48	6399.29		

1	*****					
		.0	3997.66	-13046.47		
		.8	3997.66	-10048.22		
		1.5	3997.66	-7049.98		
		2.3	3997.66	-4051.73		
		3.0	3997.66	-1053.49		

1	*****					
		.0	-1276.33	2004.54		
		.8	-1276.33	1047.29		
		1.5	-1276.33	90.04		
		2.3	-1276.33	-867.20		
		3.0	-1276.33	-1824.45		

1	-83976.31					
		.0	-1218.65	2018.11		
		.8	-1218.65	1104.12		
		1.5	-1218.65	190.13		
		2.3	-1218.65	-723.86		
		3.0	-1218.65	-1637.85		

5	1-62384.07					
		.0	-1561.26	2857.67		
		.8	-1561.26	1686.73		
		1.5	-1561.26	515.79		
		2.3	-1561.26	-655.15		
		3.0	-1561.26	-1826.09		

9	1-43610.61					
		.0	-1904.67	3622.78		
		.8	-1904.67	2194.27		
		1.5	-1904.67	765.77		
		2.3	-1904.67	-662.74		
		3.0	-1904.67	-2091.25		

3	1-27682.22					
		.0	-2374.42	4564.68		
		.8	-2374.42	2783.86		
		1.5	-2374.42	1003.05		
		2.3	-2374.42	-777.77		
		3.0	-2374.42	-2558.59		

17	1-14969.28					

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-2927.51	5542.72			
		.8	-2927.51	3347.09			
		1.5	-2927.51	1151.45			
		2.3	-2927.51	-1044.18			
		3.0	-2927.51	-3239.82			

1	-5525.92						
		.0	-4178.67	6488.53			
		.8	-4178.67	3354.52			
		1.5	-4178.67	220.52			
		2.3	-4178.67	-2913.49			
		3.0	-4178.67	-6047.49			

	1*****						
		.0	11964.71	-23961.24			
		.8	11964.71	-14987.71			
		1.5	11964.71	-6014.17			
		2.3	11964.71	2959.36			
		3.0	11964.71	11932.90			

3	1*****						
		.0	5200.79	-6502.98			
		.8	5200.79	-2602.39			
		1.5	5200.79	1298.21			
		2.3	5200.79	5198.80			
		3.0	5200.79	9099.40			

2	1*****						
		.0	5878.66	-8474.08			
		.8	5878.66	-4065.08			
		1.5	5878.66	343.91			
		2.3	5878.66	4752.91			
		3.0	5878.66	9161.90			

6	1-78958.20						
		.0	5776.13	-8068.03			
		.8	5776.13	-3735.94			
		1.5	5776.13	596.16			
		2.3	5776.13	4928.25			
		3.0	5776.13	9260.35			

0	1-57191.86						
		.0	5523.86	-7403.05			
		.8	5523.86	-3260.16			
		1.5	5523.86	882.73			
		2.3	5523.86	5025.63			
		3.0	5523.86	9168.52			

4	1-37807.39						
		.0	5163.19	-6711.23			
		.8	5163.19	-2838.83			
		1.5	5163.19	1033.56			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	5163.19	4905.96			
		3.0	5163.19	8778.35			

1	-21202.12						
		.0	4931.55	-6184.00			
		.8	4931.55	-2485.34			
		1.5	4931.55	1213.32			
		2.3	4931.55	4911.99			
		3.0	4931.55	8610.65			

1	-6936.03						
		.0	4405.48	-5315.96			
		.8	4405.48	-2011.85			
		1.5	4405.48	1292.26			
		2.3	4405.48	4596.36			
		3.0	4405.48	7900.47			

1	16766.10						
		.0	10143.78	-8917.76			
		1.8	4508.78	3903.22			
		3.5	-1126.22	6862.96			
		5.3	-6761.22	-38.56			
		7.0	-12396.22	-16801.32			

1	21727.57						
		.0	9944.13	-8164.13			
		1.8	4309.13	4307.47			
		3.5	-1325.87	6917.81			
		5.3	-6960.87	-333.09			
		7.0	-12595.87	-17445.24			

1	20083.61						
		.0	9981.49	-8247.97			
		1.8	4346.49	4289.01			
		3.5	-1288.51	6964.75			
		5.3	-6923.51	-220.76			
		7.0	-12558.51	-17267.52			

1	15291.48						
		.0	10136.68	-8779.13			
		1.8	4501.68	4029.43			
		3.5	-1133.32	6976.75			
		5.3	-6768.32	62.81			
		7.0	-12403.32	-16712.38			

1	9213.12						
		.0	10352.07	-9528.33			
		1.8	4717.07	3657.17			
		3.5	-917.93	6981.42			
		5.3	-6552.93	444.43			
		7.0	-12187.93	-15953.82			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
1	1961.22	.0	10617.14	-10451.05			
		1.8	4982.14	3198.31			
		3.5	-652.86	6986.43			
		5.3	-6287.86	913.30			
		7.0	-11922.86	-15021.08			
1	-5100.42	.0	10950.42	-11662.77			
		1.8	5315.42	2569.85			
		3.5	-319.58	6941.21			
		5.3	-5954.58	1451.33			
		7.0	-11589.58	-13899.81			
1	13459.18	.0	1768.80	452.80			
		.8	66.30	1140.96			
		1.5	-1636.20	552.25			
		2.3	-3338.70	-1313.33			
		3.0	-5041.20	-4455.79			
1	19605.25	.0	1903.32	349.77			
		.8	200.82	1138.83			
		1.5	-1501.68	651.01			
		2.3	-3204.18	-1113.69			
		3.0	-4906.68	-4155.26			
1	15986.83	.0	2121.98	33.89			
		.8	419.48	986.94			
		1.5	-1283.02	663.12			
		2.3	-2985.52	-937.58			
		3.0	-4688.02	-3815.16			
1	10684.75	.0	2385.92	-357.99			
		.8	683.42	793.02			
		1.5	-1019.08	667.14			
		2.3	-2721.58	-735.60			
		3.0	-4424.08	-3415.22			
1	4923.26	.0	2682.06	-800.30			
		.8	979.56	572.81			
		1.5	-722.94	669.04			
		2.3	-2425.44	-511.60			
		3.0	-4127.94	-2969.11			
1	-1140.63	.0	3030.94	-1323.76			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	1328.44	311.01			
		1.5	-374.06	668.90			
		2.3	-2076.56	-250.08			
		3.0	-3779.06	-2445.94			

1	-6711.38						
		.0	3424.95	-1915.93			
		.8	1722.45	14.34			
		1.5	19.95	667.74			
		2.3	-1682.55	44.27			
		3.0	-3385.05	-1856.08			

1	25661.29						
		.0	9747.04	-7671.30			
		1.8	4112.04	4455.40			
		3.5	-1522.96	6720.84			
		5.3	-7157.96	-874.97			
		7.0	-12792.96	-18332.02			

1	29264.46						
		.0	9939.21	-8172.81			
		1.8	4304.21	4290.18			
		3.5	-1330.79	6891.92			
		5.3	-6965.79	-367.59			
		7.0	-12600.79	-17488.35			

1	25793.30						
		.0	10028.34	-8463.01			
		1.8	4393.34	4155.97			
		3.5	-1241.66	6913.70			
		5.3	-6876.66	-189.83			
		7.0	-12511.66	-17154.60			

1	21658.57						
		.0	10183.23	-8993.71			
		1.8	4548.23	3896.31			
		3.5	-1086.77	6925.08			
		5.3	-6721.77	92.60			
		7.0	-12356.77	-16601.14			

1	16504.13						
		.0	10397.63	-9727.56			
		1.8	4762.63	3537.67			
		3.5	-872.37	6941.64			
		5.3	-6507.37	484.36			
		7.0	-12142.37	-15834.16			

1	10757.26						
		.0	10653.30	-10618.08			
		1.8	5018.30	3094.57			
		3.5	-616.70	6945.96			
		5.3	-6251.70	936.10			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		7.0	-11886.70	-14935.00			
1	5687.51						
		.0	10942.52	-11621.58			
		1.8	5307.52	2597.19			
		3.5	-327.48	6954.72			
		5.3	-5962.48	1451.00			
		7.0	-11597.48	-13913.97			
1	-9859.04						
		.0	6662.33	-7034.63			
		1.8	3267.33	1653.83			
		3.5	-127.67	4401.04			
		5.3	-3522.67	1207.00			
		7.0	-6917.67	-7928.29			
1	-6440.39						
		.0	2019.44	-1507.21			
		.8	1126.94	-327.32			
		1.5	234.44	183.20			
		2.3	-658.06	24.34			
		3.0	-1550.56	-803.89			
1	-4405.48						
		.0	6643.97	-6878.23			
		1.8	3248.97	1778.09			
		3.5	-146.03	4493.15			
		5.3	-3541.03	1266.96			
		7.0	-6936.03	-7900.47			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-11507.42	.0	17270.03	-38371.40			
	.8	17270.03	-25418.88			
	1.5	17270.03	-12466.36			
	2.3	17270.03	486.16			
	3.0	17270.03	13438.68			

1 -2817.15	.0	-7794.74	17207.34			
	.8	-7794.74	11361.28			
	1.5	-7794.74	5515.23			
	2.3	-7794.74	-330.83			
	3.0	-7794.74	-6176.88			

1 -7152.89	.0	-1980.90	2289.37			
	.8	-1980.90	803.69			
	1.5	-1980.90	-681.99			
	2.3	-1980.90	-2167.66			
	3.0	-1980.90	-3653.34			

1 -9227.04	.0	-2716.48	4521.48			
	.8	-2716.48	2484.12			
	1.5	-2716.48	446.76			
	2.3	-2716.48	-1590.60			
	3.0	-2716.48	-3627.96			

1-10614.60	.0	-2943.95	5130.15			
	.8	-2943.95	2922.19			
	1.5	-2943.95	714.22			
	2.3	-2943.95	-1493.74			
	3.0	-2943.95	-3701.71			

1-10429.52	.0	-3233.19	5814.95			
	.8	-3233.19	3390.05			
	1.5	-3233.19	965.16			
	2.3	-3233.19	-1459.74			
	3.0	-3233.19	-3884.64			

5 1 -8241.25	.0	-3395.91	6563.60			
	.8	-3395.91	4016.67			
	1.5	-3395.91	1469.74			
	2.3	-3395.91	-1077.19			
	3.0	-3395.91	-3624.12			

9 1 -3381.10	.0	-5029.84	8046.77			

sa Stasis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	-5029.84	4274.39			
		1.5	-5029.84	502.01			
		2.3	-5029.84	-3270.37			
		3.0	-5029.84	-7042.75			

1-72799.24		.0	26881.74	-50312.84			
		.8	26881.74	-30151.53			
		1.5	26881.74	-9990.23			
		2.3	26881.74	10171.08			
		3.0	26881.74	30332.38			

1-45016.73		.0	712.34	4553.09			
		.8	712.34	5087.35			
		1.5	712.34	5621.61			
		2.3	712.34	6155.87			
		3.0	712.34	6690.13			

1-49586.93		.0	6660.40	-10513.57			
		.8	6660.40	-5518.27			
		1.5	6660.40	-522.97			
		2.3	6660.40	4472.34			
		3.0	6660.40	9467.64			

1-46052.60		.0	5691.76	-7988.12			
		.8	5691.76	-3719.30			
		1.5	5691.76	549.52			
		2.3	5691.76	4818.34			
		3.0	5691.76	9087.16			

1-40192.55		.0	5358.16	-7292.10			
		.8	5358.16	-3273.48			
		1.5	5358.16	745.14			
		2.3	5358.16	4763.75			
		3.0	5358.16	8782.37			

1-32095.41		.0	4913.76	-6374.94			
		.8	4913.76	-2689.62			
		1.5	4913.76	995.71			
		2.3	4913.76	4681.03			
		3.0	4913.76	8366.35			

1-21826.48		.0	4310.48	-5312.24			
		.8	4310.48	-2079.38			
		1.5	4310.48	1153.47			
		2.3	4310.48	4386.33			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 3.0	1-2 PLANE SHEAR 4310.48	MOMENT 7619.19	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	-9048.77						
		.0	3569.02	-4330.05			
		.8	3569.02	-1653.29			
		1.5	3569.02	1023.47			
		2.3	3569.02	3700.24			
		3.0	3569.02	6377.00			

	1*****						
		.0	24020.29	-49048.59			
		.8	24020.29	-31033.37			
		1.5	24020.29	-13018.15			
		2.3	24020.29	4997.07			
		3.0	24020.29	23012.29			

7	1*****						
		.0	-7278.21	17570.39			
		.8	-7278.21	12111.73			
		1.5	-7278.21	6653.08			
		2.3	-7278.21	1194.42			
		3.0	-7278.21	-4264.23			

1	1-84334.53						
		.0	-458.42	201.32			
		.8	-458.42	-142.49			
		1.5	-458.42	-486.30			
		2.3	-458.42	-830.11			
		3.0	-458.42	-1173.93			

5	1-62236.39						
		.0	-1711.46	3227.47			
		.8	-1711.46	1943.88			
		1.5	-1711.46	660.29			
		2.3	-1711.46	-623.31			
		3.0	-1711.46	-1906.90			

9	1-43517.60						
		.0	-1888.45	3567.47			
		.8	-1888.45	2151.14			
		1.5	-1888.45	734.80			
		2.3	-1888.45	-681.53			
		3.0	-1888.45	-2097.86			

3	1-27637.02						
		.0	-2374.42	4557.84			
		.8	-2374.42	2777.02			
		1.5	-2374.42	996.21			
		2.3	-2374.42	-784.60			
		3.0	-2374.42	-2565.41			

7	1-14948.31						

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-2921.51	5527.16			
		.8	-2921.51	3336.02			
		1.5	-2921.51	1144.89			
		2.3	-2921.51	-1046.25			
		3.0	-2921.51	-3237.39			

1	-5491.80						
		.0	-4169.00	6474.92			
		.8	-4169.00	3348.17			
		1.5	-4169.00	221.43			
		2.3	-4169.00	-2905.32			
		3.0	-4169.00	-6032.07			

	1*****						
		.0	28591.94	-55364.01			
		.8	28591.94	-33920.06			
		1.5	28591.94	-12476.10			
		2.3	28591.94	8967.85			
		3.0	28591.94	30411.81			

	1*****						
		.0	860.05	6628.11			
		.8	860.05	7273.15			
		1.5	860.05	7918.18			
		2.3	860.05	8563.22			
		3.0	860.05	9208.25			

	1*****						
		.0	5658.94	-8144.81			
		.8	5658.94	-3900.61			
		1.5	5658.94	343.60			
		2.3	5658.94	4587.80			
		3.0	5658.94	8832.01			

	1-79045.07						
		.0	5883.68	-8362.97			
		.8	5883.68	-3950.21			
		1.5	5883.68	462.55			
		2.3	5883.68	4875.30			
		3.0	5883.68	9288.06			

	1-57275.70						
		.0	5511.14	-7375.76			
		.8	5511.14	-3242.40			
		1.5	5511.14	890.95			
		2.3	5511.14	5024.31			
		3.0	5511.14	9157.67			

	1-37878.85						
		.0	5166.43	-6725.46			
		.8	5166.43	-2850.64			
		1.5	5166.43	1024.18			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	5166.43	4899.00			
		3.0	5166.43	8773.83			

1	-21243.61						
		.0	4939.97	-6198.56			
		.8	4939.97	-2493.58			
		1.5	4939.97	1211.40			
		2.3	4939.97	4916.38			
		3.0	4939.97	8621.36			

1	-6939.05						
		.0	4409.36	-5317.85			
		.8	4409.36	-2010.83			
		1.5	4409.36	1296.20			
		2.3	4409.36	4603.22			
		3.0	4409.36	7910.24			

1	22274.76						
		.0	8690.27	-3768.66			
		1.8	3055.27	6508.69			
		3.5	-2579.73	6924.80			
		5.3	-8214.73	-2520.35			
		7.0	-13849.73	-21826.75			

1	21690.14						
		.0	10070.12	-8583.08			
		1.8	4435.12	4109.02			
		3.5	-1199.88	6939.86			
		5.3	-6834.88	-90.55			
		7.0	-12469.88	-16982.20			

1	19782.32						
		.0	9985.89	-8265.75			
		1.8	4350.89	4278.94			
		3.5	-1284.11	6962.38			
		5.3	-6919.11	-215.44			
		7.0	-12554.11	-17254.50			

1	15055.86						
		.0	10155.08	-8843.37			
		1.8	4520.08	3997.41			
		3.5	-1114.92	6976.93			
		5.3	-6749.92	95.20			
		7.0	-12384.92	-16647.78			

1	8943.46						
		.0	10369.19	-9588.35			
		1.8	4734.19	3627.10			
		3.5	-900.81	6981.31			
		5.3	-6535.81	474.26			
		7.0	-12170.81	-15894.04			

sa Statis Portal (8 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	

1	1753.33						
		.0	10631.85	-10503.33			
		1.8	4996.85	3171.78			
		3.5	-638.15	6985.63			
		5.3	-6273.15	938.24			
		7.0	-11908.15	-14970.41			

1	-5238.49						
		.0	10961.91	-11703.33			
		1.8	5326.91	2549.38			
		3.5	-308.09	6940.85			
		5.3	-5943.09	1471.06			
		7.0	-11578.09	-13859.98			

1	14914.18						
		.0	-473.09	3818.20			
		.8	-2175.59	2824.95			
		1.5	-3878.09	554.82			
		2.3	-5580.59	-2992.18			
		3.0	-7283.09	-7816.06			

1	18773.65						
		.0	2098.37	61.99			
		.8	395.87	997.33			
		1.5	-1306.63	655.80			
		2.3	-3009.13	-962.61			
		3.0	-4711.63	-3857.89			

1	16485.04						
		.0	2086.62	83.21			
		.8	384.12	1009.74			
		1.5	-1318.38	659.39			
		2.3	-3020.88	-967.83			
		3.0	-4723.38	-3871.93			

1	10740.52						
		.0	2392.77	-365.18			
		.8	690.27	790.97			
		1.5	-1012.23	670.23			
		2.3	-2714.73	-727.37			
		3.0	-4417.23	-3401.86			

1	4898.81						
		.0	2684.11	-803.19			
		.8	981.61	571.46			
		1.5	-720.89	669.22			
		2.3	-2423.39	-509.89			
		3.0	-4125.89	-2965.87			

1	-1102.52						
		.0	3031.11	-1323.64			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	1328.61	311.25			
		1.5	-373.89	669.27			
		2.3	-2076.39	-249.59			
		3.0	-3778.89	-2445.32			

1	-6652.84						
		.0	3422.74	-1911.92			
		.8	1720.24	16.69			
		1.5	17.74	668.43			
		2.3	-1684.76	43.30			
		3.0	-3387.26	-1858.72			

1	15095.37						
		.0	8258.20	-2530.28			
		1.8	2623.20	6990.94			
		3.5	-3011.80	6650.92			
		5.3	-8646.80	-3550.35			
		7.0	-14281.80	-23612.87			

1	32166.66						
		.0	10019.82	-8516.79			
		1.8	4384.82	4087.26			
		3.5	-1250.18	6830.07			
		5.3	-6885.18	-288.38			
		7.0	-12520.18	-17268.08			

1	26035.57						
		.0	10028.22	-8428.71			
		1.8	4393.22	4190.06			
		3.5	-1241.78	6947.58			
		5.3	-6876.78	-156.15			
		7.0	-12511.78	-17121.13			

4	21548.78						
		.0	10184.72	-9004.45			
		1.8	4549.72	3888.19			
		3.5	-1085.28	6919.58			
		5.3	-6720.28	89.72			
		7.0	-12355.28	-16601.39			

7	16546.65						
		.0	10396.67	-9724.16			
		1.8	4761.67	3539.39			
		3.5	-873.33	6941.69			
		5.3	-6508.33	482.74			
		7.0	-12143.33	-15837.46			

0	10825.92						
		.0	10650.58	-10608.87			
		1.8	5015.58	3099.02			
		3.5	-619.42	6945.65			
		5.3	-6254.42	931.04			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		7.0	-11889.42	-14944.83			

1	5764.10						
		.0	10938.88	-11608.49			
		1.8	5303.88	2603.91			
		3.5	-331.12	6955.07			
		5.3	-5966.12	1444.98			
		7.0	-11601.12	-13926.36			

1	-9987.01						
		.0	6670.76	-7063.80			
		1.8	3275.76	1639.40			
		3.5	-119.24	4401.35			
		5.3	-3514.24	1222.06			
		7.0	-6909.24	-7898.49			

5	-6535.08						
		.0	2015.71	-1499.91			
		.8	1123.21	-322.81			
		1.5	230.71	184.91			
		2.3	-661.79	23.26			
		3.0	-1554.29	-807.77			

6	-4409.36						
		.0	6640.95	-6866.90			
		1.8	3245.95	1784.14			
		3.5	-149.05	4493.93			
		5.3	-3544.05	1262.47			
		7.0	-6939.05	-7910.24			

Sta Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-14718.33							
	.0		20566.63	-61144.56			
	.8		20566.63	-45719.58			
	1.5		20566.63	-30294.60			
	2.3		20566.63	-14869.63			
	3.0		20566.63	555.35			

1-10119.08							
	.0		14409.96	-10134.53			
	.8		14409.96	672.94			
	1.5		14409.96	11480.40			
	2.3		14409.96	22287.87			
	3.0		14409.96	33095.34			

1 -2871.39							
	.0		-13077.93	31836.54			
	.8		-13077.93	22028.10			
	1.5		-13077.93	12219.65			
	2.3		-13077.93	2411.21			
	3.0		-13077.93	-7397.24			

1 -9990.21							
	.0		-1602.21	1627.05			
	.8		-1602.21	425.39			
	1.5		-1602.21	-776.26			
	2.3		-1602.21	-1977.91			
	3.0		-1602.21	-3179.57			

1-10989.84							
	.0		-3052.06	5473.61			
	.8		-3052.06	3184.56			
	1.5		-3052.06	895.51			
	2.3		-3052.06	-1393.54			
	3.0		-3052.06	-3682.59			

1-10787.22							
	.0		-3255.60	5869.22			
	.8		-3255.60	3427.52			
	1.5		-3255.60	985.82			
	2.3		-3255.60	-1455.88			
	3.0		-3255.60	-3897.58			

1 -8456.99							
	.0		-3419.07	6596.31			
	.8		-3419.07	4032.01			
	1.5		-3419.07	1467.70			
	2.3		-3419.07	-1096.60			
	3.0		-3419.07	-3660.91			

1 -3474.40							
	.0		-5040.54	8049.64			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		.8	-5040.54	4269.24			
		1.5	-5040.54	488.84			
		2.3	-5040.54	-3291.57			
		3.0	-5040.54	-7071.97			

	1-77781.98						
		.0	27897.48	-68330.40			
		.8	27897.48	-47407.29			
		1.5	27897.48	-26484.17			
		2.3	27897.48	-5561.06			
		3.0	27897.48	15362.05			

	1-67513.09						
		.0	29377.24	-34072.18			
		.8	29377.24	-12039.25			
		1.5	29377.24	9993.68			
		2.3	29377.24	32026.60			
		3.0	29377.24	54059.53			

	1-37359.79						
		.0	-4944.81	20030.43			
		.8	-4944.81	16321.82			
		1.5	-4944.81	12613.22			
		2.3	-4944.81	8904.61			
		3.0	-4944.81	5196.01			

	1-45453.85						
		.0	6849.55	-10897.81			
		.8	6849.55	-5760.65			
		1.5	6849.55	-623.48			
		2.3	6849.55	4513.68			
		3.0	6849.55	9650.85			

	1-39646.72						
		.0	5246.05	-6948.81			
		.8	5246.05	-3014.28			
		1.5	5246.05	920.26			
		2.3	5246.05	4854.79			
		3.0	5246.05	8789.33			

	1-31728.19						
		.0	4888.30	-6331.36			
		.8	4888.30	-2665.13			
		1.5	4888.30	1001.10			
		2.3	4888.30	4667.32			
		3.0	4888.30	8333.55			

	1-21583.42						
		.0	4299.83	-5305.79			
		.8	4299.83	-2080.91			
		1.5	4299.83	1143.96			
		2.3	4299.83	4368.83			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 3.0	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	

1	-8957.19						
		.0	3561.83	-4326.54			
		.8	3561.83	-1655.17			
		1.5	3561.83	1016.21			
		2.3	3561.83	3687.58			
		3.0	3561.83	6358.95			

1	*****						
		.0	22971.77	-63995.73			
		.8	22971.77	-46766.90			
		1.5	22971.77	-29538.08			
		2.3	22971.77	-12309.25			
		3.0	22971.77	4919.58			

1	*****						
		.0	22462.43	-22904.53			
		.8	22462.43	-6057.71			
		1.5	22462.43	10789.11			
		2.3	22462.43	27635.93			
		3.0	22462.43	44482.75			

1	-76753.20						
		.0	-12210.41	31178.06			
		.8	-12210.41	22020.25			
		1.5	-12210.41	12862.45			
		2.3	-12210.41	3704.64			
		3.0	-12210.41	-5453.17			

1	-63665.05						
		.0	-370.91	65.48			
		.8	-370.91	-212.71			
		1.5	-370.91	-490.89			
		2.3	-370.91	-769.08			
		3.0	-370.91	-1047.27			

1	-43618.55						
		.0	-2159.40	4235.80			
		.8	-2159.40	2616.25			
		1.5	-2159.40	996.70			
		2.3	-2159.40	-622.85			
		3.0	-2159.40	-2242.40			

1	-27699.23						
		.0	-2332.00	4438.72			
		.8	-2332.00	2689.72			
		1.5	-2332.00	940.73			
		2.3	-2332.00	-808.27			
		3.0	-2332.00	-2557.27			

1	-14983.64						

Statis Portal (8 lantai) Kg/m

M E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		.0	-2915.76	5517.69		
		.8	-2915.76	3330.87		
		1.5	-2915.76	1144.04		
		2.3	-2915.76	-1042.78		
		3.0	-2915.76	-3229.60		

1	-5469.47					
		.0	-4160.15	6464.14		
		.8	-4160.15	3344.03		
		1.5	-4160.15	223.91		
		2.3	-4160.15	-2896.20		
		3.0	-4160.15	-6016.31		

	1*****					
		.0	24372.11	-65617.71		
		.8	24372.11	-47338.62		
		1.5	24372.11	-29059.53		
		2.3	24372.11	-10780.45		
		3.0	24372.11	7498.64		

	1*****					
		.0	26783.38	-29833.85		
		.8	26783.38	-9746.31		
		1.5	26783.38	10341.22		
		2.3	26783.38	30428.76		
		3.0	26783.38	50516.29		

2	1*****					
		.0	-4434.49	21689.36		
		.8	-4434.49	18363.50		
		1.5	-4434.49	15037.63		
		2.3	-4434.49	11711.76		
		3.0	-4434.49	8385.89		

6	1-79080.07					
		.0	5632.72	-8264.35		
		.8	5632.72	-4039.81		
		1.5	5632.72	184.73		
		2.3	5632.72	4409.27		
		3.0	5632.72	8633.81		

0	1-57356.55					
		.0	5729.53	-7962.32		
		.8	5729.53	-3665.17		
		1.5	5729.53	631.98		
		2.3	5729.53	4929.13		
		3.0	5729.53	9226.28		

4	1-37935.00					
		.0	5143.98	-6665.02		
		.8	5143.98	-2807.04		
		1.5	5143.98	1050.94		

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		2.3	5143.98	4908.93			
		3.0	5143.98	8766.91			

	1-21275.10						
		.0	4948.28	-6213.65			
		.8	4948.28	-2502.45			
		1.5	4948.28	1208.76			
		2.3	4948.28	4919.97			
		3.0	4948.28	8631.18			

	1 -6941.16						
		.0	4411.73	-5317.94			
		.8	4411.73	-2009.14			
		1.5	4411.73	1299.66			
		2.3	4411.73	4608.46			
		3.0	4411.73	7917.25			

	1 3381.68						
		.0	4599.25	10689.88			
		1.8	-1025.25	13817.12			
		3.5	-6649.75	7101.49			
		5.3	-12274.25	-9457.02			
		7.0	-17898.75	-35858.41			

	1 1901.92						
		.0	-7629.86	13575.82			
		.8	-8844.86	7397.80			
		1.5	-10059.86	308.53			
		2.3	-11274.86	-7691.99			
		3.0	-12489.86	-16603.77			

	1 2411.26						
		.0	4312.88	11220.34			
		1.8	-1311.62	13846.45			
		3.5	-6936.12	6629.68			
		5.3	-12560.62	-10429.96			
		7.0	-18185.12	-37332.48			

	1 21936.88						
		.0	7247.69	1258.79			
		1.8	1612.69	9011.63			
		3.5	-4022.31	6903.21			
		5.3	-9657.31	-5066.46			
		7.0	-15292.31	-26897.37			

	1 19928.68						
		.0	10241.96	-9165.43			
		1.8	4606.96	3827.38			
		3.5	-1028.04	6958.94			
		5.3	-6663.04	229.25			
		7.0	-12298.04	-16361.69			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	15371.86					
	.0	10124.97	-8734.32			
	1.8	4489.97	4053.75			
	3.5	-1145.03	6980.58			
	5.3	-6780.03	46.15			
	7.0	-12415.03	-16749.53			

1	8921.80					
	.0	10380.09	-9623.73			
	1.8	4745.09	3610.80			
	3.5	-889.91	6984.08			
	5.3	-6524.91	496.11			
	7.0	-12159.91	-15853.11			

1	1549.69					
	.0	10644.83	-10548.17			
	1.8	5009.83	3149.66			
	3.5	-625.17	6986.24			
	5.3	-6260.17	961.58			
	7.0	-11895.17	-14924.34			

1	-5398.51					
	.0	10972.89	-11742.21			
	1.8	5337.89	2529.72			
	3.5	-297.11	6940.40			
	5.3	-5932.11	1489.83			
	7.0	-11567.11	-13821.99			

7						
1	15863.52					
	.0	-2499.79	6933.59			
	.8	-4202.29	4420.31			
	1.5	-5904.79	630.15			
	2.3	-7607.29	-4436.88			
	3.0	-9309.79	-10780.79			

0						
1	16059.74					
	.0	2425.07	-429.31			
	.8	722.57	751.06			
	1.5	-979.93	654.55			
	2.3	-2682.43	-718.84			
	3.0	-4384.93	-3369.09			

3						
1	12341.59					
	.0	2310.31	-244.19			
	.8	607.81	850.11			
	1.5	-1094.69	667.53			
	2.3	-2797.19	-791.93			
	3.0	-4499.69	-3528.25			

6						
1	5129.55					
	.0	2685.98	-800.21			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	983.48	575.84			
		1.5	-719.02	675.01			
		2.3	-2421.52	-502.69			
		3.0	-4124.02	-2957.27			

1	-1066.77						
		.0	3025.86	-1315.93			
		.8	1323.36	315.02			
		1.5	-379.14	669.10			
		2.3	-2081.64	-253.69			
		3.0	-3784.14	-2453.36			

2	-6568.82						
		.0	3416.97	-1902.60			
		.8	1714.47	21.69			
		1.5	11.97	669.11			
		2.3	-1690.53	39.65			
		3.0	-3393.03	-1866.68			

8	16700.94						
		.0	6855.92	2305.27			
		1.8	1220.92	9372.52			
		3.5	-4414.08	6578.51			
		5.3	-10049.08	-6076.75			
		7.0	-15684.08	-28593.26			

1	33069.56						
		.0	10200.37	-9092.28			
		1.8	4565.37	3827.74			
		3.5	-1069.63	6886.52			
		5.3	-6704.63	84.05			
		7.0	-12339.63	-16579.67			

4	21914.21						
		.0	10185.38	-8944.15			
		1.8	4550.38	3949.64			
		3.5	-1084.62	6982.19			
		5.3	-6719.62	153.49			
		7.0	-12354.62	-16536.47			

7	16365.46						
		.0	10397.65	-9738.74			
		1.8	4762.65	3526.53			
		3.5	-872.35	6930.55			
		5.3	-6507.35	473.31			
		7.0	-12142.35	-15845.17			

50	10908.42						
		.0	10648.14	-10599.84			
		1.8	5013.14	3103.78			
		3.5	-621.86	6946.15			
		5.3	-6256.86	927.28			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		7.0	-11891.86	-14952.85		

1	5819.95					
		.0	10936.02	-11598.26		
		1.8	5301.02	2609.15		
		3.5	-333.98	6955.32		
		5.3	-5968.98	1440.23		
		7.0	-11603.98	-13936.11		

1	-10129.63					
		.0	6679.06	-7092.46		
		1.8	3284.06	1625.27		
		3.5	-110.94	4401.74		
		5.3	-3505.94	1236.97		
		7.0	-6900.94	-7869.05		

1	-6608.08					
		.0	2009.41	-1488.82		
		.8	1116.91	-316.45		
		1.5	224.41	186.55		
		2.3	-668.09	20.17		
		3.0	-1560.59	-815.59		

1	-4411.73					
		.0	6638.84	-6859.12		
		1.8	3243.84	1788.22		
		3.5	-151.16	4494.31		
		5.3	-3546.16	1259.15		
		7.0	-6941.16	-7917.25		

a Statis Portal (8 lantai) Kg/m

1 E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-16223.18							
	.0		19094.80	-66999.57			
	.8		19094.80	-52678.47			
	1.5		19094.80	-38357.37			
	2.3		19094.80	-24036.27			
	3.0		19094.80	-9715.17			

1-13370.94							
	.0		15981.76	-26716.96			
	.8		15981.76	-14730.65			
	1.5		15981.76	-2744.33			
	2.3		15981.76	9241.99			
	3.0		15981.76	21228.30			

1-11278.78							
	.0		12471.35	1790.31			
	.8		12471.35	11143.82			
	1.5		12471.35	20497.33			
	2.3		12471.35	29850.85			
	3.0		12471.35	39204.36			

1 -4377.50							
	.0		-14824.08	36722.81			
	.8		-14824.08	25604.74			
	1.5		-14824.08	14486.68			
	2.3		-14824.08	3368.62			
	3.0		-14824.08	-7749.45			

1-11497.49							
	.0		-1667.53	1893.85			
	.8		-1667.53	643.20			
	1.5		-1667.53	-607.45			
	2.3		-1667.53	-1858.10			
	3.0		-1667.53	-3108.75			

1-10922.26							
	.0		-3379.32	6282.75			
	.8		-3379.32	3748.27			
	1.5		-3379.32	1213.78			
	2.3		-3379.32	-1320.71			
	3.0		-3379.32	-3855.19			

1 -8645.32							
	.0		-3439.93	6654.47			
	.8		-3439.93	4074.52			
	1.5		-3439.93	1494.57			
	2.3		-3439.93	-1085.38			
	3.0		-3439.93	-3665.33			

1 -3559.12							
	.0		-5059.48	8077.59			

Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		.8	-5059.48	4282.98			
		1.5	-5059.48	488.36			
		2.3	-5059.48	-3306.25			
		3.0	-5059.48	-7100.86			

1-79978.25		.0	28075.82	-75266.68			
		.8	28075.82	-54209.81			
		1.5	28075.82	-33152.95			
		2.3	28075.82	-12096.09			
		3.0	28075.82	8960.78			

1-70392.24		.0	30499.23	-50075.57			
		.8	30499.23	-27201.15			
		1.5	30499.23	-4326.73			
		2.3	30499.23	18547.69			
		3.0	30499.23	41422.11			

1-61355.48		.0	27835.84	-22481.84			
		.8	27835.84	-1604.96			
		1.5	27835.84	19271.92			
		2.3	27835.84	40148.80			
		3.0	27835.84	61025.68			

1-31177.37		.0	-6780.26	25033.65			
		.8	-6780.26	19948.45			
		1.5	-6780.26	14863.26			
		2.3	-6780.26	9778.07			
		3.0	-6780.26	4692.87			

1-39186.69		.0	6605.78	-10347.84			
		.8	6605.78	-5393.50			
		1.5	6605.78	-439.17			
		2.3	6605.78	4515.17			
		3.0	6605.78	9469.50			

1-31284.42		.0	4768.88	-5945.04			
		.8	4768.88	-2368.38			
		1.5	4768.88	1208.28			
		2.3	4768.88	4784.94			
		3.0	4768.88	8361.60			

1-21328.01		.0	4277.74	-5263.50			
		.8	4277.74	-2055.19			
		1.5	4277.74	1153.12			
		2.3	4277.74	4361.43			

3 Statis Portal (8 lantai) Kg/m

1 E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE ENDI	1-2 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
	3.0	4277.74	7569.73	

1	-8834.33			
	.0	3557.45	-4328.16	
	.8	3557.45	-1660.07	
	1.5	3557.45	1008.02	
	2.3	3557.45	3676.11	
	3.0	3557.45	6344.20	

1	*****			
	.0	23233.66	-70647.16	
	.8	23233.66	-53221.92	
	1.5	23233.66	-35796.67	
	2.3	23233.66	-18371.42	
	3.0	23233.66	-946.18	

1	*****			
	.0	22217.96	-37456.93	
	.8	22217.96	-20793.46	
	1.5	22217.96	-4129.99	
	2.3	22217.96	12533.47	
	3.0	22217.96	29196.94	

1	*****			
	.0	21615.68	-13000.00	
	.8	21615.68	3211.76	
	1.5	21615.68	19423.52	
	2.3	21615.68	35635.28	
	3.0	21615.68	51847.04	

1	-54916.88			
	.0	-14134.88	36421.41	
	.8	-14134.88	25820.25	
	1.5	-14134.88	15219.09	
	2.3	-14134.88	4617.93	
	3.0	-14134.88	-5983.23	

1	-45348.16			
	.0	-609.40	585.44	
	.8	-609.40	128.39	
	1.5	-609.40	-328.66	
	2.3	-609.40	-785.72	
	3.0	-609.40	-1242.77	

1	-27835.17			
	.0	-2641.86	5207.61	
	.8	-2641.86	3226.21	
	1.5	-2641.86	1244.81	
	2.3	-2641.86	-736.59	
	3.0	-2641.86	-2717.98	

1	-15043.52			

Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		.0	-2861.64	5377.28		
		.8	-2861.64	3231.05		
		1.5	-2861.64	1084.82		
		2.3	-2861.64	-1061.42		
		3.0	-2861.64	-3207.65		

1	-5455.58					
		.0	-4154.20	6462.62		
		.8	-4154.20	3346.97		
		1.5	-4154.20	231.32		
		2.3	-4154.20	-2884.34		
		3.0	-4154.20	-5999.99		

1	*****					
		.0	24447.72	-71608.16		
		.8	24447.72	-53272.37		
		1.5	24447.72	-34936.58		
		2.3	24447.72	-16600.79		
		3.0	24447.72	1735.00		

1	*****					
		.0	23386.06	-40571.38		
		.8	23386.06	-23031.84		
		1.5	23386.06	-5492.29		
		2.3	23386.06	12047.25		
		3.0	23386.06	29586.80		

1	*****					
		.0	24628.13	-16846.87		
		.8	24628.13	1624.22		
		1.5	24628.13	20095.32		
		2.3	24628.13	38566.42		
		3.0	24628.13	57037.52		

1	-77497.90					
		.0	-6279.61	26888.99		
		.8	-6279.61	22179.28		
		1.5	-6279.61	17469.57		
		2.3	-6279.61	12759.86		
		3.0	-6279.61	8050.14		

1	-57383.17					
		.0	5457.19	-7905.09		
		.8	5457.19	-3812.20		
		1.5	5457.19	280.69		
		2.3	5457.19	4373.58		
		3.0	5457.19	8466.47		

1	-37998.36					
		.0	5402.18	-7351.46		
		.8	5402.18	-3299.83		
		1.5	5402.18	751.80		

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	5402.18	4803.43			
		3.0	5402.18	8855.06			

1	-21299.51						
		.0	4925.00	-6139.56			
		.8	4925.00	-2445.81			
		1.5	4925.00	1247.95			
		2.3	4925.00	4941.70			
		3.0	4925.00	8635.45			

1	-6942.78						
		.0	4414.85	-5321.81			
		.8	4414.85	-2010.67			
		1.5	4414.85	1300.47			
		2.3	4414.85	4611.60			
		3.0	4414.85	7922.74			

1	346.04						
		.0	2852.24	17001.79			
		1.8	-2772.26	17071.77			
		3.5	-8396.76	7298.88			
		5.3	-14021.26	-12316.89			
		7.0	-19645.76	-41775.53			

1	-2023.59						
		.0	2092.16	19437.99			
		1.8	-3532.34	18177.84			
		3.5	-9156.84	7074.81			
		5.3	-14781.34	-13871.10			
		7.0	-20405.84	-44659.88			

1	-2077.36						
		.0	-10059.76	17260.82			
		.8	-11274.76	9260.38			
		1.5	-12489.76	348.68			
		2.3	-13704.76	-9474.26			
		3.0	-14919.76	-20208.45			

1	639.79						
		.0	-11369.08	19244.06			
		.8	-12584.08	10261.63			
		1.5	-13799.08	367.94			
		2.3	-15014.08	-10436.99			
		3.0	-16229.08	-22153.17			

1	-1061.66						
		.0	2876.33	16302.30			
		1.8	-2748.17	16414.44			
		3.5	-8372.67	6683.71			
		5.3	-13997.17	-12889.90			
		7.0	-19621.67	-42306.38			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
1	1242.07			
	.0	1752.22	20043.78	
	1.8	-3872.28	18188.73	
	3.5	-9496.78	6490.80	
	5.3	-15121.28	-15050.00	
	7.0	-20745.78	-46433.67	
1	18994.43			
	.0	6901.28	2481.55	
	1.8	1266.28	9628.17	
	3.5	-4368.72	6913.53	
	5.3	-10003.72	-5662.35	
	7.0	-15638.72	-28099.48	
1	15704.32			
	.0	10420.88	-9783.72	
	1.8	4785.88	3522.19	
	3.5	-849.12	6966.86	
	5.3	-6484.12	550.28	
	7.0	-12119.12	-15727.56	
1	9506.88			
	.0	10335.03	-9459.40	
	1.8	4700.03	3696.27	
	3.5	-934.97	6990.69	
	5.3	-6569.97	423.86	
	7.0	-12204.97	-16004.22	
1	1638.77			
	.0	10650.60	-10564.69	
	1.8	5015.60	3143.23	
	3.5	-619.40	6989.91	
	5.3	-6254.40	975.34	
	7.0	-11889.40	-14900.49	
1	-5555.82			
	.0	10982.32	-11774.05	
	1.8	5347.32	2514.38	
	3.5	-287.68	6941.56	
	5.3	-5922.68	1507.49	
	7.0	-11557.68	-13787.83	
1	12798.66			
	.0	-3001.47	7682.50	
	.8	-4703.97	4792.95	
	1.5	-6406.47	626.53	
	2.3	-8108.97	-4816.76	
	3.0	-9811.47	-11536.93	
3	12060.56			
	.0	2698.92	-833.25	

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	.8	996.42	552.50			
	1.5	-706.08	661.37			
	2.3	-2408.58	-506.63			
	3.0	-4111.08	-2951.51			
5	1 7084.80					
	.0	2587.59	-655.45			
	.8	885.09	646.81			
	1.5	-817.41	672.19			
	2.3	-2519.91	-579.31			
	3.0	-4222.41	-3107.67			
9	1 -770.17					
	.0	3025.24	-1308.35			
	.8	1322.74	322.15			
	1.5	-379.76	675.76			
	2.3	-2082.26	-247.49			
	3.0	-3784.76	-2447.62			
2	1 -6527.11					
	.0	3408.66	-1890.16			
	.8	1706.16	27.90			
	1.5	3.66	669.08			
	2.3	-1698.84	33.38			
	3.0	-3401.34	-1879.18			
.1	1 16072.84					
	.0	6476.10	3654.34			
	1.8	841.10	10056.88			
	3.5	-4793.90	6598.18			
	5.3	-10428.90	-6721.77			
	7.0	-16063.90	-29902.97			
14	1 30269.50					
	.0	10386.01	-9711.68			
	1.8	4751.01	3533.21			
	3.5	-883.99	6916.86			
	5.3	-6518.99	439.25			
	7.0	-12153.99	-15899.60			
47	1 16810.41					
	.0	10396.98	-9663.78			
	1.8	4761.98	3600.31			
	3.5	-873.02	7003.14			
	5.3	-6508.02	544.73			
	7.0	-12143.02	-15774.93			
50	1 10717.06					
	.0	10647.95	-10612.01			
	1.8	5012.95	3091.29			
	3.5	-622.05	6933.33			
	5.3	-6257.05	914.13			

sa Statis Portal (8 lantai) Kg/m

M E L E M E N T F O R C E S

LOAD	AXIAL DIST	1-2 PLANE		1-3 PLANE	AXIAL
COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	TORQ
	7.0	-11892.05	-14966.33		

5	1 5893.34				
	.0	10933.41	-11587.99		
	1.8	5298.41	2614.84		
	3.5	-336.59	6956.43		
	5.3	-5971.59	1436.77		
	7.0	-11606.59	-13944.14		

4	1-10287.94				
	.0	6687.17	-7120.79		
	1.8	3292.17	1611.14		
	3.5	-102.83	4401.81		
	5.3	-3497.83	1251.24		
	7.0	-6892.83	-7840.59		

5	1 -6664.14				
	.0	2001.50	-1475.47		
	.8	1109.00	-309.03		
	1.5	216.50	188.04		
	2.3	-676.00	15.73		
	3.0	-1568.50	-825.96		

6	1 -4414.85				
	.0	6637.22	-6853.27		
	1.8	3242.22	1791.24		
	3.5	-152.78	4494.49		
	5.3	-3547.78	1256.50		
	7.0	-6942.78	-7922.74		

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-17058.26							
	.0		18656.23	-68474.22			
	.8		18656.23	-54482.04			
	1.5		18656.23	-40489.87			
	2.3		18656.23	-26497.70			
	3.0		18656.23	-12505.52			

1-14713.64							
	.0		14599.04	-31282.30			
	.8		14599.04	-20333.02			
	1.5		14599.04	-9383.75			
	2.3		14599.04	1565.53			
	3.0		14599.04	12514.81			

1-14528.06							
	.0		13893.68	-13805.86			
	.8		13893.68	-3385.60			
	1.5		13893.68	7034.66			
	2.3		13893.68	17454.91			
	3.0		13893.68	27875.17			

1-12770.87							
	.0		10408.95	7252.14			
	.8		10408.95	15058.85			
	1.5		10408.95	22865.57			
	2.3		10408.95	30672.28			
	3.0		10408.95	38479.00			

1-5658.87							
	.0		-14812.57	36727.60			
	.8		-14812.57	25618.18			
	1.5		-14812.57	14508.75			
	2.3		-14812.57	3399.33			
	3.0		-14812.57	-7710.10			

1-11213.31							
	.0		-1975.37	2657.14			
	.8		-1975.37	1175.62			
	1.5		-1975.37	-305.91			
	2.3		-1975.37	-1787.44			
	3.0		-1975.37	-3268.97			

1-8612.68							
	.0		-3558.83	7063.84			
	.8		-3558.83	4394.72			
	1.5		-3558.83	1725.60			
	2.3		-3558.83	-943.53			
	3.0		-3558.83	-3612.65			

1-3620.71							
	.0		-5083.32	8132.31			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	-5083.32	4319.82			
		1.5	-5083.32	507.33			
		2.3	-5083.32	-3305.16			
		3.0	-5083.32	-7117.64			

	1-80702.88	.0	27845.71	-77065.86			
		.8	27845.71	-56181.57			
		1.5	27845.71	-35297.28			
		2.3	27845.71	-14413.00			
		3.0	27845.71	6471.29			

	1-71416.51	.0	30722.50	-55557.58			
		.8	30722.50	-32515.70			
		1.5	30722.50	-9473.82			
		2.3	30722.50	13568.05			
		3.0	30722.50	36609.93			

	1-63145.98	.0	28965.08	-37804.24			
		.8	28965.08	-16080.43			
		1.5	28965.08	5643.38			
		2.3	28965.08	27367.19			
		3.0	28965.08	49090.99			

	1-54246.97	.0	25520.98	-16678.40			
		.8	25520.98	2462.33			
		1.5	25520.98	21603.07			
		2.3	25520.98	40743.80			
		3.0	25520.98	59884.53			

	1-25300.91	.0	-6853.47	25151.75			
		.8	-6853.47	20011.65			
		1.5	-6853.47	14871.56			
		2.3	-6853.47	9731.46			
		3.0	-6853.47	4591.36			

	1-31044.66	.0	6112.62	-9297.65			
		.8	6112.62	-4713.19			
		1.5	6112.62	-128.72			
		2.3	6112.62	4455.74			
		3.0	6112.62	9040.20			

	1-21076.66	.0	4166.21	-4892.05			
		.8	4166.21	-1767.40			
		1.5	4166.21	1357.26			
		2.3	4166.21	4481.91			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 3.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	-8750.17		4166.21	7606.57			
		.0	3542.64	-4292.50			
		.8	3542.64	-1635.52			
		1.5	3542.64	1021.46			
		2.3	3542.64	3678.44			
		3.0	3542.64	6335.42			

1	*****						
		.0	23137.81	-72614.02			
		.8	23137.81	-55260.67			
		1.5	23137.81	-37907.31			
		2.3	23137.81	-20553.95			
		3.0	23137.81	-3200.60			

1	*****						
		.0	22503.87	-42682.31			
		.8	22503.87	-25804.41			
		1.5	22503.87	-8926.51			
		2.3	22503.87	7951.39			
		3.0	22503.87	24829.29			

1	-99588.87						
		.0	21460.03	-26965.64			
		.8	21460.03	-10870.62			
		1.5	21460.03	5224.40			
		2.3	21460.03	21319.42			
		3.0	21460.03	37414.44			

5	1-81496.57						
		.0	19111.04	-6780.79			
		.8	19111.04	7552.49			
		1.5	19111.04	21885.77			
		2.3	19111.04	36219.05			
		3.0	19111.04	50552.32			

9	1-36686.96						
		.0	-14177.33	36441.23			
		.8	-14177.33	25808.23			
		1.5	-14177.33	15175.24			
		2.3	-14177.33	4542.24			
		3.0	-14177.33	-6090.76			

3	1-29506.66						
		.0	-1116.97	1619.48			
		.8	-1116.97	781.75			
		1.5	-1116.97	-55.98			
		2.3	-1116.97	-893.70			
		3.0	-1116.97	-1731.43			

7	1-15128.87						

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3163.30	6133.60			
		.8	-3163.30	3761.12			
		1.5	-3163.30	1388.65			
		2.3	-3163.30	-983.83			
		3.0	-3163.30	-3356.31			

1	-5434.59						
		.0	-4105.41	6334.71			
		.8	-4105.41	3255.65			
		1.5	-4105.41	176.60			
		2.3	-4105.41	-2902.46			
		3.0	-4105.41	-5981.52			

	1*****						
		.0	24252.25	-73499.68			
		.8	24252.25	-55310.49			
		1.5	24252.25	-37121.31			
		2.3	24252.25	-18932.12			
		3.0	24252.25	-742.94			

	1*****						
		.0	23302.59	-44833.69			
		.8	23302.59	-27356.74			
		1.5	23302.59	-9879.80			
		2.3	23302.59	7597.15			
		3.0	23302.59	25074.09			

	1*****						
		.0	21280.22	-26891.12			
		.8	21280.22	-10930.96			
		1.5	21280.22	5029.21			
		2.3	21280.22	20989.37			
		3.0	21280.22	36949.54			

	1-89775.60						
		.0	22264.03	-10730.36			
		.8	22264.03	5967.66			
		1.5	22264.03	22665.68			
		2.3	22264.03	39363.70			
		3.0	22264.03	56061.72			

	1-55800.61						
		.0	-6295.85	26781.25			
		.8	-6295.85	22059.37			
		1.5	-6295.85	17337.48			
		2.3	-6295.85	12615.59			
		3.0	-6295.85	7893.71			

	1-37994.33						
		.0	5135.71	-7296.42			
		.8	5135.71	-3444.63			
		1.5	5135.71	407.15			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		2.3	5135.71	4258.93		
		3.0	5135.71	8110.72		

1-21328.99		.0	5180.59	-6813.00		
		.8	5180.59	-2927.56		
		1.5	5180.59	957.88		
		2.3	5180.59	4843.32		
		3.0	5180.59	8728.75		

1 -6943.80		.0	4388.28	-5239.84		
		.8	4388.28	-1948.63		
		1.5	4388.28	1342.58		
		2.3	4388.28	4633.78		
		3.0	4388.28	7924.99		

1 1293.19		.0	2344.62	18776.78		
		1.8	-3279.88	17958.43		
		3.5	-8904.38	7297.20		
		5.3	-14528.88	-13206.90		
		7.0	-20153.38	-43553.87		

1 -4823.64		.0	185.58	26320.67		
		1.8	-5438.92	21723.99		
		3.5	-11063.42	7284.45		
		5.3	-16687.92	-16997.97		
		7.0	-22312.42	-51123.27		

1 -4809.28		.0	1757.19	20623.03		
		1.8	-3867.31	18776.67		
		3.5	-9491.81	7087.44		
		5.3	-15116.31	-14444.67		
		7.0	-20740.81	-45819.65		

1 -1583.59		.0	-10867.01	18474.99		
		.8	-12082.01	9869.11		
		1.5	-13297.01	351.98		
		2.3	-14512.01	-10076.40		
		3.0	-15727.01	-21416.03		

1 -3066.21		.0	-14041.89	23290.90		
		.8	-15256.89	12303.85		
		1.5	-16471.89	405.56		
		2.3	-17686.89	-12403.98		
		3.0	-18901.89	-26124.77		

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ELEMENT FORCES

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
1	-1365.18					
	.0	-11841.80	19949.74			
	.8	-13056.80	10612.77			
	1.5	-14271.80	364.55			
	2.3	-15486.80	-10794.93			
	3.0	-16701.80	-22865.65			
1	-949.65					
	.0	2369.51	18065.69			
	1.8	-3254.99	17290.89			
	3.5	-8879.49	6673.22			
	5.3	-14503.99	-13787.33			
	7.0	-20128.49	-44090.75			
1	-2022.37					
	.0	158.23	25670.17			
	1.8	-5466.27	21025.64			
	3.5	-11090.77	6538.23			
	5.3	-16715.27	-17792.05			
	7.0	-22339.77	-51965.21			
1	983.81					
	.0	1390.50	21329.58			
	1.8	-4234.00	18841.52			
	3.5	-9858.50	6510.59			
	5.3	-15483.00	-15663.22			
	7.0	-21107.50	-47679.90			
1	14162.52					
	.0	7112.00	1751.40			
	1.8	1477.00	9266.78			
	3.5	-4158.00	6920.91			
	5.3	-9793.00	-5286.20			
	7.0	-15428.00	-27354.57			
5	9881.03					
	.0	10625.30	-10493.76			
	1.8	4990.30	3169.89			
	3.5	-644.70	6972.29			
	5.3	-6279.70	913.44			
	7.0	-11914.70	-15006.67			
3	2310.11					
	.0	10601.18	-10384.25			
	1.8	4966.18	3237.19			
	3.5	-668.82	6997.38			
	5.3	-6303.82	896.32			
	7.0	-11938.82	-15065.99			
1	-5410.56					
	.0	10984.23	-11777.08			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.8	5349.23	2514.70			
		3.5	-285.77	6945.22			
		5.3	-5920.77	1514.49			
		7.0	-11555.77	-13777.48			

1	8891.73						
		.0	-2661.68	7182.23			
		.8	-4364.18	4547.54			
		1.5	-6066.68	635.97			
		2.3	-7769.18	-4552.47			
		3.0	-9471.68	-11017.79			

1	6802.81						
		.0	2969.73	-1235.15			
		.8	1267.23	353.71			
		1.5	-435.27	665.70			
		2.3	-2137.77	-299.18			
		3.0	-3840.27	-2540.95			

1	1175.40						
		.0	2927.70	-1164.98			
		.8	1225.20	392.36			
		1.5	-477.30	672.82			
		2.3	-2179.80	-323.59			
		3.0	-3882.30	-2596.87			

1	-6242.67						
		.0	3407.06	-1880.94			
		.8	1704.56	35.92			
		1.5	2.06	675.90			
		2.3	-1700.44	39.00			
		3.0	-3402.94	-1874.77			

1	13652.59						
		.0	6709.19	2873.61			
		1.8	1074.19	9684.07			
		3.5	-4560.81	6633.28			
		5.3	-10195.81	-6278.77			
		7.0	-15830.81	-29052.06			

1	25073.09						
		.0	10593.43	-10415.04			
		1.8	4958.43	3192.84			
		3.5	-676.57	6939.46			
		5.3	-6311.57	824.83			
		7.0	-11946.57	-15151.05			

1	11159.99						
		.0	10646.84	-10536.42			
		1.8	5011.84	3164.94			
		3.5	-623.16	7005.04			
		5.3	-6258.16	983.89			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR MOMENT		1-3 PLANE SHEAR MOMENT	AXIAL TORQ
	7.0	-11893.16	-14898.51		

1	5678.38				
	.0	10933.85	-11601.94		
	1.8	5298.85	2601.67		
	3.5	-336.15	6944.04		
	5.3	-5971.15	1425.16		
	7.0	-11606.15	-13954.98		

1-10432.36					
	.0	6692.73	-7137.35		
	1.8	3297.73	1604.29		
	3.5	-97.27	4404.69		
	5.3	-3492.27	1263.84		
	7.0	-6887.27	-7818.27		

1 -6753.55					
	.0	1992.56	-1461.90		
	.8	1100.06	-302.17		
	1.5	207.56	188.19		
	2.3	-684.94	9.17		
	3.0	-1577.44	-839.22		

1 -4388.28					
	.0	6636.20	-6848.38		
	1.8	3241.20	1794.34		
	3.5	-153.80	4495.81		
	5.3	-3548.80	1256.04		
	7.0	-6943.80	-7924.99		

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-17835.77							
	.0		18425.18	-68525.10			
	.8		18425.18	-54706.22			
	1.5		18425.18	-40887.33			
	2.3		18425.18	-27068.44			
	3.0		18425.18	-13249.56			

1-15588.18							
	.0		14220.83	-32364.27			
	.8		14220.83	-21698.65			
	1.5		14220.83	-11033.02			
	2.3		14220.83	-367.40			
	3.0		14220.83	10298.23			

1-15890.10							
	.0		12616.59	-17729.72			
	.8		12616.59	-8267.28			
	1.5		12616.59	1195.16			
	2.3		12616.59	10657.60			
	3.0		12616.59	20120.04			

1-15850.67							
	.0		11629.35	-6703.71			
	.8		11629.35	2018.30			
	1.5		11629.35	10740.32			
	2.3		11629.35	19462.33			
	3.0		11629.35	28184.34			

1-13372.05							
	.0		7973.82	10093.42			
	.8		7973.82	16073.79			
	1.5		7973.82	22054.15			
	2.3		7973.82	28034.52			
	3.0		7973.82	34014.89			

1 -5730.73							
	.0		-13860.83	34107.68			
	.8		-13860.83	23712.06			
	1.5		-13860.83	13316.43			
	2.3		-13860.83	2920.81			
	3.0		-13860.83	-7474.82			

1 -8725.01							
	.0		-2264.54	3729.67			
	.8		-2264.54	2031.26			
	1.5		-2264.54	332.86			
	2.3		-2264.54	-1365.55			
	3.0		-2264.54	-3063.96			

1 -3486.43							
	.0		-5200.35	8505.29			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	-5200.35	4605.02			
		1.5	-5200.35	704.76			
		2.3	-5200.35	-3195.51			
		3.0	-5200.35	-7095.78			

	1-80536.02	.0	27628.77	-77149.73			
		.8	27628.77	-56428.15			
		1.5	27628.77	-35706.57			
		2.3	27628.77	-14985.00			
		3.0	27628.77	5736.58			

	1-71318.04	.0	30517.08	-56879.61			
		.8	30517.08	-33991.80			
		1.5	30517.08	-11103.99			
		2.3	30517.08	11783.82			
		3.0	30517.08	34671.62			

	1-63329.42	.0	29155.88	-42605.61			
		.8	29155.88	-20738.70			
		1.5	29155.88	1128.20			
		2.3	29155.88	22995.11			
		3.0	29155.88	44862.02			

	1-55116.73	.0	26485.24	-30369.14			
		.8	26485.24	-10505.21			
		1.5	26485.24	9358.72			
		2.3	26485.24	29222.65			
		3.0	26485.24	49086.58			

	1-45797.08	.0	22241.59	-12461.53			
		.8	22241.59	4219.66			
		1.5	22241.59	20900.85			
		2.3	22241.59	37582.04			
		3.0	22241.59	54263.23			

	1-18890.00	.0	-5989.54	22636.33			
		.8	-5989.54	18144.18			
		1.5	-5989.54	13652.03			
		2.3	-5989.54	9159.88			
		3.0	-5989.54	4667.73			

	1-21113.57	.0	5375.37	-7901.09			
		.8	5375.37	-3869.56			
		1.5	5375.37	161.96			
		2.3	5375.37	4193.49			

. Statis Portal (8 lantai) Kg/m

E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE ENDI	DIST 3.0	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
			5375.37	8225.01			

1	-8692.38						
		.0	3436.92	-3959.69			
		.8	3436.92	-1382.00			
		1.5	3436.92	1195.69			
		2.3	3436.92	3773.38			
		3.0	3436.92	6351.06			

1	*****						
		.0	22904.25	-72694.56			
		.8	22904.25	-55516.37			
		1.5	22904.25	-38338.18			
		2.3	22904.25	-21159.99			
		3.0	22904.25	-3981.80			

1	*****						
		.0	22383.02	-44071.54			
		.8	22383.02	-27284.28			
		1.5	22383.02	-10497.02			
		2.3	22383.02	6290.25			
		3.0	22383.02	23077.51			

1	-97173.69						
		.0	21689.20	-31535.31			
		.8	21689.20	-15268.41			
		1.5	21689.20	998.50			
		2.3	21689.20	17265.40			
		3.0	21689.20	33532.30			

1	-78113.58						
		.0	18958.46	-19306.56			
		.8	18958.46	-5087.71			
		1.5	18958.46	9131.13			
		2.3	18958.46	23349.98			
		3.0	18958.46	37568.82			

1	-60419.27						
		.0	15679.22	-2343.01			
		.8	15679.22	9416.40			
		1.5	15679.22	21175.82			
		2.3	15679.22	32935.23			
		3.0	15679.22	44694.64			

3	-21599.85						
		.0	-13299.93	33824.64			
		.8	-13299.93	23849.69			
		1.5	-13299.93	13874.74			
		2.3	-13299.93	3899.79			
		3.0	-13299.93	-6075.15			

7	-16555.27						

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-1803.02	2924.44			
		.8	-1803.02	1572.17			
		1.5	-1803.02	219.91			
		2.3	-1803.02	-1132.36			
		3.0	-1803.02	-2484.63			

1	-5468.38						
		.0	-4344.03	7003.45			
		.8	-4344.03	3745.43			
		1.5	-4344.03	487.41			
		2.3	-4344.03	-2770.61			
		3.0	-4344.03	-6028.63			

4	1*****						
		.0	23978.80	-73546.24			
		.8	23978.80	-55562.14			
		1.5	23978.80	-37578.05			
		2.3	23978.80	-19593.95			
		3.0	23978.80	-1609.85			

8	1*****						
		.0	23047.07	-46065.70			
		.8	23047.07	-28780.40			
		1.5	23047.07	-11495.09			
		2.3	23047.07	5790.21			
		3.0	23047.07	23075.52			

2	1*****						
		.0	21168.34	-30577.19			
		.8	21168.34	-14700.94			
		1.5	21168.34	1175.31			
		2.3	21168.34	17051.56			
		3.0	21168.34	32927.82			

6	1-87175.03						
		.0	19249.95	-19736.31			
		.8	19249.95	-5298.85			
		1.5	19249.95	9138.62			
		2.3	19249.95	23576.08			
		3.0	19249.95	38013.55			

0	1-66811.60						
		.0	19351.38	-7018.36			
		.8	19351.38	7495.17			
		1.5	19351.38	22008.70			
		2.3	19351.38	36522.23			
		3.0	19351.38	51035.77			

4	1-36539.10						
		.0	-5421.08	23868.00			
		.8	-5421.08	19802.19			
		1.5	-5421.08	15736.38			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	-5421.08	11670.57			
		3.0	-5421.08	7604.75			

1	-21289.74	.0	4943.35	-6757.13			
		.8	4943.35	-3049.62			
		1.5	4943.35	657.90			
		2.3	4943.35	4365.41			
		3.0	4943.35	8072.93			

1	-6941.71	.0	4588.25	-5828.29			
		.8	4588.25	-2387.11			
		1.5	4588.25	1054.08			
		2.3	4588.25	4495.26			
		3.0	4588.25	7936.45			

1	1435.35	.0	2247.59	19114.72			
		1.8	-3376.91	18126.56			
		3.5	-9001.41	7295.53			
		5.3	-14625.91	-13378.38			
		7.0	-20250.41	-43895.16			

1	-3933.75	.0	-301.92	28027.95			
		1.8	-5926.42	22578.15			
		3.5	-11550.92	7285.48			
		5.3	-17175.42	-17850.07			
		7.0	-22799.92	-52828.49			

1	-7319.76	.0	39.43	26823.75			
		1.8	-5585.07	21971.31			
		3.5	-11209.57	7276.00			
		5.3	-16834.07	-17262.19			
		7.0	-22458.57	-51643.25			

1	-7421.47	.0	2478.63	18090.92			
		1.8	-3145.87	17507.08			
		3.5	-8770.37	7080.36			
		5.3	-14394.87	-13189.23			
		7.0	-20019.37	-43301.70			

1	-1452.96	.0	-11032.44	18721.02			
		.8	-12247.44	9991.07			
		1.5	-13462.44	349.87			
		2.3	-14677.44	-10202.58			
		3.0	-15892.44	-21666.28			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	-2572.55						
		.0	-14811.30	24448.75			
		.8	-16026.30	12884.64			
		1.5	-17241.30	409.29			
		2.3	-18456.30	-12977.31			
		3.0	-19671.30	-27275.16			

1	-4649.13						
		.0	-14245.87	23587.90			
		.8	-15460.87	12447.87			
		1.5	-16675.87	396.59			
		2.3	-17890.87	-12565.93			
		3.0	-19105.87	-26439.71			

1	-3177.82						
		.0	-10699.73	18246.41			
		.8	-11914.73	9765.98			
		1.5	-13129.73	374.31			
		2.3	-14344.73	-9928.61			
		3.0	-15559.73	-21142.78			

1	-931.72						
		.0	2266.24	18423.46			
		1.8	-3358.26	17467.94			
		3.5	-8982.76	6669.55			
		5.3	-14607.26	-13971.71			
		7.0	-20231.76	-44455.85			

1	-1878.74						
		.0	-321.05	27337.65			
		1.8	-5945.55	21854.38			
		3.5	-11570.05	6528.22			
		5.3	-17194.55	-18640.81			
		7.0	-22819.05	-53652.71			

1	-1918.38						
		.0	-45.75	26399.15			
		1.8	-5670.25	21397.64			
		3.5	-11294.75	6553.26			
		5.3	-16919.25	-18134.00			
		7.0	-22543.75	-52664.13			

1	101.42						
		.0	2134.58	18769.05			
		1.8	-3489.92	17583.12			
		3.5	-9114.42	6554.32			
		5.3	-14738.92	-14317.36			
		7.0	-20363.42	-45031.91			

1	7988.66						
		.0	7641.31	-92.80			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
	1.8	2006.31	8348.88			
	3.5	-3628.69	6929.30			
	5.3	-9263.69	-4351.53			
	7.0	-14898.69	-25493.61			

1	2697.80					
	.0	10859.46	-11308.78			
	1.8	5224.46	2764.64			
	3.5	-410.54	6976.82			
	5.3	-6045.54	1327.75			
	7.0	-11680.54	-14182.58			

1	-4717.48					
	.0	10935.32	-11598.50			
	1.8	5300.32	2607.69			
	3.5	-334.68	6952.62			
	5.3	-5969.68	1436.31			
	7.0	-11604.68	-13941.26			

5	3984.68					
	.0	-1845.34	5966.74			
	.8	-3547.84	3944.30			
	1.5	-5250.34	644.98			
	2.3	-6952.84	-3931.21			
	3.0	-8655.34	-9784.28			

9	847.01					
	.0	3274.77	-1691.62			
	.8	1572.27	126.02			
	1.5	-130.23	666.78			
	2.3	-1832.73	-69.32			
	3.0	-3535.23	-2082.31			

2	-4531.25					
	.0	3322.59	-1757.68			
	.8	1620.09	95.83			
	1.5	-82.41	672.46			
	2.3	-1784.91	-27.79			
	3.0	-3487.41	-2004.90			

7	10172.01					
	.0	7288.46	897.40			
	1.8	1653.46	8721.58			
	3.5	-3981.54	6684.51			
	5.3	-9616.54	-5213.81			
	7.0	-15251.54	-26973.38			

10	18586.62					
	.0	10822.63	-11208.59			
	1.8	5187.63	2800.39			
	3.5	-447.37	6948.12			
	5.3	-6082.37	1234.61			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		7.0	-11717.37	-14340.16		

1	6029.41	.0	10933.34	-11534.05		
		1.8	5298.34	2668.67		
		3.5	-336.66	7010.15		
		5.3	-5971.66	1490.37		
		7.0	-11606.66	-13890.66		

1	10268.18	.0	6688.02	-7116.77		
		1.8	3293.02	1616.64		
		3.5	-101.98	4408.81		
		5.3	-3496.98	1259.72		
		7.0	-6891.98	-7830.62		

1	-6628.73	.0	1998.54	-1460.94		
		.8	1106.04	-296.72		
		1.5	213.54	198.12		
		2.3	-678.96	23.58		
		3.0	-1571.46	-820.32		

1	-4588.25	.0	6638.29	-6874.44		
		1.8	3243.29	1771.93		
		3.5	-151.71	4477.05		
		5.3	-3546.71	1240.93		
		7.0	-6941.71	-7936.45		

Statis Portal (8 lantai) Kg/m

ELEMENT FORCES

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ

1-18936.49	.0	18203.08	-68016.90			
	.8	18203.08	-54364.59			
	1.5	18203.08	-40712.28			
	2.3	18203.08	-27059.97			
	3.0	18203.08	-13407.65			

1-16651.74	.0	14002.49	-32390.45			
	.8	14002.49	-21888.58			
	1.5	14002.49	-11386.72			
	2.3	14002.49	-884.85			
	3.0	14002.49	9617.01			

1-17003.52	.0	12273.57	-18583.21			
	.8	12273.57	-9378.04			
	1.5	12273.57	-172.86			
	2.3	12273.57	9032.31			
	3.0	12273.57	18237.48			

1-17340.03	.0	10538.56	-9901.49			
	.8	10538.56	-1997.57			
	1.5	10538.56	5906.35			
	2.3	10538.56	13810.26			
	3.0	10538.56	21714.18			

1-16257.27	.0	8904.02	-1417.18			
	.8	8904.02	5260.83			
	1.5	8904.02	11938.85			
	2.3	8904.02	18616.86			
	3.0	8904.02	25294.87			

1-12444.20	.0	5088.39	11890.91			
	.8	5088.39	15707.20			
	1.5	5088.39	19523.49			
	2.3	5088.39	23339.79			
	3.0	5088.39	27156.08			

5 1 -4060.67	.0	-12145.12	29837.66			
	.8	-12145.12	20728.82			
	1.5	-12145.12	11619.99			
	2.3	-12145.12	2511.15			
	3.0	-12145.12	-6597.69			

9 1 -3553.73	.0	-4176.47	5681.86			

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E E L E M E N T F O R C E S

LOAD :OMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	.8	-4176.47	2549.51			
	1.5	-4176.47	-582.85			
	2.3	-4176.47	-3715.20			
	3.0	-4176.47	-6847.55			

1-79602.23	.0	27380.52	-76615.52			
	.8	27380.52	-56080.12			
	1.5	27380.52	-35544.73			
	2.3	27380.52	-15009.34			
	3.0	27380.52	5526.05			

1-70364.67	.0	30277.78	-56876.77			
	.8	30277.78	-34168.44			
	1.5	30277.78	-11460.10			
	2.3	30277.78	11248.23			
	3.0	30277.78	33956.57			

1-62408.18	.0	28926.77	-43618.40			
	.8	28926.77	-21923.32			
	1.5	28926.77	-228.24			
	2.3	28926.77	21466.84			
	3.0	28926.77	43161.91			

1-54398.60	.0	26586.62	-34257.27			
	.8	26586.62	-14317.30			
	1.5	26586.62	5622.67			
	2.3	26586.62	25562.63			
	3.0	26586.62	45502.60			

1-45613.16	.0	22950.10	-23695.76			
	.8	22950.10	-6483.18			
	1.5	22950.10	10729.39			
	2.3	22950.10	27941.97			
	3.0	22950.10	45154.54			

1-35543.59	.0	18114.26	-8622.42			
	.8	18114.26	4963.28			
	1.5	18114.26	18548.97			
	2.3	18114.26	32134.67			
	3.0	18114.26	45720.37			

1-11435.32	.0	-4608.31	18474.09			
	.8	-4608.31	15017.86			
	1.5	-4608.31	11561.63			
	2.3	-4608.31	8105.40			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
		3.0	-4608.31	4649.17			

1	-8785.88						
		.0	4369.59	-6404.35			
		.8	4369.59	-3127.16			
		1.5	4369.59	150.03			
		2.3	4369.59	3427.22			
		3.0	4369.59	6704.41			

1	*****						
		.0	22651.28	-72153.86			
		.8	22651.28	-55165.41			
		1.5	22651.28	-38176.95			
		2.3	22651.28	-21188.49			
		3.0	22651.28	-4200.03			

1	*****						
		.0	22126.51	-44062.27			
		.8	22126.51	-27467.39			
		1.5	22126.51	-10872.52			
		2.3	22126.51	5722.36			
		3.0	22126.51	22317.24			

1	-95591.89						
		.0	21524.68	-32593.59			
		.8	21524.68	-16450.08			
		1.5	21524.68	-306.57			
		2.3	21524.68	15836.93			
		3.0	21524.68	31980.44			

1	-76315.47						
		.0	19066.68	-22986.27			
		.8	19066.68	-8686.26			
		1.5	19066.68	5613.75			
		2.3	19066.68	19913.77			
		3.0	19066.68	34213.78			

1	-57850.86						
		.0	15494.75	-12664.29			
		.8	15494.75	-1043.23			
		1.5	15494.75	10577.83			
		2.3	15494.75	22198.90			
		3.0	15494.75	33819.96			

1	-40856.85						
		.0	11315.65	1823.31			
		.8	11315.65	10310.05			
		1.5	11315.65	18796.78			
		2.3	11315.65	27283.52			
		3.0	11315.65	35770.25			

1	-9825.54						

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
	.0	-11819.16	29461.19			
	.8	-11819.16	20596.81			
	1.5	-11819.16	11732.44			
	2.3	-11819.16	2868.07			
	3.0	-11819.16	-5996.30			

1	-6583.99					
	.0	-3409.83	4459.96			
	.8	-3409.83	1902.59			
	1.5	-3409.83	-654.78			
	2.3	-3409.83	-3212.15			
	3.0	-3409.83	-5769.52			

1	1*****					
	.0	23746.12	-73021.27			
	.8	23746.12	-55211.68			
	1.5	23746.12	-37402.09			
	2.3	23746.12	-19592.51			
	3.0	23746.12	-1782.92			

3	1*****					
	.0	22794.23	-46092.15			
	.8	22794.23	-28996.48			
	1.5	22794.23	-11900.81			
	2.3	22794.23	5194.86			
	3.0	22794.23	22290.53			

2	1*****					
	.0	20915.99	-31539.65			
	.8	20915.99	-15852.66			
	1.5	20915.99	-165.67			
	2.3	20915.99	15521.32			
	3.0	20915.99	31208.31			

6	1-86167.91					
	.0	19108.14	-22732.71			
	.8	19108.14	-8401.61			
	1.5	19108.14	5929.49			
	2.3	19108.14	20260.60			
	3.0	19108.14	34591.70			

0	1-64644.70					
	.0	16830.13	-14463.07			
	.8	16830.13	-1840.47			
	1.5	16830.13	10782.13			
	2.3	16830.13	23404.73			
	3.0	16830.13	36027.33			

4	1-45665.36					
	.0	15758.70	-4095.01			
	.8	15758.70	7724.01			
	1.5	15758.70	19543.04			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	2.3	15758.70	31362.06			
	3.0	15758.70	43181.09			

1-20126.28	.0	-3773.87	18935.63			
	.8	-3773.87	16105.22			
	1.5	-3773.87	13274.82			
	2.3	-3773.87	10444.41			
	3.0	-3773.87	7614.01			

1 -6941.61	.0	4557.76	-5837.04			
	.8	4557.76	-2418.72			
	1.5	4557.76	999.60			
	2.3	4557.76	4417.92			
	3.0	4557.76	7836.24			

1 1420.60	.0	2284.75	18982.79			
	1.8	-3339.75	18059.66			
	3.5	-8964.25	7293.66			
	5.3	-14588.75	-13315.22			
	7.0	-20213.25	-43766.98			

1 -3831.08	.0	-351.77	28200.23			
	1.8	-5976.27	22663.18			
	3.5	-11600.77	7283.27			
	5.3	-17225.27	-17939.53			
	7.0	-22849.77	-53005.19			

1 -6605.99	.0	-336.51	28138.97			
	1.8	-5961.01	22628.64			
	3.5	-11585.51	7275.43			
	5.3	-17210.01	-17920.66			
	7.0	-22834.51	-52959.62			

1 -9486.46	.0	1082.75	23131.36			
	1.8	-4541.75	20104.75			
	3.5	-10166.25	7235.26			
	5.3	-15790.75	-15477.11			
	7.0	-21415.25	-48032.35			

1-10086.37	.0	3813.08	13403.96			
	1.8	-1811.42	15155.42			
	3.5	-7435.92	7063.99			
	5.3	-13060.42	-10870.31			
	7.0	-18684.92	-38647.48			

a Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
1	-1476.66	.0	-10975.69	18635.85			
		.8	-12190.69	9948.45			
		1.5	-13405.69	349.81			
		2.3	-14620.69	-10160.08			
		3.0	-15835.69	-21581.23			
1	-2480.07	.0	-14893.29	24569.77			
		.8	-16108.29	12944.18			
		1.5	-17323.29	407.33			
		2.3	-18538.29	-13040.76			
		3.0	-19753.29	-27400.10			
1	-4265.84	.0	-14824.93	24459.56			
		.8	-16039.93	12885.24			
		1.5	-17254.93	399.67			
		2.3	-18469.93	-12997.16			
		3.0	-19684.93	-27305.23			
1	-5849.94	.0	-12629.81	21166.00			
		.8	-13844.81	11238.02			
		1.5	-15059.81	398.79			
		2.3	-16274.81	-11351.69			
		3.0	-17489.81	-24013.42			
1	-5250.54	.0	-8615.35	15129.48			
		.8	-9830.35	8212.34			
		1.5	-11045.35	383.96			
		2.3	-12260.35	-8355.68			
		3.0	-13475.35	-18006.57			
1	-951.89	.0	2307.54	18281.01			
		1.8	-3316.96	17397.76			
		3.5	-8941.46	6671.64			
		5.3	-14565.96	-13897.36			
		7.0	-20190.46	-44309.24			
1	-1878.24	.0	-371.13	27510.73			
		1.8	-5995.63	21939.82			
		3.5	-11620.13	6526.03			
		5.3	-17244.63	-18730.64			
		7.0	-22869.13	-53830.18			
1	-1807.85	.0	-408.50	27661.48			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.8	-6033.00	22025.17			
		3.5	-11657.50	6545.98			
		5.3	-17282.00	-18776.08			
		7.0	-22906.50	-53941.02			

1	-2278.01	.0	974.80	22864.65			
		1.8	-4649.70	19649.11			
		3.5	-10274.20	6590.69			
		5.3	-15898.70	-16310.60			
		7.0	-21523.20	-49054.77			

1	-1071.43	.0	3518.66	13990.07			
		1.8	-2105.84	15226.28			
		3.5	-7730.34	6619.62			
		5.3	-13354.84	-11829.92			
		7.0	-18979.34	-40122.34			

1	551.51	.0	8383.52	-2681.58			
		1.8	2748.52	7058.96			
		3.5	-2886.48	6938.25			
		5.3	-8521.48	-3043.71			
		7.0	-14156.48	-22886.92			

1	-4230.20	.0	11144.33	-12352.68			
		1.8	5509.33	2219.28			
		3.5	-125.67	6929.98			
		5.3	-5760.67	1779.44			
		7.0	-11395.67	-13232.35			

1	-1480.37	.0	-685.60	4234.11			
		.8	-2388.10	3081.47			
		1.5	-4090.60	651.97			
		2.3	-5793.10	-3054.42			
		3.0	-7495.60	-8037.68			

1	-4976.47	.0	3627.51	-2218.82			
		.8	1925.01	-136.63			
		1.5	222.51	668.69			
		2.3	-1479.99	197.13			
		3.0	-3182.49	-1551.30			

1	6275.26	.0	8095.50	-1874.21			
		1.8	2460.50	7362.29			
		3.5	-3174.50	6737.54			
		5.3	-8809.50	-3748.46			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		7.0	-14444.50	-24095.70			

1	12336.44						
		.0	11076.98	-12091.00			
		1.8	5441.98	2363.10			
		3.5	-193.02	6955.95			
		5.3	-5828.02	1687.54			
		7.0	-11463.02	-13442.11			

1	-9607.91						
		.0	6620.22	-6867.85			
		1.8	3225.22	1746.92			
		3.5	-169.78	4420.43			
		5.3	-3564.78	1152.69			
		7.0	-6959.78	-8056.30			

1	-5152.74						
		.0	1902.75	-1330.48			
		.8	1010.25	-238.10			
		1.5	117.75	184.90			
		2.3	-774.75	-61.48			
		3.0	-1667.25	-977.23			

1	-4557.76						
		.0	6638.39	-6775.00			
		1.8	3243.39	1871.57			
		3.5	-151.61	4576.88			
		5.3	-3546.61	1340.94			
		7.0	-6941.61	-7836.24			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-21068.33							
	.0		17982.50	-67314.40			
	.8		17982.50	-53827.53			
	1.5		17982.50	-40340.65			
	2.3		17982.50	-26853.78			
	3.0		17982.50	-13366.91			

1-18702.00							
	.0		13797.56	-32053.46			
	.8		13797.56	-21705.29			
	1.5		13797.56	-11357.12			
	2.3		13797.56	-1008.95			
	3.0		13797.56	9339.23			

1-18963.06							
	.0		12054.16	-18534.04			
	.8		12054.16	-9493.42			
	1.5		12054.16	-452.80			
	2.3		12054.16	8587.82			
	3.0		12054.16	17628.44			

1-19275.83							
	.0		10218.60	-10417.62			
	.8		10218.60	-2753.66			
	1.5		10218.60	4910.29			
	2.3		10218.60	12574.24			
	3.0		10218.60	20238.20			

1-18406.68							
	.0		8027.68	-3631.70			
	.8		8027.68	2389.06			
	1.5		8027.68	8409.82			
	2.3		8027.68	14430.59			
	3.0		8027.68	20451.35			

1-15552.69							
	.0		5625.13	3586.23			
	.8		5625.13	7805.08			
	1.5		5625.13	12023.93			
	2.3		5625.13	16242.77			
	3.0		5625.13	20461.62			

1 -9931.85							
	.0		1731.49	13415.64			
	.8		1731.49	14714.25			
	1.5		1731.49	16012.87			
	2.3		1731.49	17311.49			
	3.0		1731.49	18610.10			

1 -589.54							
	.0		-10862.60	24680.29			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		.8	-10862.60	16533.34			
		1.5	-10862.60	8386.39			
		2.3	-10862.60	239.44			
		3.0	-10862.60	-7907.51			

	1-83441.01						
		.0	27022.56	-75782.55			
		.8	27022.56	-55515.63			
		1.5	27022.56	-35248.71			
		2.3	27022.56	-14981.78			
		3.0	27022.56	5285.14			

	1-73179.29						
		.0	29832.26	-56170.01			
		.8	29832.26	-33795.81			
		1.5	29832.26	-11421.61			
		2.3	29832.26	10952.58			
		3.0	29832.26	33326.78			

	1-64195.95						
		.0	28491.32	-43247.73			
		.8	28491.32	-21879.24			
		1.5	28491.32	-510.75			
		2.3	28491.32	20857.74			
		3.0	28491.32	42226.23			

	1-55197.76						
		.0	26143.77	-34584.07			
		.8	26143.77	-14976.25			
		1.5	26143.77	4631.58			
		2.3	26143.77	24239.40			
		3.0	26143.77	43847.22			

	1-45544.73						
		.0	22743.62	-26113.90			
		.8	22743.62	-9056.19			
		1.5	22743.62	8001.53			
		2.3	22743.62	25059.24			
		3.0	22743.62	42116.96			

	1-34848.56						
		.0	18320.63	-16411.39			
		.8	18320.63	-2670.92			
		1.5	18320.63	11069.55			
		2.3	18320.63	24810.03			
		3.0	18320.63	38550.50			

	1-22799.52						
		.0	12845.22	-4243.92			
		.8	12845.22	5389.99			
		1.5	12845.22	15023.91			
		2.3	12845.22	24657.82			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	12845.22	34291.73			

1	-2518.24						
		.0	-2471.31	12579.64			
		.8	-2471.31	10726.15			
		1.5	-2471.31	8872.67			
		2.3	-2471.31	7019.18			
		3.0	-2471.31	5165.69			

3	1*****						
		.0	22489.84	-71505.62			
		.8	22489.84	-54638.25			
		1.5	22489.84	-37770.87			
		2.3	22489.84	-20903.49			
		3.0	22489.84	-4036.11			

7	1*****						
		.0	22012.28	-43870.92			
		.8	22012.28	-27361.71			
		1.5	22012.28	-10852.50			
		2.3	22012.28	5656.71			
		3.0	22012.28	22165.92			

1	1-97677.66						
		.0	21398.07	-32696.46			
		.8	21398.07	-16647.90			
		1.5	21398.07	-599.35			
		2.3	21398.07	15449.21			
		3.0	21398.07	31497.76			

5	1-77450.18						
		.0	19002.18	-23839.44			
		.8	19002.18	-9587.81			
		1.5	19002.18	4663.83			
		2.3	19002.18	18915.46			
		3.0	19002.18	33167.10			

9	1-57906.55						
		.0	15601.41	-15417.46			
		.8	15601.41	-3716.40			
		1.5	15601.41	7984.66			
		2.3	15601.41	19685.72			
		3.0	15601.41	31386.78			

3	1-39434.19						
		.0	11257.05	-5872.12			
		.8	11257.05	2570.67			
		1.5	11257.05	11013.46			
		2.3	11257.05	19456.25			
		3.0	11257.05	27899.04			

7	1-22429.11						

sa Statis Portal (8 lantai) Kg/m

M E M B E R E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	6034.10	6059.97			
		.8	6034.10	10585.54			
		1.5	6034.10	15111.12			
		2.3	6034.10	19636.70			
		3.0	6034.10	24162.28			

1	-847.30						
		.0	-10133.39	23441.94			
		.8	-10133.39	15841.90			
		1.5	-10133.39	8241.86			
		2.3	-10133.39	641.81			
		3.0	-10133.39	-6958.23			

4	1*****						
		.0	23530.10	-72324.03			
		.8	23530.10	-54676.46			
		1.5	23530.10	-37028.88			
		2.3	23530.10	-19381.31			
		3.0	23530.10	-1733.73			

8	1*****						
		.0	22584.90	-45744.24			
		.8	22584.90	-28805.57			
		1.5	22584.90	-11866.90			
		2.3	22584.90	5071.78			
		3.0	22584.90	22010.45			

2	1*****						
		.0	20686.45	-31511.96			
		.8	20686.45	-15997.12			
		1.5	20686.45	-482.29			
		2.3	20686.45	15032.55			
		3.0	20686.45	30547.38			

6	1-86114.24						
		.0	18869.45	-23352.75			
		.8	18869.45	-9200.66			
		1.5	18869.45	4951.43			
		2.3	18869.45	19103.52			
		3.0	18869.45	33255.61			

0	1-64374.04						
		.0	16666.29	-16568.70			
		.8	16666.29	-4068.98			
		1.5	16666.29	8430.73			
		2.3	16666.29	20930.45			
		3.0	16666.29	33430.16			

4	1-44590.55						
		.0	13842.19	-9482.49			
		.8	13842.19	899.15			
		1.5	13842.19	11280.79			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	13842.19	21662.43			
		3.0	13842.19	32044.07			

1	-27459.52						
		.0	11642.19	-1547.14			
		.8	11642.19	7184.50			
		1.5	11642.19	15916.15			
		2.3	11642.19	24647.79			
		3.0	11642.19	33379.44			

1	-6801.59						
		.0	-1739.60	12687.29			
		.8	-1739.60	11382.59			
		1.5	-1739.60	10077.89			
		2.3	-1739.60	8773.19			
		3.0	-1739.60	7468.49			

1	1386.93						
		.0	2366.33	18686.55			
		1.8	-3258.17	17906.19			
		3.5	-8882.67	7282.95			
		5.3	-14507.17	-13183.16			
		7.0	-20131.67	-43492.14			

1	-3853.60						
		.0	-261.06	27873.26			
		1.8	-5885.56	22494.98			
		3.5	-11510.06	7273.82			
		5.3	-17134.56	-17790.21			
		7.0	-22759.06	-52697.12			

1	-6560.45						
		.0	-312.77	28046.05			
		1.8	-5937.27	22577.27			
		3.5	-11561.77	7265.62			
		5.3	-17186.27	-17888.92			
		7.0	-22810.77	-52886.32			

1	-9004.08						
		.0	869.14	23869.90			
		1.8	-4755.36	20469.46			
		3.5	-10379.86	7226.15			
		5.3	-16004.36	-15860.04			
		7.0	-21628.86	-48789.10			

1	-11591.45						
		.0	2853.99	16865.12			
		1.8	-2770.51	16938.16			
		3.5	-8395.01	7168.33			
		5.3	-14019.51	-12444.37			
		7.0	-19644.01	-41899.95			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	-12898.36					
	.0	5620.85	7045.98			
	1.8	-3.65	11961.02			
	3.5	-5628.15	7033.19			
	5.3	-11252.65	-7737.52			
	7.0	-16877.15	-32351.10			

4	1 -1422.76					
	.0	-9869.95	17963.00			
	.8	-11572.45	9922.11			
	1.5	-13274.95	604.33			
	2.3	-14977.45	-9990.31			
	3.0	-16679.95	-21861.84			

7	1 -2512.65					
	.0	-13775.72	23877.38			
	.8	-15478.22	12907.15			
	1.5	-17180.72	660.05			
	2.3	-18883.22	-12863.93			
	3.0	-20585.72	-27664.78			

0	1 -4212.89					
	.0	-13812.57	23923.98			
	.8	-15515.07	12926.12			
	1.5	-17217.57	651.38			
	2.3	-18920.07	-12900.24			
	3.0	-20622.57	-27728.73			

3	1 -5603.93					
	.0	-11975.83	21172.03			
	.8	-13678.33	11551.72			
	1.5	-15380.83	654.53			
	2.3	-17083.33	-11519.53			
	3.0	-18785.83	-24970.46			

6	1 -7168.46					
	.0	-8947.84	16628.40			
	.8	-10650.34	9279.08			
	1.5	-12352.84	652.88			
	2.3	-14055.34	-9250.19			
	3.0	-15757.84	-20430.13			

9	1 -7422.94					
	.0	-4828.12	10443.31			
	.8	-6530.62	6183.79			
	1.5	-8233.12	647.39			
	2.3	-9935.62	-6165.88			
	3.0	-11638.12	-14256.03			

15	1 -945.21					
	.0	2394.22	17972.98			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.8	-3230.28	17241.42			
		3.5	-8854.78	6666.98			
		5.3	-14479.28	-13750.33			
		7.0	-20103.78	-44010.51			
3	1	-1898.45					
		.0	-282.43	27197.59			
		1.8	-5906.93	21781.91			
		3.5	-11531.43	6523.34			
		5.3	-17155.93	-18578.10			
		7.0	-22780.43	-53522.41			
1	1	-1817.00					
		.0	-395.09	27608.47			
		1.8	-6019.59	21995.63			
		3.5	-11644.09	6539.92			
		5.3	-17268.59	-18758.67			
		7.0	-22893.09	-53900.13			
4	1	-2203.17					
		.0	757.80	23614.09			
		1.8	-4866.70	20018.81			
		3.5	-10491.20	6580.64			
		5.3	-16115.70	-16700.39			
		7.0	-21740.20	-49824.30			
7	1	-2824.10					
		.0	2714.51	16828.76			
		1.8	-2909.99	16657.72			
		3.5	-8534.49	6643.81			
		5.3	-14158.99	-13212.98			
		7.0	-19783.49	-42912.65			
10	1	-2199.99					
		.0	5366.96	7583.04			
		1.8	-257.54	12053.79			
		3.5	-5882.04	6681.66			
		5.3	-11506.54	-8533.34			
		7.0	-17131.04	-33591.21			
11	1	-6996.91					
		.0	9342.31	-6070.18			
		1.8	3707.31	5348.23			
		3.5	-1927.69	6905.40			
		5.3	-7562.69	-1398.68			
		7.0	-13197.69	-19564.02			
2	1	-6393.80					
		.0	760.29	2069.24			
		.8	-942.21	2001.02			
		1.5	-2644.71	655.93			
		2.3	-4347.21	-1966.04			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	-6049.71	-5864.88			

1	3350.99	.0	9076.45	-5237.17			
		1.8	3441.45	5716.00			
		3.5	-2193.55	6807.91			
		5.3	-7828.55	-1961.43			
		7.0	-13463.55	-20592.02			

1	-8811.18	.0	6912.84	-7951.52			
		1.8	3517.84	1175.33			
		3.5	122.84	4360.92			
		5.3	-3272.16	1605.27			
		7.0	-6667.16	-7091.63			

1	-4859.79	.0	2306.73	-1932.17			
		.8	1414.23	-536.81			
		1.5	521.73	189.17			
		2.3	-370.77	245.78			
		3.0	-1263.27	-366.98			

1	1739.60	.0	6778.41	-7387.38			
		1.8	3383.41	1504.22			
		3.5	-11.59	4454.56			
		5.3	-3406.59	1463.66			
		7.0	-6801.59	-7468.49			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-21982.90						
	.0	17775.25	-66826.86			
	.8	17775.25	-53495.42			
	1.5	17775.25	-40163.98			
	2.3	17775.25	-26832.54			
	3.0	17775.25	-13501.10			

1-19504.31						
	.0	13619.80	-31792.07			
	.8	13619.80	-21577.22			
	1.5	13619.80	-11362.37			
	2.3	13619.80	-1147.52			
	3.0	13619.80	9067.33			

1-19643.75						
	.0	11890.85	-18375.36			
	.8	11890.85	-9457.22			
	1.5	11890.85	-539.09			
	2.3	11890.85	8379.05			
	3.0	11890.85	17297.19			

1-19864.31						
	.0	10057.51	-10421.55			
	.8	10057.51	-2878.41			
	1.5	10057.51	4664.72			
	2.3	10057.51	12207.85			
	3.0	10057.51	19750.98			

1-18979.69						
	.0	7813.28	-4060.75			
	.8	7813.28	1799.21			
	1.5	7813.28	7659.17			
	2.3	7813.28	13519.13			
	3.0	7813.28	19379.09			

1-16311.07						
	.0	5075.73	1867.56			
	.8	5075.73	5674.36			
	1.5	5075.73	9481.15			
	2.3	5075.73	13287.95			
	3.0	5075.73	17094.74			

1-11449.44						
	.0	1866.45	7326.53			
	.8	1866.45	8726.37			
	1.5	1866.45	10126.20			
	2.3	1866.45	11526.04			
	3.0	1866.45	12925.88			

1 -4297.42						
	.0	-3315.55	11296.68			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	-3315.55	8810.01			
		1.5	-3315.55	6323.35			
		2.3	-3315.55	3836.68			
		3.0	-3315.55	1350.02			

1-75234.55		.0	26858.10	-75332.02			
		.8	26858.10	-55188.45			
		1.5	26858.10	-35044.87			
		2.3	26858.10	-14901.29			
		3.0	26858.10	5242.28			

1-65866.46		.0	29711.79	-56011.06			
		.8	29711.79	-33727.22			
		1.5	29711.79	-11443.37			
		2.3	29711.79	10840.47			
		3.0	29711.79	33124.31			

1-57735.41		.0	28365.12	-45142.87			
		.8	28365.12	-21869.03			
		1.5	28365.12	-595.18			
		2.3	28365.12	20678.66			
		3.0	28365.12	41952.50			

1-49570.11		.0	26036.87	-34667.66			
		.8	26036.87	-15140.01			
		1.5	26036.87	4387.64			
		2.3	26036.87	23915.29			
		3.0	26036.87	43442.94			

1-40758.58		.0	22654.42	-26722.91			
		.8	22654.42	-9732.10			
		1.5	22654.42	7258.72			
		2.3	22654.42	24249.53			
		3.0	22654.42	41240.34			

1-30974.37		.0	18401.16	-18603.64			
		.8	18401.16	-4802.77			
		1.5	18401.16	8998.10			
		2.3	18401.16	22798.97			
		3.0	18401.16	36599.84			

1-20028.99		.0	13237.63	-10604.22			
		.8	13237.63	-676.00			
		1.5	13237.63	9252.22			
		2.3	13237.63	19180.43			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 3.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	-7949.62		13237.63	29108.65			
		.0	8229.77	-5066.08			
		.8	8229.77	1106.25			
		1.5	8229.77	7278.58			
		2.3	8229.77	13450.91			
		3.0	8229.77	19623.23			

1	*****						
		.0	22120.23	-70859.65			
		.8	22120.23	-54269.48			
		1.5	22120.23	-37679.31			
		2.3	22120.23	-21089.13			
		3.0	22120.23	-4498.96			

1	*****						
		.0	21544.77	-43175.08			
		.8	21544.77	-27016.50			
		1.5	21544.77	-10857.92			
		2.3	21544.77	5300.66			
		3.0	21544.77	21459.24			

1	-95547.46						
		.0	20930.60	-32085.05			
		.8	20930.60	-16387.10			
		1.5	20930.60	-689.16			
		2.3	20930.60	15008.79			
		3.0	20930.60	30706.74			

5	1-76410.95						
		.0	18537.15	-23396.98			
		.8	18537.15	-9494.12			
		1.5	18537.15	4408.74			
		2.3	18537.15	18311.60			
		3.0	18537.15	32214.46			

9	1-57946.10						
		.0	15191.94	-15519.26			
		.8	15191.94	-4125.31			
		1.5	15191.94	7268.65			
		2.3	15191.94	18662.60			
		3.0	15191.94	30056.56			

3	1-40474.05						
		.0	10942.53	-7425.02			
		.8	10942.53	781.88			
		1.5	10942.53	8988.78			
		2.3	10942.53	17195.68			
		3.0	10942.53	25402.58			

7	1-24183.10						

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	5847.89	490.14			
		.8	5847.89	4876.05			
		1.5	5847.89	9261.97			
		2.3	5847.89	13647.88			
		3.0	5847.89	18033.80			

1	-9069.22						
		.0	449.83	6043.12			
		.8	449.83	6380.49			
		1.5	449.83	6717.87			
		2.3	449.83	7055.24			
		3.0	449.83	7392.61			

1	1*****						
		.0	23315.41	-71822.38			
		.8	23315.41	-54335.81			
		1.5	23315.41	-36849.25			
		2.3	23315.41	-19362.69			
		3.0	23315.41	-1876.13			

3	1*****						
		.0	22396.64	-45474.39			
		.8	22396.64	-28676.91			
		1.5	22396.64	-11879.44			
		2.3	22396.64	4918.04			
		3.0	22396.64	21715.52			

2	1*****						
		.0	20493.43	-31311.81			
		.8	20493.43	-15941.74			
		1.5	20493.43	-571.67			
		2.3	20493.43	14798.41			
		3.0	20493.43	30168.48			

6	1-84308.63						
		.0	18658.48	-23305.87			
		.8	18658.48	-9312.01			
		1.5	18658.48	4681.85			
		2.3	18658.48	18675.71			
		3.0	18658.48	32669.57			

0	1-62613.63						
		.0	16443.36	-16987.31			
		.8	16443.36	-4654.79			
		1.5	16443.36	7677.73			
		2.3	16443.36	20010.25			
		3.0	16443.36	32342.77			

4	1-42682.51						
		.0	13699.58	-11083.12			
		.8	13699.58	-808.44			
		1.5	13699.58	9466.25			

sa Statis Portal (8 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.3	13699.58	19740.94			
		3.0	13699.58	30015.62			

1	-24924.47	.0	10386.04	-5737.51			
		.8	10386.04	2052.02			
		1.5	10386.04	9841.55			
		2.3	10386.04	17631.07			
		3.0	10386.04	25420.60			

1	-9413.74	.0	6395.95	-2385.61			
		.8	6395.95	2411.36			
		1.5	6395.95	7208.32			
		2.3	6395.95	12005.29			
		3.0	6395.95	16802.25			

3	1359.45	.0	2478.58	18290.98			
		1.8	-3145.92	17707.06			
		3.5	-8770.42	7280.27			
		5.3	-14394.92	-12989.40			
		7.0	-20019.42	-43101.94			

5	-3864.05	.0	-139.44	27442.69			
		1.8	-5763.94	22277.23			
		3.5	-11388.44	7268.90			
		5.3	-17012.94	-17582.30			
		7.0	-22637.44	-52276.39			

9	-6556.66	.0	-220.56	27718.73			
		1.8	-5845.06	22411.32			
		3.5	-11469.56	7261.03			
		5.3	-17094.06	-17732.13			
		7.0	-22718.56	-52568.17			

2	-8942.77	.0	884.62	23811.73			
		1.8	-4739.88	20438.38			
		3.5	-10364.38	7222.15			
		5.3	-15988.88	-15836.96			
		7.0	-21613.38	-48738.94			

5	-11246.45	.0	2668.62	17511.52			
		1.8	-2955.88	17260.18			
		3.5	-8580.38	7165.95			
		5.3	-14204.88	-12771.14			
		7.0	-19829.38	-42551.12			

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TIME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

3	1	-13571.73					
		.0	4861.63	9768.21			
		1.8	-762.87	13354.63			
		3.5	-6387.37	7098.17			
		5.3	-12011.87	-9001.17			
		7.0	-17636.37	-34943.38			

1	1	-14395.99					
		.0	7152.02	1629.20			
		1.8	1527.52	9223.80			
		3.5	-4096.98	6975.52			
		5.3	-9721.48	-5115.63			
		7.0	-15345.98	-27049.66			

4	1	-1494.23					
		.0	-10651.33	18151.40			
		.8	-11866.33	9707.28			
		1.5	-13081.33	351.92			
		2.3	-14296.33	-9914.70			
		3.0	-15511.33	-21092.57			

7	1	-2517.38					
		.0	-14506.38	23990.79			
		.8	-15721.38	12655.38			
		1.5	-16936.38	408.71			
		2.3	-18151.38	-12749.20			
		3.0	-19366.38	-26818.36			

0	1	-4228.40					
		.0	-14553.26	24051.99			
		.8	-15768.26	12681.42			
		1.5	-16983.26	399.60			
		2.3	-18198.26	-12793.48			
		3.0	-19413.26	-26897.80			

3	1	-5560.32					
		.0	-12801.85	21426.91			
		.8	-14016.85	11369.90			
		1.5	-15231.85	401.65			
		2.3	-16446.85	-11477.86			
		3.0	-17661.85	-24268.62			

6	1	-6993.19					
		.0	-10045.17	17292.87			
		.8	-11260.17	9303.36			
		1.5	-12475.17	402.61			
		2.3	-13690.17	-9409.40			
		3.0	-14905.17	-20132.65			

9	1	-8408.19					
		.0	-6690.99	12260.69			

Analisa Statis Portal (8 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.8	-7905.99	6786.82			
		1.5	-9120.99	401.70			
		2.3	-10335.99	-6894.66			
		3.0	-11550.99	-15102.28			

2	1	-9388.14					
		.0	-3266.62	7125.07			
		.8	-4481.62	4219.49			
		1.5	-5696.62	402.65			
		2.3	-6911.62	-4325.44			
		3.0	-8126.62	-9964.77			

5	1	-918.78					
		.0	2508.74	17583.55			
		1.8	-3115.76	17052.41			
		3.5	-8740.26	6678.40			
		5.3	-14364.76	-13538.49			
		7.0	-19989.26	-43598.26			

8	1	-1903.21					
		.0	-144.32	26725.93			
		1.8	-5768.82	21551.93			
		3.5	-11393.32	6535.05			
		5.3	-17017.82	-18324.70			
		7.0	-22642.32	-53027.33			

1	1	-1834.95					
		.0	-276.75	27205.92			
		1.8	-5901.25	21800.16			
		3.5	-11525.75	6551.54			
		5.3	-17150.25	-18539.97			
		7.0	-22774.75	-53474.35			

4	1	-2215.12					
		.0	803.00	23465.10			
		1.8	-4821.50	19948.92			
		3.5	-10446.00	6589.86			
		5.3	-16070.50	-16612.07			
		7.0	-21695.00	-49656.88			

7	1	-2743.78					
		.0	2566.88	17348.92			
		1.8	-3057.62	16919.53			
		3.5	-8682.12	6647.27			
		5.3	-14306.62	-13467.88			
		7.0	-19931.12	-43425.89			

0	1	-3313.54					
		.0	4739.96	9810.17			
		1.8	-884.54	13183.66			
		3.5	-6509.04	6714.27			
		5.3	-12133.54	-9598.00			

Isa Statis Portal (8 lantai) Kg/m

A M E E L E M E N T F O R C E S

T D	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
3			7.0	-17758.04	-35753.13			

	1	-3990.09						
			.0	6987.27	2025.90			
			1.8	1362.77	9332.19			
			3.5	-4261.73	6795.60			
			5.3	-9886.23	-5583.87			
			7.0	-15510.73	-27806.21			

4								
	1	-15075.55						
			.0	4297.42	1350.02			
			1.8	902.42	5899.88			
			3.5	-2492.58	4508.48			
			5.3	-5887.58	-2824.16			
			7.0	-9282.58	-16098.05			

5								
	1	-6845.78						
			.0	-1332.96	3525.19			
			.8	-2225.46	2190.78			
			1.5	-3117.96	187.00			
			2.3	-4010.46	-2486.15			
			3.0	-4902.96	-5828.68			

6								
	1	-6395.95						
			.0	4166.26	1563.93			
			1.8	771.26	5884.26			
			3.5	-2623.74	4263.34			
			5.3	-6018.74	-3298.83			
			7.0	-9413.74	-16802.25			

sa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.001186	-.000037	-.000823
6	.001670	-.000294	-.000545
7	.001920	-.000749	-.000961
8	.002661	-.000858	-.000820
9	.003640	-.000059	-.001001
0	.004267	-.000575	-.000670
1	.004631	-.001348	-.000949
2	.005476	-.001568	-.000643
3	.006416	-.000093	-.001037
4	.006996	-.000854	-.000661
5	.007293	-.001816	-.000923
6	.008038	-.002139	-.000596
7	.009129	-.000141	-.000984
8	.009571	-.001113	-.000596
9	.009769	-.002163	-.000852
0	.010394	-.002578	-.000515
1	.011585	-.000196	-.000886
2	.011851	-.001339	-.000494
3	.011942	-.002405	-.000748
4	.012419	-.002896	-.000394
5	.013662	-.000252	-.000754
6	.013718	-.001519	-.000357
7	.013697	-.002560	-.000611
8	.014008	-.003107	-.000253
9	.015218	-.000297	-.000552
0	.015070	-.001641	-.000199
1	.014946	-.002643	-.000454
2	.015110	-.003225	-.000088
3	.016168	-.000315	-.000482
4	.015884	-.001692	-.000058
5	.015764	-.002674	-.000424
6	.015637	-.003264	.000088

sa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.004614	-.000064	-.001699
6	.005258	-.000405	-.001362
7	.005534	-.000835	-.001774
8	.005970	-.000888	-.001701
9	.007652	-.000080	-.000947
0	.008279	-.000656	-.000595
1	.008627	-.001410	-.000868
2	.009556	-.001595	-.000621
3	.010397	-.000120	-.001040
4	.010968	-.000932	-.000667
5	.011274	-.001880	-.000934
6	.012026	-.002166	-.000574
7	.013102	-.000171	-.000980
8	.013537	-.001188	-.000592
9	.013736	-.002226	-.000844
0	.014359	-.002606	-.000511
1	.015543	-.000230	-.000882
2	.015801	-.001412	-.000490
3	.015892	-.002468	-.000744
4	.016370	-.002925	-.000390
5	.017605	-.000288	-.000751
6	.017656	-.001590	-.000355
7	.017636	-.002622	-.000608
8	.017948	-.003136	-.000250
9	.019151	-.000334	-.000550
0	.018999	-.001712	-.000197
1	.018876	-.002705	-.000452
2	.019042	-.003254	-.000085
3	.020097	-.000353	-.000482
4	.019809	-.001762	-.000058
5	.019688	-.002736	-.000422
6	.019560	-.003292	.000091

sa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.008654	-.000082	-.004129
	.008751	-.000433	-.003610
	.008787	-.000800	-.004026
	.008856	-.000867	-.003961
	.020418	-.000138	-.002565
	.021051	-.000809	-.002248
	.021346	-.001506	-.002556
	.021828	-.001633	-.002551
	.024049	-.000154	-.000899
	.024624	-.001017	-.000528
	.024923	-.001933	-.000802
	.025878	-.002197	-.000502
	.026713	-.000210	-.001005
	.027157	-.001270	-.000613
	.027386	-.002287	-.000869
	.028019	-.002637	-.000477
	.029179	-.000271	-.000883
	.029436	-.001490	-.000488
	.029531	-.002530	-.000733
	.030004	-.002956	-.000390
	.031236	-.000331	-.000748
	.031280	-.001667	-.000352
	.031261	-.002684	-.000605
	.031576	-.003167	-.000247
	.032771	-.000378	-.000548
	.032615	-.001787	-.000196
	.032494	-.002768	-.000449
	.032662	-.003285	-.000082
	.033713	-.000397	-.000482
	.033421	-.001837	-.000057
	.033298	-.002798	-.000419
	.033171	-.003324	.000095

sa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
1	.010126	-.000090	-.005228
2	.010136	-.000445	-.004519
3	.010098	-.000777	-.004879
4	.010067	-.000858	-.004762
5	.028284	-.000165	-.005602
6	.028225	-.000837	-.005109
7	.028237	-.001455	-.005442
8	.028273	-.001606	-.005511
9	.042392	-.000227	-.002809
0	.042940	-.001178	-.002482
1	.043178	-.002032	-.002795
2	.043642	-.002240	-.002772
3	.046082	-.000252	-.000834
4	.046536	-.001352	-.000456
5	.046760	-.002338	-.000720
6	.047634	-.002671	-.000390
7	.048509	-.000316	-.000917
8	.048783	-.001570	-.000516
9	.048915	-.002590	-.000765
0	.049400	-.002990	-.000352
1	.050607	-.000377	-.000751
2	.050654	-.001744	-.000351
3	.050640	-.002745	-.000595
4	.050949	-.003202	-.000250
5	.052143	-.000425	-.000548
6	.051983	-.001863	-.000194
7	.051862	-.002829	-.000448
8	.052032	-.003320	-.000080
9	.053081	-.000445	-.000481
0	.052784	-.001912	-.000056
1	.052660	-.002859	-.000416
2	.052533	-.003359	.000098

sa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
.	.000000	.000000	.000000
.	.000000	.000000	.000000
.	.000000	.000000	.000000
.	.000000	.000000	.000000
.	.010510	-.000095	-.005519
.	.010547	-.000449	-.004811
.	.010518	-.000761	-.005167
.	.010490	-.000854	-.005060
.	.030731	-.000177	-.006798
.	.030592	-.000847	-.006103
.	.030535	-.001421	-.006384
.	.030477	-.001595	-.006407
.	.051349	-.000258	-.005839
.	.051210	-.001198	-.005333
.	.051185	-.001976	-.005672
.	.051213	-.002212	-.005721
.	.065437	-.000329	-.002723
.	.065846	-.001500	-.002389
.	.066011	-.002429	-.002688
.	.066405	-.002712	-.002632
.	.068866	-.000360	-.000745
.	.069152	-.001641	-.000362
.	.069278	-.002634	-.000620
.	.070002	-.003022	-.000268
.	.070927	-.000423	-.000787
.	.070994	-.001814	-.000379
.	.071016	-.002798	-.000628
.	.071338	-.003234	-.000213
.	.072508	-.000471	-.000551
.	.072352	-.001931	-.000194
.	.072236	-.002882	-.000438
.	.072400	-.003353	-.000082
.	.073451	-.000491	-.000482
.	.073149	-.001980	-.000055
.	.073024	-.002912	-.000414
.	.072897	-.003391	.000101

aa Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.010563	-.000099	-.005573
	.010605	-.000448	-.004867
	.010578	-.000750	-.005226
	.010551	-.000850	-.005122
	.031240	-.000186	-.007077
	.031126	-.000845	-.006381
	.031078	-.001398	-.006657
	.031024	-.001588	-.006689
	.053735	-.000274	-.006914
	.053524	-.001198	-.006227
	.053438	-.001939	-.006520
	.053382	-.002199	-.006529
	.073673	-.000363	-.005450
	.073459	-.001504	-.004951
	.073400	-.002374	-.005276
	.073403	-.002684	-.005283
	.086468	-.000437	-.002444
	.086699	-.001759	-.002102
	.086773	-.002710	-.002389
	.087066	-.003056	-.002283
	.089420	-.000469	-.000629
	.089498	-.001864	-.000241
	.089513	-.002830	-.000498
	.090050	-.003259	-.000138
	.090968	-.000518	-.000584
	.090832	-.001982	-.000219
	.090747	-.002922	-.000468
	.090922	-.003378	-.000049
	.091953	-.000537	-.000488
	.091656	-.002030	-.000056
	.091533	-.002953	-.000402
	.091400	-.003416	.000095

Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.010501	-.000105	-.005549
	.010542	-.000443	-.004845
	.010515	-.000741	-.005204
	.010487	-.000847	-.005098
	.031152	-.000198	-.007101
	.031041	-.000835	-.006407
	.030995	-.001381	-.006686
	.030941	-.001581	-.006720
	.053959	-.000293	-.007125
	.053769	-.001182	-.006438
	.053690	-.001913	-.006728
	.053637	-.002188	-.006743
	.075387	-.000389	-.006320
	.075113	-.001485	-.005672
	.075005	-.002338	-.005962
	.074939	-.002668	-.005935
	.092971	-.000480	-.004693
	.092680	-.001739	-.004209
	.092582	-.002660	-.004521
	.092551	-.003028	-.004465
	.103665	-.000549	-.002031
	.103681	-.001937	-.001681
	.103654	-.002887	-.001958
	.103835	-.003282	-.001801
	.105931	-.000572	-.000448
	.105809	-.002000	-.000105
	.105717	-.002942	-.000359
	.106073	-.003394	.000008
	.106894	-.000591	-.000527
	.106616	-.002049	-.000085
	.106520	-.002978	-.000448
	.106389	-.003432	.000145

la Statis Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.010399	-.000117	-.005499
	.010439	-.000464	-.004805
	.010412	-.000763	-.005148
	.010385	-.000845	-.005047
	.030867	-.000221	-.007047
	.030756	-.000872	-.006361
	.030709	-.001419	-.006628
	.030654	-.001579	-.006665
	.053542	-.000327	-.007108
	.053352	-.001229	-.006431
	.053274	-.001963	-.006709
	.053221	-.002186	-.006731
	.075085	-.000434	-.006439
	.074825	-.001536	-.005800
	.074721	-.002394	-.006074
	.074657	-.002665	-.006056
	.093643	-.000537	-.005293
	.093308	-.001790	-.004709
	.093175	-.002717	-.004985
	.093094	-.003023	-.004906
	.107736	-.000623	-.003654
	.107364	-.001984	-.003200
	.107226	-.002936	-.003484
	.107162	-.003271	-.003369
	.115631	-.000678	-.001471
	.115429	-.002111	-.001152
	.115310	-.003061	-.001424
	.115407	-.003424	-.001199
	.117031	-.000682	-.000328
	.116776	-.002125	.000057
	.116686	-.003066	-.000301
	.116736	-.003462	.000174



Statistika Portal (8 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.010245	-.000121	-.005425
	.010284	-.000415	-.004734
	.010256	-.000732	-.005090
	.010230	-.000826	-.004978
	.030437	-.000229	-.006960
	.030325	-.000778	-.006280
	.030279	-.001365	-.006556
	.030224	-.001541	-.006582
	.052836	-.000337	-.007033
	.052647	-.001097	-.006360
	.052568	-.001892	-.006649
	.052515	-.002132	-.006659
	.074182	-.000447	-.006403
	.073923	-.001370	-.005767
	.073820	-.002314	-.006054
	.073756	-.002597	-.006027
	.092764	-.000551	-.005368
	.092439	-.001595	-.004787
	.092310	-.002634	-.005072
	.092230	-.002942	-.004990
	.107549	-.000641	-.004088
	.107157	-.001766	-.003571
	.107001	-.002857	-.003858
	.106906	-.003178	-.003711
	.117981	-.000705	-.002720
	.117566	-.001876	-.002322
	.117391	-.002990	-.002607
	.117276	-.003315	-.002382
	.124466	-.000728	-.001866
	.124031	-.001920	-.001338
	.123904	-.003040	-.001699
	.123719	-.003367	-.001408

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STRUCTURAL ANALYSIS PROGRAMS

VERSION 5.20

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sa Statis Portal (4 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
:	.000000	.000000	.000000
:	.000000	.000000	.000000
:	.010920	.001528	-.004298
:	.014625	-.000958	-.003457
:	.020393	-.002187	-.002679
:	.028797	.002544	-.004213
3	.031007	-.001631	-.002369
2	.034150	-.003318	-.000568
1	.042229	.002886	-.002436
1	.039917	-.001940	-.000968
2	.040484	-.003790	.000520
3	.042852	.002821	-.000556
4	.042452	-.002066	-.000371
5	.042336	-.003906	.000320

sa Statis Portal (4 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.094476	.001094	-.015394
	.097079	-.001688	-.014591
	.101310	-.002625	-.013778
	.114568	.002284	-.002234
1	.117134	-.002088	-.000599
2	.120961	-.003684	.001003
3	.127974	.002596	-.002857
4	.125701	-.002441	-.001287
5	.126225	-.004173	.000338
6	.128762	.002531	-.000512
7	.128349	-.002554	-.000326
8	.128240	-.004288	.000333

sa Statis Portal (4 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

U(X)	U(Y)	R(Z)
.000000	.000000	.000000
.000000	.000000	.000000
.000000	.000000	.000000
.161980	.000908	-.041845
.160302	-.001514	-.041117
.160011	-.002422	-.040105
.356645	.001533	-.019836
.356410	-.002701	-.018987
.358615	-.004150	-.017911
.372993	.002200	.000320
.371636	-.002672	.001548
.373302	-.004492	.003097
.374034	.002116	-.001141
.373652	-.002889	-.000765
.373418	-.004621	.000104

a Statis Portal (4 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

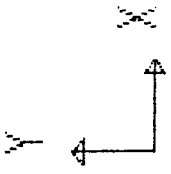
U(X)	U(Y)	R(Z)
.000000	.000000	.000000
.000000	.000000	.000000
.000000	.000000	.000000
.222114	.002078	-.061725
.220299	-.001366	-.061137
.219862	-.003542	-.059464
.584035	.003634	-.063654
.583602	-.002401	-.063619
.583542	-.006154	-.062685
.859410	.004645	-.026308
.858011	-.003114	-.025191
.860385	-.007804	-.023657
.878611	.005627	.001330
.877791	-.002480	.001812
.878761	-.007810	.002522

sa Statis Portal (4 lantai) Kg/m

N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

	U(X)	U(Y)	R(Z)
	.000000	.000000	.000000
	.000000	.000000	.000000
	.000000	.000000	.000000
	.178006	.000845	-.049117
	.176110	-.001173	-.048039
	.175647	-.002316	-.046474
	.457046	.001321	-.047003
	.452896	-.002014	-.046148
	.451533	-.003858	-.044687
	.660380	.001452	-.025134
	.654492	-.002525	-.024873
	.652656	-.004648	-.023920
	.728209	.001499	-.004015
	.727777	-.002711	-.004550
	.727518	-.004938	-.003664



B-P
UNDEFORMED
SHAPE

OPTIONS
JOINT IDS
ELEMENT IDS
WIRE FRAME

SAP90

49	53	89	54	90	55	91	52
45	48	86	50	87	51	88	52
45	45	83	46	84	47	85	48
41	41	82	42	81	43	82	44
37	37	77	38	78	39	79	40
33	33	74	34	75	35	76	36
29	29	71	30	72	31	73	32
25	25	68	26	69	27	70	28
21	21	65	22	66	23	67	24
17	17	62	18	63	19	64	20
13	13	59	14	60	15	61	16
9	9	56	10	57	11	58	12
5	5	53	6	54	7	55	8
1	1	50	2	51	3	52	4

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ

1	56984.77					
		.0	5450.63	-34238.32		
		1.0	5450.63	-28787.69		
		2.0	5450.63	-23337.06		
		3.0	5450.63	-17886.42		
		4.0	5450.63	-12435.79		
3	-----					
1	42781.04					
		.0	1313.76	-9415.44		
		1.0	1313.76	-8101.68		
		2.0	1313.76	-6787.93		
		3.0	1313.76	-5474.17		
		4.0	1313.76	-4160.41		
9	-----					
1	16843.36					
		.0	501.47	-3503.39		
		1.0	501.47	-3001.93		
		2.0	501.47	-2500.46		
		3.0	501.47	-1999.00		
		4.0	501.47	-1497.53		
3	-----					
1	-8036.64					
		.0	-522.11	359.57		
		1.0	-522.11	-162.54		
		2.0	-522.11	-684.65		
		3.0	-522.11	-1206.76		
		4.0	-522.11	-1728.87		
7	-----					
1	-29193.87					
		.0	-1271.10	2637.13		
		1.0	-1271.10	1366.03		
		2.0	-1271.10	94.93		
		3.0	-1271.10	-1176.16		
		4.0	-1271.10	-2447.26		
1	-----					
1	-46074.70					
		.0	-1963.20	4613.65		
		1.0	-1963.20	2650.45		
		2.0	-1963.20	687.25		
		3.0	-1963.20	-1275.95		
		4.0	-1963.20	-3239.15		
5	-----					
1	-58499.81					
		.0	-2683.68	6615.48		
		1.0	-2683.68	3931.80		
		2.0	-2683.68	1248.11		
		3.0	-2683.68	-1435.57		
		4.0	-2683.68	-4119.26		
9	-----					
1	-66216.19					
		.0	-3448.58	8674.16		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORG
		1.0	-3448.58	5225.57			
		2.0	-3448.58	1776.99			
		3.0	-3448.58	-1671.59			
		4.0	-3448.58	-5120.18			

	1-68897.62	.0	-4265.75	10799.85			
		1.0	-4265.75	6534.10			
		2.0	-4265.75	2268.35			
		3.0	-4265.75	-1997.41			
		4.0	-4265.75	-6263.16			

	1-66151.45	.0	-5143.80	13009.38			
		1.0	-5143.80	7865.58			
		2.0	-5143.80	2721.78			
		3.0	-5143.80	-2422.02			
		4.0	-5143.80	-7565.82			

	1-57518.56	.0	-6088.47	15325.73			
		1.0	-6088.47	9237.26			
		2.0	-6088.47	3148.79			
		3.0	-6088.47	-2939.68			
		4.0	-6088.47	-9028.15			

	1-42451.65	.0	-7059.86	17814.51			
		1.0	-7059.86	10754.65			
		2.0	-7059.86	3694.79			
		3.0	-7059.86	-3365.07			
		4.0	-7059.86	-10424.93			

	1-20226.74	.0	-10501.64	20746.29			
		1.0	-10501.64	10244.65			
		2.0	-10501.64	-256.98			
		3.0	-10501.64	-10758.62			
		4.0	-10501.64	-21260.25			

	1*****	.0	11797.85	-48654.21			
		1.0	11797.85	-36856.36			
		2.0	11797.85	-25058.50			
		3.0	11797.85	-13260.65			
		4.0	11797.85	-1462.79			

	1*****	.0	3289.83	-10329.15			
		1.0	3289.83	-7039.31			
		2.0	3289.83	-3749.48			
		3.0	3289.83	-459.64			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD) COMB	AXIAL FORCE ENDI	DIST 4.0	1-2 PLANE		1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1*****							
	.0		3850.16	-9630.17			
	1.0		3850.16	-5780.01			
	2.0		3850.16	-1929.85			
	3.0		3850.16	1920.31			
	4.0		3850.16	5770.48			
4	-----						
1*****							
	.0		2936.11	-6395.96			
	1.0		2936.11	-3459.85			
	2.0		2936.11	-523.73			
	3.0		2936.11	2412.38			
	4.0		2936.11	5348.49			
8	-----						
1*****							
	.0		2226.88	-4387.73			
	1.0		2226.88	-2160.85			
	2.0		2226.88	66.04			
	3.0		2226.88	2292.92			
	4.0		2226.88	4519.80			
2	-----						
1*****							
	.0		1374.04	-2127.55			
	1.0		1374.04	-753.52			
	2.0		1374.04	620.52			
	3.0		1374.04	1994.56			
	4.0		1374.04	3368.60			
16	-----						
1*****							
	.0		370.11	408.12			
	1.0		370.11	778.23			
	2.0		370.11	1148.35			
	3.0		370.11	1518.46			
	4.0		370.11	1888.57			
30	-----						
1*****							
	.0		-783.84	3212.83			
	1.0		-783.84	2428.99			
	2.0		-783.84	1645.15			
	3.0		-783.84	861.30			
	4.0		-783.84	77.46			
54	-----						
1*****							
	.0		-2085.96	6280.72			
	1.0		-2085.96	4194.76			
	2.0		-2085.96	2108.80			
	3.0		-2085.96	22.85			
	4.0		-2085.96	-2063.11			
58	-----						
1-92060.27							

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3532.80	9604.83			
		1.0	-3532.80	6072.04			
		2.0	-3532.80	2539.24			
		3.0	-3532.80	-993.56			
		4.0	-3532.80	-4526.35			

	1-64373.96						
		.0	-5113.27	13187.59			
		1.0	-5113.27	8074.32			
		2.0	-5113.27	2961.04			
		3.0	-5113.27	-2152.23			
		4.0	-5113.27	-7265.51			

	1-39727.02						
		.0	-7041.53	17086.48			
		1.0	-7041.53	10044.95			
		2.0	-7041.53	3003.42			
		3.0	-7041.53	-4038.11			
		4.0	-7041.53	-11079.63			

	1-16446.49						
		.0	-10102.15	20252.63			
		1.0	-10102.15	10150.48			
		2.0	-10102.15	48.33			
		3.0	-10102.15	-10053.82			
		4.0	-10102.15	-20155.96			

	1*****						
		.0	31420.11	-84796.92			
		1.0	31420.11	-53376.82			
		2.0	31420.11	-21956.71			
		3.0	31420.11	9463.39			
		4.0	31420.11	40883.50			

	1*****						
		.0	21266.33	-43788.15			
		1.0	21266.33	-22521.82			
		2.0	21266.33	-1255.49			
		3.0	21266.33	20010.84			
		4.0	21266.33	41277.16			

	1*****						
		.0	21047.99	-42600.84			
		1.0	21047.99	-21552.84			
		2.0	21047.99	-504.85			
		3.0	21047.99	20543.15			
		4.0	21047.99	41591.14			

	1*****						
		.0	20973.82	-42105.22			
		1.0	20973.82	-21131.41			
		2.0	20973.82	-157.59			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
			SHEAR	MOMENT			
)		3.0	20973.82	20816.22			
		4.0	20973.82	41790.04			

1*****							
3		.0	20658.19	-40957.53			
		1.0	20658.19	-20299.34			
		2.0	20658.19	358.85			
		3.0	20658.19	21017.04			
		4.0	20658.19	41675.23			

1*****							
7		.0	20096.03	-39301.32			
		1.0	20096.03	-19205.29			
		2.0	20096.03	890.74			
		3.0	20096.03	20986.77			
		4.0	20096.03	41082.80			

1*****							
1		.0	19337.94	-37290.15			
		1.0	19337.94	-17952.21			
		2.0	19337.94	1385.73			
		3.0	19337.94	20723.68			
		4.0	19337.94	40061.62			

1*****							
5		.0	18384.65	-34917.29			
		1.0	18384.65	-16532.65			
		2.0	18384.65	1852.00			
		3.0	18384.65	20236.65			
		4.0	18384.65	38621.30			

1*****							
9		.0	17241.30	-32198.66			
		1.0	17241.30	-14957.35			
		2.0	17241.30	2283.95			
		3.0	17241.30	19525.25			
		4.0	17241.30	36766.56			

1*****							
3		.0	15922.13	-29157.20			
		1.0	15922.13	-13235.07			
		2.0	15922.13	2687.06			
		3.0	15922.13	18609.19			
		4.0	15922.13	34531.32			

1*****							
3		.0	14370.08	-25781.85			
		1.0	14370.08	-11411.77			
		2.0	14370.08	2958.30			
		3.0	14370.08	17328.38			
		4.0	14370.08	31698.45			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD) COMB	AXIAL FORCE ENDI	DIST	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

	1-72460.85						
	.0		13116.53	-22381.35			
	1.0		13116.53	-9264.82			
	2.0		13116.53	3851.71			
	3.0		13116.53	16968.25			
	4.0		13116.53	30084.78			
1	-----						
	1-26291.49						
	.0		10402.59	-16061.06			
	1.0		10402.59	-5658.47			
	2.0		10402.59	4744.11			
	3.0		10402.59	15146.70			
	4.0		10402.59	25549.28			
4	-----						
	1*****						
	.0		34953.18	-99476.17			
	1.0		34953.18	-64522.99			
	2.0		34953.18	-29569.81			
	3.0		34953.18	5383.37			
	4.0		34953.18	40336.54			
8	-----						
	1*****						
	.0		11041.29	-18176.96			
	1.0		11041.29	-7135.66			
	2.0		11041.29	3905.63			
	3.0		11041.29	14946.92			
	4.0		11041.29	25988.21			
2	-----						
	1*****						
	.0		13419.73	-27581.70			
	1.0		13419.73	-14161.98			
	2.0		13419.73	-742.25			
	3.0		13419.73	12677.48			
	4.0		13419.73	26097.21			
6	-----						
	1*****						
	.0		13257.46	-26670.28			
	1.0		13257.46	-13412.82			
	2.0		13257.46	-155.37			
	3.0		13257.46	13102.09			
	4.0		13257.46	26359.55			
0	-----						
	1*****						
	.0		12945.66	-25418.21			
	1.0		12945.66	-12472.55			
	2.0		12945.66	473.11			
	3.0		12945.66	13418.77			
	4.0		12945.66	26364.43			
4	-----						
	1*****						
	.0		12565.87	-24088.04			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	12565.87	-11522.16			
		2.0	12565.87	1043.71			
		3.0	12565.87	13609.58			
		4.0	12565.87	26175.45			

		.0	12097.69	-22610.80			
		1.0	12097.69	-10513.11			
		2.0	12097.69	1584.59			
		3.0	12097.69	13682.28			
		4.0	12097.69	25779.97			

		.0	11539.14	-20986.49			
		1.0	11539.14	-9447.35			
		2.0	11539.14	2091.79			
		3.0	11539.14	13630.93			
		4.0	11539.14	25170.07			

		.0	10882.90	-19204.56			
		1.0	10882.90	-8321.67			
		2.0	10882.90	2561.23			
		3.0	10882.90	13444.13			
		4.0	10882.90	24327.02			

		.0	10114.86	-17264.03			
		1.0	10114.86	-7149.17			
		2.0	10114.86	2965.70			
		3.0	10114.86	13080.56			
		4.0	10114.86	23195.42			

		.0	9336.31	-15213.32			
		1.0	9336.31	-5877.02			
		2.0	9336.31	3459.29			
		3.0	9336.31	12795.59			
		4.0	9336.31	22131.90			

		.0	8560.80	-12589.46			
		1.0	8560.80	-4028.65			
		2.0	8560.80	4532.15			
		3.0	8560.80	13092.96			
		4.0	8560.80	21653.76			

		.0	5812.70	-8035.73			
		1.0	5812.70	-2223.03			
		2.0	5812.70	3589.66			
		3.0	5812.70	9402.36			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR 5812.70	MOMENT 15215.06	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	53285.20	.0	12260.52	-3131.51			
		2.0	5120.52	14249.53			
		4.0	-10219.48	17350.58			
		6.0	-17359.48	-10228.38			
		8.0	-24499.48	-52087.34			

1	68816.25	.0	11848.96	-833.20			
		2.0	4708.96	15724.71			
		4.0	-10631.04	18002.63			
		6.0	-17771.04	-10399.45			
		8.0	-24911.04	-53081.54			

1	63830.52	.0	12177.93	-2026.65			
		2.0	5037.93	15189.22			
		4.0	-10302.07	18125.08			
		6.0	-17442.07	-9619.05			
		8.0	-24582.07	-51643.18			

1	53638.18	.0	12810.10	-4520.80			
		2.0	5670.10	13959.41			
		4.0	-9669.90	18159.62			
		6.0	-16809.90	-8320.18			
		8.0	-23949.90	-49079.97			

1	42630.38	.0	13477.86	-7198.02			
		2.0	6337.86	12617.70			
		4.0	-9002.14	18153.42			
		6.0	-16142.14	-6990.87			
		8.0	-23282.14	-46415.15			

1	31393.56	.0	14168.19	-9973.67			
		2.0	7028.19	11222.72			
		4.0	-8311.81	18139.10			
		6.0	-15451.81	-5624.52			
		8.0	-22591.81	-43668.14			

1	19735.74	.0	14892.85	-12893.73			
		2.0	7752.85	9751.98			
		4.0	-7587.15	18117.69			
		6.0	-14727.15	-4196.60			
		8.0	-21867.15	-40790.90			

1	7524.81						

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR MOMENT		1-3 PLANE SHEAR MOMENT		AXIAL TORQ
		.0	15662.52	-16000.68			
		2.0	8522.52	8184.36			
		4.0	-6817.48	18089.39			
		6.0	-13957.48	-2685.57			
		8.0	-21097.48	-37740.53			
7 -----							
1	-5353.87						
		.0	16486.87	-19332.33			
		2.0	9346.87	6501.41			
		4.0	-5993.13	18055.15			
		6.0	-13133.13	-1071.10			
		8.0	-20273.13	-34477.36			
) -----							
1	-19013.21						
		.0	17376.26	-22929.05			
		2.0	10236.26	4683.46			
		4.0	-5103.74	18015.98			
		6.0	-12243.74	668.50			
		8.0	-19383.74	-30958.98			
3 -----							
1	-33642.58						
		.0	18347.95	-26856.10			
		2.0	11207.95	2699.80			
		4.0	-4132.05	17975.69			
		6.0	-11272.05	2571.58			
		8.0	-18412.05	-27112.52			
5 -----							
1	-47141.64						
		.0	19386.24	-31157.62			
		2.0	12246.24	474.85			
		4.0	-3093.76	17827.32			
		6.0	-10233.76	4499.79			
		8.0	-17373.76	-23107.73			
4 -----							
1	64626.87						
		.0	25962.85	-43520.03			
		3.0	15402.85	18528.54			
		6.0	-3357.15	32497.10			
		9.0	-22117.15	-1614.34			
		12.0	-32677.15	-83805.77			
7 -----							
1	92416.09						
		.0	25600.71	-40990.37			
		3.0	15040.71	19971.77			
		6.0	-3719.29	32853.91			
		9.0	-22479.29	-2343.95			
		12.0	-33039.29	-85621.82			
) -----							
1	95954.88						
		.0	25444.01	-39846.26			
		3.0	14884.01	20645.76			
		6.0	-3875.99	33057.78			

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I M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		9.0	-22635.99	-2610.20			
		12.0	-33195.99	-86358.18			

3	1	93362.56					
		.0	25428.07	-39698.28			
		3.0	14868.07	20745.94			
		6.0	-3891.93	33110.16			
		9.0	-22651.93	-2605.62			
		12.0	-33211.93	-86401.40			

6	1	88865.08					
		.0	25502.43	-40102.55			
		3.0	14942.43	20564.75			
		6.0	-3817.57	33152.05			
		9.0	-22577.57	-2340.64			
		12.0	-33137.57	-85913.34			

9	1	82550.83					
		.0	25661.39	-41017.62			
		3.0	15101.39	20126.55			
		6.0	-3658.61	33190.71			
		9.0	-22418.61	-1825.12			
		12.0	-32978.61	-84920.95			

2	1	74531.47					
		.0	25896.66	-42395.28			
		3.0	15336.66	19454.70			
		6.0	-3423.34	33224.68			
		9.0	-22183.34	-1085.35			
		12.0	-32743.34	-83475.37			

5	1	64959.78					
		.0	26200.58	-44189.18			
		3.0	15640.58	18572.54			
		6.0	-3119.42	33254.27			
		9.0	-21879.42	-144.00			
		12.0	-32439.42	-81622.28			

8	1	53996.22					
		.0	26565.09	-46351.21			
		3.0	16005.09	17504.06			
		6.0	-2754.91	33279.33			
		9.0	-21514.91	974.60			
		12.0	-32074.91	-79410.13			

11	1	41828.65					
		.0	26982.53	-48834.72			
		3.0	16422.53	16272.88			
		6.0	-2337.47	33300.48			
		9.0	-21097.47	2248.07			
		12.0	-31657.47	-76884.33			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD , COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	29000.44						
		.0	27439.51	-51576.98			
		3.0	16879.51	14901.55			
		6.0	-1880.49	33300.08			
		9.0	-20640.49	3618.62			
		12.0	-31200.49	-74142.85			

7	1 14430.20						
		.0	27949.16	-54513.51			
		3.0	17389.16	13493.97			
		6.0	-1370.84	33421.45			
		9.0	-20130.84	5268.93			
		12.0	-30690.84	-70963.59			

5	1 83215.83						
		.0	11022.35	556.81			
		2.0	3882.35	15461.52			
		4.0	-11457.65	16086.22			
		6.0	-18597.65	-13969.08			
		8.0	-25737.65	-58304.38			

8	1100692.64						
		.0	11972.59	-2136.54			
		2.0	4832.59	14668.63			
		4.0	-10507.41	17193.80			
		6.0	-17647.41	-10961.03			
		8.0	-24787.41	-53395.86			

.1	1 94677.84						
		.0	12184.08	-3033.45			
		2.0	5044.08	14194.70			
		4.0	-10295.92	17142.86			
		6.0	-17435.92	-10588.99			
		8.0	-24575.92	-52600.84			

.4	1 90008.35						
		.0	12429.17	-4013.34			
		2.0	5289.17	13704.99			
		4.0	-10050.83	17143.33			
		6.0	-17190.83	-10098.33			
		8.0	-24330.83	-51619.99			

.57	1 84425.22						
		.0	12751.62	-5278.62			
		2.0	5611.62	13084.62			
		4.0	-9728.38	17167.85			
		6.0	-16868.38	-9428.91			
		8.0	-24008.38	-50305.67			

.70	1 77824.50						
		.0	13156.73	-6866.24			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		2.0	6016.73	12307.22		
		4.0	-9323.27	17200.68		
		6.0	-16463.27	-8585.86		
		8.0	-23603.27	-48652.40		

1	70193.50	.0	13647.26	-8785.67		
		2.0	6507.26	11368.84		
		4.0	-8832.74	17243.35		
		6.0	-15972.74	-7562.13		
		8.0	-23112.74	-46647.62		

1	61484.81	.0	14228.05	-11057.40		
		2.0	7088.05	10258.71		
		4.0	-8251.95	17294.81		
		6.0	-15391.95	-6349.08		
		8.0	-22531.95	-44272.97		

1	51641.03	.0	14904.20	-13702.50		
		2.0	7764.20	8965.91		
		4.0	-7575.80	17354.32		
		6.0	-14715.80	-4937.27		
		8.0	-21855.80	-41508.86		

1	40747.43	.0	15679.45	-16743.68		
		2.0	8539.45	7475.22		
		4.0	-6800.55	17414.11		
		6.0	-13940.55	-3326.99		
		8.0	-21080.55	-38348.09		

1	28385.10	.0	16567.80	-20189.59		
		2.0	9427.80	5806.02		
		4.0	-5912.20	17521.63		
		6.0	-13052.20	-1442.76		
		8.0	-20192.20	-34687.15		

1	9485.00	.0	17780.69	-24895.09		
		2.0	10640.69	3526.28		
		4.0	-4699.31	17667.65		
		6.0	-11839.31	1129.02		
		8.0	-18979.31	-29689.61		

1	55166.00	.0	12663.71	-21242.01		
		2.0	8283.71	-294.58		
		4.0	-1976.29	11892.84		
		6.0	-6356.29	3560.27		

sa Statis Portal (13 lantai) Kg/m

TIME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT
1	-5812.70	8.0	-10736.29	-13532.31		
		.0	12361.26	-20505.13		
		2.0	7981.26	-162.62		
		4.0	-2278.74	11419.90		
		6.0	-6658.74	2482.42		
		8.0	-11038.74	-15215.06		
0	1-28448.39					
		.0	17995.11	-33770.72		
		3.0	11425.11	10359.61		
		6.0	-1024.89	23019.93		
		9.0	-13474.89	4210.25		
		12.0	-20044.89	-46069.43		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
COMB			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	42118.07						
		.0	61816.12	-171863.76			
		1.0	61816.12	-110047.64			
		2.0	61816.12	-48231.52			
		3.0	61816.12	13584.60			
		4.0	61816.12	75400.72			

5	47035.66						
		.0	-14579.05	48642.70			
		1.0	-14579.05	34063.66			
		2.0	-14579.05	19484.61			
		3.0	-14579.05	4905.56			
		4.0	-14579.05	-9673.49			

9	13211.74						
		.0	1532.31	-7106.65			
		1.0	1532.31	-5574.34			
		2.0	1532.31	-4042.03			
		3.0	1532.31	-2509.73			
		4.0	1532.31	-977.42			

3	11023.57						
		.0	-650.91	835.83			
		1.0	-650.91	184.91			
		2.0	-650.91	-466.00			
		3.0	-650.91	-1116.91			
		4.0	-650.91	-1767.82			

7	31916.43						
		.0	-1339.23	2808.71			
		1.0	-1339.23	1469.48			
		2.0	-1339.23	130.26			
		3.0	-1339.23	-1208.97			
		4.0	-1339.23	-2548.19			

1	48390.18						
		.0	-2025.74	4743.44			
		1.0	-2025.74	2717.70			
		2.0	-2025.74	691.96			
		3.0	-2025.74	-1333.77			
		4.0	-2025.74	-3359.51			

5	60424.34						
		.0	-2736.04	6713.37			
		1.0	-2736.04	3977.33			
		2.0	-2736.04	1241.29			
		3.0	-2736.04	-1494.76			
		4.0	-2736.04	-4230.80			

9	67777.20						
		.0	-3496.29	8761.26			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		1.0	-3496.29	5264.97		
		2.0	-3496.29	1768.68		
		3.0	-3496.29	-1727.62		
		4.0	-3496.29	-5223.91		

	1-70122.34	.0	-4308.99	10875.91		
		1.0	-4308.99	6566.91		
		2.0	-4308.99	2257.92		
		3.0	-4308.99	-2051.07		
		4.0	-4308.99	-6360.07		

7	1-67067.46	.0	-5181.78	13073.14		
		1.0	-5181.78	7891.36		
		2.0	-5181.78	2709.57		
		3.0	-5181.78	-2472.21		
		4.0	-5181.78	-7654.00		

1	1-58154.58	.0	-6122.55	15378.63		
		1.0	-6122.55	9256.07		
		2.0	-6122.55	3133.52		
		3.0	-6122.55	-2989.03		
		4.0	-6122.55	-9111.58		

5	1-42837.38	.0	-7087.08	17851.65		
		1.0	-7087.08	10764.56		
		2.0	-7087.08	3677.48		
		3.0	-7087.08	-3409.60		
		4.0	-7087.08	-10496.69		

9	1-20401.21	.0	-10527.84	20764.53		
		1.0	-10527.84	10236.68		
		2.0	-10527.84	-291.16		
		3.0	-10527.84	-10819.01		
		4.0	-10527.84	-21346.85		

2	1*****	.0	74448.65	-196437.46		
		1.0	74448.65	-121988.81		
		2.0	74448.65	-47540.17		
		3.0	74448.65	26908.48		
		4.0	74448.65	101357.13		

6	1*****	.0	-12553.70	45303.62		
		1.0	-12553.70	32749.92		
		2.0	-12553.70	20196.22		
		3.0	-12553.70	7642.51		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		4.0	-12553.70	-4911.19			

	1*****	.0	4928.23	-13471.32			
		1.0	4928.23	-8543.09			
		2.0	4928.23	-3614.86			
		3.0	4928.23	1313.38			
		4.0	4928.23	6241.61			

	1*****	.0	2624.80	-5541.30			
		1.0	2624.80	-2916.49			
		2.0	2624.80	-291.69			
		3.0	2624.80	2333.11			
		4.0	2624.80	4957.91			

	1*****	.0	1992.29	-3897.98			
		1.0	1992.29	-1905.69			
		2.0	1992.29	86.59			
		3.0	1992.29	2078.88			
		4.0	1992.29	4071.16			

	1*****	.0	1154.03	-1691.22			
		1.0	1154.03	-537.18			
		2.0	1154.03	616.85			
		3.0	1154.03	1770.89			
		4.0	1154.03	2924.92			

	1*****	.0	156.26	830.40			
		1.0	156.26	986.66			
		2.0	156.26	1142.92			
		3.0	156.26	1299.18			
		4.0	156.26	1455.44			

	1*****	.0	-991.33	3620.34			
		1.0	-991.33	2629.01			
		2.0	-991.33	1637.68			
		3.0	-991.33	646.35			
		4.0	-991.33	-344.98			

	1*****	.0	-2286.48	6672.22			
		1.0	-2286.48	4385.74			
		2.0	-2286.48	2099.25			
		3.0	-2286.48	-187.23			
		4.0	-2286.48	-2473.71			

	1-92554.38						

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3726.00	9979.49			
		1.0	-3726.00	6253.49			
		2.0	-3726.00	2527.49			
		3.0	-3726.00	-1198.51			
		4.0	-3726.00	-4924.51			

	1-64725.99						
		.0	-5296.91	13543.68			
		1.0	-5296.91	8246.77			
		2.0	-5296.91	2949.87			
		3.0	-5296.91	-2347.04			
		4.0	-5296.91	-7643.95			

	1-39918.35						
		.0	-7231.07	17431.67			
		1.0	-7231.07	10200.59			
		2.0	-7231.07	2969.52			
		3.0	-7231.07	-4261.55			
		4.0	-7231.07	-11492.63			

	1-16448.97						
		.0	-10196.45	20507.99			
		1.0	-10196.45	10311.54			
		2.0	-10196.45	115.08			
		3.0	-10196.45	-10081.37			
		4.0	-10196.45	-20277.82			

	1*****						
		.0	94997.89	-234293.18			
		1.0	94997.89	-139295.29			
		2.0	94997.89	-44297.39			
		3.0	94997.89	50700.50			
		4.0	94997.89	145698.39			

	1*****						
		.0	5302.22	12260.65			
		1.0	5302.22	17562.87			
		2.0	5302.22	22865.09			
		3.0	5302.22	28167.32			
		4.0	5302.22	33469.54			

	1*****						
		.0	22795.66	-48022.87			
		1.0	22795.66	-25227.22			
		2.0	22795.66	-2431.56			
		3.0	22795.66	20364.10			
		4.0	22795.66	43159.75			

	1*****						
		.0	20786.99	-41358.67			
		1.0	20786.99	-20571.68			
		2.0	20786.99	215.30			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		3.0	20786.99	21002.29		
		4.0	20786.99	41789.28		

	1*****					
		.0	20818.27	-41350.96		
		1.0	20818.27	-20532.69		
		2.0	20818.27	285.59		
		3.0	20818.27	21103.86		
		4.0	20818.27	41922.14		
5	-----					
	1*****					
		.0	20209.88	-39527.33		
		1.0	20209.88	-19317.45		
		2.0	20209.88	892.43		
		3.0	20209.88	21102.31		
		4.0	20209.88	41312.20		
7	-----					
	1*****					
		.0	19458.08	-37537.40		
		1.0	19458.08	-18079.32		
		2.0	19458.08	1378.76		
		3.0	19458.08	20836.84		
		4.0	19458.08	40294.93		
1	-----					
	1*****					
		.0	18509.03	-35174.99		
		1.0	18509.03	-16665.96		
		2.0	18509.03	1843.06		
		3.0	18509.03	20352.09		
		4.0	18509.03	38861.12		
5	-----					
	1*****					
		.0	17370.73	-32468.28		
		1.0	17370.73	-15097.55		
		2.0	17370.73	2273.19		
		3.0	17370.73	19643.92		
		4.0	17370.73	37014.66		
9	-----					
	1*****					
		.0	16057.68	-29440.64		
		1.0	16057.68	-13382.96		
		2.0	16057.68	2674.72		
		3.0	16057.68	18732.40		
		4.0	16057.68	34790.07		
3	-----					
	1*****					
		.0	14511.67	-26080.06		
		1.0	14511.67	-11568.40		
		2.0	14511.67	2943.27		
		3.0	14511.67	17454.93		
		4.0	14511.67	31966.60		

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	

7	1-72438.21						
		.0	13280.52	-22698.60			
		1.0	13280.52	-9418.08			
		2.0	13280.52	3862.43			
		3.0	13280.52	17142.95			
		4.0	13280.52	30423.47			

1	1-26135.74						
		.0	10479.96	-16301.33			
		1.0	10479.96	-5821.37			
		2.0	10479.96	4658.59			
		3.0	10479.96	15138.55			
		4.0	10479.96	25618.51			

4	1*****						
		.0	88578.34	-231790.35			
		1.0	88578.34	-143212.01			
		2.0	88578.34	-54633.67			
		3.0	88578.34	33944.66			
		4.0	88578.34	122523.00			

8	1*****						
		.0	-1934.36	33445.44			
		1.0	-1934.36	31511.08			
		2.0	-1934.36	29576.72			
		3.0	-1934.36	27642.36			
		4.0	-1934.36	25708.00			

2	1*****						
		.0	12942.29	-26767.24			
		1.0	12942.29	-13824.96			
		2.0	12942.29	-882.67			
		3.0	12942.29	12059.61			
		4.0	12942.29	25001.90			

6	1*****						
		.0	13520.91	-27594.10			
		1.0	13520.91	-14073.19			
		2.0	13520.91	-552.28			
		3.0	13520.91	12968.64			
		4.0	13520.91	26489.55			

20	1*****						
		.0	12911.54	-25301.16			
		1.0	12911.54	-12389.61			
		2.0	12911.54	521.93			
		3.0	12911.54	13433.48			
		4.0	12911.54	26345.02			

24	1*****						
		.0	12570.90	-24109.47			

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M E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	12570.90	-11538.57			
		2.0	12570.90	1032.33			
		3.0	12570.90	13603.22			
		4.0	12570.90	26174.12			
3	1*****	.0	12100.05	-22622.59			
		1.0	12100.05	-10522.54			
		2.0	12100.05	1577.51			
		3.0	12100.05	13677.56			
		4.0	12100.05	25777.60			
2	1*****	.0	11544.30	-21006.89			
		1.0	11544.30	-9462.59			
		2.0	11544.30	2081.71			
		3.0	11544.30	13626.02			
		4.0	11544.30	25170.32			
6	1*****	.0	10890.87	-19232.66			
		1.0	10890.87	-8341.79			
		2.0	10890.87	2549.09			
		3.0	10890.87	13439.96			
		4.0	10890.87	24330.83			
0	1*****	.0	10126.42	-17300.95			
		1.0	10126.42	-7174.54			
		2.0	10126.42	2951.88			
		3.0	10126.42	13078.30			
		4.0	10126.42	23204.72			
.4	1-71218.05	.0	9353.11	-15259.05			
		1.0	9353.11	-5905.94			
		2.0	9353.11	3447.17			
		3.0	9353.11	12800.28			
		4.0	9353.11	22153.38			
18	1-36296.71	.0	8584.11	-12637.02			
		1.0	8584.11	-4052.91			
		2.0	8584.11	4531.19			
		3.0	8584.11	13115.30			
		4.0	8584.11	21699.41			
12	1-11059.89	.0	5835.52	-8053.80			
		1.0	5835.52	-2218.29			
		2.0	5835.52	3617.23			
		3.0	5835.52	9452.75			

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M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		4.0	5835.52	15288.26			
3	1	72915.17					
		.0	4917.59	26758.02			
		2.0	-2222.41	29453.19			
		4.0	-17562.41	17868.37			
		6.0	-24702.41	-24396.46			
		8.0	-31842.41	-80941.29			
6	1	68643.41					
		.0	12343.40	-2792.53			
		2.0	5203.40	14754.27			
		4.0	-10136.60	18021.08			
		6.0	-17276.60	-9392.12			
		8.0	-24416.60	-51085.31			
9	1	63721.90					
		.0	12162.89	-1978.82			
		2.0	5022.89	15206.97			
		4.0	-10317.11	18112.75			
		6.0	-17457.11	-9661.47			
		8.0	-24597.11	-51715.68			
12	1	53213.10					
		.0	12860.24	-4730.78			
		2.0	5720.24	13849.69			
		4.0	-9619.76	18150.16			
		6.0	-16759.76	-8229.37			
		8.0	-23899.76	-48888.90			
15	1	41999.61					
		.0	13532.08	-7427.54			
		2.0	6392.08	12496.62			
		4.0	-8947.92	18140.79			
		6.0	-16087.92	-6895.05			
		8.0	-23227.92	-46210.89			
18	1	30799.35					
		.0	14218.85	-10190.73			
		2.0	7078.85	11106.97			
		4.0	-8261.15	18124.67			
		6.0	-15401.15	-5537.64			
		8.0	-22541.15	-43479.94			
21	1	19209.25					
		.0	14938.71	-13091.29			
		2.0	7798.71	9646.13			
		4.0	-7541.29	18103.56			
		6.0	-14681.29	-4119.02			
		8.0	-21821.29	-40621.59			
24	1	7058.63					

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	15703.73	-16179.49			
		2.0	8563.73	8087.98			
		4.0	-6776.27	18075.44			
		6.0	-13916.27	-2617.09			
		8.0	-21056.27	-37589.62			
7	1	-5758.11					
		.0	16523.38	-19492.13			
		2.0	9383.38	6414.63			
		4.0	-5956.62	18041.39			
		6.0	-13096.62	-1011.84			
		8.0	-20236.62	-34345.08			
0	1	-19353.43					
		.0	17407.92	-23069.35			
		2.0	10267.92	4606.49			
		4.0	-5072.08	18002.32			
		6.0	-12212.08	718.15			
		8.0	-19352.08	-30846.01			
3	1	-33920.16					
		.0	18374.73	-26976.01			
		2.0	11234.73	2633.45			
		4.0	-4105.27	17962.91			
		6.0	-11245.27	2612.36			
		8.0	-18385.27	-27018.18			
16	1	-47335.38					
		.0	19404.51	-31247.07			
		2.0	12264.51	421.95			
		4.0	-3075.49	17810.98			
		6.0	-10215.49	4520.00			
		8.0	-17355.49	-23050.97			
54	1	67709.76					
		.0	23107.81	-25169.57			
		3.0	12397.81	28088.85			
		6.0	-6512.19	32817.27			
		9.0	-25422.19	-10984.31			
		12.0	-36132.19	-103315.89			
57	1	92548.57					
		.0	26131.99	-42932.85			
		3.0	15421.99	19398.13			
		6.0	-3488.01	33199.10			
		9.0	-22398.01	-1529.93			
		12.0	-33108.01	-84788.95			
50	1	96767.93					
		.0	25721.26	-40299.16			
		3.0	15011.26	20799.62			
		6.0	-3898.74	33368.39			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
COMB			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		9.0	-22808.74	-2592.83			
		12.0	-33518.74	-87084.06			
3	1 93139.59						
		.0	25747.97	-40386.87			
		3.0	15037.97	20792.03			
		6.0	-3872.03	33440.93			
		9.0	-22782.03	-2440.18			
		12.0	-33492.03	-86851.28			
6	1 88472.79						
		.0	25818.47	-40781.45			
		3.0	15108.47	20608.97			
		6.0	-3801.53	33469.40			
		9.0	-22711.53	-2200.18			
		12.0	-33421.53	-86399.76			
9	1 82211.88						
		.0	25977.12	-41693.87			
		3.0	15267.12	20172.47			
		6.0	-3642.88	33508.82			
		9.0	-22552.88	-1684.83			
		12.0	-33262.88	-85408.48			
12	1 74225.89						
		.0	26211.30	-43065.22			
		3.0	15501.30	19503.67			
		6.0	-3408.70	33542.56			
		9.0	-22318.70	-948.54			
		12.0	-33028.70	-83969.65			
75	1 64694.96						
		.0	26513.82	-44850.98			
		3.0	15803.82	18625.48			
		6.0	-3106.18	33571.95			
		9.0	-22016.18	-11.59			
		12.0	-32726.18	-82125.13			
78	1 53778.63						
		.0	26876.66	-47003.13			
		3.0	16166.66	17561.85			
		6.0	-2743.34	33596.83			
		9.0	-21653.34	1101.81			
		12.0	-32363.34	-79923.20			
31	1 41663.03						
		.0	27292.17	-49475.12			
		3.0	16582.17	16336.39			
		6.0	-2327.83	33617.89			
		9.0	-21237.83	2369.40			
		12.0	-31947.83	-77409.09			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
4	1	28896.06					
		.0	27747.09	-52205.58			
		3.0	17037.09	14970.69			
		6.0	-1872.91	33616.95			
		9.0	-20782.91	3733.22			
		12.0	-31492.91	-74680.52			
7	1	14285.61					
		.0	28254.93	-55124.89			
		3.0	17544.93	13574.90			
		6.0	-1365.07	33744.70			
		9.0	-20275.07	5384.49			
		12.0	-30985.07	-71505.71			
5	1	43300.64					
		.0	3560.74	29818.85			
		2.0	-3579.26	29800.34			
		4.0	-18919.26	15501.82			
		6.0	-26059.26	-29476.70			
		8.0	-33199.26	-88735.21			
8		1110695.10					
		.0	12308.96	-3733.68			
		2.0	5168.96	13744.23			
		4.0	-10171.04	16942.14			
		6.0	-17311.04	-10539.94			
		8.0	-24451.04	-52302.03			
11	1	95355.98					
		.0	12193.48	-2939.38			
		2.0	5053.48	14307.58			
		4.0	-10286.52	17274.54			
		6.0	-17426.52	-10438.50			
		8.0	-24566.52	-52431.55			
34	1	89802.67					
		.0	12434.39	-4067.75			
		2.0	5294.39	13661.03			
		4.0	-10045.61	17109.81			
		6.0	-17185.61	-10121.40			
		8.0	-24325.61	-51632.62			
57	1	84544.47					
		.0	12752.95	-5291.26			
		2.0	5612.95	13074.64			
		4.0	-9727.05	17160.53			
		6.0	-16867.05	-9433.57			
		8.0	-24007.05	-50307.67			
70	1	77921.83					
		.0	13156.67	-6876.12			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
			SHEAR	MOMENT			
		2.0	6016.67	12297.22			
		4.0	-9323.33	17190.56			
		6.0	-16463.33	-8596.11			
		8.0	-23603.33	-48662.77			

1	70319.34	.0	13645.32	-8788.06			
		2.0	6505.32	11362.57			
		4.0	-8834.68	17233.21			
		6.0	-15974.68	-7576.16			
		8.0	-23114.68	-46665.52			

5	61640.31	.0	14223.61	-11050.03			
		2.0	7083.61	10257.19			
		4.0	-8256.39	17284.42			
		6.0	-15396.39	-6368.35			
		8.0	-22536.39	-44301.12			

9	51832.57	.0	14896.76	-13683.40			
		2.0	7756.76	8970.12			
		4.0	-7583.24	17343.63			
		6.0	-14723.24	-4962.86			
		8.0	-21863.24	-41549.34			

2	40982.15	.0	15668.53	-16711.00			
		2.0	8528.53	7486.05			
		4.0	-6811.47	17403.11			
		6.0	-13951.47	-3359.83			
		8.0	-21091.47	-38402.78			

15	28646.45	.0	16553.22	-20141.64			
		2.0	9413.22	5824.80			
		4.0	-5926.78	17511.25			
		6.0	-13066.78	-1482.31			
		8.0	-20206.78	-34755.86			

38	9744.77	.0	17768.12	-24857.71			
		2.0	10628.12	3538.53			
		4.0	-4711.88	17654.77			
		6.0	-11851.88	1091.00			
		8.0	-18991.88	-29752.76			

39	55369.23	.0	12686.62	-21328.32			
		2.0	8306.62	-335.08			
		4.0	-1953.38	11898.16			
		6.0	-6333.38	3611.40			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
COMB		8.0	-10713.38	-13435.36			

1	1	-5835.52					
		.0	12340.11	-20409.16			
		2.0	7960.11	-108.94			
		4.0	-2299.89	11431.29			
		6.0	-6679.89	2451.51			
		8.0	-11059.89	-15288.26			

0	1	-28808.84					
		.0	17999.43	-33795.18			
		3.0	11429.43	10348.10			
		6.0	-1020.57	23021.39			
		9.0	-13470.57	4224.68			
		12.0	-20040.57	-46042.03			

Analisa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

ELEM	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	AXIAL TORQ
				SHEAR	MOMENT		
1	1	51170.92	.0	71696.07	-254019.05		
			1.0	71696.07	-182322.99		
			2.0	71696.07	-110626.92		
3			3.0	71696.07	-38930.85		
			4.0	71696.07	32765.21		
5	1	34350.67	.0	55933.47	-73651.11		
			1.0	55933.47	-17717.64		
			2.0	55933.47	38215.82		
7			3.0	55933.47	94149.29		
			4.0	55933.47	150082.75		
9	1	33382.92	.0	-28430.03	99374.48		
			1.0	-28430.03	70944.44		
			2.0	-28430.03	42514.41		
11			3.0	-28430.03	14084.38		
			4.0	-28430.03	-14345.65		
13	1	-5146.53	.0	1988.81	-8493.65		
			1.0	1988.81	-6504.84		
			2.0	1988.81	-4516.03		
15			3.0	1988.81	-2527.22		
			4.0	1988.81	-538.41		
17	1	-25686.16	.0	-1425.33	3299.67		
			1.0	-1425.33	1874.33		
			2.0	-1425.33	449.00		
19			3.0	-1425.33	-976.34		
			4.0	-1425.33	-2401.67		
21	1	-42704.38	.0	-1999.78	4647.82		
			1.0	-1999.78	2648.04		
			2.0	-1999.78	648.26		
23			3.0	-1999.78	-1351.52		
			4.0	-1999.78	-3351.30		
25	1	-55080.65	.0	-2672.85	6477.51		
			1.0	-2672.85	3804.66		
			2.0	-2672.85	1131.81		
27			3.0	-2672.85	-1541.04		
			4.0	-2672.85	-4213.89		
29	1	-62907.26	.0	-3401.49	8459.95		

isa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
COMB			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	1	51170.92					
		.0	71696.07	-254019.05			
		1.0	71696.07	-182322.99			
		2.0	71696.07	-110626.92			
		3.0	71696.07	-38930.85			
		4.0	71696.07	32765.21			

5	1	34350.67					
		.0	55933.47	-73651.11			
		1.0	55933.47	-17717.64			
		2.0	55933.47	38215.82			
		3.0	55933.47	94149.29			
		4.0	55933.47	150082.75			

9	1	33382.92					
		.0	-28430.03	99374.48			
		1.0	-28430.03	70944.44			
		2.0	-28430.03	42514.41			
		3.0	-28430.03	14084.38			
		4.0	-28430.03	-14345.65			

13	1	-5146.53					
		.0	1988.81	-8493.65			
		1.0	1988.81	-6504.84			
		2.0	1988.81	-4516.03			
		3.0	1988.81	-2527.22			
		4.0	1988.81	-538.41			

17	1	-25686.16					
		.0	-1425.33	3299.67			
		1.0	-1425.33	1874.33			
		2.0	-1425.33	449.00			
		3.0	-1425.33	-976.34			
		4.0	-1425.33	-2401.67			

21	1	-42704.38					
		.0	-1999.78	4647.82			
		1.0	-1999.78	2648.04			
		2.0	-1999.78	648.26			
		3.0	-1999.78	-1351.52			
		4.0	-1999.78	-3351.30			

25	1	-55080.65					
		.0	-2672.85	6477.51			
		1.0	-2672.85	3804.66			
		2.0	-2672.85	1131.81			
		3.0	-2672.85	-1541.04			
		4.0	-2672.85	-4213.89			

29	1	-62907.26					
		.0	-3401.49	8459.95			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-3401.49	5058.46			
		2.0	-3401.49	1656.97			
		3.0	-3401.49	-1744.52			
		4.0	-3401.49	-5146.01			
3	1-65858.76	.0	-4197.96	10560.17			
		1.0	-4197.96	6362.21			
		2.0	-4197.96	2164.24			
		3.0	-4197.96	-2033.72			
		4.0	-4197.96	-6231.68			
7	1-63522.24	.0	-5056.93	12745.77			
		1.0	-5056.93	7688.85			
		2.0	-5056.93	2631.92			
		3.0	-5056.93	-2425.01			
		4.0	-5056.93	-7481.94			
11	1-55422.77	.0	-5986.66	15042.32			
		1.0	-5986.66	9055.66			
		2.0	-5986.66	3068.99			
		3.0	-5986.66	-2917.67			
		4.0	-5986.66	-8904.33			
15	1-41002.65	.0	-6928.39	17503.22			
		1.0	-6928.39	10574.83			
		2.0	-6928.39	3646.44			
		3.0	-6928.39	-3281.96			
		4.0	-6928.39	-10210.35			
19	1-19529.09	.0	-10341.83	20472.19			
		1.0	-10341.83	10130.37			
		2.0	-10341.83	-211.46			
		3.0	-10341.83	-10553.29			
		4.0	-10341.83	-20895.12			
2	1*****	.0	82057.61	-268271.24			
		1.0	82057.61	-186213.63			
		2.0	82057.61	-104156.02			
		3.0	82057.61	-22098.41			
		4.0	82057.61	59959.20			
6	1*****	.0	78163.91	-121986.13			
		1.0	78163.91	-43822.22			
		2.0	78163.91	34341.69			
		3.0	78163.91	112505.60			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
			78163.91	190669.51			

	1*****						
	.0		-25551.36	92599.34			
	1.0		-25551.36	67047.97			
	2.0		-25551.36	41496.61			
	3.0		-25551.36	15945.25			
	4.0		-25551.36	-9606.11			
1	-----						
	1*****						
	.0		5541.88	-15171.71			
	1.0		5541.88	-9629.83			
	2.0		5541.88	-4087.95			
	3.0		5541.88	1453.93			
	4.0		5541.88	6995.80			
3	-----						
	1*****						
	.0		1879.65	-3283.12			
	1.0		1879.65	-1403.48			
	2.0		1879.65	476.17			
	3.0		1879.65	2355.82			
	4.0		1879.65	4235.47			
2	-----						
	1*****						
	.0		1254.76	-1985.55			
	1.0		1254.76	-730.79			
	2.0		1254.76	523.97			
	3.0		1254.76	1778.73			
	4.0		1254.76	3033.50			
6	-----						
	1*****						
	.0		306.44	408.78			
	1.0		306.44	715.23			
	2.0		306.44	1021.67			
	3.0		306.44	1328.11			
	4.0		306.44	1634.55			
10	-----						
	1*****						
	.0		-813.95	3160.63			
	1.0		-813.95	2346.68			
	2.0		-813.95	1532.73			
	3.0		-813.95	718.78			
	4.0		-813.95	-95.17			
14	-----						
	1*****						
	.0		-2086.42	6182.70			
	1.0		-2086.42	4096.28			
	2.0		-2086.42	2009.86			
	3.0		-2086.42	-76.56			
	4.0		-2086.42	-2162.98			
18	-----						
	1-93116.94						

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E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3506.44	9466.14			
		1.0	-3506.44	5959.70			
		2.0	-3506.44	2453.27			
		3.0	-3506.44	-1053.17			
		4.0	-3506.44	-4559.60			
2	1-64976.15	.0	-5060.81	13012.18			
		1.0	-5060.81	7951.37			
		2.0	-5060.81	2890.56			
		3.0	-5060.81	-2170.24			
		4.0	-5060.81	-7231.05			
6	1-39804.44	.0	-6975.46	16884.00			
		1.0	-6975.46	9908.54			
		2.0	-6975.46	2933.07			
		3.0	-6975.46	-4042.39			
		4.0	-6975.46	-11017.86			
10	1-16037.37	.0	-9863.53	19976.00			
		1.0	-9863.53	10112.48			
		2.0	-9863.53	248.95			
		3.0	-9863.53	-9614.58			
		4.0	-9863.53	-19478.10			
3	1*****	.0	89997.73	-281039.53			
		1.0	89997.73	-191041.80			
		2.0	89997.73	-101044.07			
		3.0	89997.73	-11046.34			
		4.0	89997.73	78951.38			
7	1*****	.0	101330.46	-168157.84			
		1.0	101330.46	-66827.38			
		2.0	101330.46	34503.08			
		3.0	101330.46	135833.54			
		4.0	101330.46	237164.00			
11	1*****	.0	-7730.70	60214.07			
		1.0	-7730.70	52483.37			
		2.0	-7730.70	44752.67			
		3.0	-7730.70	37021.97			
		4.0	-7730.70	29291.27			
15	1*****	.0	23238.94	-49512.73			
		1.0	23238.94	-26273.79			
		2.0	23238.94	-3034.85			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	23238.94	20204.09			
		4.0	23238.94	43443.02			

	1*****						
		.0	20112.95	-39509.82			
		1.0	20112.95	-19396.87			
		2.0	20112.95	716.09			
		3.0	20112.95	20829.04			
		4.0	20112.95	40942.00			

	1*****						
		.0	20085.83	-39558.57			
		1.0	20085.83	-19472.74			
		2.0	20085.83	613.09			
		3.0	20085.83	20698.92			
		4.0	20085.83	40784.75			

	1*****						
		.0	19235.73	-37210.97			
		1.0	19235.73	-17975.23			
		2.0	19235.73	1260.50			
		3.0	19235.73	20496.24			
		4.0	19235.73	39731.97			

	1*****						
		.0	18289.35	-34848.43			
		1.0	18289.35	-16559.08			
		2.0	18289.35	1730.28			
		3.0	18289.35	20019.63			
		4.0	18289.35	38308.99			

	1*****						
		.0	17151.95	-32125.66			
		1.0	17151.95	-14973.71			
		2.0	17151.95	2178.23			
		3.0	17151.95	19330.18			
		4.0	17151.95	36482.12			

	1*****						
		.0	15843.67	-29089.02			
		1.0	15843.67	-13245.35			
		2.0	15843.67	2598.33			
		3.0	15843.67	18442.00			
		4.0	15843.67	34285.67			

	1*****						
		.0	14302.60	-25723.32			
		1.0	14302.60	-11420.71			
		2.0	14302.60	2881.89			
		3.0	14302.60	17184.49			
		4.0	14302.60	31487.10			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-76753.27							
	.0		13071.13	-22356.46			
	1.0		13071.13	-9285.33			
	2.0		13071.13	3785.80			
	3.0		13071.13	16856.93			
	4.0		13071.13	29928.07			

1-27975.85							
	.0		10333.33	-16125.95			
	1.0		10333.33	-5792.61			
	2.0		10333.33	4540.72			
	3.0		10333.33	14874.06			
	4.0		10333.33	25207.39			

4	1*****						
		.0	74409.60	-262500.27			
		1.0	74409.60	-188090.67			
		2.0	74409.60	-113681.07			
		3.0	74409.60	-39271.48			
		4.0	74409.60	35138.12			

8	1*****						
		.0	79266.16	-121659.04			
		1.0	79266.16	-42392.88			
		2.0	79266.16	36873.29			
		3.0	79266.16	116139.45			
		4.0	79266.16	195405.62			

2	1*****						
		.0	-14851.79	81635.75			
		1.0	-14851.79	66783.95			
		2.0	-14851.79	51932.16			
		3.0	-14851.79	37080.36			
		4.0	-14851.79	22228.57			

.6	1*****						
		.0	12714.62	-27088.17			
		1.0	12714.62	-14373.54			
		2.0	12714.62	-1658.92			
		3.0	12714.62	11055.70			
		4.0	12714.62	23770.32			

20	1*****						
		.0	13131.53	-26652.10			
		1.0	13131.53	-13520.56			
		2.0	13131.53	-389.03			
		3.0	13131.53	12742.51			
		4.0	13131.53	25874.04			

24	1*****						
		.0	12224.69	-23481.60			

sa Statis Portal (13 lantai) Kg/m

M E L E M E N T F O R C E S

LOAD) COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	12224.69	-11256.91			
		2.0	12224.69	967.78			
		3.0	12224.69	13192.46			
		4.0	12224.69	25417.15			
3	1*****	.0	11803.82	-22177.53			
		1.0	11803.82	-10373.71			
		2.0	11803.82	1430.11			
		3.0	11803.82	13233.92			
		4.0	11803.82	25037.74			
2	1*****	.0	11229.36	-20494.14			
		1.0	11229.36	-9264.79			
		2.0	11229.36	1964.57			
		3.0	11229.36	13193.93			
		4.0	11229.36	24423.29			
6	1*****	.0	10571.24	-18693.54			
		1.0	10571.24	-8122.30			
		2.0	10571.24	2448.94			
		3.0	10571.24	13020.18			
		4.0	10571.24	23591.42			
0	1*****	.0	9805.81	-16741.09			
		1.0	9805.81	-6935.28			
		2.0	9805.81	2870.53			
		3.0	9805.81	12676.34			
		4.0	9805.81	22482.15			
14	1-83581.21	.0	9034.90	-14685.82			
		1.0	9034.90	-5650.92			
		2.0	9034.90	3383.98			
		3.0	9034.90	12418.87			
		4.0	9034.90	21453.77			
18	1-42447.55	.0	8206.74	-12061.59			
		1.0	8206.74	-3854.85			
		2.0	8206.74	4351.89			
		3.0	8206.74	12558.63			
		4.0	8206.74	20765.36			
22	1-10846.82	.0	5595.63	-7876.28			
		1.0	5595.63	-2280.65			
		2.0	5595.63	3314.98			
		3.0	5595.63	8910.60			

sa Statis Portal (13 lantai) Kg/m

TIME ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
COMB			SHEAR	MOMENT	SHEAR	TORQ
		4.0	5595.63	14506.23		
3	1	12295.60				
		.0	-16820.25	106416.32		
		2.0	-22360.25	67235.82		
		4.0	-36100.25	16975.33		
		6.0	-41640.25	-60765.16		
		8.0	-47180.25	-149585.66		
4	1	16189.30				
		.0	10316.48	32359.68		
		3.0	2006.48	50844.11		
		6.0	-14503.52	27998.55		
		9.0	-31013.52	-36177.01		
		12.0	-39323.52	-141682.57		
5	1	4856.57				
		.0	-17597.98	105426.65		
		2.0	-23137.98	64690.70		
		4.0	-36877.98	12874.75		
		6.0	-42417.98	-66421.21		
		8.0	-47957.98	-156797.16		
6	1	77428.50				
		.0	-967.75	50708.28		
		2.0	-8107.75	41632.77		
		4.0	-23447.75	18277.27		
		6.0	-30587.75	-35758.24		
		8.0	-37727.75	-104073.74		
9	1	61707.70				
		.0	13181.69	-6107.90		
		2.0	6041.69	13115.48		
		4.0	-9298.31	18058.85		
		6.0	-16438.31	-7677.78		
		8.0	-23578.31	-47694.41		
2	1	54922.33				
		.0	12674.42	-3986.95		
		2.0	5534.42	14221.88		
		4.0	-9805.58	18150.71		
		6.0	-16945.58	-8600.45		
		8.0	-24085.58	-49631.62		
5	1	42932.07				
		.0	13476.12	-7187.92		
		2.0	6336.12	12624.31		
		4.0	-9003.88	18156.55		
		6.0	-16143.88	-6991.22		
		8.0	-23283.88	-46418.98		
8	1	31409.99				

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	14159.45	-9947.95			
		2.0	7019.45	11230.96			
		4.0	-8320.55	18129.87			
		6.0	-15460.55	-5651.22			
		8.0	-22600.55	-43712.31			
1	20064.49						
		.0	14860.24	-12774.71			
		2.0	7720.24	9805.76			
		4.0	-7619.76	18106.24			
		6.0	-14759.76	-4273.29			
		8.0	-21899.76	-40932.81			
4	8171.45						
		.0	15606.88	-15787.96			
		2.0	8466.88	8285.81			
		4.0	-6873.12	18079.58			
		6.0	-14013.12	-2806.66			
		8.0	-21153.12	-37972.89			
7	-4436.49						
		.0	16411.33	-19038.87			
		2.0	9271.33	6643.79			
		4.0	-6068.67	18046.45			
		6.0	-13208.67	-1230.90			
		8.0	-20348.67	-34788.24			
0	-17849.85						
		.0	17282.98	-22563.81			
		2.0	10142.98	4862.15			
		4.0	-5197.02	18008.11			
		6.0	-12337.02	474.07			
		8.0	-19477.02	-31339.98			
3	-32270.12						
		.0	18238.32	-26423.45			
		2.0	11098.32	2913.18			
		4.0	-4241.68	17969.82			
		6.0	-11381.68	2346.45			
		8.0	-18521.68	-27556.92			
6	-45557.90						
		.0	19262.10	-30671.67			
		2.0	12122.10	712.53			
		4.0	-3217.90	17816.73			
		6.0	-10357.90	4240.93			
		8.0	-17497.90	-23614.87			
7	77879.23						
		.0	19976.71	-6378.61			
		3.0	9266.71	37486.52			
		6.0	-9643.29	32821.64			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		9.0	-28553.29	-20373.23			
		12.0	-39263.29	-122098.11			
) -----							
1	99093.75						
		.0	26086.94	-42574.14			
		3.0	15376.94	19621.67			
		6.0	-3533.06	33287.48			
		9.0	-22443.06	-1576.71			
		12.0	-33153.06	-84970.90			
3 -----							
1	96677.57						
		.0	25634.51	-39708.91			
		3.0	14924.51	21129.62			
		6.0	-3985.49	33438.14			
		9.0	-22895.49	-2783.33			
		12.0	-33605.49	-87534.80			
6 -----							
1	89545.27						
		.0	25778.08	-40535.98			
		3.0	15068.08	20733.24			
		6.0	-3841.92	33472.47			
		9.0	-22751.92	-2318.31			
		12.0	-33461.92	-86639.08			
9 -----							
1	83123.40						
		.0	25924.18	-41399.88			
		3.0	15214.18	20307.67			
		6.0	-3695.82	33485.22			
		9.0	-22605.82	-1867.23			
		12.0	-33315.82	-85749.68			
2 -----							
1	75348.85						
		.0	26153.65	-42742.54			
		3.0	15443.65	19653.42			
		6.0	-3466.35	33519.39			
		9.0	-22376.35	-1144.65			
		12.0	-33086.35	-84338.69			
75 -----							
1	65953.79						
		.0	26451.34	-44500.42			
		3.0	15741.34	18788.62			
		6.0	-3168.66	33547.65			
		9.0	-22078.66	-223.32			
		12.0	-32788.66	-82524.28			
78 -----							
1	55154.93						
		.0	26809.99	-46628.23			
		3.0	16099.99	17736.76			
		6.0	-2810.01	33571.74			
		9.0	-21720.01	876.72			
		12.0	-32430.01	-80348.30			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T D	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
1		1	43137.32					
			.0	27221.94	-49079.14			
			3.0	16511.94	16521.68			
			6.0	-2398.06	33592.49			
			9.0	-21308.06	2133.31			
			12.0	-32018.06	-77855.88			
34		1	30443.09					
			.0	27673.63	-51790.55			
			3.0	16963.63	15165.35			
			6.0	-1946.37	33591.25			
			9.0	-20856.37	3487.15			
			12.0	-31566.37	-75146.95			
37		1	16049.96					
			.0	28177.01	-54688.84			
			3.0	17467.01	13777.21			
			6.0	-1442.99	33713.25			
			9.0	-20352.99	5119.29			
			12.0	-31062.99	-72004.67			
58		1	52102.91					
			.0	-2589.94	54436.49			
			2.0	-9729.94	42116.61			
			4.0	-25069.94	15516.73			
			6.0	-32209.94	-41763.15			
			8.0	-47549.94	-113323.03			
51			1115331.34					
			.0	13065.37	-6644.01			
			2.0	5925.37	12346.73			
			4.0	-9414.63	17057.46			
			6.0	-16554.63	-8911.81			
			8.0	-31894.63	-49161.07			
64		1	88567.26					
			.0	12713.63	-4941.77			
			2.0	5573.63	13345.48			
			4.0	-9766.37	17352.74			
			6.0	-16906.37	-9320.01			
			8.0	-32246.37	-50272.76			
67		1	81633.79					
			.0	13037.76	-6475.44			
			2.0	5897.76	12460.07			
			4.0	-9442.24	17115.58			
			6.0	-16582.24	-8908.91			
			8.0	-31922.24	-49213.40			
70		1	75274.19					
			.0	13455.45	-8070.11			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE SHEAR MOMENT		1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		2.0	6315.45	11700.79			
		4.0	-9024.55	17191.69			
		6.0	-16164.55	-7997.40			
		8.0	-31504.55	-47466.50			
3	1	67361.87					
		.0	13958.20	-10044.71			
		2.0	6818.20	10731.69			
		4.0	-8521.80	17228.09			
		6.0	-15661.80	-6955.51			
		8.0	-31001.80	-45419.11			
6	1	58509.71					
		.0	14545.02	-12341.71			
		2.0	7405.02	9608.33			
		4.0	-7934.98	17278.37			
		6.0	-15074.98	-5731.59			
		8.0	-30414.98	-43021.55			
9	1	48571.09					
		.0	15221.65	-14990.15			
		2.0	8081.65	8313.16			
		4.0	-7258.35	17336.47			
		6.0	-14398.35	-4320.23			
		8.0	-29738.35	-40256.92			
12	1	37635.69					
		.0	15992.78	-18016.23			
		2.0	8852.78	6829.33			
		4.0	-6487.22	17394.88			
		6.0	-13627.22	-2719.56			
		8.0	-28967.22	-37114.01			
35	1	25253.73					
		.0	16872.38	-21426.89			
		2.0	9732.38	5177.87			
		4.0	-5607.62	17502.63			
		6.0	-12747.62	-852.61			
		8.0	-28087.62	-33487.85			
38	1	6781.19					
		.0	18052.16	-26024.54			
		2.0	10912.16	2939.78			
		4.0	-4427.84	17624.11			
		6.0	-11567.84	1628.44			
		8.0	-26907.84	-28647.23			
39	1	53546.20					
		.0	12578.75	-20879.48			
		2.0	8198.75	-101.98			
		4.0	-2061.25	11915.53			
		6.0	-6441.25	3413.04			

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M M E E L E M E N T F O R C E S

L O -	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
-			8.0	-10821.25	-13849.45			
L	1	-5595.63						
			.0	12553.18	-21331.64			
			2.0	8173.18	-605.29			
			4.0	-2086.82	11361.06			
			6.0	-6466.82	2807.42			
3			8.0	-10846.82	-14506.23			
0	1	-25321.27						
			.0	17924.43	-33412.64			
			3.0	11354.43	10505.65			
			6.0	-1095.57	22953.93			
			9.0	-13545.57	3932.21			
9			12.0	-20115.57	-46559.50			

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sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-3614.61	5401.17			
		2.0	-3614.61	1786.56			
		3.0	-3614.61	-1828.06			
		4.0	-3614.61	-5442.67			

	1-72398.57	.0	-4400.03	11050.70			
		1.0	-4400.03	6650.67			
		2.0	-4400.03	2250.63			
		3.0	-4400.03	-2149.40			
		4.0	-4400.03	-6549.44			

	1-68735.66	.0	-5261.08	13221.32			
		1.0	-5261.08	7960.24			
		2.0	-5261.08	2699.16			
		3.0	-5261.08	-2561.92			
		4.0	-5261.08	-7823.00			

	1-59294.81	.0	-6190.74	15499.55			
		1.0	-6190.74	9308.81			
		2.0	-6190.74	3118.07			
		3.0	-6190.74	-3072.67			
		4.0	-6190.74	-9263.41			

	1-43520.87	.0	-7148.03	17947.27			
		1.0	-7148.03	10799.25			
		2.0	-7148.03	3651.22			
		3.0	-7148.03	-3496.80			
		4.0	-7148.03	-10644.83			

	1-20697.35	.0	-10578.13	20818.16			
		1.0	-10578.13	10240.04			
		2.0	-10578.13	-338.09			
		3.0	-10578.13	-10916.21			
		4.0	-10578.13	-21494.34			

	1*****	.0	82274.06	-288751.15			
		1.0	82274.06	-206477.09			
		2.0	82274.06	-124203.03			
		3.0	82274.06	-41928.97			
		4.0	82274.06	40345.09			

	1*****	.0	85193.49	-184362.92			
		1.0	85193.49	-99169.43			
		2.0	85193.49	-13975.93			
		3.0	85193.49	71217.56			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		4.0	85193.49	156411.05			
) -----							
	1*****						
		.0	77285.87	-92936.80			
		1.0	77285.87	-15650.93			
		2.0	77285.87	61634.94			
		3.0	77285.87	138920.82			
		4.0	77285.87	216206.69			
4	-----						
	1*****						
		.0	-29823.46	107921.48			
		1.0	-29823.46	78098.02			
		2.0	-29823.46	48274.57			
		3.0	-29823.46	18451.11			
		4.0	-29823.46	-11372.35			
8	-----						
	1*****						
		.0	5060.67	-13718.48			
		1.0	5060.67	-8657.82			
		2.0	5060.67	-3597.15			
		3.0	5060.67	1463.52			
		4.0	5060.67	6524.18			
2	-----						
	1*****						
		.0	795.61	-278.45			
		1.0	795.61	517.16			
		2.0	795.61	1312.77			
		3.0	795.61	2108.38			
		4.0	795.61	2903.99			
26	-----						
	1*****						
		.0	44.32	1106.45			
		1.0	44.32	1150.76			
		2.0	44.32	1195.08			
		3.0	44.32	1239.40			
		4.0	44.32	1283.72			
50	-----						
	1*****						
		.0	-1073.28	3784.05			
		1.0	-1073.28	2710.77			
		2.0	-1073.28	1637.49			
		3.0	-1073.28	564.21			
		4.0	-1073.28	-509.08			
54	-----						
	1*****						
		.0	-2356.96	6808.65			
		1.0	-2356.96	4451.68			
		2.0	-2356.96	2094.72			
		3.0	-2356.96	-262.24			
		4.0	-2356.96	-2619.21			
38	-----						
	1-90927.88						

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		.0	-3785.31	10088.66			
		1.0	-3785.31	6303.35			
		2.0	-3785.31	2518.03			
		3.0	-3785.31	-1267.28			
		4.0	-3785.31	-5052.59			
2		-----					
		1-63645.88					
		.0	-5344.20	13624.04			
		1.0	-5344.20	8279.83			
		2.0	-5344.20	2935.63			
		3.0	-5344.20	-2408.57			
		4.0	-5344.20	-7752.78			
6		-----					
		1-39323.21					
		.0	-7268.83	17483.02			
		1.0	-7268.83	10214.19			
		2.0	-7268.83	2945.36			
		3.0	-7268.83	-4323.47			
		4.0	-7268.83	-11592.30			
50		-----					
		1-16266.76					
		.0	-10209.75	20511.31			
		1.0	-10209.75	10301.56			
		2.0	-10209.75	91.81			
		3.0	-10209.75	-10117.93			
		4.0	-10209.75	-20327.68			
3		-----					
		1*****					
		.0	91381.99	-300086.77			
		1.0	91381.99	-208704.77			
		2.0	91381.99	-117322.78			
		3.0	91381.99	-25940.79			
		4.0	91381.99	65441.21			
7		-----					
		1*****					
		.0	99236.56	-217196.86			
		1.0	99236.56	-117960.30			
		2.0	99236.56	-18723.74			
		3.0	99236.56	80512.81			
		4.0	99236.56	179749.37			
11		-----					
		1*****					
		.0	101363.53	-139783.72			
		1.0	101363.53	-38420.19			
		2.0	101363.53	62943.34			
		3.0	101363.53	164306.86			
		4.0	101363.53	265670.39			
15		-----					
		1*****					
		.0	-11590.38	75155.12			
		1.0	-11590.38	63564.75			
		2.0	-11590.38	51974.37			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	-11590.38	40383.99			
		4.0	-11590.38	28793.62			

	1*****						
		.0	23301.88	-49190.27			
		1.0	23301.88	-25888.39			
		2.0	23301.88	-2586.50			
		3.0	23301.88	20715.38			
		4.0	23301.88	44017.26			
5	-----						
	1*****						
		.0	19711.97	-37977.52			
		1.0	19711.97	-18265.56			
		2.0	19711.97	1446.41			
		3.0	19711.97	21158.38			
		4.0	19711.97	40870.34			
7	-----						
	1*****						
		.0	19543.84	-37845.72			
		1.0	19543.84	-18301.88			
		2.0	19543.84	1241.96			
		3.0	19543.84	20785.79			
		4.0	19543.84	40329.63			
1	-----						
	1*****						
		.0	18497.01	-35142.03			
		1.0	18497.01	-16645.02			
		2.0	18497.01	1851.99			
		3.0	18497.01	20349.00			
		4.0	18497.01	38846.00			
5	-----						
	1*****						
		.0	17367.92	-32472.59			
		1.0	17367.92	-15104.67			
		2.0	17367.92	2263.25			
		3.0	17367.92	19631.17			
		4.0	17367.92	36999.10			
9	-----						
	1*****						
		.0	16060.05	-29459.34			
		1.0	16060.05	-13399.29			
		2.0	16060.05	2660.75			
		3.0	16060.05	18720.80			
		4.0	16060.05	34780.85			
3	-----						
	1*****						
		.0	14521.25	-26117.02			
		1.0	14521.25	-11595.77			
		2.0	14521.25	2925.48			
		3.0	14521.25	17446.74			
		4.0	14521.25	31967.99			

Essa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	

	1-72624.64						
		.0	13305.85	-22757.60			
3		1.0	13305.85	-9451.75			
		2.0	13305.85	3854.10			
		3.0	13305.85	17159.96			
		4.0	13305.85	30465.81			
1	-----						
	1-26167.12						
		.0	10505.63	-16338.53			
2		1.0	10505.63	-5832.90			
		2.0	10505.63	4672.73			
		3.0	10505.63	15178.36			
		4.0	10505.63	25683.99			
4	-----						
	1*****						
		.0	74802.26	-278850.80			
		1.0	74802.26	-204048.54			
		2.0	74802.26	-129246.27			
		3.0	74802.26	-54444.01			
		4.0	74802.26	20358.25			
8	-----						
	1*****						
		.0	67756.59	-156621.87			
		1.0	67756.59	-88865.28			
		2.0	67756.59	-21108.68			
		3.0	67756.59	46647.91			
		4.0	67756.59	114404.50			
12	-----						
	1*****						
		.0	76246.28	-85820.66			
		1.0	76246.28	-9574.37			
		2.0	76246.28	66671.91			
		3.0	76246.28	142918.20			
		4.0	76246.28	219164.48			
16	-----						
	1*****						
		.0	-18948.41	97602.49			
		1.0	-18948.41	78654.09			
		2.0	-18948.41	59705.68			
		3.0	-18948.41	40757.27			
		4.0	-18948.41	21808.87			
20	-----						
	1*****						
		.0	12713.66	-26699.92			
		1.0	12713.66	-13986.26			
		2.0	12713.66	-1272.60			
		3.0	12713.66	11441.06			
		4.0	12713.66	24154.72			
24	-----						
	1*****						
		.0	13032.01	-25774.25			

sa spa Statis Portal (13 lantai) Kg/m

M E M E E L E M E N T F O R C E S

LOA COM	LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR 5830.53	MOMENT 15276.34	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
	1	3728.33						
			.0	-23699.66	134806.86			
			2.0	-29239.66	81867.53			
5			4.0	-42979.66	17848.20			
			6.0	-48519.66	-73651.13			
			8.0	-54059.66	-176230.46			
	1	2709.04						
			.0	-26415.62	145209.76			
			2.0	-31955.62	86838.52			
8			4.0	-45695.62	17387.28			
			6.0	-51235.62	-79543.96			
			8.0	-56775.62	-187555.20			
	1	808.89						
			.0	7720.02	48477.55			
			3.0	-589.98	59172.61			
1			6.0	-17099.98	28537.67			
			9.0	-33609.98	-43427.27			
			12.0	-41919.98	-156722.21			
	1	10616.66						
			.0	5387.63	61792.65			
			3.0	-2922.37	65490.53			
4			6.0	-19432.37	27858.42			
			9.0	-35942.37	-51103.70			
			12.0	-44252.37	-171395.82			
	1	-7045.67						
			.0	-22682.00	125915.86			
			2.0	-28222.00	75011.86			
7			4.0	-41962.00	13027.87			
			6.0	-47502.00	-76436.13			
			8.0	-53042.00	-176980.12			
	1	8489.69						
			.0	-28365.30	148137.27			
			2.0	-33905.30	85866.66			
30			4.0	-47645.30	12516.06			
			6.0	-53185.30	-88314.55			
			8.0	-58725.30	-200225.16			
	1	73333.89						
			.0	-2049.62	55113.68			
			2.0	-9189.62	43874.44			
33			4.0	-24529.62	18355.21			
			6.0	-31669.62	-37844.02			
			8.0	-38809.62	-108323.26			
	1	51313.66						

sa Statis Portal (13 lantai) Kg/m

M E E L F M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		6.0	-11183.83	2735.11		
		8.0	-18323.83	-26772.54		

5	1-47720.68					
		.0	19454.72	-31447.60		
		2.0	12314.72	321.84		
		4.0	-3025.28	17811.29		
		6.0	-10165.28	4620.73		
		8.0	-17305.28	-22849.83		

0	1 80436.57					
		.0	18988.78	-427.09		
		3.0	8278.78	40474.26		
		6.0	-10631.22	32845.60		
		9.0	-29541.22	-23313.05		
		12.0	-40251.22	-128001.70		

3	1 99181.30					
		.0	26093.37	-42525.28		
		3.0	15383.37	19689.82		
		6.0	-3526.63	33374.93		
		9.0	-22436.63	-1469.97		
		12.0	-33146.63	-84844.86		

6	1 92688.00					
		.0	25709.32	-40076.38		
		3.0	14999.32	20986.57		
		6.0	-3910.68	33519.52		
		9.0	-22820.68	-2477.54		
		12.0	-33530.68	-87004.59		

9	1 82851.76					
		.0	25961.15	-41568.26		
		3.0	15251.15	20250.19		
		6.0	-3658.85	33538.64		
		9.0	-22568.85	-1702.92		
		12.0	-33278.85	-85474.47		

2	1 74462.34					
		.0	26193.34	-42954.69		
		3.0	15483.34	19560.32		
		6.0	-3426.66	33545.34		
		9.0	-22336.66	-999.64		
		12.0	-33046.66	-84074.62		

5	1 64963.73					
		.0	26499.60	-44761.56		
		3.0	15789.60	18672.24		
		6.0	-3120.40	33576.03		
		9.0	-22030.40	-50.18		
		12.0	-32740.40	-82206.38		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	54050.42						
		.0	26863.77	-46922.47			
		3.0	16153.77	17603.83			
		6.0	-2756.23	33600.14			
		9.0	-21666.23	1066.45			
		12.0	-32376.23	-79997.24			
1	-----						
1	41966.19						
		.0	27279.32	-49395.37			
		3.0	16569.32	16377.60			
		6.0	-2340.68	33620.56			
		9.0	-21250.68	2333.52			
		12.0	-31960.68	-77483.51			
4	-----						
1	29253.94						
		.0	27733.11	-52119.53			
		3.0	17023.11	15014.79			
		6.0	-1886.89	33619.11			
		9.0	-20796.89	3693.43			
		12.0	-31506.89	-74762.26			
17	-----						
1	14658.02						
		.0	28239.28	-55026.55			
		3.0	17529.28	13626.29			
		6.0	-1380.72	33749.12			
		9.0	-20290.72	5341.96			
		12.0	-31000.72	-71595.21			
51	-----						
1	52740.13						
		.0	-4513.82	62064.82			
		2.0	-11653.82	45897.18			
		4.0	-26993.82	15449.55			
		6.0	-34133.82	-45678.09			
		8.0	-41273.82	-121085.72			
54	-----						
	1115094.58						
		.0	13252.32	-7340.57			
		2.0	6112.32	12024.06			
		4.0	-9227.68	17108.70			
		6.0	-16367.68	-8486.67			
		8.0	-23507.68	-48362.03			
67	-----						
1	84589.80						
		.0	12825.56	-5351.58			
		2.0	5685.56	13159.54			
		4.0	-9654.44	17390.66			
		6.0	-16794.44	-9058.21			
		8.0	-23934.44	-49787.09			
70	-----						
1	77099.26						
		.0	13193.74	-7072.51			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIA
COMB			SHEAR	MOMENT	SHEAR	MOMENT	TOR
		2.0	6053.74	12174.97			
		4.0	-9286.26	17142.45			
		6.0	-16426.26	-8570.07			
		8.0	-23566.26	-48562.59			
3	-----						
	1	70119.96					
		.0	13671.77	-8891.72			
		2.0	6531.77	11311.83			
		4.0	-8808.23	17235.37			
		6.0	-15948.23	-7521.08			
		8.0	-23088.23	-46557.54			
6	-----						
	1	61420.82					
		.0	14245.95	-11142.12			
		2.0	7105.95	10209.78			
		4.0	-8234.05	17281.69			
		6.0	-15374.05	-6326.41			
		8.0	-22514.05	-44214.51			
19	-----						
	1	51684.19					
		.0	14913.94	-13754.46			
		2.0	7773.94	8933.43			
		4.0	-7566.06	17341.32			
		6.0	-14706.06	-4930.79			
		8.0	-21846.06	-41482.90			
32	-----						
	1	40905.99					
		.0	15679.66	-16758.01			
		2.0	8539.66	7461.31			
		4.0	-6800.34	17400.62			
		6.0	-13940.34	-3340.06			
		8.0	-21080.34	-38360.74			
35	-----						
	1	28637.50					
		.0	16558.21	-20163.57			
		2.0	9418.21	5812.86			
		4.0	-5921.79	17509.29			
		6.0	-13061.79	-1474.28			
		8.0	-20201.79	-34737.85			
88	-----						
	1	9715.17					
		.0	17770.47	-24868.40			
		2.0	10630.47	3532.55			
		4.0	-4709.53	17653.49			
		6.0	-11849.53	1094.43			
		8.0	-18989.53	-29744.62			
89	-----						
	1	55738.31					
		.0	12724.61	-21474.91			
		2.0	8344.61	-405.68			
		4.0	-1915.39	11903.54			
		6.0	-6295.39	3692.77			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIA
			SHEAR	MOMENT	SHEAR	TOR
	8.0		-10675.39	-13278.00		

1	-5830.53					
		.0	12344.27	-20430.54		
		2.0	7964.27	-121.99		
		4.0	-2295.73	11426.56		
		6.0	-6675.73	2455.11		
		8.0	-11055.73	-15276.34		

0	1-28807.39					
		.0	17983.30	-33688.64		
		3.0	11413.30	10406.27		
		6.0	-1036.70	23031.17		
		9.0	-13486.70	4186.07		
		12.0	-20056.70	-46129.03		

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ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	1	63182.77					
		.0	66991.69	-274112.14			
		1.0	66991.69	-207120.45			
		2.0	66991.69	-140128.76			
		3.0	66991.69	-73137.07			
		4.0	66991.69	-6145.38			

5	1	37867.46					
		.0	57206.58	-147424.71			
		1.0	57206.58	-90218.12			
		2.0	57206.58	-33011.54			
		3.0	57206.58	24195.05			
		4.0	57206.58	81401.63			

9	1	3959.80					
		.0	56268.07	-94726.45			
		1.0	56268.07	-38458.38			
		2.0	56268.07	17809.69			
		3.0	56268.07	74077.76			
		4.0	56268.07	130345.84			

13	1	23936.27					
		.0	47637.57	-20846.70			
		1.0	47637.57	26790.87			
		2.0	47637.57	74428.44			
		3.0	47637.57	122066.01			
		4.0	47637.57	169703.57			

17	1	25378.58					
		.0	-33210.05	116949.05			
		1.0	-33210.05	83739.00			
		2.0	-33210.05	50528.95			
		3.0	-33210.05	17318.91			
		4.0	-33210.05	-15891.14			

21	1	56703.18					
		.0	506.43	-3959.10			
		1.0	506.43	-3452.67			
		2.0	506.43	-2946.24			
		3.0	506.43	-2439.81			
		4.0	506.43	-1933.38			

25	1	67204.37					
		.0	-3116.24	8155.85			
		1.0	-3116.24	5039.61			
		2.0	-3116.24	1923.37			
		3.0	-3116.24	-1192.87			
		4.0	-3116.24	-4309.11			

29	1	73541.12					
		.0	-3741.48	9402.33			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-3741.48	5660.84			
		2.0	-3741.48	1919.36			
		3.0	-3741.48	-1822.12			
		4.0	-3741.48	-5563.61			

5	1-74588.57	.0	-4497.25	11273.62			
		1.0	-4497.25	6776.37			
		2.0	-4497.25	2279.12			
		3.0	-4497.25	-2218.13			
		4.0	-4497.25	-6715.38			

7	1-70377.64	.0	-5334.56	13370.93			
		1.0	-5334.56	8036.38			
		2.0	-5334.56	2701.82			
		3.0	-5334.56	-2632.74			
		4.0	-5334.56	-7967.29			

1	1-60452.09	.0	-6257.34	15633.43			
		1.0	-6257.34	9376.09			
		2.0	-6257.34	3118.74			
		3.0	-6257.34	-3138.60			
		4.0	-6257.34	-9395.94			

5	1-44243.10	.0	-7210.24	18065.35			
		1.0	-7210.24	10855.11			
		2.0	-7210.24	3644.87			
		3.0	-7210.24	-3565.37			
		4.0	-7210.24	-10775.61			

9	1-21026.68	.0	-10645.86	20911.39			
		1.0	-10645.86	10265.53			
		2.0	-10645.86	-380.33			
		3.0	-10645.86	-11026.19			
		4.0	-10645.86	-21672.05			

2	1*****	.0	81722.62	-292989.55			
		1.0	81722.62	-211266.93			
		2.0	81722.62	-129544.31			
		3.0	81722.62	-47821.69			
		4.0	81722.62	33900.93			

6	1*****	.0	85542.73	-201863.35			
		1.0	85542.73	-116320.62			
		2.0	85542.73	-30777.89			
		3.0	85542.73	54764.85			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	1*****						
	.0		85542.73	140307.58			
	1.0		84792.41	-155918.73			
	2.0		84792.41	-71126.33			
	3.0		84792.41	13666.08			
	4.0		84792.41	98458.49			
4	-----						
4	1*****						
	.0		74201.17	-79361.76			
	1.0		74201.17	-5160.60			
	2.0		74201.17	69040.57			
	3.0		74201.17	143241.74			
	4.0		74201.17	217442.91			
8	-----						
8	1*****						
	.0		-30722.49	110712.68			
	1.0		-30722.49	79990.19			
	2.0		-30722.49	49267.70			
	3.0		-30722.49	18545.21			
	4.0		-30722.49	-12177.28			
12	-----						
12	1*****						
	.0		4229.20	-11476.61			
	1.0		4229.20	-7247.41			
	2.0		4229.20	-3018.22			
	3.0		4229.20	1210.98			
	4.0		4229.20	5440.18			
26	-----						
26	1*****						
	.0		-243.53	2363.62			
	1.0		-243.53	2120.10			
	2.0		-243.53	1876.57			
	3.0		-243.53	1633.05			
	4.0		-243.53	1389.52			
30	-----						
30	1*****						
	.0		-1121.63	3937.97			
	1.0		-1121.63	2816.34			
	2.0		-1121.63	1694.71			
	3.0		-1121.63	573.07			
	4.0		-1121.63	-548.56			
34	-----						
34	1*****						
	.0		-2384.19	6870.96			
	1.0		-2384.19	4486.77			
	2.0		-2384.19	2102.58			
	3.0		-2384.19	-281.61			
	4.0		-2384.19	-2665.80			
38	-----						
38	1-88664.31						

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	.0	-3807.50	10137.44			
	1.0	-3807.50	6329.94			
	2.0	-3807.50	2522.44			
	3.0	-3807.50	-1285.06			
	4.0	-3807.50	-5092.57			
2	1-62094.15					
	.0	-5362.30	13661.73			
	1.0	-5362.30	8299.42			
	2.0	-5362.30	2937.12			
	3.0	-5362.30	-2425.18			
	4.0	-5362.30	-7787.49			
6	1-38423.52					
	.0	-7287.18	17509.92			
	1.0	-7287.18	10222.74			
	2.0	-7287.18	2935.56			
	3.0	-7287.18	-4351.62			
	4.0	-7287.18	-11638.80			
0	1-15952.70					
	.0	-10209.45	20501.55			
	1.0	-10209.45	10292.10			
	2.0	-10209.45	82.65			
	3.0	-10209.45	-10126.81			
	4.0	-10209.45	-20336.26			
3	1*****					
	.0	91450.85	-305405.10			
	1.0	91450.85	-213954.25			
	2.0	91450.85	-122503.40			
	3.0	91450.85	-31052.55			
	4.0	91450.85	60398.29			
7	1*****					
	.0	100792.55	-233232.36			
	1.0	100792.55	-132439.81			
	2.0	100792.55	-31647.25			
	3.0	100792.55	69145.30			
	4.0	100792.55	169937.86			
11	1*****					
	.0	99101.72	-188459.25			
	1.0	99101.72	-89357.53			
	2.0	99101.72	9744.18			
	3.0	99101.72	108845.90			
	4.0	99101.72	207947.61			
15	1*****					
	.0	99051.03	-127641.58			
	1.0	99051.03	-28590.55			
	2.0	99051.03	70460.48			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	99051.03	169511.51			
		4.0	99051.03	268562.53			

9	1*****	.0	-12229.86	77599.53			
		1.0	-12229.86	65369.67			
		2.0	-12229.86	53139.81			
		3.0	-12229.86	40909.95			
		4.0	-12229.86	28680.09			

3	1*****	.0	22804.61	-47684.16			
		1.0	22804.61	-24879.55			
		2.0	22804.61	-2074.93			
		3.0	22804.61	20729.68			
		4.0	22804.61	43534.29			

17	1*****	.0	18999.32	-36055.61			
		1.0	18999.32	-17056.29			
		2.0	18999.32	1943.03			
		3.0	18999.32	20942.35			
		4.0	18999.32	39941.67			

51	1*****	.0	18634.45	-35561.28			
		1.0	18634.45	-16926.83			
		2.0	18634.45	1707.63			
		3.0	18634.45	20342.08			
		4.0	18634.45	38976.54			

35	1*****	.0	17396.77	-32512.62			
		1.0	17396.77	-15115.85			
		2.0	17396.77	2280.93			
		3.0	17396.77	19677.70			
		4.0	17396.77	37074.48			

39	1*****	.0	16093.85	-29526.60			
		1.0	16093.85	-13432.75			
		2.0	16093.85	2661.11			
		3.0	16093.85	18754.96			
		4.0	16093.85	34848.81			

43	1*****	.0	14554.45	-26186.60			
		1.0	14554.45	-11632.15			
		2.0	14554.45	2922.29			
		3.0	14554.45	17476.74			
		4.0	14554.45	32031.19			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	1-2 PLANE MOMENT	1-3 PLANE SHEAR	1-3 PLANE MOMENT	AXIAL TORQ
1-73074.63						
	.0	13348.20	-22833.74			
	1.0	13348.20	-9485.55			
	2.0	13348.20	3862.65			
	3.0	13348.20	17210.85			
	4.0	13348.20	30559.05			
1-26289.53						
	.0	10536.15	-16371.31			
	1.0	10536.15	-5835.16			
	2.0	10536.15	4700.99			
	3.0	10536.15	15237.15			
	4.0	10536.15	25773.30			
1*****						
	.0	74634.84	-283906.31			
	1.0	74634.84	-209271.47			
	2.0	74634.84	-134636.62			
	3.0	74634.84	-60001.78			
	4.0	74634.84	14633.06			
1*****						
	.0	67809.13	-168872.03			
	1.0	67809.13	-101062.90			
	2.0	67809.13	-33253.77			
	3.0	67809.13	34555.36			
	4.0	67809.13	102364.48			
1*****						
	.0	64289.81	-119775.86			
	1.0	64289.81	-55486.05			
	2.0	64289.81	8803.75			
	3.0	64289.81	73093.56			
	4.0	64289.81	137383.37			
1*****						
	.0	73214.24	-71687.06			
	1.0	73214.24	1527.18			
	2.0	73214.24	74741.42			
	3.0	73214.24	147955.65			
	4.0	73214.24	221169.89			
1*****						
	.0	-19656.09	100142.92			
	1.0	-19656.09	80486.83			
	2.0	-19656.09	60830.74			
	3.0	-19656.09	41174.65			
	4.0	-19656.09	21518.56			
1*****						
	.0	12348.18	-25474.59			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL DIST	1-2 PLANE		1-3 PLANE	AXIAL
COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	TORQ
	1.0	12348.18	-13126.40		
	2.0	12348.18	-778.22		
	3.0	12348.18	11569.96		
	4.0	12348.18	23918.14		
8	1*****				
	.0	12546.59	-24258.68		
	1.0	12546.59	-11712.09		
	2.0	12546.59	834.49		
	3.0	12546.59	13381.08		
	4.0	12546.59	25927.67		
12	1*****				
	.0	11434.52	-20683.85		
	1.0	11434.52	-9249.34		
	2.0	11434.52	2185.18		
	3.0	11434.52	13619.70		
	4.0	11434.52	25054.21		
36	1*****				
	.0	10858.57	-19181.75		
	1.0	10858.57	-8323.18		
	2.0	10858.57	2535.39		
	3.0	10858.57	13393.96		
	4.0	10858.57	24252.53		
10	1*****				
	.0	10094.86	-17249.18		
	1.0	10094.86	-7154.32		
	2.0	10094.86	2940.55		
	3.0	10094.86	13035.41		
	4.0	10094.86	23130.27		
14	1-71023.26				
	.0	9336.73	-15239.06		
	1.0	9336.73	-5902.33		
	2.0	9336.73	3434.39		
	3.0	9336.73	12771.12		
	4.0	9336.73	22107.84		
48	1-36172.99				
	.0	8577.77	-12621.20		
	1.0	8577.77	-4043.42		
	2.0	8577.77	4534.35		
	3.0	8577.77	13112.12		
	4.0	8577.77	21689.90		
52	1-11040.02				
	.0	5807.84	-8007.12		
	1.0	5807.84	-2199.28		
	2.0	5807.84	3608.56		
	3.0	5807.84	9416.40		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	6336.11	.0	5807.84	15224.24			
		2.0	-25315.31	141279.33			
		4.0	-30855.31	85108.72			
		6.0	-44595.31	17858.11			
		8.0	-50135.31	-76872.51			
			-55675.31	-182683.12			

5	1	-5960.49	.0	-33907.66	176128.08		
			2.0	-39447.66	102772.77		
			4.0	-53187.66	18337.45		
			6.0	-58727.66	-93577.87		
			8.0	-64267.66	-216573.18		

9	1	-1717.50	.0	-27896.08	151192.54		
			2.0	-33436.08	89860.38		
			4.0	-47176.08	17448.23		
			6.0	-52716.08	-82443.93		
			8.0	-58256.08	-193416.09		

4	1	2515.99	.0	6956.58	53081.17		
			3.0	-1353.42	61485.91		
			6.0	-17863.42	28560.66		
			9.0	-34373.42	-45694.59		
			12.0	-42683.42	-161279.85		

7	1	-5210.16	.0	2507.19	79653.13		
			3.0	-5802.81	74709.70		
			6.0	-22312.81	28436.28		
			9.0	-38822.81	-59167.14		
			12.0	-47132.81	-188100.57		

50	1	8873.74	.0	4156.44	69196.57		
			3.0	-4153.56	69200.88		
			6.0	-20663.56	27875.18		
			9.0	-37173.56	-54780.51		
			12.0	-45483.56	-178766.20		

35	1	-6825.71	.0	-24301.99	132350.81		
			2.0	-29841.99	78206.83		
			4.0	-43581.99	12982.86		
			6.0	-49121.99	-79721.11		
			8.0	-54661.99	-183505.09		

58	1	-3519.32					

sa Statis Portal (13 lantai) Kg/m

M E		E L E M E N T			F O R C E S	
LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE	AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	TORQ
		.0	-33874.61	170296.54		
		2.0	-39414.61	97007.32		
		4.0	-53154.61	12638.10		
		6.0	-58694.61	-99211.12		
		8.0	-64234.61	-222140.35		
1	8924.43	.0	-30556.68	156822.99		
		2.0	-36096.68	90169.63		
		4.0	-49836.68	12436.28		
		6.0	-55376.68	-92777.07		
		8.0	-60916.68	-209070.42		
2	67049.61	.0	-1442.30	52754.53		
		2.0	-8582.30	42729.92		
		4.0	-23922.30	18425.31		
		6.0	-31062.30	-36559.29		
		8.0	-38202.30	-105823.90		
5	39754.83	.0	14719.73	-12164.27		
		2.0	7579.73	10135.19		
		4.0	-7760.27	18154.65		
		6.0	-14900.27	-4505.89		
		8.0	-22040.27	-41446.43		
8	30862.97	.0	14224.01	-10196.83		
		2.0	7084.01	11111.19		
		4.0	-8255.99	18139.20		
		6.0	-15395.99	-5512.79		
		8.0	-22535.99	-43444.77		
11	17646.06	.0	15124.24	-13807.82		
		2.0	7984.24	9300.67		
		4.0	-7355.76	18129.16		
		6.0	-14495.76	-3722.35		
		8.0	-21635.76	-39853.87		
74	5042.75	.0	15888.31	-16912.30		
		2.0	8748.31	7724.33		
		4.0	-6591.69	18080.95		
		6.0	-13731.69	-2242.43		
		8.0	-20871.69	-36845.81		
77	-7480.31	.0	16686.04	-20141.08		
		2.0	9546.04	6090.99		
		4.0	-5793.96	18043.06		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		6.0	-12933.96	-684.87			
		8.0	-20073.96	-33692.80			

1	20779.57						
		.0	17549.61	-23633.87			
		2.0	10409.61	4325.35			
		4.0	-4930.39	18004.57			
		6.0	-12070.39	1003.78			
		8.0	-19210.39	-30277.00			

3	35099.98						
		.0	18499.14	-27470.98			
		2.0	11359.14	2387.31			
		4.0	-3980.86	17965.59			
		6.0	-11120.86	2863.87			
		8.0	-18260.86	-26517.85			

6	48288.90						
		.0	19510.90	-31670.21			
		2.0	12370.90	211.58			
		4.0	-2969.10	17813.37			
		6.0	-10109.10	4735.16			
		8.0	-17249.10	-22623.05			

3	80035.97						
		.0	18834.21	533.08			
		3.0	8124.21	40970.70			
		6.0	-10785.79	32878.33			
		9.0	-29695.79	-23744.05			
		12.0	-40405.79	-128896.43			

6	96477.04						
		.0	26110.69	-42571.81			
		3.0	15400.69	19695.24			
		6.0	-3509.31	33432.30			
		9.0	-22419.31	-1360.64			
		12.0	-33129.31	-84683.59			

9	87426.21						
		.0	25817.49	-40677.66			
		3.0	15107.49	20709.80			
		6.0	-3802.51	33567.25			
		9.0	-22712.51	-2105.29			
		12.0	-33422.51	-86307.83			

72	75722.33						
		.0	26153.05	-42681.54			
		3.0	15443.05	19712.61			
		6.0	-3466.95	33576.76			
		9.0	-22376.95	-1089.09			
		12.0	-33086.95	-84284.93			

a Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
1	65718.24					
	.0	26457.78	-44508.07			
	3.0	15747.78	18800.27			
	6.0	-3162.22	33578.61			
	9.0	-22072.22	-173.05			
	12.0	-32782.22	-82454.71			
3	1	54790.97				
	.0	26827.36	-46700.02			
	3.0	16117.36	17717.05			
	6.0	-2792.64	33604.13			
	9.0	-21702.64	961.20			
	12.0	-32412.64	-80211.73			
1	1	42661.49				
	.0	27245.92	-49191.54			
	3.0	16535.92	16481.21			
	6.0	-2374.08	33623.97			
	9.0	-21284.08	2236.72			
	12.0	-31994.08	-77680.53			
4	1	29931.35				
	.0	27701.42	-51926.68			
	3.0	16991.42	15112.58			
	6.0	-1918.58	33621.85			
	9.0	-20828.58	3601.11			
	12.0	-31538.58	-74949.63			
7	1	15283.14				
	.0	28208.58	-54836.97			
	3.0	17498.58	13723.75			
	6.0	-1411.42	33754.48			
	9.0	-20321.42	5255.21			
	12.0	-31031.42	-71774.06			
64	1	50242.65				
	.0	-4391.55	61614.52			
	2.0	-11531.55	45691.42			
	4.0	-26871.55	15488.31			
	6.0	-34011.55	-45394.80			
	8.0	-41151.55	-120557.90			
67	1	1109591.21				
	.0	13620.72	-8784.09			
	2.0	6480.72	11317.35			
	4.0	-8859.28	17138.79			
	6.0	-15999.28	-7719.76			
	8.0	-23139.28	-46858.32			
70	1	77786.30				
	.0	13253.66	-7037.58			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	AXIAL TORQ
			SHEAR	MOMENT		
		2.0	6113.66	12329.74		
		4.0	-9226.34	17417.07		
		6.0	-16366.34	-8175.60		
		8.0	-23506.34	-48048.28		

1	69339.77	.0	13701.96	-9068.66		
		2.0	6561.96	11195.26		
		4.0	-8778.04	17179.18		
		6.0	-15918.04	-7516.91		
		8.0	-23058.04	-46492.99		

5	61329.16	.0	14265.84	-11221.70		
		2.0	7125.84	10169.98		
		4.0	-8214.16	17281.66		
		6.0	-15354.16	-6286.66		
		8.0	-22494.16	-44134.98		

9	51538.61	.0	14930.96	-13827.56		
		2.0	7790.96	8894.35		
		4.0	-7549.04	17336.27		
		6.0	-14689.04	-4901.82		
		8.0	-21829.04	-41419.90		

2	40801.28	.0	15693.75	-16818.75		
		2.0	8553.75	7428.74		
		4.0	-6786.25	17396.24		
		6.0	-13926.25	-3316.27		
		8.0	-21066.25	-38308.78		

5	28547.62	.0	16569.76	-20212.90		
		2.0	9429.76	5786.62		
		4.0	-5910.24	17506.13		
		6.0	-13050.24	-1454.35		
		8.0	-20190.24	-34694.83		

8	9546.11	.0	17783.18	-24922.44		
		2.0	10643.18	3503.92		
		4.0	-4696.82	17650.28		
		6.0	-11836.82	1116.64		
		8.0	-18976.82	-29697.00		

9	1-56306.23	.0	12771.07	-21651.55		
		2.0	8391.07	-489.41		
		4.0	-1868.93	11912.72		
		6.0	-6248.93	3794.86		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

1-3 PLANE AXIAL
SHEAR MOMENT TORQ

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT
	8.0	-10628.93	-13083.00

1	-5807.84		
	.0	12359.98	-20504.08
	2.0	7979.98	-164.12
	4.0	-2280.02	11415.84
	6.0	-6660.02	2475.80
	8.0	-11040.02	-15224.24

1	-28660.04		
	.0	17954.29	-33503.87
	3.0	11384.29	10504.01
	6.0	-1065.71	23041.89
	9.0	-13515.71	4109.76
	12.0	-20085.71	-46292.36

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT
		3.0	96588.46	115410.99		
		4.0	96588.46	211999.45		

9	1*****	.0	95258.93	-119538.64		
		1.0	95258.93	-24279.71		
		2.0	95258.93	70979.23		
		3.0	95258.93	166238.16		
		4.0	95258.93	261497.10		

3	1*****	.0	-11671.21	75371.99		
		1.0	-11671.21	63700.78		
		2.0	-11671.21	52029.56		
		3.0	-11671.21	40358.35		
		4.0	-11671.21	28687.14		

17	1*****	.0	22050.18	-45581.01		
		1.0	22050.18	-23530.82		
		2.0	22050.18	-1480.64		
		3.0	22050.18	20569.54		
		4.0	22050.18	42619.73		

1	1*****	.0	18138.99	-33883.31		
		1.0	18138.99	-15744.31		
		2.0	18138.99	2394.68		
		3.0	18138.99	20533.68		
		4.0	18138.99	38672.67		

15	1*****	.0	17561.68	-32979.02		
		1.0	17561.68	-15417.34		
		2.0	17561.68	2144.33		
		3.0	17561.68	19706.01		
		4.0	17561.68	37267.69		

19	1*****	.0	16151.79	-29622.33		
		1.0	16151.79	-13470.54		
		2.0	16151.79	2681.24		
		3.0	16151.79	18833.03		
		4.0	16151.79	34984.81		

3	1*****	.0	14614.30	-26305.11		
		1.0	14614.30	-11690.81		
		2.0	14614.30	2923.49		
		3.0	14614.30	17537.79		
		4.0	14614.30	32152.09		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

1-3 PLANE AXIAL
SHEAR MOMENT TORQ

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT
		4.0	85120.54	134918.13

1*****

.0	85087.84	-172662.62
1.0	85087.84	-87574.78
2.0	85087.84	-2486.94
3.0	85087.84	82600.90
4.0	85087.84	167688.73

4 -----

1*****

.0	81692.41	-140631.99
1.0	81692.41	-58939.58
2.0	81692.41	22752.83
3.0	81692.41	104445.24
4.0	81692.41	186137.64

8 -----

1*****

.0	69824.49	-70162.47
1.0	69824.49	-337.98
2.0	69824.49	69486.50
3.0	69824.49	139310.99
4.0	69824.49	209135.48

2 -----

1*****

.0	-30412.47	108911.74
1.0	-30412.47	78499.26
2.0	-30412.47	48086.79
3.0	-30412.47	17674.31
4.0	-30412.47	-12738.16

16 -----

1*****

.0	3132.05	-8610.01
1.0	3132.05	-5477.96
2.0	3132.05	-2345.91
3.0	3132.05	786.13
4.0	3132.05	3918.18

50 -----

1*****

.0	-1407.19	5191.00
1.0	-1407.19	3783.81
2.0	-1407.19	2376.63
3.0	-1407.19	969.44
4.0	-1407.19	-437.75

34 -----

1*****

.0	-2423.08	7006.59
1.0	-2423.08	4583.51
2.0	-2423.08	2160.42
3.0	-2423.08	-262.66
4.0	-2423.08	-2685.74

38 -----

1-85049.64

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	76121.94						
		.0	66562.71	-274236.61			
		1.0	66562.71	-207673.90			
		2.0	66562.71	-141111.18			
		3.0	66562.71	-74548.47			
		4.0	66562.71	-7985.76			

1	50561.44						
		.0	56272.08	-150243.72			
		1.0	56272.08	-93971.63			
		2.0	56272.08	-37699.55			
		3.0	56272.08	18572.53			
		4.0	56272.08	74844.62			

1	15029.67						
		.0	52640.81	-107808.42			
		1.0	52640.81	-55167.61			
		2.0	52640.81	-2526.80			
		3.0	52640.81	50114.01			
		4.0	52640.81	102754.82			

1	20130.07						
		.0	52758.54	-78418.68			
		1.0	52758.54	-25660.13			
		2.0	52758.54	27098.41			
		3.0	52758.54	79856.95			
		4.0	52758.54	132615.50			

1	46623.05						
		.0	43801.08	-12958.40			
		1.0	43801.08	30842.69			
		2.0	43801.08	74643.77			
		3.0	43801.08	118444.85			
		4.0	43801.08	162245.94			

1	46764.87						
		.0	-32626.19	114650.11			
		1.0	-32626.19	82023.92			
		2.0	-32626.19	49397.73			
		3.0	-32626.19	16771.53			
		4.0	-32626.19	-15854.66			

1	72143.80						
		.0	-553.76	-916.27			
		1.0	-553.76	-1470.03			
		2.0	-553.76	-2023.80			
		3.0	-553.76	-2577.56			
		4.0	-553.76	-3131.32			

1	77201.96						
		.0	-3972.63	10376.03			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-19062.55	78536.11			
		2.0	-19062.55	59473.56			
		3.0	-19062.55	40411.02			
		4.0	-19062.55	21348.47			
3	-----						
	1*****						
		.0	11882.09	-23961.02			
		1.0	11882.09	-12078.93			
		2.0	11882.09	-196.84			
		3.0	11882.09	11685.25			
		4.0	11882.09	23567.34			
2	-----						
	1*****						
		.0	11953.57	-22530.82			
		1.0	11953.57	-10577.25			
		2.0	11953.57	1376.32			
		3.0	11953.57	13329.89			
		4.0	11953.57	25283.47			
6	-----						
	1*****						
		.0	10767.51	-18880.92			
		1.0	10767.51	-8113.41			
		2.0	10767.51	2654.11			
		3.0	10767.51	13421.62			
		4.0	10767.51	24189.14			
0	-----						
	1*****						
		.0	10082.42	-17226.69			
		1.0	10082.42	-7144.27			
		2.0	10082.42	2938.15			
		3.0	10082.42	13020.56			
		4.0	10082.42	23102.98			
4	-----						
	1-70759.25						
		.0	9322.05	-15196.99			
		1.0	9322.05	-5874.93			
		2.0	9322.05	3447.12			
		3.0	9322.05	12769.18			
		4.0	9322.05	22091.23			
18	-----						
	1-35995.67						
		.0	8563.04	-12573.15			
		1.0	8563.04	-4010.12			
		2.0	8563.04	4552.92			
		3.0	8563.04	13115.96			
		4.0	8563.04	21678.99			
12	-----						
	1-11013.74						
		.0	5769.03	-7939.25			
		1.0	5769.03	-2170.23			
		2.0	5769.03	3598.80			
		3.0	5769.03	9367.82			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

	1-73813.53						
	.0		13419.30	-22956.37			
	1.0		13419.30	-9537.07			
	2.0		13419.30	3882.24			
	3.0		13419.30	17301.54			
	4.0		13419.30	30720.85			
1	-----						
	1-26493.50						
	.0		10584.32	-16419.33			
	1.0		10584.32	-5835.01			
	2.0		10584.32	4749.31			
	3.0		10584.32	15333.64			
	4.0		10584.32	25917.96			
4	-----						
	1*****						
	.0		74129.83	-284127.08			
	1.0		74129.83	-209997.25			
	2.0		74129.83	-135867.43			
	3.0		74129.83	-61737.60			
	4.0		74129.83	12392.22			
8	-----						
	1*****						
	.0		67460.61	-172479.05			
	1.0		67460.61	-105018.44			
	2.0		67460.61	-37557.83			
	3.0		67460.61	29902.79			
	4.0		67460.61	97363.40			
2	-----						
	1*****						
	.0		64332.99	-131355.59			
	1.0		64332.99	-67022.59			
	2.0		64332.99	-2689.60			
	3.0		64332.99	61643.40			
	4.0		64332.99	125976.39			
.6	-----						
	1*****						
	.0		61413.59	-104225.10			
	1.0		61413.59	-42811.51			
	2.0		61413.59	18602.08			
	3.0		61413.59	80015.67			
	4.0		61413.59	141429.26			
20	-----						
	1*****						
	.0		69791.49	-64234.62			
	1.0		69791.49	5556.88			
	2.0		69791.49	75348.37			
	3.0		69791.49	145139.86			
	4.0		69791.49	214931.36			
24	-----						
	1*****						
	.0		-19062.55	97598.66			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3829.03	10192.67			
		1.0	-3829.03	6363.64			
		2.0	-3829.03	2534.61			
		3.0	-3829.03	-1294.43			
		4.0	-3829.03	-5123.46			
2	1-59607.94						
		.0	-5379.13	13703.42			
		1.0	-5379.13	8324.30			
		2.0	-5379.13	2945.17			
		3.0	-5379.13	-2433.96			
		4.0	-5379.13	-7813.08			
6	1-36971.49						
		.0	-7308.59	17544.06			
		1.0	-7308.59	10235.47			
		2.0	-7308.59	2926.87			
		3.0	-7308.59	-4381.72			
		4.0	-7308.59	-11690.31			
0	1-15438.41						
		.0	-10206.02	20485.64			
		1.0	-10206.02	10279.62			
		2.0	-10206.02	73.60			
		3.0	-10206.02	-10132.41			
		4.0	-10206.02	-20338.43			
3	1*****						
		.0	91062.59	-305749.81			
		1.0	91062.59	-214687.21			
		2.0	91062.59	-123624.62			
		3.0	91062.59	-32562.03			
		4.0	91062.59	58500.56			
7	1*****						
		.0	100820.76	-237491.95			
		1.0	100820.76	-136671.20			
		2.0	100820.76	-35850.44			
		3.0	100820.76	64970.32			
		4.0	100820.76	165791.08			
1	1*****						
		.0	100724.36	-203919.92			
		1.0	100724.36	-103195.56			
		2.0	100724.36	-2471.20			
		3.0	100724.36	98253.15			
		4.0	100724.36	198977.51			
5	1*****						
		.0	96588.46	-174354.38			
		1.0	96588.46	-77765.92			
		2.0	96588.46	18822.54			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-3972.63	6403.39			
		2.0	-3972.63	2430.76			
		3.0	-3972.63	-1541.88			
		4.0	-3972.63	-5514.51			

	1-77807.45	.0	-4650.09	11714.61			
		1.0	-4650.09	7064.52			
		2.0	-4650.09	2414.44			
		3.0	-4650.09	-2235.65			
		4.0	-4650.09	-6885.74			
7	-----						
	1-72827.30	.0	-5456.43	13646.71			
		1.0	-5456.43	8190.28			
		2.0	-5456.43	2733.85			
		3.0	-5456.43	-2722.59			
		4.0	-5456.43	-8179.02			
1	-----						
	1-62193.25	.0	-6355.80	15835.66			
		1.0	-6355.80	9479.86			
		2.0	-6355.80	3124.06			
		3.0	-6355.80	-3231.74			
		4.0	-6355.80	-9587.54			
5	-----						
	1-45341.31	.0	-7303.78	18250.65			
		1.0	-7303.78	10946.87			
		2.0	-7303.78	3643.09			
		3.0	-7303.78	-3660.69			
		4.0	-7303.78	-10964.48			
19	-----						
	1-21533.79	.0	-10754.44	21067.28			
		1.0	-10754.44	10312.84			
		2.0	-10754.44	-441.59			
		3.0	-10754.44	-11196.03			
		4.0	-10754.44	-21950.46			
2	-----						
	1*****	.0	81362.87	-293262.47			
		1.0	81362.87	-211899.60			
		2.0	81362.87	-130536.73			
		3.0	81362.87	-49173.86			
		4.0	81362.87	32189.01			
6	-----						
	1*****	.0	85120.54	-205564.04			
		1.0	85120.54	-120443.50			
		2.0	85120.54	-35322.96			
		3.0	85120.54	49797.59			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		8.0	-10556.00	-12776.34			

1	-5769.03						
		.0	12386.26	-20626.95			
		2.0	8006.26	-234.43			
		4.0	-2253.74	11398.10			
		6.0	-6633.74	2510.62			
		8.0	-11013.74	-15136.85			

0							
	1-28409.38						
		.0	17906.79	-33202.11			
		3.0	11336.79	10663.25			
		6.0	-1113.21	23058.61			
		9.0	-13563.21	3983.96			
		12.0	-20133.21	-46560.68			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		2.0	6891.24	10524.73			
		4.0	-8448.76	17167.21			
		6.0	-15588.76	-6870.31			
		8.0	-22728.76	-45187.83			
3	1	70001.98					
		.0	13763.12	-9049.79			
		2.0	6623.12	11336.46			
		4.0	-8716.88	17442.71			
		6.0	-15856.88	-7131.04			
		8.0	-22996.88	-45984.79			
16	1	60567.38					
		.0	14298.14	-11408.39			
		2.0	7158.14	10047.89			
		4.0	-8181.86	17224.17			
		6.0	-15321.86	-6279.55			
		8.0	-22461.86	-44063.28			
79	1	51443.90					
		.0	14952.99	-13918.39			
		2.0	7812.99	8847.59			
		4.0	-7527.01	17333.57			
		6.0	-14667.01	-4860.46			
		8.0	-21807.01	-41334.48			
32	1	40653.29					
		.0	15712.89	-16902.84			
		2.0	8572.89	7382.93			
		4.0	-6767.11	17388.70			
		6.0	-13907.11	-3285.53			
		8.0	-21047.11	-38239.76			
35	1	28409.08					
		.0	16587.09	-20287.27			
		2.0	9447.09	5746.90			
		4.0	-5892.91	17501.07			
		6.0	-13032.91	-1424.75			
		8.0	-20172.91	-34630.58			
88	1	9262.03					
		.0	17804.05	-25011.20			
		2.0	10664.05	3456.90			
		4.0	-4675.95	17645.00			
		6.0	-11815.95	1153.10			
		8.0	-18955.95	-29618.80			
89	1	-57239.27					
		.0	12844.00	-21928.31			
		2.0	8464.00	-620.32			
		4.0	-1796.00	11927.67			
		6.0	-6176.00	3955.66			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	80703.28						
		.0	25985.38	-41644.15			
		3.0	15275.38	20247.01			
		6.0	-3634.62	33608.16			
		9.0	-22544.62	-1560.69			
		12.0	-33254.62	-85259.54			

5	67447.65						
		.0	26394.07	-44093.69			
		3.0	15684.07	19023.53			
		6.0	-3225.93	33610.74			
		9.0	-22135.93	-332.05			
		12.0	-32845.93	-82804.83			

78	55997.88						
		.0	26763.81	-46314.44			
		3.0	16053.81	17912.00			
		6.0	-2856.19	33608.44			
		9.0	-21766.19	774.88			
		12.0	-32476.19	-80588.68			

31	43831.43						
		.0	27188.94	-48844.25			
		3.0	16478.94	16657.58			
		6.0	-2431.06	33629.42			
		9.0	-21341.06	2071.25			
		12.0	-32051.06	-78016.92			

34	31046.82						
		.0	27648.06	-51602.07			
		3.0	16938.06	15277.12			
		6.0	-1971.94	33626.32			
		9.0	-20881.94	3445.51			
		12.0	-31591.94	-75265.30			

87	16294.86						
		.0	28157.73	-54523.37			
		3.0	17447.73	13884.81			
		6.0	-1462.27	33763.00			
		9.0	-20372.27	5111.18			
		12.0	-31082.27	-72070.64			

67	46595.31						
		.0	-3493.75	58104.72			
		2.0	-10633.75	43977.22			
		4.0	-25973.75	15569.71			
		6.0	-33113.75	-43517.79			
		8.0	-40253.75	-116885.30			

70	1102042.58						
		.0	14031.24	-10397.75			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		6.0	-14552.14	-3846.19			
		8.0	-21692.14	-40090.46			

1	4332.79						
		.0	15992.43	-17303.26			
		2.0	8852.43	7541.60			
		4.0	-6487.57	18106.47			
		6.0	-13627.57	-2008.66			
		8.0	-20767.57	-36403.80			

7							
1	-8773.20						
		.0	16799.10	-20584.44			
		2.0	9659.10	5873.76			
		4.0	-5680.90	18051.96			
		6.0	-12820.90	-449.84			
		8.0	-19960.90	-33231.64			

0							
1	-21956.68						
		.0	17653.60	-24045.19			
		2.0	10513.60	4122.01			
		4.0	-4826.40	18009.20			
		6.0	-11966.40	1216.40			
		8.0	-19106.40	-29856.40			

3							
1	-36139.73						
		.0	18594.08	-27845.52			
		2.0	11454.08	2202.65			
		4.0	-3885.92	17970.81			
		6.0	-11025.92	3058.98			
		8.0	-18165.92	-26132.86			

6							
1	-49216.80						
		.0	19597.49	-32012.83			
		2.0	12457.49	42.14			
		4.0	-2882.51	17817.11			
		6.0	-10022.51	4912.08			
		8.0	-17162.51	-22272.95			

6							
1	77875.83						
		.0	19046.77	-700.32			
		3.0	8336.77	40374.98			
		6.0	-10573.23	32920.28			
		9.0	-29483.23	-23064.42			
		12.0	-40193.23	-127579.12			

9							
1	91820.95						
		.0	26178.32	-42925.57			
		3.0	15468.32	19544.40			
		6.0	-3441.68	33484.36			
		9.0	-22351.68	-1105.67			
		12.0	-33061.68	-84225.70			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	4279.43	68490.15			
		3.0	-4030.57	68863.44			
		6.0	-20540.57	27906.72			
		9.0	-37050.57	-54379.99			
		12.0	-45360.57	-177996.71			
5	1	-6669.21					
		.0	-24637.65	133669.96			
		2.0	-30177.65	78854.65			
		4.0	-43917.65	12959.34			
		6.0	-49457.65	-80415.97			
		8.0	-54997.65	-184871.28			
8	1	-3127.62					
		.0	-35507.98	176784.86			
		2.0	-41047.98	100228.90			
		4.0	-54787.98	12592.93			
		6.0	-60327.98	-102523.03			
		8.0	-65867.98	-228718.99			
31	1	-2919.41					
		.0	-35866.89	178173.62			
		2.0	-41406.89	100899.85			
		4.0	-55146.89	12546.07			
		6.0	-60686.89	-103287.71			
		8.0	-66226.89	-230201.49			
54	1	8377.90					
		.0	-29720.66	153541.38			
		2.0	-35260.66	88560.07			
		4.0	-49000.66	12498.76			
		6.0	-54540.66	-91042.56			
		8.0	-60080.66	-205663.87			
55	1	59205.28					
		.0	-141.82	47595.83			
		2.0	-7281.82	40172.19			
		4.0	-22621.82	18468.54			
		6.0	-29761.82	-33915.10			
		8.0	-36901.82	-100578.74			
68	1	27366.98					
		.0	15473.47	-15146.67			
		2.0	8333.47	8660.27			
		4.0	-7006.53	18187.21			
		6.0	-14146.53	-2965.84			
		8.0	-21286.53	-38398.90			
71	1	17816.05					
		.0	15067.86	-13593.36			
		2.0	7927.86	9402.36			
		4.0	-7412.14	18118.09			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	6846.63						
		.0	-25560.50	142257.96			
		2.0	-31100.50	85596.96			
		4.0	-44840.50	17855.96			
		6.0	-50380.50	-77365.04			
		8.0	-55920.50	-183666.04			

5	1	-3256.73					
		.0	-35531.76	182653.04			
		2.0	-41071.76	106049.51			
		4.0	-54811.76	18365.98			
		6.0	-60351.76	-96797.55			
		8.0	-65891.76	-223041.08			

9	1	-10450.73					
		.0	-35159.74	181173.49			
		2.0	-40699.74	105314.01			
		4.0	-54439.74	18374.53			
		6.0	-59979.74	-96044.94			
		8.0	-65519.74	-221544.42			

2	1	-4819.54					
		.0	-26492.98	145573.90			
		2.0	-32032.98	87047.93			
		4.0	-45772.98	17441.97			
		6.0	-51312.98	-79644.00			
		8.0	-56852.98	-187809.96			

4	1	3088.96					
		.0	6785.87	54087.02			
		3.0	-1524.13	61979.62			
		6.0	-18034.13	28542.23			
		9.0	-34544.13	-46225.16			
		12.0	-42854.13	-162322.55			

7	1	-3224.02					
		.0	1697.85	84539.68			
		3.0	-6612.15	77168.22			
		6.0	-23122.15	28466.77			
		9.0	-39632.15	-61564.69			
		12.0	-47942.15	-192926.14			

0	1	-7055.30					
		.0	1325.45	86776.30			
		3.0	-6984.55	78287.66			
		6.0	-23494.55	28469.02			
		9.0	-40004.55	-62679.62			
		12.0	-48314.55	-195158.26			

3	1	7048.38					

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ

1	89320.55						
		.0	66156.58	-273263.22			
		1.0	66156.58	-207106.64			
		2.0	66156.58	-140950.06			
		3.0	66156.58	-74793.48			
		4.0	66156.58	-8636.90			

1	63870.74						
		.0	55836.09	-150447.37			
		1.0	55836.09	-94611.28			
		2.0	55836.09	-38775.18			
		3.0	55836.09	17060.91			
		4.0	55836.09	72897.00			

1	28182.34						
		.0	51685.56	-110383.44			
		1.0	51685.56	-58697.88			
		2.0	51685.56	-7012.32			
		3.0	51685.56	44673.24			
		4.0	51685.56	96358.80			

1	-8446.69						
		.0	49241.58	-90725.03			
		1.0	49241.58	-41483.45			
		2.0	49241.58	7758.13			
		3.0	49241.58	56999.71			
		4.0	49241.58	106241.29			

1	-41772.00						
		.0	48900.42	-67541.88			
		1.0	48900.42	-18641.47			
		2.0	48900.42	30258.95			
		3.0	48900.42	79159.37			
		4.0	48900.42	128059.78			

1	-65651.13						
		.0	39495.94	-6998.02			
		1.0	39495.94	32497.92			
		2.0	39495.94	71993.85			
		3.0	39495.94	111489.79			
		4.0	39495.94	150985.72			

1	-64131.13						
		.0	-31429.82	110009.12			
		1.0	-31429.82	78579.31			
		2.0	-31429.82	47149.49			
		3.0	-31429.82	15719.67			
		4.0	-31429.82	-15710.15			

1	-82799.98						
		.0	-1733.52	2398.77			

sa Statis Portal (13 lantai) Kg/m

M E M B E R E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-1733.52	665.25			
		2.0	-1733.52	-1068.27			
		3.0	-1733.52	-2801.79			
		4.0	-1733.52	-4535.31			

	1-81952.42	.0	-4897.81	12679.43			
		1.0	-4897.81	7781.62			
		2.0	-4897.81	2883.81			
		3.0	-4897.81	-2014.01			
		4.0	-4897.81	-6911.82			
7	-----						
	1-76293.35	.0	-5639.64	14147.11			
		1.0	-5639.64	8507.47			
		2.0	-5639.64	2867.84			
		3.0	-5639.64	-2771.80			
		4.0	-5639.64	-8411.43			
1	-----						
	1-64685.39	.0	-6509.06	16175.09			
		1.0	-6509.06	9666.03			
		2.0	-6509.06	3156.97			
		3.0	-6509.06	-3352.09			
		4.0	-6509.06	-9861.15			
5	-----						
	1-46921.59	.0	-7437.12	18518.31			
		1.0	-7437.12	11081.19			
		2.0	-7437.12	3644.07			
		3.0	-7437.12	-3793.05			
		4.0	-7437.12	-11230.17			
9	-----						
	1-22267.87	.0	-10914.36	21300.79			
		1.0	-10914.36	10386.43			
		2.0	-10914.36	-527.93			
		3.0	-10914.36	-11442.28			
		4.0	-10914.36	-22356.64			
2	-----						
	1*****	.0	80933.52	-292264.53			
		1.0	80933.52	-211331.01			
		2.0	80933.52	-130397.48			
		3.0	80933.52	-49463.96			
		4.0	80933.52	31469.56			
6	-----						
	1*****	.0	84740.44	-205853.63			
		1.0	84740.44	-121113.19			
		2.0	84740.44	-36372.74			
		3.0	84740.44	48367.70			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
			84740.44	133108.14			

	1*****						
	.0		84667.19	-176156.25			
	1.0		84667.19	-91489.05			
	2.0		84667.19	-6821.86			
	3.0		84667.19	77845.33			
	4.0		84667.19	162512.53			
4	-----						
	1*****						
	.0		81888.51	-156418.49			
	1.0		81888.51	-74529.98			
	2.0		81888.51	7358.53			
	3.0		81888.51	89247.04			
	4.0		81888.51	171135.55			
8	-----						
	1*****						
	.0		77148.80	-128422.10			
	1.0		77148.80	-51273.30			
	2.0		77148.80	25875.50			
	3.0		77148.80	103024.29			
	4.0		77148.80	180173.09			
12	-----						
	1*****						
	.0		64365.37	-61480.36			
	1.0		64365.37	2885.01			
	2.0		64365.37	67250.38			
	3.0		64365.37	131615.74			
	4.0		64365.37	195981.11			
16	-----						
	1*****						
	.0		-29563.44	104955.26			
	1.0		-29563.44	75391.82			
	2.0		-29563.44	45828.38			
	3.0		-29563.44	16264.94			
	4.0		-29563.44	-13298.51			
20	-----						
	1*****						
	.0		1825.60	-5284.00			
	1.0		1825.60	-3458.40			
	2.0		1825.60	-1632.81			
	3.0		1825.60	192.79			
	4.0		1825.60	2018.39			
24	-----						
	1*****						
	.0		-2704.78	8236.87			
	1.0		-2704.78	5532.09			
	2.0		-2704.78	2827.31			
	3.0		-2704.78	122.54			
	4.0		-2704.78	-2582.24			
28	-----						
	1-79791.50						

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-3862.58	10316.23			
		1.0	-3862.58	6453.64			
		2.0	-3862.58	2591.06			
		3.0	-3862.58	-1271.52			
		4.0	-3862.58	-5134.10			

	1-55959.58						
		.0	-5398.95	13759.38			
		1.0	-5398.95	8360.44			
		2.0	-5398.95	2961.49			
		3.0	-5398.95	-2437.45			
		4.0	-5398.95	-7836.40			

	1-34838.99						
		.0	-7335.89	17590.15			
		1.0	-7335.89	10254.26			
		2.0	-7335.89	2918.36			
		3.0	-7335.89	-4417.53			
		4.0	-7335.89	-11753.42			

	1-14680.33						
		.0	-10201.07	20465.48			
		1.0	-10201.07	10264.41			
		2.0	-10201.07	63.34			
		3.0	-10201.07	-10137.73			
		4.0	-10201.07	-20338.80			

	1*****						
		.0	90635.35	-304751.21			
		1.0	90635.35	-214115.86			
		2.0	90635.35	-123480.52			
		3.0	90635.35	-32845.17			
		4.0	90635.35	57790.18			

	1*****						
		.0	100443.39	-237902.94			
		1.0	100443.39	-137459.56			
		2.0	100443.39	-37016.17			
		3.0	100443.39	63427.21			
		4.0	100443.39	163870.60			

	1*****						
		.0	100766.53	-208018.48			
		1.0	100766.53	-107251.95			
		2.0	100766.53	-6485.41			
		3.0	100766.53	94281.12			
		4.0	100766.53	195047.65			

	1*****						
		.0	98202.34	-189004.40			
		1.0	98202.34	-90802.06			
		2.0	98202.34	7400.28			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

1-3 PLANE SHEAR MOMENT AXIAL TORQ

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT
		3.0	98202.34	105602.61
		4.0	98202.34	203804.95

1*****

.0	92610.94	-163260.65
1.0	92610.94	-70649.70
2.0	92610.94	21961.24
3.0	92610.94	114572.18
4.0	92610.94	207183.13

3 1*****

.0	90255.68	-111699.70
1.0	90255.68	-21444.02
2.0	90255.68	68811.65
3.0	90255.68	159067.33
4.0	90255.68	249323.01

27 1*****

.0	-10599.61	70980.69
1.0	-10599.61	60381.08
2.0	-10599.61	49781.47
3.0	-10599.61	39181.86
4.0	-10599.61	28582.25

31 1*****

.0	21086.57	-43033.37
1.0	21086.57	-21946.80
2.0	21086.57	-860.23
3.0	21086.57	20226.34
4.0	21086.57	41312.91

35 1*****

.0	17129.01	-31455.99
1.0	17129.01	-14326.98
2.0	17129.01	2802.03
3.0	17129.01	19931.05
4.0	17129.01	37060.06

39 1*****

.0	16344.34	-30133.43
1.0	16344.34	-13789.09
2.0	16344.34	2555.25
3.0	16344.34	18899.59
4.0	16344.34	35243.93

43 1*****

.0	14701.95	-26461.30
1.0	14701.95	-11759.35
2.0	14701.95	2942.61
3.0	14701.95	17644.56
4.0	14701.95	32346.52

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-74879.10							
	.0		13526.67	-23138.06			
	1.0		13526.67	-9611.39			
	2.0		13526.67	3915.28			
	3.0		13526.67	17441.96			
	4.0		13526.67	30968.63			
1	-----						
1-26785.85							
	.0		10655.44	-16488.21			
	1.0		10655.44	-5832.76			
	2.0		10655.44	4822.68			
	3.0		10655.44	15478.12			
	4.0		10655.44	26133.56			
4	-----						
1*****							
	.0		73716.55	-283137.28			
	1.0		73716.55	-209420.73			
	2.0		73716.55	-135704.18			
	3.0		73716.55	-61987.63			
	4.0		73716.55	11728.91			
8	-----						
1*****							
	.0		66980.08	-172827.92			
	1.0		66980.08	-105847.84			
	2.0		66980.08	-38867.76			
	3.0		66980.08	28112.33			
	4.0		66980.08	95092.41			
12	-----						
1*****							
	.0		63996.71	-134768.61			
	1.0		63996.71	-70771.90			
	2.0		63996.71	-6775.18			
	3.0		63996.71	57221.53			
	4.0		63996.71	121218.24			
16	-----						
1*****							
	.0		61457.57	-115081.40			
	1.0		61457.57	-53623.82			
	2.0		61457.57	7833.75			
	3.0		61457.57	69291.32			
	4.0		61457.57	130748.89			
20	-----						
1*****							
	.0		58361.84	-94634.42			
	1.0		58361.84	-36272.58			
	2.0		58361.84	22089.27			
	3.0		58361.84	80451.11			
	4.0		58361.84	138812.96			
24	-----						
1*****							
	.0		65695.02	-58272.86			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		1.0	65695.02	7422.16		
		2.0	65695.02	73117.18		
		3.0	65695.02	138812.20		
		4.0	65695.02	204507.21		

1	1*****	.0	-17874.72	92607.90		
		1.0	-17874.72	74733.18		
		2.0	-17874.72	56858.46		
		3.0	-17874.72	38983.74		
		4.0	-17874.72	21109.03		

2	1*****	.0	11327.46	-22255.78		
		1.0	11327.46	-10928.33		
		2.0	11327.46	399.13		
		3.0	11327.46	11726.59		
		4.0	11327.46	23054.04		

6	1*****	.0	11257.20	-20620.17		
		1.0	11257.20	-9362.97		
		2.0	11257.20	1894.23		
		3.0	11257.20	13151.42		
		4.0	11257.20	24408.62		

0	1*****	.0	9993.74	-16933.26		
		1.0	9993.74	-6939.52		
		2.0	9993.74	3054.23		
		3.0	9993.74	13047.97		
		4.0	9993.74	23041.72		

14	1-70398.17	.0	9313.45	-15169.90		
		1.0	9313.45	-5856.45		
		2.0	9313.45	3457.01		
		3.0	9313.45	12770.46		
		4.0	9313.45	22083.92		

48	1-35743.97	.0	8542.51	-12498.70		
		1.0	8542.51	-3956.19		
		2.0	8542.51	4586.31		
		3.0	8542.51	13128.82		
		4.0	8542.51	21671.33		

52	1-10975.86	.0	5712.11	-7837.19		
		1.0	5712.11	-2125.08		
		2.0	5712.11	3587.02		
		3.0	5712.11	9299.13		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	6878.49		5712.11	15011.23			
		.0	-25449.80	141810.47			
		2.0	-30989.80	85370.87			
		4.0	-44729.80	17851.26			
		6.0	-50269.80	-77148.34			
		8.0	-55809.80	-183227.94			

6	1	-2733.47					
		.0	-35688.40	183280.44			
		2.0	-41228.40	106363.63			
		4.0	-54968.40	18366.83			
		6.0	-60508.40	-97109.97			
		8.0	-66048.40	-223666.78			

9	1	-7882.02					
		.0	-36629.03	187083.83			
		2.0	-42169.03	108285.77			
		4.0	-55909.03	18407.70			
		6.0	-61449.03	-98950.36			
		8.0	-66989.03	-227388.43			

2	1	-13426.84					
		.0	-33325.31	173783.17			
		2.0	-38865.31	101592.56			
		4.0	-52605.31	18321.95			
		6.0	-58145.31	-92428.66			
		8.0	-63685.31	-214259.27			

5	1	-7805.52					
		.0	-23879.14	135057.81			
		2.0	-29419.14	81759.53			
		4.0	-43159.14	17381.25			
		6.0	-48699.14	-74477.02			
		8.0	-54239.14	-177415.30			

54	1	3071.57					
		.0	6784.61	54095.24			
		3.0	-1525.39	61984.09			
		6.0	-18035.39	28542.93			
		9.0	-34545.39	-46228.23			
		12.0	-42855.39	-162329.39			

57	1	-2660.22					
		.0	1518.88	85597.61			
		3.0	-6791.12	77689.24			
		6.0	-23301.12	28450.87			
		9.0	-39811.12	-62117.49			
		12.0	-48121.12	-194015.86			

50	1	-5103.34					

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT
		.0	536.07	91542.59		
		3.0	-7773.93	80685.80		
		6.0	-24283.93	28499.01		
		9.0	-40793.93	-65017.78		
		12.0	-49103.93	-199864.58		
3	1	-8687.12				
		.0	1577.77	85298.37		
		3.0	-6732.23	77566.68		
		6.0	-23242.23	28504.98		
		9.0	-39752.23	-61886.71		
		12.0	-48062.23	-193608.41		
56	1	4977.91				
		.0	4994.47	64238.15		
		3.0	-3315.53	66756.56		
		6.0	-19825.53	27944.98		
		9.0	-36335.53	-52196.61		
		12.0	-44645.53	-173668.19		
55	1	-6736.47				
		.0	-24560.07	133363.74		
		2.0	-30100.07	78703.59		
		4.0	-43840.07	12963.45		
		6.0	-49380.07	-80256.69		
		8.0	-54920.07	-184556.84		
58	1	-2983.37				
		.0	-35786.78	177873.21		
		2.0	-41326.78	100759.66		
		4.0	-55066.78	12566.10		
		6.0	-60606.78	-103107.46		
		8.0	-66146.78	-229861.02		
51	1	-2539.14				
		.0	-37380.89	184187.47		
		2.0	-42920.89	103885.69		
		4.0	-56660.89	12503.91		
		6.0	-62200.89	-106357.86		
		8.0	-67740.89	-236299.64		
54	1	-3095.73				
		.0	-34675.06	173457.19		
		2.0	-40215.06	98567.06		
		4.0	-53955.06	12596.94		
		6.0	-59495.06	-100853.19		
		8.0	-65035.06	-225383.32		
57	1	7333.17				
		.0	-27607.56	145214.64		
		2.0	-33147.56	84459.52		
		4.0	-46887.56	12624.41		

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ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
COMB			SHEAR	MOMENT	SHEAR	TORQ
		6.0	-52427.56	-86690.70		
		8.0	-57967.56	-197085.81		

3	1	50273.75				
		.0	1520.01	40976.60		
		2.0	-5619.99	36876.62		
		4.0	-20959.99	18496.63		
		6.0	-28099.99	-30563.36		
		8.0	-35239.99	-93903.34		

1	1	14252.94				
		.0	16265.73	-18289.80		
		2.0	9125.73	7101.66		
		4.0	-6214.27	18213.12		
		6.0	-13354.27	-1355.42		
		8.0	-20494.27	-35203.96		

74	1	3923.05				
		.0	15982.44	-17277.81		
		2.0	8842.44	7547.08		
		4.0	-6497.56	18091.97		
		6.0	-13637.56	-2043.14		
		8.0	-20777.56	-36458.25		

77	1	-9895.79				
		.0	16937.23	-21109.09		
		2.0	9797.23	5625.38		
		4.0	-5542.77	18079.84		
		6.0	-12682.77	-145.69		
		8.0	-19822.77	-32651.22		

30	1	-23616.90				
		.0	17798.48	-24613.47		
		2.0	10658.48	3843.49		
		4.0	-4681.52	18020.45		
		6.0	-11821.52	1517.41		
		8.0	-18961.52	-29265.63		

33	1	-37680.86				
		.0	18730.48	-28383.40		
		2.0	11590.48	1937.55		
		4.0	-3749.52	17978.50		
		6.0	-10889.52	3339.45		
		8.0	-18029.52	-25579.60		

36	1	-50599.18				
		.0	19722.92	-32508.94		
		2.0	12582.92	-203.09		
		4.0	-2757.08	17822.76		
		6.0	-9897.08	5168.61		
		8.0	-17037.08	-21765.54		

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ

1	74456.33					
		.0	19469.44	-3188.31		
		3.0	8759.44	39155.02		
		6.0	-10150.56	32968.35		
		9.0	-29060.56	-21748.33		
		12.0	-39770.56	-124995.00		

2	85663.06					
		.0	26296.35	-43584.21		
		3.0	15586.35	19239.85		
		6.0	-3323.65	33533.91		
		9.0	-22233.65	-702.04		
		12.0	-32943.65	-83467.98		

5	72747.06					
		.0	26204.83	-42924.52		
		3.0	15494.83	19624.96		
		6.0	-3415.17	33644.44		
		9.0	-22325.17	-866.09		
		12.0	-33035.17	-83906.61		

8	58194.18					
		.0	26676.34	-45757.23		
		3.0	15966.34	18206.80		
		6.0	-2943.66	33640.82		
		9.0	-21853.66	544.84		
		12.0	-32563.66	-81081.13		

11	45514.45					
		.0	27102.77	-48320.94		
		3.0	16392.77	16922.38		
		6.0	-2517.23	33635.70		
		9.0	-21427.23	1819.01		
		12.0	-32137.23	-78527.67		

34	32689.05					
		.0	27568.72	-51119.31		
		3.0	16858.72	15521.84		
		6.0	-2051.28	33632.99		
		9.0	-20961.28	3214.14		
		12.0	-31671.28	-75734.70		

37	17770.97					
		.0	28082.65	-54060.28		
		3.0	17372.65	14122.67		
		6.0	-1537.35	33775.62		
		9.0	-20447.35	4898.57		
		12.0	-31157.35	-72508.48		

70	42271.70					
		.0	-2170.87	52925.21		

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.0	-9310.87	41443.46			
		4.0	-24650.87	15681.72			
		6.0	-31790.87	-40760.03			
		8.0	-38930.87	-111481.78			
5	-----						
	1	92994.30					
		.0	14505.39	-12261.10			
		2.0	7365.39	9609.67			
		4.0	-7974.61	17200.45			
		6.0	-15114.61	-5888.77			
		8.0	-22254.61	-43258.00			
6	-----						
	1	61197.89					
		.0	14357.82	-11400.58			
		2.0	7217.82	10175.07			
		4.0	-8122.18	17470.71			
		6.0	-15262.18	-5913.64			
		8.0	-22402.18	-43578.00			
9	-----						
	1	50721.94					
		.0	14985.92	-14107.78			
		2.0	7845.92	8724.06			
		4.0	-7494.08	17275.89			
		6.0	-14634.08	-4852.27			
		8.0	-21774.08	-41260.44			
12	-----						
	1	40561.68					
		.0	15736.29	-17002.17			
		2.0	8596.29	7330.40			
		4.0	-6743.71	17382.97			
		6.0	-13883.71	-3244.45			
		8.0	-21023.71	-38151.88			
15	-----						
	1	28223.17					
		.0	16609.32	-20383.93			
		2.0	9469.32	5694.72			
		4.0	-5870.68	17493.37			
		6.0	-13010.68	-1387.99			
		8.0	-20150.68	-34549.34			
18	-----						
	1	8856.77					
		.0	17833.19	-25135.38			
		2.0	10693.19	3390.99			
		4.0	-4646.81	17637.37			
		6.0	-11786.81	1203.74			
		8.0	-18926.81	-29509.88			
19	-----						
	1	58633.20					
		.0	12950.43	-22332.06			
		2.0	8570.43	-811.21			
		4.0	-1689.57	11949.65			
		6.0	-6069.57	4190.50			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	8.0		-10449.57	-12328.65			

1	-5712.11						
	.0		12424.14	-20804.38			
	2.0		8044.14	-336.09			
	4.0		-2215.86	11372.20			
	6.0		-6595.86	2560.48			
	8.0		-10975.86	-15011.23			

0	1-28054.71						
	.0		17836.99	-32758.77			
	3.0		11266.99	10897.21			
	6.0		-1183.01	23083.19			
	9.0		-13633.01	3799.18			
	12.0		-20203.01	-46954.84			

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M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1102507.65							
	.0		65748.65	-271929.07			
	1.0		65748.65	-206180.42			
	2.0		65748.65	-140431.76			
	3.0		65748.65	-74683.11			
	4.0		65748.65	-8934.45			
5	-----						
1 77277.33							
	.0		55428.06	-149859.05			
	1.0		55428.06	-94430.99			
	2.0		55428.06	-39002.93			
	3.0		55428.06	16425.13			
	4.0		55428.06	71853.19			
9	-----						
1 41805.35							
	.0		51222.17	-110558.45			
	1.0		51222.17	-59336.28			
	2.0		51222.17	-8114.11			
	3.0		51222.17	43108.05			
	4.0		51222.17	94330.22			
3	-----						
1 5118.26							
	.0		48269.50	-92987.99			
	1.0		48269.50	-44718.49			
	2.0		48269.50	3551.01			
	3.0		48269.50	51820.52			
	4.0		48269.50	100090.02			
7	-----						
1-29487.09							
	.0		45536.57	-78850.53			
	1.0		45536.57	-33313.96			
	2.0		45536.57	12222.61			
	3.0		45536.57	57759.17			
	4.0		45536.57	103295.74			
21	-----						
1-59646.90							
	.0		44496.18	-57703.73			
	1.0		44496.18	-13207.55			
	2.0		44496.18	31288.63			
	3.0		44496.18	75784.81			
	4.0		44496.18	120280.99			
25	-----						
1-80203.02							
	.0		34609.81	-1391.02			
	1.0		34609.81	33218.79			
	2.0		34609.81	67828.60			
	3.0		34609.81	102438.42			
	4.0		34609.81	137048.23			
29	-----						
1-76749.43							
	.0		-29833.81	103789.14			

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ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	-29833.81	73955.33			
		2.0	-29833.81	44121.52			
		3.0	-29833.81	14287.71			
		4.0	-29833.81	-15546.10			
3	1-87993.21						
		.0	-3031.28	5963.68			
		1.0	-3031.28	2932.41			
		2.0	-3031.28	-98.87			
		3.0	-3031.28	-3130.15			
		4.0	-3031.28	-6161.42			
57	1-80682.73						
		.0	-5909.41	15106.13			
		1.0	-5909.41	9196.72			
		2.0	-5909.41	3287.30			
		3.0	-5909.41	-2622.11			
		4.0	-5909.41	-8531.53			
41	1-68110.11						
		.0	-6730.93	16746.78			
		1.0	-6730.93	10015.85			
		2.0	-6730.93	3284.93			
		3.0	-6730.93	-3446.00			
		4.0	-6730.93	-10176.93			
45	1-49113.63						
		.0	-7633.97	18936.34			
		1.0	-7633.97	11302.37			
		2.0	-7633.97	3668.40			
		3.0	-7633.97	-3965.57			
		4.0	-7633.97	-11599.54			
49	1-23289.24						
		.0	-11137.92	21627.86			
		1.0	-11137.92	10489.94			
		2.0	-11137.92	-647.98			
		3.0	-11137.92	-11785.90			
		4.0	-11137.92	-22923.81			
2	1*****						
		.0	80477.58	-290871.42			
		1.0	80477.58	-210393.84			
		2.0	80477.58	-129916.27			
		3.0	80477.58	-49438.69			
		4.0	80477.58	31038.88			
6	1*****						
		.0	84274.57	-205126.98			
		1.0	84274.57	-120852.41			
		2.0	84274.57	-36577.84			
		3.0	84274.57	47696.73			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TOR
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		4.0	84274.57	131971.31			

	1*****						
		.0	84270.98	-176441.22			
		1.0	84270.98	-92170.24			
		2.0	84270.98	-7899.26			
		3.0	84270.98	76371.71			
		4.0	84270.98	160642.69			
4	-----						
	1*****						
		.0	81456.83	-159598.86			
		1.0	81456.83	-78142.03			
		2.0	81456.83	3314.79			
		3.0	81456.83	84771.62			
		4.0	81456.83	166228.44			
18	-----						
	1*****						
		.0	77221.58	-142961.18			
		1.0	77221.58	-65739.60			
		2.0	77221.58	11481.99			
		3.0	77221.58	88703.57			
		4.0	77221.58	165925.15			
22	-----						
	1*****						
		.0	71408.99	-115782.25			
		1.0	71408.99	-44373.26			
		2.0	71408.99	27035.74			
		3.0	71408.99	98444.73			
		4.0	71408.99	169853.72			
26	-----						
	1*****						
		.0	57894.03	-52187.74			
		1.0	57894.03	5706.29			
		2.0	57894.03	63600.32			
		3.0	57894.03	121494.35			
		4.0	57894.03	179388.38			
30	-----						
	1*****						
		.0	-28382.37	99585.54			
		1.0	-28382.37	71203.17			
		2.0	-28382.37	42820.81			
		3.0	-28382.37	14438.44			
		4.0	-28382.37	-13943.93			
34	-----						
	1-97182.28						
		.0	325.16	-1548.89			
		1.0	325.16	-1223.74			
		2.0	325.16	-898.58			
		3.0	325.16	-573.43			
		4.0	325.16	-248.27			
38	-----						
	1-72583.80						

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NAME ELEMENT FORCES		1-2 PLANE		1-3 PLANE		AXIAL	
LOAD	AXIAL DIST		SHEAR	MOMENT	SHEAR	MOMENT	TORQ
COMB	FORCE ENDI						
	.0	-4141.68	11514.62				
	1.0	-4141.68	7372.94				
	2.0	-4141.68	3231.26				
	3.0	-4141.68	-910.42				
	4.0	-4141.68	-5052.11				
2	-----						
	1-50914.78						
	.0	-5431.53	13878.90				
	1.0	-5431.53	8447.37				
	2.0	-5431.53	3015.83				
	3.0	-5431.53	-2415.70				
	4.0	-5431.53	-7847.23				
46	-----						
	1-31868.95						
	.0	-7375.23	17661.05				
	1.0	-7375.23	10285.82				
	2.0	-7375.23	2910.59				
	3.0	-7375.23	-4464.64				
	4.0	-7375.23	-11839.87				
50	-----						
	1-13628.00						
	.0	-10196.81	20442.86				
	1.0	-10196.81	10246.05				
	2.0	-10196.81	49.24				
	3.0	-10196.81	-10147.57				
	4.0	-10196.81	-20344.38				
3	-----						
	1*****						
	.0	90200.26	-303385.59				
	1.0	90200.26	-213185.33				
	2.0	90200.26	-122985.06				
	3.0	90200.26	-32784.80				
	4.0	90200.26	57415.47				
7	-----						
	1*****						
	.0	100013.51	-237244.44				
	1.0	100013.51	-137230.93				
	2.0	100013.51	-37217.42				
	3.0	100013.51	62796.09				
	4.0	100013.51	162809.59				
11	-----						
	1*****						
	.0	100412.45	-208495.44				
	1.0	100412.45	-108082.99				
	2.0	100412.45	-7670.55				
	3.0	100412.45	92741.90				
	4.0	100412.45	193154.35				
15	-----						
	1*****						
	.0	98245.35	-192838.58				
	1.0	98245.35	-94593.23				
	2.0	98245.35	3652.12				

sa Statis Portal (13 lantai) Kg/m

TIME ELEMENT FORCES

LOAD COMB	AXIAL FORCE ENDI	DIST	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	3.0		98245.35	101897.47			
	4.0		98245.35	200142.82			
) -----							
	1*****						
	.0		94178.21	-176836.77			
	1.0		94178.21	-82658.57			
	2.0		94178.21	11519.64			
	3.0		94178.21	105697.85			
	4.0		94178.21	199876.05			
3 -----							
	1*****						
	.0		87437.93	-151699.88			
	1.0		87437.93	-64261.95			
	2.0		87437.93	23175.99			
	3.0		87437.93	110613.92			
	4.0		87437.93	198051.85			
27 -----							
	1*****						
	.0		84124.92	-103009.17			
	1.0		84124.92	-18884.26			
	2.0		84124.92	65240.66			
	3.0		84124.92	149365.57			
	4.0		84124.92	233490.49			
51 -----							
	1*****						
	.0		-9213.37	65157.95			
	1.0		-9213.37	55944.58			
	2.0		-9213.37	46731.22			
	3.0		-9213.37	37517.85			
	4.0		-9213.37	28304.48			
55 -----							
	1*****						
	.0		19932.22	-40100.63			
	1.0		19932.22	-20168.41			
	2.0		19932.22	-236.19			
	3.0		19932.22	19696.02			
	4.0		19932.22	39628.24			
39 -----							
	1*****						
	.0		15986.03	-28798.86			
	1.0		15986.03	-12812.83			
	2.0		15986.03	3173.21			
	3.0		15986.03	19159.24			
	4.0		15986.03	35145.27			
43 -----							
	1*****						
	.0		14921.43	-27017.93			
	1.0		14921.43	-12096.50			
	2.0		14921.43	2824.93			
	3.0		14921.43	17746.36			
	4.0		14921.43	32667.79			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

	1-76311.38						
	.0		13674.29	-23371.96			
	1.0		13674.29	-9697.67			
	2.0		13674.29	3976.62			
	3.0		13674.29	17650.90			
	4.0		13674.29	31325.19			
1	-----						
	1-27171.29						
	.0		10755.59	-16583.72			
	1.0		10755.59	-5828.13			
	2.0		10755.59	4927.47			
	3.0		10755.59	15683.06			
	4.0		10755.59	26438.66			
4	-----						
	1*****						
	.0		73333.50	-281836.17			
	1.0		73333.50	-208502.66			
	2.0		73333.50	-135169.16			
	3.0		73333.50	-61835.65			
	4.0		73333.50	11497.85			
8	-----						
	1*****						
	.0		66601.86	-172300.21			
	1.0		66601.86	-105698.34			
	2.0		66601.86	-39096.48			
	3.0		66601.86	27505.38			
	4.0		66601.86	94107.24			
12	-----						
	1*****						
	.0		63528.41	-135158.97			
	1.0		63528.41	-71630.56			
	2.0		63528.41	-8102.16			
	3.0		63528.41	55426.25			
	4.0		63528.41	118954.65			
16	-----						
	1*****						
	.0		61136.32	-118235.48			
	1.0		61136.32	-57099.16			
	2.0		61136.32	4037.16			
	3.0		61136.32	65173.48			
	4.0		61136.32	126309.81			
20	-----						
	1*****						
	.0		58402.65	-104573.89			
	1.0		58402.65	-46171.25			
	2.0		58402.65	12231.40			
	3.0		58402.65	70634.04			
	4.0		58402.65	129036.69			
24	-----						
	1*****						
	.0		54784.90	-86007.67			

sa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT
		1.0	54784.90	-31222.77		
		2.0	54784.90	23562.12		
		3.0	54784.90	78347.02		
		4.0	54784.90	133131.92		
8	-----					
	1*****					
		.0	60846.24	-52291.23		
		1.0	60846.24	8555.01		
		2.0	60846.24	69401.25		
		3.0	60846.24	130247.49		
		4.0	60846.24	191093.73		
32	-----					
	1*****					
		.0	-16303.63	85952.42		
		1.0	-16303.63	69648.79		
		2.0	-16303.63	53345.15		
		3.0	-16303.63	37041.52		
		4.0	-16303.63	20737.89		
36	-----					
	1*****					
		.0	10682.47	-20382.73		
		1.0	10682.47	-9700.27		
		2.0	10682.47	982.20		
		3.0	10682.47	11664.66		
		4.0	10682.47	22347.13		
40	-----					
	1*****					
		.0	10446.88	-18540.11		
		1.0	10446.88	-8093.24		
		2.0	10446.88	2353.64		
		3.0	10446.88	12800.51		
		4.0	10446.88	23247.39		
44	-----					
	1-69947.69					
		.0	9236.48	-14891.91		
		1.0	9236.48	-5655.42		
		2.0	9236.48	3581.06		
		3.0	9236.48	12817.55		
		4.0	9236.48	22054.03		
48	-----					
	1-35418.08					
		.0	8527.10	-12429.70		
		1.0	8527.10	-3902.60		
		2.0	8527.10	4624.51		
		3.0	8527.10	13151.61		
		4.0	8527.10	21678.72		
52	-----					
	1-10925.43					
		.0	5634.20	-7691.90		
		1.0	5634.20	-2057.71		
		2.0	5634.20	3576.49		
		3.0	5634.20	9210.69		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
			5634.20	14844.88			

1	6878.60	.0	-25230.32	140924.59			
		2.0	-30770.32	84923.95			
		4.0	-44510.32	17843.31			
		6.0	-50050.32	-76717.34			
		8.0	-55590.32	-182357.98			

6	1	-2678.11	.0	-35471.98	182411.64		
			2.0	-41011.98	105927.69		
			4.0	-54751.98	18363.73		
			6.0	-60291.98	-96680.22		
			8.0	-65831.98	-222804.17		

39	1	-7373.33	.0	-36687.09	187318.21		
			2.0	-42227.09	108404.03		
			4.0	-55967.09	18409.84		
			6.0	-61507.09	-99064.34		
			8.0	-67047.09	-227618.53		

32	1	-11036.06	.0	-34605.35	178940.55		
			2.0	-40145.35	104189.85		
			4.0	-53885.35	18359.15		
			6.0	-59425.35	-94951.55		
			8.0	-64965.35	-219342.25		

35	1	-16170.61	.0	-30159.81	160999.47		
			2.0	-35699.81	95139.84		
			4.0	-49439.81	18200.21		
			6.0	-54979.81	-86219.42		
			8.0	-60519.81	-201719.05		

38	1	-10766.63	.0	-20556.12	121672.01		
			2.0	-26096.12	75019.77		
			4.0	-39836.12	17287.53		
			6.0	-45376.12	-67924.71		
			8.0	-50916.12	-164216.94		

54	1	3081.60	.0	6832.85	53807.88		
			3.0	-1477.15	61841.42		
			6.0	-17987.15	28544.96		
			9.0	-34497.15	-46081.50		
			12.0	-42807.15	-162037.96		

57	1	-2674.52					

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
		.0	1517.52	85608.36			
		3.0	-6792.48	77695.91			
		6.0	-23302.48	28453.46			
		9.0	-39812.48	-62118.99			
		12.0	-48122.48	-194021.43			

1	-4559.18	.0	353.86	92623.02			
		3.0	-7956.14	81219.60			
		6.0	-24466.14	28486.18			
		9.0	-40976.14	-65577.24			
		12.0	-49286.14	-200970.66			

3	-6800.82	.0	824.51	89847.37			
		3.0	-7485.49	79855.91			
		6.0	-23995.49	28534.44			
		9.0	-40505.49	-64117.02			
		12.0	-48815.49	-198098.49			

6	-10358.02	.0	2468.58	79988.35			
		3.0	-5841.42	74929.09			
		6.0	-22351.42	28539.83			
		9.0	-38861.42	-59179.43			
		12.0	-47171.42	-188228.69			

9	2748.33	.0	6070.52	57824.51			
		3.0	-2239.48	63571.07			
		6.0	-18749.48	27987.63			
		9.0	-35259.48	-48925.82			
		12.0	-43569.48	-167169.26			

15	-6731.64	.0	-24372.50	132621.95			
		2.0	-29912.50	78336.94			
		4.0	-43652.50	12971.94			
		6.0	-49192.50	-79873.06			
		8.0	-54732.50	-183798.06			

38	-3073.46	.0	-35638.73	177283.60			
		2.0	-41178.73	100466.15			
		4.0	-54918.73	12568.69			
		6.0	-60458.73	-102808.76			
		8.0	-65998.73	-229266.21			

31	-2392.08	.0	-37596.55	185022.28			
		2.0	-43136.55	104289.17			
		4.0	-56876.55	12476.07			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		6.0	-62416.55	-106817.03			
		8.0	-67956.55	-237190.14			

1	-2733.68						
		.0	-36040.60	178881.10			
		2.0	-41580.60	101259.90			
		4.0	-55320.60	12558.70			
		6.0	-60860.60	-103622.50			
		8.0	-66400.60	-230883.70			

7	-3617.75						
		.0	-32118.95	163347.25			
		2.0	-37658.95	93569.35			
		4.0	-51398.95	12711.44			
		6.0	-56938.95	-95626.46			
		8.0	-62478.95	-215044.36			

10	6061.34						
		.0	-24734.36	133891.76			
		2.0	-30274.36	78883.04			
		4.0	-44014.36	12794.31			
		6.0	-49554.36	-80774.42			
		8.0	-55094.36	-185423.14			

11	40347.62						
		.0	3453.60	33259.09			
		2.0	-3686.40	33026.28			
		4.0	-19026.40	18513.47			
		6.0	-26166.40	-26679.34			
		8.0	-33306.40	-86152.15			

14	321.93						
		.0	17113.65	-21660.28			
		2.0	9973.65	5427.03			
		4.0	-5366.35	18234.33			
		6.0	-12506.35	361.64			
		8.0	-19646.35	-31791.05			

17	-10965.82						
		.0	16982.17	-21305.98			
		2.0	9842.17	5518.37			
		4.0	-5497.83	18062.72			
		6.0	-12637.83	-72.94			
		8.0	-19777.83	-32488.59			

30	-25236.50						
		.0	17978.27	-25302.38			
		2.0	10838.27	3514.15			
		4.0	-4501.73	18050.69			
		6.0	-11641.73	1907.22			
		8.0	-18781.73	-28516.24			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

	1-39800.12					
	.0	18916.41	-29112.69			
	2.0	11776.41	1580.12			
	4.0	-3563.59	17992.94			
	6.0	-10703.59	3725.76			
	8.0	-17843.59	-24821.43			
6	-----					
	1-52564.76					
	.0	19897.84	-33201.07			
	2.0	12757.84	-545.38			
	4.0	-2582.16	17830.31			
	6.0	-9722.16	5526.00			
	8.0	-16862.16	-21058.31			
'2	-----					
	1 70014.56					
	.0	20050.46	-6620.46			
	3.0	9340.46	37465.92			
	6.0	-9569.54	33022.30			
	9.0	-28479.54	-19951.31			
	12.0	-39189.54	-121454.93			
'5	-----					
	1 78253.15					
	.0	26459.15	-44513.15			
	3.0	15749.15	18799.30			
	6.0	-3160.85	33581.76			
	9.0	-22070.85	-165.79			
	12.0	-32780.85	-82443.34			
'8	-----					
	1 63772.78					
	.0	26466.37	-44461.70			
	3.0	15756.37	18872.41			
	6.0	-3153.63	33676.52			
	9.0	-22063.63	-49.37			
	12.0	-32773.63	-82305.26			
31	-----					
	1 48206.47					
	.0	26990.11	-47612.42			
	3.0	16280.11	17292.90			
	6.0	-2629.89	33668.22			
	9.0	-21539.89	1513.54			
	12.0	-32249.89	-79171.15			
34	-----					
	1 34908.72					
	.0	27457.49	-50444.05			
	3.0	16747.49	15863.43			
	6.0	-2162.51	33640.91			
	9.0	-21072.51	2888.38			
	12.0	-31782.51	-76394.14			
37	-----					
	1 19800.82					
	.0	27978.70	-53418.77			

ASTA S.H. & YAZID, GONDOLAYU PROJECT

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

1-3 PLANE SHEAR MOMENT AXIAL TORQ

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	AXIAL TORQ
3	1	37388.06			
		.0	17268.70	14452.32	
		2.0	-1641.30	33793.42	
		4.0	-20551.30	4604.52	
		6.0	-31261.30	-73114.39	
76	1	37388.06			
		.0	-525.14	46481.06	
		2.0	-7665.14	38290.78	
		4.0	-23005.14	15820.51	
		6.0	-30145.14	-37329.76	
79	1	82575.74			
		.0	15052.40	-14409.72	
		2.0	7912.40	8555.08	
		4.0	-7427.60	17239.87	
		6.0	-14567.60	-4755.33	
89	1	51316.66			
		.0	15042.07	-14106.96	
		2.0	7902.07	8837.19	
		4.0	-7437.93	17501.34	
		6.0	-14577.93	-4514.51	
32	1	39899.88			
		.0	15768.76	-17189.58	
		2.0	8628.76	7207.94	
		4.0	-6711.24	17325.46	
		6.0	-13851.24	-3237.02	
85	1	28087.88			
		.0	16635.28	-20493.40	
		2.0	9495.28	5637.17	
		4.0	-5844.72	17487.73	
		6.0	-12984.72	-1341.71	
88	1	8317.70			
		.0	17870.05	-25293.43	
		2.0	10730.05	3306.67	
		4.0	-4609.95	17626.77	
		6.0	-11749.95	1266.86	
89	1	60602.66			
		.0	13098.95	-22895.85	
		2.0	8718.95	-1077.96	
		4.0	-1541.05	11979.94	
		6.0	-5921.05	4517.83	

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL
		SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	8.0	-10301.05	-11704.27			

'1	1	-5634.20				
	.0	12474.57	-21041.43			
	2.0	8094.57	-472.29			
	4.0	-2165.43	11336.85			
	6.0	-6545.43	2625.98			
	8.0	-10925.43	-14844.88			

90	1	-27600.40				
	.0	17740.58	-32145.49			
	3.0	11170.58	11221.25			
	6.0	-1279.42	23118.00			
	9.0	-13729.42	3544.75			
	12.0	-20299.42	-47498.50			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
COMB			SHEAR	MOMENT	SHEAR	TORQ
	1115638.95	.0	65345.98	-270499.56		
		1.0	65345.98	-205153.58		
		2.0	65345.98	-139807.60		
		3.0	65345.98	-74461.62		
		4.0	65345.98	-9115.64		
5	1 90659.85	.0	55026.73	-149025.92		
		1.0	55026.73	-93999.19		
		2.0	55026.73	-38972.45		
		3.0	55026.73	16054.28		
		4.0	55026.73	71081.01		
9	1 55515.05	.0	50784.37	-110015.32		
		1.0	50784.37	-59230.95		
		2.0	50784.37	-8446.59		
		3.0	50784.37	42337.78		
		4.0	50784.37	93122.15		
13	1 19121.51	.0	47770.63	-93020.97		
		1.0	47770.63	-45250.34		
		2.0	47770.63	2520.28		
		3.0	47770.63	50290.91		
		4.0	47770.63	98061.53		
17	1-15441.33	.0	44546.40	-80712.42		
		1.0	44546.40	-36166.02		
		2.0	44546.40	8380.38		
		3.0	44546.40	52926.78		
		4.0	44546.40	97473.18		
21	1-46662.56	.0	41319.41	-67812.78		
		1.0	41319.41	-26493.37		
		2.0	41319.41	14826.04		
		3.0	41319.41	56145.45		
		4.0	41319.41	97464.86		
25	1-72842.09	.0	39444.20	-47446.22		
		1.0	39444.20	-8002.02		
		2.0	39444.20	31442.18		
		3.0	39444.20	70886.39		
		4.0	39444.20	110330.59		
29	1-89499.47	.0	29081.31	4374.40		

Analisa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
3	1-83850.96	1.0	29081.31	33455.71		
		2.0	29081.31	62537.03		
		3.0	29081.31	91618.34		
		4.0	29081.31	120699.65		
37	1-86864.31	1.0	-27904.91	96211.42		
		2.0	-27904.91	68306.50		
		3.0	-27904.91	40401.59		
		4.0	-27904.91	12496.68		
41	1-72390.84	1.0	-27904.91	-15408.23		
		2.0	-4464.56	9812.07		
		3.0	-4464.56	5347.51		
		4.0	-4464.56	882.95		
45	1-52065.40	1.0	-4464.56	-3581.60		
		2.0	-4464.56	-8046.16		
		3.0	-7033.85	17712.91		
		4.0	-7033.85	10679.06		
49	1-24676.12	1.0	-7033.85	3645.21		
		2.0	-7033.85	-3388.63		
		3.0	-7033.85	-10422.48		
		4.0	-7910.43	19597.65		
2	1*****	1.0	-7910.43	11687.22		
		2.0	-7910.43	3776.78		
		3.0	-7910.43	-4133.65		
		4.0	-7910.43	-12044.08		
6	1*****	1.0	-11453.16	22115.26		
		2.0	-11453.16	10662.10		
		3.0	-11453.16	-791.07		
		4.0	-11453.16	-12244.23		
6	1*****	1.0	-11453.16	-23697.39		
		2.0	80017.32	-289370.65		
		3.0	80017.32	-209353.33		
		4.0	80017.32	-129336.02		
6	1*****	1.0	80017.32	-49318.70		
		2.0	80017.32	30698.61		
		3.0	83782.03	-204098.34		
		4.0	83782.03	-120316.31		

isa Statis Portal (13 lantai) Kg/m

AME ELEMENT FORCES

ELEM	LOAD	COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
					SHEAR	MOMENT	SHEAR	TORQ
7	1-78138.42							
			.0		13961.53	-23994.61		
			1.0		13961.53	-10033.08		
			2.0		13961.53	3928.45		
			3.0		13961.53	17889.98		
			4.0		13961.53	31851.52		
11	1-27654.76							
			.0		10888.22	-16693.79		
			1.0		10888.22	-5805.57		
			2.0		10888.22	5082.65		
			3.0		10888.22	15970.87		
			4.0		10888.22	26859.08		
4	1*****							
			.0		72955.74	-280445.68		
			1.0		72955.74	-207489.94		
			2.0		72955.74	-134534.19		
			3.0		72955.74	-61578.45		
			4.0		72955.74	11377.30		
8	1*****							
			.0		66254.42	-171551.08		
			1.0		66254.42	-105296.66		
			2.0		66254.42	-39042.24		
			3.0		66254.42	27212.18		
			4.0		66254.42	93466.60		
12	1*****							
			.0		63161.86	-134760.67		
			1.0		63161.86	-71598.81		
			2.0		63161.86	-8436.95		
			3.0		63161.86	54724.90		
			4.0		63161.86	117886.76		
16	1*****							
			.0		60679.43	-118565.96		
			1.0		60679.43	-57886.53		
			2.0		60679.43	2792.90		
			3.0		60679.43	63472.33		
			4.0		60679.43	124151.76		
20	1*****							
			.0		58096.34	-107385.23		
			1.0		58096.34	-49288.89		
			2.0		58096.34	8807.45		
			3.0		58096.34	66903.79		
			4.0		58096.34	125000.12		
24	1*****							
			.0		54819.24	-94857.48		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		.0	-1367.40	2582.81		
		1.0	-1367.40	1215.42		
		2.0	-1367.40	-151.98		
		3.0	-1367.40	-1519.37		
		4.0	-1367.40	-2886.77		
2	1-44203.29	.0	-5710.13	15041.32		
		1.0	-5710.13	9331.19		
		2.0	-5710.13	3621.06		
		3.0	-5710.13	-2089.06		
		4.0	-5710.13	-7799.19		
46	1-27893.87	.0	-7434.52	17793.32		
		1.0	-7434.52	10358.80		
		2.0	-7434.52	2924.28		
		3.0	-7434.52	-4510.24		
		4.0	-7434.52	-11944.76		
50	1-12212.01	.0	-10198.90	20428.24		
		1.0	-10198.90	10229.34		
		2.0	-10198.90	30.45		
		3.0	-10198.90	-10168.45		
		4.0	-10198.90	-20367.34		
3	1*****	.0	89761.96	-301916.99		
		1.0	89761.96	-212155.03		
		2.0	89761.96	-122393.07		
		3.0	89761.96	-32631.11		
		4.0	89761.96	57130.85		
7	1*****	.0	99572.82	-236301.00		
		1.0	99572.82	-136728.18		
		2.0	99572.82	-37155.36		
		3.0	99572.82	62417.45		
		4.0	99572.82	161990.27		
11	1*****	.0	100008.29	-207989.50		
		1.0	100008.29	-107981.20		
		2.0	100008.29	-7972.91		
		3.0	100008.29	92035.38		
		4.0	100008.29	192043.68		
15	1*****	.0	97918.68	-193293.59		
		1.0	97918.68	-95374.91		
		2.0	97918.68	2543.77		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

I LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		4.0	83782.03	131029.78		

	1*****	.0	83791.48	-175787.91		
		1.0	83791.48	-91996.43		
		2.0	83791.48	-8204.95		
		3.0	83791.48	75586.53		
		4.0	83791.48	159378.01		
4	-----					
	1*****	.0	81042.26	-159780.37		
		1.0	81042.26	-78738.11		
		2.0	81042.26	2304.16		
		3.0	81042.26	83346.42		
		4.0	81042.26	164388.68		
18	-----					
	1*****	.0	76771.08	-145728.18		
		1.0	76771.08	-68957.10		
		2.0	76771.08	7813.97		
		3.0	76771.08	84585.05		
		4.0	76771.08	161356.13		
22	-----					
	1*****	.0	71336.35	-128816.24		
		1.0	71336.35	-57479.89		
		2.0	71336.35	13856.47		
		3.0	71336.35	85192.82		
		4.0	71336.35	156529.17		
26	-----					
	1*****	.0	64559.76	-101714.01		
		1.0	64559.76	-37154.25		
		2.0	64559.76	27405.51		
		3.0	64559.76	91965.27		
		4.0	64559.76	156525.03		
30	-----					
	1*****	.0	50411.66	-41910.60		
		1.0	50411.66	8501.07		
		2.0	50411.66	58912.73		
		3.0	50411.66	109324.39		
		4.0	50411.66	159736.05		
34	-----					
	1-73457.22	.0	-26930.27	93011.26		
		1.0	-26930.27	66081.00		
		2.0	-26930.27	39150.73		
		3.0	-26930.27	12220.46		
		4.0	-26930.27	-14709.81		
38	-----					
	1-63604.29					

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		1.0	54819.24	-40038.24		
		2.0	54819.24	14781.00		
		3.0	54819.24	69600.24		
		4.0	54819.24	124419.48		

1	1*****	.0	50581.35	-76902.25		
		1.0	50581.35	-26320.90		
		2.0	50581.35	24260.45		
		3.0	50581.35	74841.80		
		4.0	50581.35	125423.16		

2	1*****	.0	55221.90	-45853.47		
		1.0	55221.90	9368.43		
		2.0	55221.90	64590.33		
		3.0	55221.90	119812.23		
		4.0	55221.90	175034.13		

36	1*****	.0	-14416.21	77864.46		
		1.0	-14416.21	63448.25		
		2.0	-14416.21	49032.03		
		3.0	-14416.21	34615.82		
		4.0	-14416.21	20199.61		

0	1*****	.0	9934.99	-18354.99		
		1.0	9934.99	-8420.00		
		2.0	9934.99	1514.99		
		3.0	9934.99	11449.98		
		4.0	9934.99	21384.98		

4	1-69440.92	.0	9657.59	-16347.86		
		1.0	9657.59	-6690.27		
		2.0	9657.59	2967.32		
		3.0	9657.59	12624.91		
		4.0	9657.59	22282.50		

18	1-35019.32	.0	8449.94	-12121.37		
		1.0	8449.94	-3671.43		
		2.0	8449.94	4778.51		
		3.0	8449.94	13228.46		
		4.0	8449.94	21678.40		

52	1-10862.07	.0	5540.85	-7525.22		
		1.0	5540.85	-1984.37		
		2.0	5540.85	3556.49		
		3.0	5540.85	9097.34		

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	97918.68	100462.45			
		4.0	97918.68	198381.13			

	1*****						
		0.0	94216.18	-180313.66			
		1.0	94216.18	-86097.47			
		2.0	94216.18	8118.71			
		3.0	94216.18	102334.90			
		4.0	94216.18	196551.08			

13	1*****						
		0.0	88928.99	-163973.13			
		1.0	88928.99	-75044.13			
		2.0	88928.99	13884.86			
		3.0	88928.99	102813.85			
		4.0	88928.99	191742.85			

27	1*****						
		0.0	81147.69	-138651.54			
		1.0	81147.69	-57503.86			
		2.0	81147.69	23643.83			
		3.0	81147.69	104791.52			
		4.0	81147.69	185939.20			

51	1*****						
		0.0	76901.12	-93165.87			
		1.0	76901.12	-16264.75			
		2.0	76901.12	60636.38			
		3.0	76901.12	137537.50			
		4.0	76901.12	214438.63			

35	1*****						
		0.0	-7564.93	58101.62			
		1.0	-7564.93	50536.69			
		2.0	-7564.93	42971.76			
		3.0	-7564.93	35406.83			
		4.0	-7564.93	27841.90			

39	1*****						
		0.0	18613.72	-36834.07			
		1.0	18613.72	-18220.35			
		2.0	18613.72	393.37			
		3.0	18613.72	19007.10			
		4.0	18613.72	37620.82			

43	1*****						
		0.0	14646.88	-25904.86			
		1.0	14646.88	-11257.98			
		2.0	14646.88	3388.89			
		3.0	14646.88	18035.77			
		4.0	14646.88	32682.65			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		8.0	-10098.83	-10855.43			

1	-5540.85						
		.0	12537.93	-21341.60			
		2.0	8157.93	-645.75			
		4.0	-2102.07	11290.10			
		6.0	-6482.07	2705.96			
		8.0	-10862.07	-14638.19			

10	1-27057.10						
		.0	17612.16	-31327.03			
		3.0	11042.16	11654.46			
		6.0	-1407.84	23165.96			
		9.0	-13857.84	3207.45			
		12.0	-20427.84	-48221.05			

sa Statis Portal (13 lantai) Kg/m

M E M B E R E L E M E N T F O R C E S

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
	4.0	5540.85	14638.19			

1	6874.25					
	.0	-24979.10	139910.28			
	2.0	-30519.10	84412.08			
	4.0	-44259.10	17833.88			
	6.0	-49799.10	-76224.32			
	8.0	-55339.10	-181362.51			

6	1 -2647.64					
	.0	-35144.80	181096.33			
	2.0	-40684.80	105266.73			
	4.0	-54424.80	18357.12			
	6.0	-59964.80	-96032.49			
	8.0	-65504.80	-221502.10			

39	1 -7321.36					
	.0	-36393.54	186143.12			
	2.0	-41933.54	107816.05			
	4.0	-55673.54	18408.97			
	6.0	-61213.54	-98478.10			
	8.0	-66753.54	-226445.17			

32	1-10556.78					
	.0	-34562.84	178773.96			
	2.0	-40102.84	104108.27			
	4.0	-53842.84	18362.59			
	6.0	-59382.84	-94863.09			
	8.0	-64922.84	-219168.77			

35	1-13999.01					
	.0	-31221.23	165285.96			
	2.0	-36761.23	97303.50			
	4.0	-50501.23	18241.04			
	6.0	-56041.23	-88301.42			
	8.0	-61581.23	-205923.88			

68	1-18795.79					
	.0	-26179.53	144911.08			
	2.0	-31719.53	87012.02			
	4.0	-45459.53	18032.97			
	6.0	-50999.53	-78426.09			
	8.0	-56539.53	-185965.14			

71	1-13754.11					
	.0	-16657.37	105956.19			
	2.0	-22197.37	67101.44			
	4.0	-35937.37	17166.69			
	6.0	-41477.37	-60248.06			
	8.0	-47017.37	-148742.80			

54	1 3109.53					

Status Portal (13 lantai) kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	

	1-14622.96						
	.0		18037.76	-25337.23			
	2.0		10897.76	3598.29			
	4.0		-4442.24	18253.81			
	6.0		-11582.24	2229.33			
	8.0		-18722.24	-28075.16			
0	-----						
	1-27089.07						
	.0		18090.59	-25770.63			
	2.0		10950.59	3270.54			
	4.0		-4389.41	18031.71			
	6.0		-11529.41	2112.89			
	8.0		-18669.41	-28085.94			
13	-----						
	1-42049.60						
	.0		19150.47	-30015.37			
	2.0		12010.47	1145.58			
	4.0		-3329.53	18026.53			
	6.0		-10469.53	4227.48			
	8.0		-17609.53	-23851.57			
36	-----						
	1-55222.12						
	.0		20132.72	-34127.30			
	2.0		12992.72	-1001.86			
	4.0		-2347.28	17843.58			
	6.0		-9487.28	6009.02			
	8.0		-16627.28	-20105.54			
45	-----						
	1 64749.77						
	.0		20767.78	-10865.32			
	3.0		10057.78	35373.01			
	6.0		-8852.22	33081.35			
	9.0		-27762.22	-17740.32			
	12.0		-38472.22	-117091.98			
78	-----						
	1 69852.65						
	.0		26656.41	-45650.43			
	3.0		15946.41	18253.79			
	6.0		-2963.59	33628.02			
	9.0		-21873.59	472.24			
	12.0		-32583.59	-81213.54			
31	-----						
	1 54080.07						
	.0		26758.04	-46182.18			
	3.0		16048.04	18026.93			
	6.0		-2861.96	33706.04			
	9.0		-21771.96	855.15			
	12.0		-32481.96	-80525.74			
34	-----						
	1 38184.07						
	.0		27315.77	-49561.81			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		6.0	-48977.38	-79433.61		
		8.0	-54517.38	-182928.37		

1	-3092.56	.0	-35380.88	176259.75		
		2.0	-40920.88	99957.99		
		4.0	-54660.88	12576.24		
		6.0	-60200.88	-102285.52		
		8.0	-65740.88	-228227.27		

1	-2482.43	.0	-37412.34	184286.03		
		2.0	-42952.34	103921.34		
		4.0	-56692.34	12476.65		
		6.0	-62232.34	-106448.03		
		8.0	-67772.34	-236452.72		

4	-2583.09	.0	-36196.78	179477.22		
		2.0	-41736.78	101543.67		
		4.0	-55476.78	12530.12		
		6.0	-61016.78	-103963.43		
		8.0	-66556.78	-231536.99		

7	-3277.09	.0	-33313.88	168093.45		
		2.0	-38853.88	95925.69		
		4.0	-52593.88	12677.92		
		6.0	-58133.88	-98049.84		
		8.0	-63673.88	-219857.61		

0	-4237.89	.0	-28727.99	149942.21		
		2.0	-34267.99	86946.22		
		4.0	-48007.99	12870.24		
		6.0	-53547.99	-88685.75		
		8.0	-59087.99	-201321.74		

3	4640.55	.0	-21249.80	120161.75		
		2.0	-26789.80	72122.16		
		4.0	-40529.80	13002.56		
		6.0	-46069.80	-73597.03		
		8.0	-51609.80	-171276.63		

74	29424.23	.0	5648.50	24488.24		
		2.0	-1491.50	28645.24		
		4.0	-16831.50	18522.25		
		6.0	-23971.50	-22280.75		
		8.0	-31111.50	-77363.75		

isa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TOR
			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	16605.77	16320.51			
		6.0	-2304.23	33672.82			
		9.0	-21214.23	2495.14			
		12.0	-31924.23	-77212.55			
7	1	22449.58					
		.0	27839.15	-52558.42			
		3.0	17129.15	14894.03			
		6.0	-1780.85	33816.49			
		9.0	-20690.85	4208.94			
		12.0	-31400.85	-73928.61			
76	1	31947.70					
		.0	1416.25	38879.42			
		2.0	-5723.75	34571.92			
		4.0	-21063.75	15984.42			
		6.0	-28203.75	-33283.08			
		8.0	-35343.75	-96830.58			
79	1	70782.58					
		.0	15677.93	-16866.76			
		2.0	8537.93	7349.10			
		4.0	-6802.07	17284.96			
		6.0	-13942.07	-3459.17			
		8.0	-21082.07	-38483.31			
82	1	40467.87					
		.0	15819.30	-17191.52			
		2.0	8679.30	7307.07			
		4.0	-6660.70	17525.67			
		6.0	-13800.70	-2935.74			
		8.0	-20940.70	-37677.15			
85	1	27439.94					
		.0	16668.32	-20678.10			
		2.0	9528.32	5518.55			
		4.0	-5811.68	17435.19			
		6.0	-12951.68	-1328.16			
		8.0	-20091.68	-34371.51			
88	1	7718.94					
		.0	17913.75	-25477.37			
		2.0	10773.75	3210.12			
		4.0	-4566.25	17617.62			
		6.0	-11706.25	1345.12			
		8.0	-18846.25	-29207.39			
89	1	-63309.74					
		.0	13301.17	-23664.81			
		2.0	8921.17	-1442.47			
		4.0	-1338.83	12019.88			
		6.0	-5718.83	4962.22			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
					MOMENT	
		.0	6895.37	53434.44		
		3.0	-1414.63	61655.54		
		6.0	-17924.63	28546.64		
		9.0	-34434.63	-45892.25		
		12.0	-42744.63	-161661.15		

1	-2657.09					
		.0	1567.03	85315.60		
		3.0	-6742.97	77551.69		
		6.0	-23252.97	28457.79		
		9.0	-39762.97	-61966.12		
		12.0	-48072.97	-193720.02		

0	-4572.04					
		.0	339.63	92713.21		
		3.0	-7970.37	81267.10		
		6.0	-24480.37	28490.99		
		9.0	-40990.37	-65615.13		
		12.0	-49300.37	-201051.24		

3	-6285.59					
		.0	639.53	90948.09		
		3.0	-7670.47	80401.68		
		6.0	-24180.47	28525.27		
		9.0	-40690.47	-64681.15		
		12.0	-49000.47	-199217.56		

6	-8564.28					
		.0	1763.40	84248.48		
		3.0	-6546.60	77073.67		
		6.0	-23056.60	28568.86		
		9.0	-39566.60	-61265.95		
		12.0	-47876.60	-192430.76		

9	-12019.20					
		.0	3759.15	72278.03		
		3.0	-4550.85	71090.48		
		6.0	-21060.85	28572.93		
		9.0	-37570.85	-55274.63		
		12.0	-45880.85	-180452.18		

2	393.98					
		.0	7433.65	49692.82		
		3.0	-876.35	59528.79		
		6.0	-17386.35	28034.75		
		9.0	-33896.35	-44789.29		
		12.0	-42206.35	-158943.32		

55	-6701.32					
		.0	-24157.38	131770.70		
		2.0	-29697.38	77915.93		
		4.0	-43437.38	12981.16		

sa Statis Portal (13 lantai) Kg/m

M E M B E R F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDJ	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-12196.15	87965.84			
		2.0	-17736.15	58033.53			
		4.0	-31476.15	17021.22			
		6.0	-37016.15	-51471.09			
		8.0	-42556.15	-131043.39			

1	3138.53						
		.0	6962.17	53035.44			
		3.0	-1347.83	61456.95			
		6.0	-17857.83	28548.46			
		9.0	-34367.83	-45690.03			
		12.0	-42677.83	-161258.53			
7							

1	2622.47						
		.0	1631.30	84934.26			
		3.0	-6678.70	77363.15			
		6.0	-23188.70	28462.04			
		9.0	-39698.70	-61769.08			
		12.0	-48008.70	-193330.19			
0							

1	4556.42						
		.0	372.79	92521.03			
		3.0	-7937.21	81174.42			
		6.0	-24447.21	28497.80			
		9.0	-40957.21	-65508.81			
		12.0	-49267.21	-200845.43			
3							

1	6297.41						
		.0	608.69	91140.61			
		3.0	-7701.31	80501.69			
		6.0	-24211.31	28532.77			
		9.0	-40721.31	-64766.15			
		12.0	-49031.31	-199395.07			
6							

1	8084.38						
		.0	1577.96	85356.35			
		3.0	-6732.04	77625.24			
		6.0	-23242.04	28564.13			
		9.0	-39752.04	-61826.98			
		12.0	-48062.04	-193548.09			
9							

1	10340.34						
		.0	3113.11	76183.18			
		3.0	-5196.89	73057.52			
		6.0	-21706.89	28601.86			
		9.0	-38216.89	-57183.80			
		12.0	-46526.89	-184299.46			
12							

1	13663.72						
		.0	5372.68	62630.07			
		3.0	-2937.32	66283.10			
		6.0	-19447.32	28606.13			

sa Statis Portal (15 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
1	6864.65	.0	5372.67	14380.85			
		2.0	-24717.85	138855.51			
		4.0	-30257.85	83879.81			
		6.0	-43997.85	17824.11			
		8.0	-49537.85	-75711.59			
			-55077.85	-180327.29			
1	-2622.00	.0					
		2.0	-34782.83	179640.23			
		4.0	-40322.83	104534.58			
		6.0	-54062.83	18348.93			
		8.0	-59602.83	-95316.72			
			-65142.83	-220062.37			
1	-7292.27	.0					
		2.0	-35993.27	184537.86			
		4.0	-41533.27	107011.33			
		6.0	-55273.27	18404.80			
		8.0	-60813.27	-97681.74			
			-66353.27	-224848.27			
1	-10516.30	.0					
		2.0	-34193.19	177296.96			
		4.0	-39733.19	103370.57			
		6.0	-53473.19	18364.19			
		8.0	-59013.19	-94122.20			
			-64553.19	-217688.59			
1	-13558.32	.0					
		2.0	-31063.38	164659.18			
		4.0	-36603.38	96992.43			
		6.0	-50343.38	18245.68			
		8.0	-55883.38	-87981.07			
			-61423.38	-205287.83			
1	-16880.96	.0					
		2.0	-26985.02	148177.31			
		4.0	-32525.02	88667.27			
		6.0	-46265.02	18077.24			
		8.0	-51805.02	-79992.79			
			-57345.02	-189142.82			
1	-21366.89	.0					
		2.0	-21514.96	126046.67			
		4.0	-27054.96	77476.75			
		6.0	-40794.96	17826.82			
		8.0	-46334.96	-69303.10			
			-51874.96	-167513.03			
1	-16774.27	.0					

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORC
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	54527.00	-42731.57			
		2.0	54527.00	11795.42			
		3.0	54527.00	66322.42			
		4.0	54527.00	120849.42			

		1.0	50604.71	-84496.67			
		1.0	50604.71	-33891.96			
		2.0	50604.71	16712.75			
		3.0	50604.71	67317.47			
		4.0	50604.71	117922.18			

		1.0	45714.19	-66891.94			
		1.0	45714.19	-21177.75			
		2.0	45714.19	24536.44			
		3.0	45714.19	70250.64			
		4.0	45714.19	115964.83			

		1.0	48809.63	-38835.45			
		1.0	48809.63	9974.18			
		2.0	48809.63	58783.82			
		3.0	48809.63	107593.45			
		4.0	48809.63	156403.09			

		1.0	-12242.04	68398.58			
		1.0	-12242.04	56156.54			
		2.0	-12242.04	43914.50			
		3.0	-12242.04	31672.46			
		4.0	-12242.04	19430.42			

		1.0	9233.86	-16232.65			
		1.0	9233.86	-6998.80			
		2.0	9233.86	2235.06			
		3.0	9233.86	11468.92			
		4.0	9233.86	20702.77			

		1.0	8817.26	-13338.84			
		1.0	8817.26	-4521.58			
		2.0	8817.26	4295.68			
		3.0	8817.26	13112.93			
		4.0	8817.26	21930.19			

		1.0	5372.67	-7109.83			
		1.0	5372.67	-1737.16			
		2.0	5372.67	3635.51			
		3.0	5372.67	9008.18			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	1-80502.53						
		.0	13837.02	-23162.46			
		1.0	13837.02	-9325.43			
		2.0	13837.02	4511.59			
		3.0	13837.02	18348.61			
		4.0	13837.02	32185.64			

1	1-28222.46						
		.0	11137.35	-17105.77			
		1.0	11137.35	-5968.42			
		2.0	11137.35	5168.93			
		3.0	11137.35	16306.28			
		4.0	11137.35	27443.63			

4	1*****						
		.0	72580.24	-279028.57			
		1.0	72580.24	-206448.33			
		2.0	72580.24	-133868.09			
		3.0	72580.24	-61287.84			
		4.0	72580.24	11292.40			

8	1*****						
		.0	65913.68	-170740.32			
		1.0	65913.68	-104826.64			
		2.0	65913.68	-38912.95			
		3.0	65913.68	27000.73			
		4.0	65913.68	92914.42			

2	1*****						
		.0	62825.38	-134167.56			
		1.0	62825.38	-71342.18			
		2.0	62825.38	-8516.80			
		3.0	62825.38	54308.58			
		4.0	62825.38	117133.95			

6	1*****						
		.0	60317.77	-118175.53			
		1.0	60317.77	-57857.77			
		2.0	60317.77	2460.00			
		3.0	60317.77	62777.77			
		4.0	60317.77	123095.54			

20	1*****						
		.0	57649.78	-107603.71			
		1.0	57649.78	-49953.93			
		2.0	57649.78	7695.85			
		3.0	57649.78	65345.63			
		4.0	57649.78	122995.40			

24	1*****						
		.0	54527.00	-97258.57			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

E L E M E N T	I T E M	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
					SHEAR	MOMENT	SHEAR	MOMENT	
2	1-35937.94			0	-25225.78	85281.63			
				1.0	-25225.78	60055.85			
				2.0	-25225.78	34830.07			
				3.0	-25225.78	9604.29			
				4.0	-25225.78	-15621.49			
16	1-22704.11			0	-3241.56	7121.11			
				1.0	-3241.56	3879.55			
				2.0	-3241.56	637.99			
				3.0	-3241.56	-2603.57			
				4.0	-3241.56	-5845.13			
50	1-10371.56			0	-10210.79	20437.36			
				1.0	-10210.79	10226.57			
				2.0	-10210.79	15.78			
				3.0	-10210.79	-10195.00			
				4.0	-10210.79	-20405.79			
3	1*****			0	89323.24	-300417.92			
				1.0	89323.24	-211094.69			
				2.0	89323.24	-121771.45			
				3.0	89323.24	-32448.21			
				4.0	89323.24	56875.02			
7	1*****			0	99128.33	-235277.64			
				1.0	99128.33	-136149.32			
				2.0	99128.33	-37020.99			
				3.0	99128.33	62107.34			
				4.0	99128.33	161235.66			
11	1*****			0	99594.16	-207225.89			
				1.0	99594.16	-107631.73			
				2.0	99594.16	-8037.57			
				3.0	99594.16	91556.60			
				4.0	99594.16	191150.76			
15	1*****			0	97545.35	-192847.85			
				1.0	97545.35	-95302.50			
				2.0	97545.35	2242.86			

sa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCE S

LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		3.0	97545.35	99788.21			
		4.0	97545.35	197333.56			
9	-----						
	1*****						
		0.0	93915.93	-180697.65			
		1.0	93915.93	-86781.72			
		2.0	93915.93	7134.21			
		3.0	93915.93	101050.14			
		4.0	93915.93	194966.07			
3	-----						
	1*****						
		0.0	88954.33	-167013.94			
		1.0	88954.33	-78059.61			
		2.0	88954.33	10894.72			
		3.0	88954.33	99849.05			
		4.0	88954.33	188803.38			
27	-----						
	1*****						
		0.0	82536.27	-149406.70			
		1.0	82536.27	-66870.43			
		2.0	82536.27	15665.84			
		3.0	82536.27	98202.11			
		4.0	82536.27	180738.38			
31	-----						
	1*****						
		0.0	73763.06	-123827.42			
		1.0	73763.06	-50064.35			
		2.0	73763.06	23698.71			
		3.0	73763.06	97461.78			
		4.0	73763.06	171224.84			
35	-----						
	1*****						
		0.0	68604.94	-82116.79			
		1.0	68604.94	-13511.85			
		2.0	68604.94	55093.09			
		3.0	68604.94	123698.02			
		4.0	68604.94	192302.96			
39	-----						
	1*****						
		0.0	-5643.66	49826.48			
		1.0	-5643.66	44182.82			
		2.0	-5643.66	38539.15			
		3.0	-5643.66	32895.49			
		4.0	-5643.66	27251.83			
43	-----						
	1*****						
		0.0	17069.49	-33239.38			
		1.0	17069.49	-16169.89			
		2.0	17069.49	899.60			
		3.0	17069.49	17969.09			
		4.0	17069.49	35038.58			

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	33688.46	-2598.16			
		2.0	33688.46	31090.30			
		3.0	33688.46	64778.75			
		4.0	33688.46	98467.21			
5	1-92631.10						
		.0	22854.73	10501.37			
		1.0	22854.73	33356.09			
		2.0	22854.73	56210.82			
		3.0	22854.73	79065.55			
		4.0	22854.73	101920.28			
17	1-84505.15						
		.0	-25665.98	87336.83			
		1.0	-25665.98	61670.84			
		2.0	-25665.98	36004.86			
		3.0	-25665.98	10338.87			
		4.0	-25665.98	-15327.11			
41	1-78293.49						
		.0	-6073.58	14032.61			
		1.0	-6073.58	7959.03			
		2.0	-6073.58	1885.45			
		3.0	-6073.58	-4188.13			
		4.0	-6073.58	-10261.71			
15	1-55715.85						
		.0	-8268.10	20592.85			
		1.0	-8268.10	12324.75			
		2.0	-8268.10	4056.64			
		3.0	-8268.10	-4211.46			
		4.0	-8268.10	-12479.56			
19	1-26518.27						
		.0	-11893.35	22857.98			
		1.0	-11893.35	10964.63			
		2.0	-11893.35	-928.72			
		3.0	-11893.35	-12822.07			
		4.0	-11893.35	-24715.42			
2	1*****						
		.0	79555.16	-287837.41			
		1.0	79555.16	-208282.24			
		2.0	79555.16	-128727.08			
		3.0	79555.16	-49171.91			
		4.0	79555.16	30383.25			
6	1*****						
		.0	83281.27	-202979.49			
		1.0	83281.27	-119698.21			
		2.0	83281.27	-36416.94			
		3.0	83281.27	46864.34			

sa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
COMB			SHEAR	MOMENT	SHEAR	TORQ
		4.0	83281.27	130145.61		
) -----						
	1*****					
		.0	83281.74	-174851.02		
		1.0	83281.74	-91569.28		
		2.0	83281.74	-8287.54		
		3.0	83281.74	74994.20		
		4.0	83281.74	158275.94		
4) -----					
	1*****					
		.0	80545.89	-159093.36		
		1.0	80545.89	-78547.47		
		2.0	80545.89	1998.43		
		3.0	80545.89	82544.32		
		4.0	80545.89	163090.22		
.8) -----					
	1*****					
		.0	76327.00	-145738.98		
		1.0	76327.00	-69411.98		
		2.0	76327.00	6915.02		
		3.0	76327.00	83242.02		
		4.0	76327.00	159569.03		
22) -----					
	1*****					
		.0	70853.07	-131075.15		
		1.0	70853.07	-60222.08		
		2.0	70853.07	10630.98		
		3.0	70853.07	81484.05		
		4.0	70853.07	152337.11		
26) -----					
	1*****					
		.0	64312.45	-112988.89		
		1.0	64312.45	-48676.44		
		2.0	64312.45	15636.01		
		3.0	64312.45	79948.45		
		4.0	64312.45	144260.90		
30) -----					
	1*****					
		.0	56609.28	-85882.19		
		1.0	56609.28	-29272.91		
		2.0	56609.28	27336.38		
		3.0	56609.28	83945.66		
		4.0	56609.28	140554.94		
34) -----					
	1*****					
		.0	41897.70	-30507.76		
		1.0	41897.70	11389.94		
		2.0	41897.70	53287.64		
		3.0	41897.70	95185.35		
		4.0	41897.70	137083.05		
38) -----					
	1-41470.52					

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR MOMENT		1-3 PLANE SHEAR MOMENT	AXIAL TOR
1	1128815.55	.0	64944.36	-269041.84	
		1.0	64944.36	-204097.48	
		2.0	64944.36	-139153.12	
		3.0	64944.36	-74208.76	
		4.0	64944.36	9264.40	
5	1104097.69	.0	54628.71	-148119.92	
		1.0	54628.71	-93491.21	
		2.0	54628.71	-38862.49	
		3.0	54628.71	15766.22	
		4.0	54628.71	70394.93	
9	1 69314.87	.0	50348.72	-109245.30	
		1.0	50348.72	-58896.58	
		2.0	50348.72	-8547.87	
		3.0	50348.72	41800.85	
		4.0	50348.72	92149.56	
13	1 33321.60	.0	47287.98	-92388.30	
		1.0	47287.98	-45100.32	
		2.0	47287.98	2187.67	
		3.0	47287.98	49475.65	
		4.0	47287.98	96763.64	
17	1 -871.59	.0	44000.29	-80533.33	
		1.0	44000.29	-36533.04	
		2.0	44000.29	7467.25	
		3.0	44000.29	51467.54	
		4.0	44000.29	95467.83	
21	1-31934.97	.0	40303.61	-69191.35	
		1.0	40303.61	-28887.74	
		2.0	40303.61	11415.86	
		3.0	40303.61	51719.47	
		4.0	40303.61	92023.08	
25	1-58919.98	.0	36478.57	-56154.23	
		1.0	36478.57	-19675.66	
		2.0	36478.57	16802.91	
		3.0	36478.57	53281.48	
		4.0	36478.57	89760.05	
29	1-80434.95	.0	33688.46	-36286.62	

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIA TOR
		SHEAR	MOMENT	SHEAR	MOMENT	
1	8.0	-9831.55	-9729.24			

	1	-5372.67				
	.0	12615.58	-21705.50			
	2.0	8235.58	-854.34			
	4.0	-2024.42	11236.83			
	6.0	-6404.42	2807.99			
	8.0	-10784.42	-14380.85			
10	-----					
	1	-26496.88				
	.0	17443.10	-30248.76			
	3.0	10873.10	12225.54			
	6.0	-1576.90	23229.84			
	9.0	-14026.90	2764.13			
	12.0	-20596.90	-49171.57			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD O COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE SHEAR	MOMENT	AXIAL TOP
		SHEAR	MOMENT			
	3.0	16163.37	17649.54			
	6.0	-2746.63	33674.65			
	9.0	-21656.63	1169.75			
	12.0	32366.63	-79865.15			
4	-----					
	1	44456.48				
	.0	27056.48	-47966.80			
	3.0	16346.48	17137.64			
	6.0	-2563.52	33712.08			
	9.0	21473.52	1756.51			
	12.0	-32183.52	-78729.05			
37	-----					
	1	26178.01				
	.0	27663.12	-51452.80			
	3.0	16953.12	15471.56			
	6.0	-1956.88	33865.92			
	9.0	-20866.88	3730.28			
	12.0	-31576.88	-74935.36			
79	-----					
	1	25913.18				
	.0	3648.59	30137.73			
	2.0	-3491.41	30294.91			
	4.0	-18831.41	16172.09			
	6.0	-25971.41	-28630.73			
	8.0	-33111.41	-87713.56			
82	-----					
	1	57756.27				
	.0	16385.50	-19656.37			
	2.0	9245.50	5974.64			
	4.0	-6094.50	17325.64			
	6.0	-13234.50	-2003.35			
	8.0	-20374.50	-35612.34			
85	-----					
	1	27907.36				
	.0	16713.64	-20682.33			
	2.0	9573.64	5604.95			
	4.0	-5766.36	17612.24			
	6.0	-12906.36	-1060.48			
	8.0	-20046.36	-34013.20			
88	-----					
	1	6542.28				
	.0	17967.30	-25743.31			
	2.0	10827.30	3051.30			
	4.0	-4512.70	17565.90			
	6.0	-11652.70	1400.50			
	8.0	-18792.70	-29044.89			
89	-----					
	1	66945.47				
	.0	13568.45	-24676.81			
	2.0	9188.45	-1919.91			
	4.0	-1071.55	12076.98			
	6.0	-5451.55	5553.87			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TOP
3	1	-4890.52					
		.0	-24650.26	133828.00			
		2.0	-30190.26	78987.47			
		4.0	-43930.26	13066.94			
		6.0	-49470.26	-80333.59			
		8.0	-55010.26	-184814.12			
6	1	3095.44					
		.0	-17191.75	104173.71			
		2.0	-22731.75	64250.21			
		4.0	-36471.75	13246.72			
		6.0	-42011.75	-65236.78			
		8.0	-47551.75	-154800.28			
77	1	17461.71					
		.0	8125.95	14583.45			
		2.0	985.95	23695.35			
		4.0	-14354.05	18527.24			
		6.0	-21494.05	-17320.87			
		8.0	-28634.05	-67448.97			
90	1	-30892.36					
		.0	19068.61	-29439.31			
		2.0	11928.61	1557.91			
		4.0	-3411.39	18275.13			
		6.0	-10551.39	4312.35			
		8.0	-17691.39	-23930.43			
33	1	-44894.25					
		.0	19350.91	-30836.23			
		2.0	12210.91	725.59			
		4.0	-3129.09	18007.42			
		6.0	-10269.09	4609.25			
		8.0	-17409.09	-23068.92			
36	1	-58226.03					
		.0	20433.47	-35299.58			
		2.0	13293.47	-1572.65			
		4.0	-2046.53	17874.28			
		6.0	-9186.53	6641.22			
		8.0	-16326.53	-18871.85			
78	1	58935.15					
		.0	21604.56	-15823.38			
		3.0	10894.56	32925.31			
		6.0	-8015.44	33144.00			
		9.0	-26925.44	-15167.31			
		12.0	-37635.44	-112008.62			
31	1	60837.77					
		.0	26873.37	-46905.56			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	MOMENT	AXIAL TOR
			SHEAR	MOMENT			
		9.0	-35957.32	-50400.84			
		12.0	-44267.32	-170737.81			

1	-2062.69						
		.0	9054.40	40019.31			
		3.0	744.40	54717.50			
		6.0	-15765.60	28085.70			
		9.0	-32275.60	-39876.11			
		12.0	-40585.60	-149167.91			

5	-6666.56						
		.0	-23935.86	130894.14			
		2.0	-29475.86	77482.43			
		4.0	-43215.86	12990.71			
		6.0	-48755.86	-78981.00			
		8.0	-54295.86	-182032.72			

8	-3088.31						
		.0	-35096.67	175131.37			
		2.0	-40636.67	99398.03			
		4.0	-54376.67	12584.69			
		6.0	-59916.67	-101708.64			
		8.0	-65456.67	-227081.98			

11	-2507.61						
		.0	-37127.83	183153.18			
		2.0	-42667.83	103357.52			
		4.0	-56407.83	12481.85			
		6.0	-61947.83	-105873.82			
		8.0	-67487.83	-235309.49			

14	-2667.99						
		.0	-35986.92	178636.14			
		2.0	-41526.92	101122.29			
		4.0	-55266.92	12528.45			
		6.0	-60806.92	-103545.40			
		8.0	-66346.92	-230699.25			

17	-3122.78						
		.0	-33405.74	168431.93			
		2.0	-38945.74	96080.45			
		4.0	-52685.74	12648.98			
		6.0	-58225.74	-98262.50			
		8.0	-63765.74	-220253.98			

70	-3922.29						
		.0	-29727.09	153910.62			
		2.0	-35267.09	88916.44			
		4.0	-49007.09	12842.27			
		6.0	-54547.09	-90711.91			
		8.0	-60087.09	-205346.08			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIA
			SHEAR	MOMENT	SHEAR	TOR

	1142314.33					
		.0	64539.87	-267567.87		
		1.0	64539.87	-203028.00		
		2.0	64539.87	-138488.12		
		3.0	64539.87	-73948.25		
		4.0	64539.87	-9408.37		
5	-----					
	1117864.01					
		.0	54226.58	-147183.90		
		1.0	54226.58	-92957.32		
		2.0	54226.58	-38730.75		
		3.0	54226.58	15495.83		
		4.0	54226.58	69722.41		
9	-----					
	1 83459.18					
		.0	49908.57	-108397.37		
		1.0	49908.57	-58488.80		
		2.0	49908.57	-8580.23		
		3.0	49908.57	41328.33		
		4.0	49908.57	91236.90		
13	-----					
	1 47906.15					
		.0	46797.04	-91534.54		
		1.0	46797.04	-44737.50		
		2.0	46797.04	2059.54		
		3.0	46797.04	48856.57		
		4.0	46797.04	95653.61		
17	-----					
	1 14188.68					
		.0	43454.83	-79739.30		
		1.0	43454.83	-36284.46		
		2.0	43454.83	7170.37		
		3.0	43454.83	50625.20		
		4.0	43454.83	94080.03		
21	-----					
	1-16412.78					
		.0	39690.95	-68735.87		
		1.0	39690.95	-29044.91		
		2.0	39690.95	10646.04		
		3.0	39690.95	50337.00		
		4.0	39690.95	90027.95		
25	-----					
	1-43097.66					
		.0	35419.10	-56954.76		
		1.0	35419.10	-21535.65		
		2.0	35419.10	13883.45		
		3.0	35419.10	49302.56		
		4.0	35419.10	84721.66		
29	-----					
	1-65113.68					
		.0	30946.24	-43377.38		

sa Status Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDT	1-2 PLANE		1-3 PLANE		AXIAL TOR	
COMB			SHEAR	MOMENT	SHEAR	MOMENT		
		1.0	30946.24	-12431.14				
		2.0	30946.24	18515.10				
		3.0	30946.24	49461.34				
		4.0	30946.24	80407.58				
3			-----					
			1 81283.16					
		.0	27179.10	-24015.13				
		1.0	27179.10	3163.97				
		2.0	27179.10	30343.08				
		3.0	27179.10	57522.18				
		4.0	27179.10	84701.29				
7			-----					
			1 83419.14					
		.0	15854.16	17144.06				
		1.0	15854.16	32998.22				
		2.0	15854.16	48852.38				
		3.0	15854.16	64706.54				
		4.0	15854.16	80560.71				
41			-----					
			1 77487.34					
		.0	-23138.58	77194.11				
		1.0	-23138.58	54055.53				
		2.0	-23138.58	30916.95				
		3.0	-23138.58	7778.37				
		4.0	-23138.58	-15360.22				
15			-----					
			1 60718.57					
		.0	-7881.50	18774.24				
		1.0	-7881.50	10892.74				
		2.0	-7881.50	3011.24				
		3.0	-7881.50	-4870.26				
		4.0	-7881.50	-12751.76				
49			-----					
			1-28751.57					
		.0	-12470.94	23876.47				
		1.0	-12470.94	11405.53				
		2.0	-12470.94	-1065.41				
		3.0	-12470.94	-13536.36				
		4.0	-12470.94	-26007.30				
2			-----					
			1*****					
		.0	79089.55	-286287.09				
		1.0	79089.55	-207197.53				
		2.0	79089.55	-128107.98				
		3.0	79089.55	-49018.42				
		4.0	79089.55	30071.13				
6			-----					
			1*****					
		.0	82773.49	-201825.82				
		1.0	82773.49	-119052.33				
		2.0	82773.49	-36278.84				
		3.0	82773.49	46494.65				

sa Statis Portal (33 lantai) Kg/m

Y M E E L E M E N T F O R C E S

LOAD	AXIAL DIST	1-3 PLANE		AXIAL
COMB	FORCE ENDI	SHEAR	MOMENT	TORQ

0	4.0	82773.49	129268.14	

1*****				
	0.0	82757.96	-173821.75	
	1.0	82757.96	-91063.79	
	2.0	82757.96	8305.83	
	3.0	82757.96	74452.13	
	4.0	82757.96	157210.07	

4				

1*****				
	0.0	80014.21	-158138.09	
	1.0	80014.21	-78123.88	
	2.0	80014.21	1890.34	
	3.0	80014.21	81904.55	
	4.0	80014.21	161918.76	

18				

1*****				
	0.0	75801.58	-144959.15	
	1.0	75801.58	-69157.56	
	2.0	75801.58	6644.02	
	3.0	75801.58	82445.60	
	4.0	75801.58	158247.19	

22				

1*****				
	0.0	70362.41	-130856.43	
	1.0	70362.41	-60494.02	
	2.0	70362.41	9868.39	
	3.0	70362.41	80230.81	
	4.0	70362.41	150593.22	

26				

1*****				
	0.0	63777.03	-114640.91	
	1.0	63777.03	-50863.88	
	2.0	63777.03	12913.15	
	3.0	63777.03	76690.17	
	4.0	63777.03	140467.20	

30				

1*****				
	0.0	56149.71	-95136.34	
	1.0	56149.71	-38986.64	
	2.0	56149.71	17163.07	
	3.0	56149.71	73312.78	
	4.0	56149.71	129462.48	

34				

1*****				
	0.0	47545.82	-68157.19	
	1.0	47545.82	-20611.36	
	2.0	47545.82	26934.46	
	3.0	47545.82	74480.28	
	4.0	47545.82	122026.11	

38				

1-68391.16				

isa Statis Portal (13 lantai) Kg/m

A M E M B E R F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST END1	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	32309.09	-17875.97			
		1.0	32309.09	14433.12			
		2.0	32309.09	46742.20			
		3.0	32309.09	79051.29			
		4.0	32309.09	111360.38			
2	1-16529.72	.0	-23258.17	76399.04			
		1.0	-23258.17	53140.86			
		2.0	-23258.17	29862.69			
		3.0	-23258.17	6624.52			
		4.0	-23258.17	-16633.65			
16	1-16336.60	.0	-5675.64	12192.34			
		1.0	-5675.64	6516.70			
		2.0	-5675.64	841.06			
		3.0	-5675.64	-4834.58			
		4.0	-5675.64	-10510.22			
50	1 -7988.10	.0	-10463.73	21365.97			
		1.0	-10463.73	10902.24			
		2.0	-10463.73	438.52			
		3.0	-10463.73	-10025.21			
		4.0	-10463.73	-20488.93			
3	1*****	.0	88884.20	-298906.17			
		1.0	88884.20	-210021.97			
		2.0	88884.20	-121137.77			
		3.0	88884.20	-32253.58			
		4.0	88884.20	56630.62			
7	1*****	.0	98684.03	-234232.07			
		1.0	98684.03	-135548.04			
		2.0	98684.03	-36864.00			
		3.0	98684.03	61820.03			
		4.0	98684.03	160504.06			
11	1*****	.0	99178.37	-206395.37			
		1.0	99178.37	-107217.00			
		2.0	99178.37	-8038.64			
		3.0	99178.37	91139.73			
		4.0	99178.37	190318.10			
15	1*****	.0	97166.24	-192177.88			
		1.0	97166.24	-95011.64			
		2.0	97166.24	2154.60			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT
9	1*****	3.0	97166.24	99320.84		
		4.0	97166.24	196487.08		
		0.0	93576.79	-180282.99		
		1.0	93576.79	-86706.20		
		2.0	93576.79	6870.58		
13	1*****	3.0	93576.79	100447.37		
		4.0	93576.79	194024.16		
		0.0	88681.97	-167303.00		
		1.0	88681.97	-78621.03		
		2.0	88681.97	10060.94		
27	1*****	3.0	88681.97	98742.90		
		4.0	88681.97	187424.87		
		0.0	82545.30	-151947.14		
		1.0	82545.30	-69401.83		
		2.0	82545.30	13143.47		
31	1*****	3.0	82545.30	95688.77		
		4.0	82545.30	178234.07		
		0.0	75032.16	-132862.05		
		1.0	75032.16	-57829.89		
		2.0	75032.16	17202.27		
35	1*****	3.0	75032.16	92234.44		
		4.0	75032.16	167266.60		
		0.0	65290.91	-107159.13		
		1.0	65290.91	-41868.21		
		2.0	65290.91	23422.70		
39	1*****	3.0	65290.91	88713.62		
		4.0	65290.91	154004.53		
		0.0	59277.97	-69916.46		
		1.0	59277.97	-10638.49		
		2.0	59277.97	48639.48		
43	1*****	3.0	59277.97	107917.44		
		4.0	59277.97	167195.41		
		0.0	-3515.64	40336.49		
		1.0	-3515.64	36820.85		
		2.0	-3515.64	33305.21		
		3.0	-3515.64	29789.57		
		4.0	-3515.64	26273.93		

sa Statis Portal (13 lantai) Kg/m

TIME ELEMENT FORCES

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1	1-85321.47	.0	16096.76	-29744.72		
		1.0	16096.76	-13647.97		
		2.0	16096.76	2448.79		
		3.0	16096.76	18545.55		
		4.0	16096.76	34642.31		

1	1-28973.38	.0	11084.29	-16255.69		
		1.0	11084.29	-5171.40		
		2.0	11084.29	5912.89		
		3.0	11084.29	16997.18		
		4.0	11084.29	28081.47		

4	1*****	.0	72205.37	-277600.23		
		1.0	72205.37	-205394.86		
		2.0	72205.37	-133189.48		
		3.0	72205.37	-60984.11		
		4.0	72205.37	11221.27		

8	1*****	.0	65575.90	-169911.86		
		1.0	65575.90	-104335.96		
		2.0	65575.90	-38760.05		
		3.0	65575.90	26815.85		
		4.0	65575.90	92391.75		

2	1*****	.0	62496.11	-133523.42		
		1.0	62496.11	-71027.32		
		2.0	62496.11	-8531.21		
		3.0	62496.11	53964.89		
		4.0	62496.11	116461.00		

6	1*****	.0	59984.51	-117614.18		
		1.0	59984.51	-57629.67		
		2.0	59984.51	2354.84		
		3.0	59984.51	62339.34		
		4.0	59984.51	122323.85		

20	1*****	.0	57290.80	-107179.46		
		1.0	57290.80	-49888.67		
		2.0	57290.80	7402.13		
		3.0	57290.80	64692.93		
		4.0	57290.80	121983.73		

24	1*****	.0	54090.67	-97336.55		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE SHEAR	AXIAL TORQ
			SHEAR	MOMENT		
		1.0	54090.67	-43245.88		
		2.0	54090.67	10844.79		
		3.0	54090.67	64935.46		
		4.0	54090.67	119026.12		
3	1*****	.0	50326.57	-86427.73		
		1.0	50326.57	-36101.17		
		2.0	50326.57	14225.40		
		3.0	50326.57	64551.97		
		4.0	50326.57	114878.54		
52	1*****	.0	45722.89	-73064.34		
		1.0	45722.89	-27341.45		
		2.0	45722.89	18381.45		
		3.0	45722.89	64104.34		
		4.0	45722.89	109827.23		
36	1*****	.0	40158.16	-55838.20		
		1.0	40158.16	-15680.05		
		2.0	40158.16	24478.11		
		3.0	40158.16	64636.27		
		4.0	40158.16	104794.43		
40	1*****	.0	41595.78	-31223.54		
		1.0	41595.78	10372.24		
		2.0	41595.78	51968.02		
		3.0	41595.78	93563.81		
		4.0	41595.78	135159.59		
44	1-65807.75	.0	-9616.57	57502.48		
		1.0	-9616.57	47885.91		
		2.0	-9616.57	38269.35		
		3.0	-9616.57	28652.78		
		4.0	-9616.57	19036.22		
48	1-33961.01	.0	8481.80	-13222.04		
		1.0	8481.80	-4740.24		
		2.0	8481.80	3741.56		
		3.0	8481.80	12223.36		
		4.0	8481.80	20705.15		
52	1-10688.30	.0	5510.01	-7918.60		
		1.0	5510.01	-2408.59		
		2.0	5510.01	3101.42		
		3.0	5510.01	8611.43		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST ENDI 4.0	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
3	1	6854.30		5510.01	14121.44		
		.0	-24450.32	137775.53			
		2.0	-29990.32	83334.89			
		4.0	-43730.32	17814.26			
		6.0	-49270.32	-75186.37			
		8.0	-54810.32	-179267.01			
36	1	-2600.99					
		.0	-34404.83	178119.78			
		2.0	-39944.83	103770.11			
		4.0	-53684.83	18340.44			
		6.0	-59224.83	-94569.22			
		8.0	-64764.83	-218558.89			
59	1	-7267.47					
		.0	-35553.03	182771.44			
		2.0	-41093.03	106125.39			
		4.0	-54833.03	18399.33			
		6.0	-60373.03	-96806.73			
		8.0	-65913.03	-223092.78			
62	1	-10495.79					
		.0	-33717.47	175392.91			
		2.0	-39257.47	102417.96			
		4.0	-52997.47	18363.02			
		6.0	-58537.47	-93171.92			
		8.0	-64077.47	-215786.87			
65	1	-13534.12					
		.0	-30601.46	162815.90			
		2.0	-36141.46	96072.98			
		4.0	-49881.46	18250.07			
		6.0	-55421.46	-87052.85			
		8.0	-60961.46	-203435.77			
68	1	-16486.15					
		.0	-26684.88	146982.71			
		2.0	-32224.88	88072.94			
		4.0	-45964.88	18083.18			
		6.0	-51504.88	-79386.58			
		8.0	-57044.88	-187936.35			
71	1	-19744.13					
		.0	-22016.02	128099.03			
		2.0	-27556.02	78526.99			
		4.0	-41296.02	17874.95			
		6.0	-46836.02	-70257.09			
		8.0	-52376.02	-169469.13			
74	1	-23909.87					

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ	
			SHEAR	MOMENT	SHEAR	MOMENT		
7	1	-10812.00	0	-16169.48	104422.71			
			2.0	-21709.48	66543.76			
			4.0	-35449.48	17584.80			
			6.0	-40989.48	58854.15			
			8.0	-46529.48	-146373.10			
54	1	-3170.36	0	-7135.98	67557.23			
			2.0	-12675.98	47745.27			
			4.0	-26415.98	16853.31			
			6.0	-31955.98	-41518.65			
			8.0	-37495.98	-110970.61			
57	1	-2585.40	0	1699.30	84531.00			
			3.0	-6610.70	77163.91			
			6.0	-23120.70	28466.83			
			9.0	-39630.70	-61560.26			
			12.0	-47940.70	-192917.35			
60	1	-4523.72	0	418.28	92255.40			
			3.0	-7891.72	81045.23			
			6.0	-24401.72	28505.05			
			9.0	-40911.72	-65365.12			
			12.0	-49221.72	-200565.29			
63	1	-6283.16	0	618.64	91091.04			
			3.0	-7691.36	80481.95			
			6.0	-24201.36	28542.86			
			9.0	-40711.36	-64726.23			
			12.0	-49021.36	-199325.32			
66	1	-8094.95	0	1527.90	85667.85			
			3.0	-6782.10	77786.56			
			6.0	-23292.10	28575.27			
			9.0	-39802.10	-61966.01			
			12.0	-48112.10	-193837.30			
69	1	-9900.76	0	2927.51	77297.78			
			3.0	-5382.49	73615.30			
			6.0	-21892.49	28602.82			

isa Statis Portal (13 lantai) kg/m

A M E E L E M E N T F O R C E S

JT LOAD ID COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		9.0	-38402.49	-57739.66			
		12.0	-46712.49	-185412.13			
72	1-12116.81						
		.0	4793.56	66134.41			
		3.0	-3516.44	68050.09			
		6.0	-20026.44	28635.77			
		9.0	-36536.44	-52108.55			
		12.0	-44846.44	-174182.87			
75	1-15305.98						
		.0	7275.80	51246.56			
		3.0	-1034.20	60608.97			
		6.0	-17544.20	28641.38			
		9.0	-34054.20	-44656.21			
		12.0	-42364.20	-159283.80			
78	1 -4575.32						
		.0	10911.25	28931.47			
		3.0	2601.25	49200.22			
		6.0	-13908.75	28138.96			
		9.0	-30418.75	-34252.29			
		12.0	-38728.75	-137973.54			
55	1 -6629.47						
		.0	-23713.37	130013.83			
		2.0	-29253.37	77047.09			
		4.0	-42993.37	13000.35			
		6.0	-48533.37	-78526.39			
		8.0	-54073.37	-181133.12			
58	1 -3079.80						
		.0	-34807.16	173982.08			
		2.0	-40347.16	98827.77			
		4.0	-54087.16	12593.46			
		6.0	-59627.16	-101120.85			
		8.0	-65167.16	-225915.17			
61	1 -2511.60						
		.0	-36820.73	181930.68			
		2.0	-42360.73	102749.22			
		4.0	-56100.73	12487.75			
		6.0	-61640.73	-105253.71			
		8.0	-67180.73	-234075.18			
64	1 -2693.71						
		.0	-35688.51	177444.76			
		2.0	-41228.51	100527.74			
		4.0	-54968.51	12530.72			
		6.0	-60508.51	-102946.30			
		8.0	-66048.51	-229503.32			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T	LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
Q	COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
7								
	1	-3200.13						
			.0	33171.27	167489.86			
			2.0	-38711.27	95607.33			
			4.0	-50451.27	12644.79			
			6.0	-57991.27	-97797.74			
			8.0	-63531.27	-219320.27			
0								
	1	-3764.10						
			.0	-29746.72	153959.87			
			2.0	-35286.72	88926.44			
			4.0	-49026.72	12813.01			
			6.0	-54566.72	-90780.42			
			8.0	-60106.72	-205453.86			
73								
	1	-4603.67						
			.0	-25427.02	136913.26			
			2.0	-30967.02	80519.23			
			4.0	-44707.02	13045.19			
			6.0	-50247.02	-81908.84			
			8.0	-55787.02	-187942.88			
76								
	1	-5564.74						
			.0	-19920.92	115141.93			
			2.0	-25460.92	69760.09			
			4.0	-39200.92	13298.24			
			6.0	-44740.92	-70643.60			
			8.0	-50280.92	-165665.44			
79								
	1	1437.63						
			.0	-12565.68	85947.44			
			2.0	-18105.68	55276.09			
			4.0	-31845.68	13524.74			
			6.0	-37385.68	-55706.62			
			8.0	-42925.68	-136017.97			
30								
	1	4396.74						
			.0	10931.80	3366.60			
			2.0	3791.80	18090.20			
			4.0	-11548.20	18533.80			
			6.0	-18688.20	-11702.60			
			8.0	-25828.20	-56219.00			
83								
	1	-49057.71						
			.0	20260.93	-34171.89			
			2.0	13120.93	-790.02			
			4.0	-2219.07	18311.84			
			6.0	-9359.07	6733.71			
			8.0	-16499.07	-19124.42			
86								
	1	-62277.56						
			.0	20745.31	-36575.12			

isa Statis Portal (15 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		2.0	13605.31	-2224.50			
		4.0	-1734.69	17846.12			
		6.0	-8874.69	7236.74			
		8.0	-16014.69	17657.64			
1	53018.63						
		.0	22541.08	-21376.06			
		3.0	11831.08	30182.17			
		6.0	-7078.92	33210.40			
		9.0	-25988.92	-12291.37			
		12.0	-36698.92	-106323.14			
34	52142.88						
		.0	27081.06	-48126.32			
		3.0	16371.06	17051.87			
		6.0	-2538.94	33700.05			
		9.0	-21448.94	1818.24			
		12.0	-32158.94	-78593.57			
87	32686.62						
		.0	27367.94	-49617.55			
		3.0	16657.94	16421.26			
		6.0	-2252.06	33930.07			
		9.0	-21162.06	2908.87			
		12.0	-31872.06	-76642.32			
32	19446.12						
		.0	6171.95	20244.59			
		2.0	-968.05	25448.49			
		4.0	-16308.05	16372.38			
		6.0	-23448.05	-23383.72			
		8.0	-30588.05	-77419.83			
35	42666.69						
		.0	17201.97	-22810.19			
		2.0	10061.97	4453.75			
		4.0	-5278.03	17437.69			
		6.0	-12418.03	-258.37			
		8.0	-19558.03	-32234.43			
88	6128.61						
		.0	18032.89	-25856.45			
		2.0	10892.89	3069.34			
		4.0	-4447.11	17715.12			
		6.0	-11587.11	1680.91			
		8.0	-18727.11	-28633.31			
89	-71369.77						
		.0	13912.77	-25962.23			
		2.0	9532.77	-2516.68			
		4.0	-727.23	12168.87			
		6.0	-5107.23	6334.42			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
1	8.0	-9487.23	-8260.04			
	1 -5510.01					
	.0	12711.70	-22215.01			
	2.0	8331.70	-1171.62			
	4.0	-1928.30	11111.78			
	6.0	-6308.30	2875.17			
	8.0	-10688.30	-14121.44			
90						
	1-25694.70					
	.0	17232.72	-28876.38			
	3.0	10662.72	12966.78			
	6.0	-1787.28	23339.94			
	9.0	-14237.28	2243.10			
	12.0	-20807.28	-50323.75			

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

	1156684.33					
	.0	64133.32	-266093.32			
	1.0	64133.32	-201960.00			
	2.0	64133.32	-137826.68			
	3.0	64133.32	-73693.36			
	4.0	64133.32	-9560.04			
5	-----					
	1132507.70					
	.0	53816.91	-146231.17			
	1.0	53816.91	-92414.26			
	2.0	53816.91	-38597.36			
	3.0	53816.91	15219.55			
	4.0	53816.91	69036.46			
9	-----					
	1 98494.81					
	.0	49455.05	-107507.47			
	1.0	49455.05	-58052.42			
	2.0	49455.05	-8597.37			
	3.0	49455.05	40857.68			
	4.0	49455.05	90312.73			
13	-----					
	1 63407.93					
	.0	46288.77	-90589.02			
	1.0	46288.77	-44300.25			
	2.0	46288.77	1988.52			
	3.0	46288.77	48277.29			
	4.0	46288.77	94566.06			
17	-----					
	1 30216.76					
	.0	42883.60	-78720.09			
	1.0	42883.60	-35836.49			
	2.0	42883.60	7047.11			
	3.0	42883.60	49930.70			
	4.0	42883.60	92814.30			
21	-----					
	1 188.81					
	.0	39054.29	-67710.48			
	1.0	39054.29	-28656.19			
	2.0	39054.29	10398.10			
	3.0	39054.29	49452.39			
	4.0	39054.29	88506.68			
25	-----					
	1-25910.56					
	.0	34708.48	-56142.21			
	1.0	34708.48	-21433.73			
	2.0	34708.48	13274.75			
	3.0	34708.48	47983.22			
	4.0	34708.48	82691.70			
29	-----					
	1-47442.28					
	.0	29810.11	-43478.31			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL DIST	1-2 PLANE		1-3 PLANE		AXIAL
D COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	1.0	29810.11	-13668.20			
	2.0	29810.11	16141.91			
	3.0	29810.11	45952.01			
	4.0	29810.11	75762.12			
3	-----					
	1-63743.28					
	.0	24655.24	-29240.42			
	1.0	24655.24	-4585.18			
	2.0	24655.24	20070.06			
	3.0	24655.24	44725.30			
	4.0	24655.24	69380.54			
57	-----					
	1-73832.15					
	.0	19851.38	-10444.75			
	1.0	19851.38	9406.63			
	2.0	19851.38	29258.00			
	3.0	19851.38	49109.38			
	4.0	19851.38	68960.75			
41	-----					
	1-75229.05					
	.0	7934.68	24548.74			
	1.0	7934.68	32483.42			
	2.0	7934.68	40418.10			
	3.0	7934.68	48352.79			
	4.0	7934.68	56287.47			
15	-----					
	1-61075.96					
	.0	-20313.60	65780.81			
	1.0	-20313.60	45467.21			
	2.0	-20313.60	25153.61			
	3.0	-20313.60	4840.01			
	4.0	-20313.60	-15473.59			
19	-----					
	1-31938.09					
	.0	-12922.02	24162.64			
	1.0	-12922.02	11240.62			
	2.0	-12922.02	-1681.40			
	3.0	-12922.02	-14603.42			
	4.0	-12922.02	-27525.44			
2	-----					
	1*****					
	.0	78623.39	-284738.23			
	1.0	78623.39	-206114.84			
	2.0	78623.39	-127491.46			
	3.0	78623.39	-48868.07			
	4.0	78623.39	29755.32			
6	-----					
	1*****					
	.0	82261.51	-200661.31			
	1.0	82261.51	-118399.80			
	2.0	82261.51	-36138.29			
	3.0	82261.51	46123.22			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
D COMB			SHEAR	MOMENT	SHEAR	MOMENT	
0		4.0	82261.51	128384.73			

	1*****						
		.0	82224.69	-172758.39			
		1.0	82224.69	-90533.70			
		2.0	82224.69	-8309.02			
		3.0	82224.69	73915.67			
		4.0	82224.69	156140.36			
4	1*****						
		.0	79465.16	-157094.29			
		1.0	79465.16	-77629.12			
		2.0	79465.16	1836.04			
		3.0	79465.16	81301.20			
		4.0	79465.16	160766.37			
18	1*****						
		.0	75237.30	-143933.24			
		1.0	75237.30	-68695.94			
		2.0	75237.30	6541.36			
		3.0	75237.30	81778.66			
		4.0	75237.30	157015.96			
22	1*****						
		.0	69792.47	-129939.70			
		1.0	69792.47	-60147.23			
		2.0	69792.47	9645.24			
		3.0	69792.47	79437.70			
		4.0	69792.47	149230.17			
26	1*****						
		.0	63220.17	-114135.98			
		1.0	63220.17	-50915.81			
		2.0	63220.17	12304.36			
		3.0	63220.17	75524.52			
		4.0	63220.17	138744.69			
30	1*****						
		.0	55540.28	-96078.59			
		1.0	55540.28	-40538.31			
		2.0	55540.28	15001.97			
		3.0	55540.28	70542.25			
		4.0	55540.28	126082.52			
34	1*****						
		.0	46826.96	-75115.57			
		1.0	46826.96	-28288.60			
		2.0	46826.96	18538.36			
		3.0	46826.96	65365.32			
		4.0	46826.96	112192.28			
38	1-84505.23						

sa Statis Portal (13 lantai) Kg/m

E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	37344.62	-48438.64			
		1.0	37344.62	-11094.01			
		2.0	37344.62	26250.61			
		3.0	37344.62	63595.24			
		4.0	37344.62	100939.86			
2	1-39769.40						
		.0	21574.97	-3844.34			
		1.0	21574.97	17730.63			
		2.0	21574.97	39305.60			
		3.0	21574.97	60880.56			
		4.0	21574.97	82455.53			
6	1 -1158.63						
		.0	-21423.25	66424.39			
		1.0	-21423.25	45001.14			
		2.0	-21423.25	23577.90			
		3.0	-21423.25	2154.65			
		4.0	-21423.25	-19268.60			
50	1 -4965.18						
		.0	-8862.08	15808.41			
		1.0	-8862.08	6946.33			
		2.0	-8862.08	-1915.74			
		3.0	-8862.08	-10777.82			
		4.0	-8862.08	-19639.90			
3	1*****						
		.0	88449.52	-297402.86			
		1.0	88449.52	-208953.34			
		2.0	88449.52	-120503.82			
		3.0	88449.52	-32054.30			
		4.0	88449.52	56395.22			
7	1*****						
		.0	98247.65	-233196.23			
		1.0	98247.65	-134948.58			
		2.0	98247.65	-36700.93			
		3.0	98247.65	61546.72			
		4.0	98247.65	159794.37			
11	1*****						
		.0	98773.27	-205568.26			
		1.0	98773.27	-106795.00			
		2.0	98773.27	-8021.73			
		3.0	98773.27	90751.53			
		4.0	98773.27	189524.80			
15	1*****						
		.0	96798.06	-191475.28			
		1.0	96798.06	-94677.22			
		2.0	96798.06	2120.83			



sa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		3.0	96798.06	98918.89			
		4.0	96798.06	195716.95			

	1*****						
		.0	93246.98	-179702.79			
		1.0	93246.98	-86455.82			
		2.0	93246.98	6791.16			
		3.0	93246.98	100038.13			
		4.0	93246.98	193285.11			

	1*****						
		.0	88390.42	-166931.81			
		1.0	88390.42	-78541.39			
		2.0	88390.42	9849.03			
		3.0	88390.42	98239.45			
		4.0	88390.42	186629.86			

	1*****						
		.0	82314.42	-152148.16			
		1.0	82314.42	-69833.74			
		2.0	82314.42	12480.68			
		3.0	82314.42	94795.11			
		4.0	82314.42	177109.53			

	1*****						
		.0	75034.82	-134865.80			
		1.0	75034.82	-59830.98			
		2.0	75034.82	15203.84			
		3.0	75034.82	90238.65			
		4.0	75034.82	165273.47			

	1*****						
		.0	66442.77	-114293.10			
		1.0	66442.77	-47850.33			
		2.0	66442.77	18592.44			
		3.0	66442.77	85035.21			
		4.0	66442.77	151477.98			

	1*****						
		.0	55760.10	-88670.06			
		1.0	55760.10	-32909.95			
		2.0	55760.10	22850.15			
		3.0	55760.10	78610.26			
		4.0	55760.10	134370.36			

	1*****						
		.0	48849.45	-56586.93			
		1.0	48849.45	-7737.48			
		2.0	48849.45	41111.97			
		3.0	48849.45	89961.42			
		4.0	48849.45	138810.86			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE	AXIAL
D COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	TORQ
7	-----					
	1-76753.19					
		.0	-245.77	29113.69		
		1.0	-245.77	28867.93		
		2.0	-245.77	28622.16		
		3.0	-245.77	28376.39		
		4.0	-245.77	28130.63		
1	-----					
	1-31356.80					
		.0	12795.14	-21387.76		
		1.0	12795.14	-8592.62		
		2.0	12795.14	4202.53		
		3.0	12795.14	16997.67		
		4.0	12795.14	29792.81		
4	-----					
	1*****					
		.0	71833.77	-276179.18		
		1.0	71833.77	-204345.41		
		2.0	71833.77	-132511.64		
		3.0	71833.77	-60677.87		
		4.0	71833.77	11155.91		
8	-----					
	1*****					
		.0	65242.93	-169087.34		
		1.0	65242.93	-103844.40		
		2.0	65242.93	-38601.47		
		3.0	65242.93	26641.46		
		4.0	65242.93	91884.39		
12	-----					
	1*****					
		.0	62174.00	-132875.63		
		1.0	62174.00	-70701.63		
		2.0	62174.00	-8527.63		
		3.0	62174.00	53646.37		
		4.0	62174.00	115820.36		
16	-----					
	1*****					
		.0	59662.01	-117018.98		
		1.0	59662.01	-57356.97		
		2.0	59662.01	2305.03		
		3.0	59662.01	61967.04		
		4.0	59662.01	121629.04		
20	-----					
	1*****					
		.0	56961.13	-106617.27		
		1.0	56961.13	-49656.14		
		2.0	56961.13	7304.98		
		3.0	56961.13	64266.11		
		4.0	56961.13	121227.24		
24	-----					
	1*****					
		.0	53735.82	-96866.84		

sa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
3		1.0	53735.82	-43131.01			
		2.0	53735.82	10604.81			
		3.0	53735.82	64340.63			
		4.0	53735.82	118076.45			

12	1*****	.0	49902.93	-86349.40			
		1.0	49902.93	-36446.47			
		2.0	49902.93	13456.47			
		3.0	49902.93	63359.40			
	4.0	49902.93	113262.34				

36	1*****	.0	45462.80	-74472.96			
		1.0	45462.80	-29010.17			
		2.0	45462.80	16452.63			
		3.0	45462.80	61915.43			
	4.0	45462.80	107378.22				

40	1*****	.0	40153.03	-60417.08			
		1.0	40153.03	-20264.05			
		2.0	40153.03	19888.97			
		3.0	40153.03	60042.00			
	4.0	40153.03	100195.03				

44	1*****	.0	33880.90	-43686.07			
		1.0	33880.90	-9807.17			
		2.0	33880.90	24073.73			
		3.0	33880.90	57954.62			
	4.0	33880.90	91835.52				

48	1-85291.05	.0	33765.90	-23082.89			
		1.0	33765.90	10683.01			
		2.0	33765.90	44448.91			
		3.0	33765.90	78214.82			
	4.0	33765.90	111980.72				

52	1-31509.28	.0	-6694.18	46074.39			
		1.0	-6694.18	39380.20			
		2.0	-6694.18	32686.02			
		3.0	-6694.18	25991.84			
	4.0	-6694.18	19297.66				

52	1-10563.68	.0	5306.85	-7750.40			
		1.0	5306.85	-2443.55			
		2.0	5306.85	2863.31			
		3.0	5306.85	8170.16			

sa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	MOMENT	
3		4.0	5306.85	13477.01			

1	6845.41						
		.0	-24176.63	136671.13			
		2.0	-29716.63	82777.88			
		4.0	-43456.63	17804.62			
		6.0	-48996.63	-74648.63			
		8.0	-54536.63	-178181.89			

6							
1	-2580.14						
		.0	-34012.90	176543.93			
		2.0	-39552.90	102978.13			
		4.0	-53292.90	18332.34			
		6.0	-58832.90	-93793.45			
		8.0	-64372.90	-216999.25			

39							
1	-7246.72						
		.0	-35086.88	180901.75			
		2.0	-40626.88	105187.99			
		4.0	-54366.88	18394.22			
		6.0	-59906.88	-95879.54			
		8.0	-65446.88	-221233.31			

52							
1	-10479.82						
		.0	-33191.16	173286.15			
		2.0	-38731.16	101363.83			
		4.0	-52471.16	18361.50			
		6.0	-58011.16	-92120.83			
		8.0	-63551.16	-213683.15			

55							
1	-13526.69						
		.0	-30027.95	160524.78			
		2.0	-35567.95	94928.87			
		4.0	-49307.95	18252.97			
		6.0	-54847.95	-85902.94			
		8.0	-60387.95	-201138.85			

58							
1	-16481.19						
		.0	-26099.37	144648.89			
		2.0	-31639.37	86910.14			
		4.0	-45379.37	18091.40			
		6.0	-50919.37	-78207.34			
		8.0	-56459.37	-185586.09			

71							
1	-19399.63						
		.0	-21531.71	126170.01			
		2.0	-27071.71	77566.58			
		4.0	-40811.71	17883.15			
		6.0	-46351.71	-69280.28			
		8.0	-51891.71	-167523.70			

74							
1	-22615.13						

isa Statis Portal (13 lantai) Kg/m

A M E M B E R F O R C E S

T LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
D COMB			SHEAR	MOMENT	SHEAR	MOMENT	
		.0	-16301.01	105002.54			
		2.0	-21841.01	66860.53			
		4.0	-35581.01	17638.52			
		6.0	-41121.01	-59063.49			
		8.0	-46661.01	-146845.50			
7	1-26437.14						
		.0	-10088.87	79825.29			
		2.0	-15628.87	54107.55			
		4.0	-29368.87	17309.81			
		6.0	-34908.87	-46967.92			
		8.0	-40448.87	-122325.66			
30	1-22795.31						
		.0	-1396.90	44412.01			
		2.0	-6936.90	36078.22			
		4.0	-20676.90	16664.42			
		6.0	-26216.90	-30229.37			
		8.0	-31756.90	-88203.17			
54	1 3207.29						
		.0	7096.38	52234.74			
		3.0	-1213.62	61058.89			
		6.0	-17723.62	28553.04			
		9.0	-34233.62	-45282.81			
		12.0	-42543.62	-160448.66			
57	1 -2543.32						
		.0	1764.78	84143.87			
		3.0	-6545.22	76973.21			
		6.0	-23055.22	28472.54			
		9.0	-39565.22	-61358.12			
		12.0	-47875.22	-192518.79			
50	1 -4487.20						
		.0	462.05	92001.34			
		3.0	-7847.95	80922.49			
		6.0	-24357.95	28513.64			
		9.0	-40867.95	-65225.20			
		12.0	-49177.95	-200294.05			
53	1 -6251.96						
		.0	632.99	91016.46			
		3.0	-7677.01	80450.42			
		6.0	-24187.01	28554.39			
		9.0	-40697.01	-64671.65			
		12.0	-49007.01	-199227.68			
66	1 -8081.86						
		.0	1505.55	85816.81			
		3.0	-6804.45	77868.47			
		6.0	-23314.45	28590.13			

Isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
D COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
		9.0	-39824.45	-62018.22			
		12.0	-48134.45	-193956.56			
9	1 -9908.88						
		.0	2849.87	77780.06			
		3.0	-5460.13	73864.67			
		6.0	-21970.13	28619.28			
		9.0	-38480.13	-57956.11			
		12.0	-46790.13	-185861.50			
2	1-11719.74						
		.0	4600.82	67299.57			
		3.0	-3709.18	68637.04			
		6.0	-20219.18	28644.51			
		9.0	-36729.18	-52678.02			
		12.0	-45039.18	-175330.55			
75	1-13901.82						
		.0	6763.47	54352.59			
		3.0	-1546.53	62177.99			
		6.0	-18056.53	28673.39			
		9.0	-34566.53	-46161.21			
		12.0	-42876.53	-162325.81			
78	1-16954.80						
		.0	9439.31	38305.26			
		3.0	1129.31	54158.19			
		6.0	-15380.69	28681.13			
		9.0	-31890.69	-38125.94			
		12.0	-40200.69	-146263.01			
31	1 -7025.65						
		.0	12978.93	16581.03			
		3.0	4668.93	43052.83			
		6.0	-11841.07	28194.63			
		9.0	-28351.07	-27993.57			
		12.0	-36661.07	-125511.77			
55	1 -6590.84						
		.0	-23493.25	129142.79			
		2.0	-29033.25	76616.28			
		4.0	-42773.25	13009.77			
		6.0	-48313.25	-78076.74			
		8.0	-53853.25	-180243.24			
58	1 -3068.93						
		.0	-34520.48	172843.85			
		2.0	-40060.48	98262.88			
		4.0	-53800.48	12601.91			
		6.0	-59340.48	-100539.05			
		8.0	-64880.48	-224760.02			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T D	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
				SHEAR	MOMENT	SHEAR	TORQ
1	1	-2511.99	.0	-36513.17	180706.03		
			2.0	-42053.17	102139.69		
			4.0	-55793.17	12493.35		
			6.0	-61333.17	-104633.00		
			8.0	-66873.17	-232839.34		
4	1	-2700.88	.0	-35374.80	176192.06		
			2.0	-40914.80	99902.47		
			4.0	-54654.80	12532.87		
			6.0	-60194.80	-102316.72		
			8.0	-65734.80	-228246.31		
57	1	-3225.30	.0	-32864.30	166260.36		
			2.0	-38404.30	94991.75		
			4.0	-52144.30	12643.14		
			6.0	-57684.30	-97185.46		
			8.0	-63224.30	-218094.07		
70	1	-3832.89	.0	-29487.80	152916.53		
			2.0	-35027.80	88400.93		
			4.0	-48767.80	12805.33		
			6.0	-54307.80	-90270.26		
			8.0	-59847.80	-204425.86		
73	1	-4440.14	.0	-25367.51	136644.77		
			2.0	-30907.51	80369.76		
			4.0	-44647.51	13014.74		
			6.0	-50187.51	-81820.28		
			8.0	-55727.51	-187735.30		
76	1	-5309.77	.0	-20449.51	117240.76		
			2.0	-25989.51	70801.75		
			4.0	-39729.51	13282.73		
			6.0	-45269.51	-71716.28		
			8.0	-50809.51	-167795.30		
79	1	-6272.13	.0	-14541.02	93885.03		
			2.0	-20081.02	59263.00		
			4.0	-33821.02	13560.97		
			6.0	-39361.02	-59621.06		
			8.0	-44901.02	-143883.09		
82	1	-114.99	.0	-7365.49	65445.52		

sa Statis Portal (13 lantai) kg/m

NAME ELEMENT FORCES

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		2.0	-12905.49	45174.53		
		4.0	-26645.49	13823.55		
		6.0	-32185.49	-45007.43		
		8.0	-37725.49	-114918.41		

3	1 -9934.72					
		.0	14153.09	-9493.34		
		2.0	7013.09	11672.84		
		4.0	-8326.91	18559.01		
		6.0	-15466.91	-5234.81		
		8.0	-22606.91	-43308.63		

36	1-67073.63					
		.0	21601.35	-39624.55		
		2.0	14461.35	-3561.84		
		4.0	-878.65	18220.86		
		6.0	-8018.65	9323.57		
		8.0	-15158.65	-13853.73		

34	1 48146.55					
		.0	23540.38	-27329.22		
		3.0	12830.38	27226.92		
		6.0	-6079.62	33253.06		
		9.0	-24989.62	-9250.79		
		12.0	-35699.62	-100284.65		

37	1 41313.35					
		.0	27283.38	-49065.56		
		3.0	16573.38	16719.59		
		6.0	-2336.62	33974.74		
		9.0	-21246.62	2699.88		
		12.0	-31956.62	-77104.97		

35	1 11558.78					
		.0	9018.34	9159.62		
		2.0	1878.34	20056.31		
		4.0	-13461.66	16673.00		
		6.0	-20601.66	-17390.31		
		8.0	-27741.66	-65733.62		

88	1 17319.40					
		.0	18468.24	-27767.86		
		2.0	11328.24	2028.63		
		4.0	-4011.76	17545.11		
		6.0	-11151.76	2381.59		
		8.0	-18291.76	-27061.93		

89	1-77530.19					
		.0	14288.86	-27464.88		
		2.0	9908.86	-3267.15		
		4.0	-351.14	12170.57		
		6.0	-4731.14	7088.30		

Statis Portal (13 lantai) Kg/m

MEME ELEMENT FORCES

T LOAD	AXIAL DIST	DIST	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT
0 COMB	FORCE	ENDI				
		8.0	-9111.14	-6753.98		

1						
	1	-5306.85				
		1.0	12836.32	-22567.57		
		2.0	8456.32	-1274.93		
		4.0	-1803.68	11257.71		
		6.0	-6183.68	3270.35		
		8.0	-10563.68	-13477.01		

0						
	1	-23420.36				
		1.0	16865.68	-26537.33		
		3.0	10295.68	14204.70		
		6.0	-2154.32	23476.73		
		9.0	-14604.32	1278.75		
		12.0	-21174.32	-52389.22		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T D	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
				SHEAR	MOMENT	SHEAR	TORQ
1	1172928.68						
		.0		63720.38	-264621.23		
		1.0		63720.38	-200900.85		
		2.0		63720.38	-157180.46		
		3.0		63720.38	73460.08		
		4.0		63720.38	-9739.69		
5	1149036.80						
		.0		53392.10	-145262.85		
		1.0		53392.10	-91870.75		
		2.0		53392.10	-38478.64		
		3.0		53392.10	14913.46		
		4.0		53392.10	68305.56		
9	1115438.30						
		.0		48975.05	-136574.11		
		1.0		48975.05	-57599.06		
		2.0		48975.05	-8624.01		
		3.0		48975.05	40351.04		
		4.0		48975.05	89326.09		
13	1 80853.07						
		.0		45742.48	-89566.02		
		1.0		45742.48	-43823.54		
		2.0		45742.48	1918.94		
		3.0		45742.48	47661.41		
		4.0		45742.48	93403.89		
17	1 48240.30						
		.0		42264.51	-77569.69		
		1.0		42264.51	-35305.18		
		2.0		42264.51	6959.34		
		3.0		42264.51	49223.85		
		4.0		42264.51	91488.37		
21	1 18865.10						
		.0		38355.49	-66430.79		
		1.0		38355.49	-28075.31		
		2.0		38355.49	10280.18		
		3.0		38355.49	48635.66		
		4.0		38355.49	86991.15		
25	1 -6512.61						
		.0		33927.55	-54781.43		
		1.0		33927.55	-20853.88		
		2.0		33927.55	13073.68		
		3.0		33927.55	47001.23		
		4.0		33927.55	80928.78		
29	1-27275.27						
		.0		28942.24	-42180.88		

sa Statis Portal (13 lantai) Kg/m

ELEMENT FORCE S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
		1.0	28942.24	-13238.64			
		2.0	28942.24	15703.60			
		3.0	28942.24	44645.84			
		4.0	28942.24	73588.08			

	1-42834.56	.0	23377.78	-28461.97			
		1.0	23377.78	-5084.19			
		2.0	23377.78	18293.58			
		3.0	23377.78	41671.36			
		4.0	23377.78	65049.13			

	1-52584.46	.0	17506.63	-13485.18			
		1.0	17506.63	4021.45			
		2.0	17506.63	21528.08			
		3.0	17506.63	39034.71			
		4.0	17506.63	56541.33			

	1-55736.08	.0	11543.48	4768.89			
		1.0	11543.48	16312.37			
		2.0	11543.48	27855.85			
		3.0	11543.48	39399.33			
		4.0	11543.48	50942.81			

	1-50559.94	.0	-1013.90	33012.62			
		1.0	-1013.90	31998.72			
		2.0	-1013.90	30984.82			
		3.0	-1013.90	29970.92			
		4.0	-1013.90	28957.03			

	1-32738.45	.0	-20835.28	53284.96			
		1.0	-20835.28	32449.68			
		2.0	-20835.28	11614.39			
		3.0	-20835.28	-9220.89			
		4.0	-20835.28	-30056.18			

	1*****	.0	78155.83	-283198.80			
		1.0	78155.83	-205042.97			
		2.0	78155.83	-126887.14			
		3.0	78155.83	-48731.30			
		4.0	78155.83	29424.53			

	1*****	.0	81746.80	-199502.30			
		1.0	81746.80	-117755.51			
		2.0	81746.80	-36008.71			
		3.0	81746.80	45738.08			

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
		4.0	81746.80	127484.88		

	1*****	.0	81685.45	-171689.72		
		1.0	81685.45	-90004.28		
		2.0	81685.45	-8318.83		
		3.0	81685.45	73366.61		
		4.0	81685.45	155052.06		
1	-----					
	1*****	.0	78905.38	-156025.36		
		1.0	78905.38	-77119.98		
		2.0	78905.38	1785.40		
		3.0	78905.38	80690.78		
		4.0	78905.38	159596.16		
8	-----					
	1*****	.0	74655.59	-142837.21		
		1.0	74655.59	-68181.62		
		2.0	74655.59	6473.98		
		3.0	74655.59	81129.57		
		4.0	74655.59	155785.17		
22	-----					
	1*****	.0	69186.55	-128822.75		
		1.0	69186.55	-59636.21		
		2.0	69186.55	9550.34		
		3.0	69186.55	78736.89		
		4.0	69186.55	147923.43		
26	-----					
	1*****	.0	62595.67	-113062.32		
		1.0	62595.67	-50466.66		
		2.0	62595.67	12129.01		
		3.0	62595.67	74724.68		
		4.0	62595.67	137320.34		
30	-----					
	1*****	.0	54904.72	-95252.53		
		1.0	54904.72	-40347.82		
		2.0	54904.72	14556.90		
		3.0	54904.72	69461.62		
		4.0	54904.72	124366.34		
34	-----					
	1*****	.0	46131.57	-75266.00		
		1.0	46131.57	-29134.43		
		2.0	46131.57	16997.14		
		3.0	46131.57	63128.72		
		4.0	46131.57	109260.29		
38	-----					
	1*****					

2. Sa Statis Portal (13 lantai) Kg/m

TIME ELEMENT FORCES

T.D	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
9			.0	36320.70	-52831.67			
			1.0	36320.70	-16510.97			
			2.0	36320.70	19809.73			
			3.0	36320.70	56130.43			
			4.0	36320.70	92451.13			

23	1-62777.50		.0	25971.73	-26562.83			
			1.0	25971.73	-591.09			
			2.0	25971.73	25380.64			
			3.0	25971.73	51352.37			
			4.0	25971.73	77324.11			

6	1-22398.69		.0	9047.78	11910.39			
			1.0	9047.78	20958.17			
			2.0	9047.78	30005.95			
			3.0	9047.78	39053.73			
			4.0	9047.78	48101.51			

50	1 3517.42		.0	-18888.74	52461.27			
			1.0	-18888.74	33572.53			
			2.0	-18888.74	14683.79			
			3.0	-18888.74	-4204.95			
			4.0	-18888.74	-23093.69			

3	1*****		.0	88020.31	-295918.92			
			1.0	88020.31	-207898.61			
			2.0	88020.31	-119878.29			
			3.0	88020.31	-31857.98			
			4.0	88020.31	56162.34			

7	1*****		.0	97824.80	-232193.52			
			1.0	97824.80	-134368.71			
			2.0	97824.80	-36543.91			
			3.0	97824.80	61280.89			
			4.0	97824.80	159105.69			

.1	1*****		.0	98389.82	-204786.75			
			1.0	98389.82	-106396.93			
			2.0	98389.82	-8007.11			
			3.0	98389.82	90382.72			
			4.0	98389.82	188772.54			

15	1*****		.0	96457.94	-190821.15			
			1.0	96457.94	-94363.21			
			2.0	96457.94	2094.73			

2 isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

ST LOAD D COMB	AXIAL FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORO
		SHEAR	MOMENT	SHEAR	MOMENT	
	3.0	96457.94	98552.67			
	4.0	96457.94	195010.60			
9	-----					
	1*****					
	.0	92949.73	-179150.26			
	1.0	92949.73	-86200.53			
	2.0	92949.73	6749.20			
	3.0	92949.73	99698.93			
	4.0	92949.73	192648.67			
23	-----					
	1*****					
	.0	88135.25	-166488.29			
	1.0	88135.25	-78353.04			
	2.0	88135.25	9782.21			
	3.0	88135.25	97917.46			
	4.0	88135.25	186052.71			
27	-----					
	1*****					
	.0	82098.49	-151875.19			
	1.0	82098.49	-69776.70			
	2.0	82098.49	12321.79			
	3.0	82098.49	94420.28			
	4.0	82098.49	176518.77			
31	-----					
	1*****					
	.0	74876.07	-135027.06			
	1.0	74876.07	-60150.99			
	2.0	74876.07	14725.09			
	3.0	74876.07	89601.16			
	4.0	74876.07	164477.24			
35	-----					
	1*****					
	.0	66466.01	-115763.71			
	1.0	66466.01	-49297.70			
	2.0	66466.01	17168.32			
	3.0	66466.01	83634.33			
	4.0	66466.01	150100.35			
39	-----					
	1*****					
	.0	56838.13	-93763.49			
	1.0	56838.13	-36925.36			
	2.0	56838.13	19912.77			
	3.0	56838.13	76750.91			
	4.0	56838.13	133589.04			
43	-----					
	1*****					
	.0	45041.29	-68259.70			
	1.0	45041.29	-23218.41			
	2.0	45041.29	21822.88			
	3.0	45041.29	66864.16			
	4.0	45041.29	111905.45			

↳ sa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE	AXIAL TORQ
COMB			SHEAR	MOMENT	SHEAR	
7	1-98819.10					
		.0	38293.11	-42673.92		
		1.0	38293.11	-4380.81		
		2.0	38293.11	33912.29		
		3.0	38293.11	72205.40		
		4.0	38293.11	110498.51		
1	1-23021.65					
		.0	1583.74	21333.28		
		1.0	1583.74	22917.02		
		2.0	1583.74	24500.76		
		3.0	1583.74	26084.49		
		4.0	1583.74	27668.23		
4	1*****					
		.0	71463.47	-274772.38		
		1.0	71463.47	-203308.91		
		2.0	71463.47	-131845.44		
		3.0	71463.47	-60381.97		
		4.0	71463.47	11081.49		
8	1*****					
		.0	64911.30	-168274.57		
		1.0	64911.30	-103363.27		
		2.0	64911.30	-38451.97		
		3.0	64911.30	26459.33		
		4.0	64911.30	91370.63		
12	1*****					
		.0	61853.68	-132236.47		
		1.0	61853.68	-70382.79		
		2.0	61853.68	-8529.11		
		3.0	61853.68	53324.57		
		4.0	61853.68	115178.26		
16	1*****					
		.0	59341.20	-116421.96		
		1.0	59341.20	-57080.76		
		2.0	59341.20	2260.45		
		3.0	59341.20	61601.65		
		4.0	59341.20	120942.86		
20	1*****					
		.0	56634.16	-106024.76		
		1.0	56634.16	-49390.60		
		2.0	56634.16	7243.56		
		3.0	56634.16	63877.71		
		4.0	56634.16	120511.87		
24	1*****					
		.0	53397.72	-96277.76		

sa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

LOAD COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE	AXIAL
		SHEAR	MOMENT	SHEAR	TORQ
3	1.0	53397.72	-42880.04		
	2.0	53397.72	10517.68		
	3.0	53397.72	63915.40		
	4.0	53397.72	117313.12		
12	1*****				
	.0	49538.29	-85806.96		
	1.0	49538.29	-36268.67		
	2.0	49538.29	13269.62		
36	1*****				
	.0	45035.97	-74195.58		
	1.0	45035.97	-29159.61		
	2.0	45035.97	15876.35		
40	1*****				
	.0	39896.64	-61215.69		
	1.0	39896.64	-21319.06		
	2.0	39896.64	18577.58		
44	1*****				
	.0	33833.54	-46441.22		
	1.0	33833.54	-12607.68		
	2.0	33833.54	21225.86		
48	1-77679.30				
	.0	27084.50	-30423.60		
	1.0	27084.50	-3339.10		
	2.0	27084.50	23745.40		
52	1-45822.27				
	.0	24970.01	-13109.97		
	1.0	24970.01	11860.04		
	2.0	24970.01	36830.05		
52	1-9996.16				
	.0	-5994.32	35624.35		
	1.0	-5994.32	29630.02		
	2.0	-5994.32	23635.70		
	3.0	-5994.32	17641.38		

sa Statis Portal (13 lantai) Kg/m

M E E L E M E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDE 4.0	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
1	6843.28	.0	-5994.32	11647.05			
		2.0	-23891.88	135523.16			
		4.0	-29431.88	82199.39			
		6.0	-43171.88	17795.63			
		8.0	-48711.88	-74088.13			
		8.0	-54251.88	-177051.90			
1	-2553.94	.0					
		2.0	-33598.50	174879.67			
		4.0	-39138.50	102142.68			
		6.0	-52878.50	18325.68			
		8.0	-58418.50	-92971.32			
		8.0	-63958.50	-215348.31			
1	-7224.43	.0					
		2.0	-34585.23	178892.10			
		4.0	-40125.23	104181.64			
		6.0	-53865.23	18391.17			
		8.0	-59405.23	-94879.29			
		8.0	-64945.23	-219229.76			
1	-10465.04	.0					
		2.0	-32612.77	170973.58			
		4.0	-38152.77	100208.05			
		6.0	-51892.77	18362.52			
		8.0	-57432.77	-90963.02			
		8.0	-62972.77	-211368.55			
1	-13519.97	.0					
		2.0	-29375.20	157919.16			
		4.0	-34915.20	93628.76			
		6.0	-48655.20	18258.36			
		8.0	-54195.20	-84592.04			
		8.0	-59735.20	-198522.45			
1	-16487.07	.0					
		2.0	-25377.71	141772.58			
		4.0	-30917.71	85477.16			
		6.0	-44657.71	18101.73			
		8.0	-50197.71	-76753.69			
		8.0	-55737.71	-182689.11			
1	-19415.69	.0					
		2.0	-20762.66	123109.66			
		4.0	-26302.66	76044.33			
		6.0	-40042.66	17899.01			
		8.0	-45582.66	-67726.32			
		8.0	-51122.66	-164431.65			
1	-22322.54	.0					

sa Statis Portal (13 lantai) Kg/m

ELEM	ELEMENT	FORCE S	1-2 PLANE		1-3 PLANE	AXIAL
			SHEAR	MOMENT	SHEAR	TORQ
7	1-25501.85					
		AXIAL DIST FORCE ENDI				
		.0	-15559.28	102050.05		
		2.0	-21099.28	65391.49		
		4.0	-34839.28	17652.92		
		6.0	-40379.28	-57565.64		
		8.0	-45919.28	-143864.20		
0	1-28894.85					
		.0	-9749.90	78534.32		
		2.0	-15289.90	53494.51		
		4.0	-29029.90	17374.70		
		6.0	-34569.90	-46225.10		
		8.0	-40109.90	-120904.91		
33	1-25786.62					
		.0	5176.14	17930.19		
		2.0	-363.86	22742.48		
		4.0	-14103.86	16474.76		
		6.0	-19643.86	-17272.95		
		8.0	-25183.86	-62100.67		
44	1-3252.32					
		.0	7156.86	51874.93		
		3.0	-1153.14	60880.50		
		6.0	-17663.14	28556.07		
		9.0	-34173.14	-45098.36		
		12.0	-42483.14	-160082.79		
57	1-2492.59					
		.0	1818.87	83826.29		
		3.0	-6491.13	76817.90		
		6.0	-23001.13	28479.50		
		9.0	-39511.13	-61188.89		
		12.0	-47821.13	-192187.28		
60	1-4444.36					
		.0	489.41	91847.66		
		3.0	-7820.59	80850.88		
		6.0	-24330.59	28524.10		
		9.0	-40840.59	-65132.68		
		12.0	-49150.59	-200119.46		
63	1-6215.25					
		.0	627.25	91064.83		
		3.0	-7682.75	80481.59		
		6.0	-24192.75	28568.35		

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL DIST	1-2 PLANE		1-3 PLANE	AXIAL
D COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	TORQ
5	1 -6552.17				
	.0	-23273.64	128273.06		
	2.0	-28813.64	76185.78		
	4.0	-42553.64	13018.50		
	6.0	-48093.64	-77628.78		
	8.0	-53633.64	-179356.07		
8	1 -3057.62				
	.0	-34234.03	171705.16		
	2.0	-39774.03	97697.09		
	4.0	-53514.03	12609.03		
	6.0	-59054.03	-99959.04		
	8.0	-64594.03	-223607.10		
51	1 -2512.48				
	.0	-36204.31	179474.23		
	2.0	-41744.31	101525.62		
	4.0	-55484.31	12497.00		
	6.0	-61024.31	-104011.61		
	8.0	-66564.31	-231600.22		
64	1 -2707.05				
	.0	-35055.05	174912.75		
	2.0	-40595.05	99262.66		
	4.0	-54335.05	12532.57		
	6.0	-59875.05	-101677.52		
	8.0	-65415.05	-226967.62		
67	1 3236.44				
	.0	-32537.07	164946.94		
	2.0	-38077.07	94332.80		
	4.0	-51817.07	12638.66		
	6.0	-57357.07	-96535.49		
	8.0	-62897.07	-216789.63		
70	1 -3859.43				
	.0	-29159.14	151593.06		
	2.0	-34699.14	87734.78		
	4.0	-48439.14	12796.49		
	6.0	-53979.14	-89621.80		
	8.0	-59519.14	-203120.08		
73	1 -4502.32				
	.0	-25065.42	135421.57		
	2.0	-30605.42	79750.73		
	4.0	-44345.42	12999.90		
	6.0	-49885.42	-81230.94		
	8.0	-55425.42	-186541.78		
76	1 -5139.33				
	.0	-20282.74	116537.90		

sa Statis Portal (13 lantai) Kg/m

ELEM	ELEMENT	FORCE	1-2 PLANE		1-3 PLANE		AXIAL
			SHEAR	MOMENT	SHEAR	MOMENT	TORQ
1	6063.13						
		2.0	-25822.74	70432.43			
		4.0	-39562.74	13246.96			
		6.0	-45102.74	-71418.51			
		8.0	-50642.74	127163.99			
2	6749.04						
		2.0	-14770.37	94790.85			
		4.0	-20310.37	59710.12			
		6.0	-34050.37	13549.39			
		8.0	-39590.37	-60091.34			
		10.0	-45130.37	-144812.07			
35	2114.49						
		2.0	-8466.93	69858.89			
		4.0	-14006.93	47385.03			
		6.0	-27746.93	13831.18			
		8.0	-33286.93	-47202.68			
		10.0	-38826.93	119316.53			
36	22008.61						
		2.0	17821.49	-24327.94			
		4.0	10681.49	4175.05			
		6.0	-4658.51	18398.03			
		8.0	-11798.51	1941.01			
		10.0	-18938.51	-28796.01			
87	41094.67						
		3.0	24565.39	-33150.71			
		6.0	13855.39	24480.47			
		9.0	-5054.61	33581.65			
		12.0	-23964.61	-5847.17			
		15.0	-34674.61	-93805.99			
88	7645.82						
		2.0	12604.94	-4856.53			
		4.0	5464.94	13213.36			
		6.0	-9875.06	17003.24			
		8.0	-17015.06	-9886.87			
		10.0	-24155.06	-51056.99			
89	85469.04						
		2.0	15150.65	-30025.89			
		4.0	10770.65	-4104.58			
		6.0	510.65	13056.72			
		8.0	-3869.35	9698.03			

isa Statis Portal (13 lantai) Kg/m

NAME ELEMENT FORCES

T LOAD	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
0	COMB	8.0	-8249.35	-2420.66			
1-----							
1	5994.32						
		1.0	13403.84	-25277.75			
		2.0	9023.84	7850.07			
		4.0	1236.16	10817.60			
		6.0	-5616.16	3965.27			
		8.0	-9996.16	-11647.05			
0-----							
0	1-18907.92						
		1.0	16751.14	-25714.04			
		3.0	10181.14	14684.38			
		6.0	-2268.86	23612.80			
		9.0	-14718.86	1071.23			
		12.0	-21288.86	-52940.35			

Wisa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LT	LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
ID	COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
1		1151699.37						
			.0	62516.48	-262066.78			
			1.0	62516.48	-199550.29			
			2.0	62516.48	-137033.81			
			3.0	62516.48	-74517.33			
			4.0	62516.48	-12000.84			
5		1131291.18						
			.0	51813.56	-142091.30			
			1.0	51813.56	-90277.74			
			2.0	51813.56	-38464.18			
			3.0	51813.56	13349.39			
			4.0	51813.56	65162.95			
9		1101441.47						
			.0	47231.64	-103239.04			
			1.0	47231.64	-56007.40			
			2.0	47231.64	-8775.76			
			3.0	47231.64	38455.89			
			4.0	47231.64	85687.53			
13		1 70800.19						
			.0	43834.39	-85950.24			
			1.0	43834.39	-42115.85			
			2.0	43834.39	1718.54			
			3.0	43834.39	45552.94			
			4.0	43834.39	89387.33			
17		1 42305.15						
			.0	40194.70	-73644.12			
			1.0	40194.70	-33449.42			
			2.0	40194.70	6745.28			
			3.0	40194.70	46939.98			
			4.0	40194.70	87134.69			
21		1 17210.11						
			.0	36128.35	-62200.54			
			1.0	36128.35	-26072.19			
			2.0	36128.35	10056.16			
			3.0	36128.35	46184.50			
			4.0	36128.35	82312.85			
25		1 -3734.44						
			.0	31548.47	-50271.97			
			1.0	31548.47	-18723.49			
			2.0	31548.47	12824.98			
			3.0	31548.47	44373.45			
			4.0	31548.47	75921.93			
29		1-19923.75						
			.0	26419.24	-37447.92			

isa Statis Portal (13 lantai) Kg/m

A M E C L E M E N T F O R C E S

T D	LOAD COMB	AXIAL FORCE	DIST ENDDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
			1.0	26419.24	-11028.68			
			2.0	26419.24	15390.55			
			3.0	26419.24	41809.79			
			4.0	26419.24	68229.02			
3		1-30793.57						
			.0	20722.14	23626.32			
			1.0	20722.14	-2904.18			
			2.0	20722.14	17817.96			
			3.0	20722.14	38540.09			
			4.0	20722.14	59262.23			
37		1-35798.20						
			.0	14449.80	-8855.71			
			1.0	14449.80	5594.10			
			2.0	14449.80	20043.90			
			3.0	14449.80	34493.71			
			4.0	14449.80	48943.51			
41		1-34428.49						
			.0	7579.64	6637.48			
			1.0	7579.64	14217.13			
			2.0	7579.64	21796.77			
			3.0	7579.64	29376.41			
			4.0	7579.64	36956.06			
45		1-26291.33						
			.0	-67.14	22065.58			
			1.0	-67.14	21998.44			
			2.0	-67.14	21931.31			
			3.0	-67.14	21864.17			
			4.0	-67.14	21797.03			
49		1-11568.11						
			.0	-12316.57	33761.63			
			1.0	-12316.57	21445.07			
			2.0	-12316.57	9128.50			
			3.0	-12316.57	-3188.07			
			4.0	-12316.57	-15504.64			
2		1*****						
			.0	76822.98	-280532.04			
			1.0	76822.98	-203709.06			
			2.0	76822.98	-126886.08			
			3.0	76822.98	-50063.10			
			4.0	76822.98	26759.88			
6		1*****						
			.0	79865.71	-195654.00			
			1.0	79865.71	-115788.29			
			2.0	79865.71	-35922.58			
			3.0	79865.71	43943.12			

isa Statis Portal (13 lantai) kg/m

A M E E L E M E N T F O R C E S

ELEM ID	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
				SHEAR	MOMENT	SHEAR	MOMENT	
10			4.0	79865.71	123808.83			
		1*****						
			.0	79758.09	-167968.06			
			1.0	79758.09	-88209.97			
			2.0	79758.09	-8451.88			
			3.0	79758.09	71306.21			
			4.0	79758.09	151064.30			
14								
		1*****						
			.0	76897.19	-152175.30			
			1.0	76897.19	-75278.11			
			2.0	76897.19	1619.08			
			3.0	76897.19	78516.27			
			4.0	76897.19	155413.46			
18								
		1*****						
			.0	72577.51	-138858.22			
			1.0	72577.51	-66280.71			
			2.0	72577.51	6296.81			
			3.0	72577.51	78874.32			
			4.0	72577.51	151451.83			
22								
		1*****						
			.0	67048.00	-124731.66			
			1.0	67048.00	-57683.67			
			2.0	67048.00	9364.33			
			3.0	67048.00	76412.32			
			4.0	67048.00	143460.32			
26								
		1*****						
			.0	60406.57	-108892.11			
			1.0	60406.57	-48485.54			
			2.0	60406.57	11921.03			
			3.0	60406.57	72327.59			
			4.0	60406.57	132734.16			
30								
		1*****						
			.0	52681.67	-91075.01			
			1.0	52681.67	-38393.34			
			2.0	52681.67	14288.32			
			3.0	52681.67	66969.99			
			4.0	52681.67	119651.66			
34								
		1*****						
			.0	43888.22	-71247.45			
			1.0	43888.22	-27359.23			
			2.0	43888.22	16528.99			
			3.0	43888.22	60417.21			
			4.0	43888.22	104305.43			
38								
		1*****						

Analisa Statis Portal (13 lantai) Kg/m

NAME ELEMENT		FORCE S			
T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE	AXIAL
		SHEAR	MOMENT	SHEAR	MOMENT
	.0	34050.64	-49523.34		
	1.0	34050.64	-15472.71		
	2.0	34050.64	18577.93		
	3.0	34050.64	52628.57		
	4.0	34050.64	86679.20		
2	-----				
	1*****				
	.0	23223.63	-26261.41		
	1.0	23223.63	-3037.78		
	2.0	23223.63	20185.85		
	3.0	23223.63	43409.48		
	4.0	23223.63	66633.11		
46	-----				
	1-68617.39				
	.0	11022.13	-2493.55		
	1.0	11022.13	8528.58		
	2.0	11022.13	19550.71		
	3.0	11022.13	30572.85		
	4.0	11022.13	41594.98		
50	-----				
	1-25093.81				
	.0	-825.54	14878.03		
	1.0	-825.54	14052.49		
	2.0	-825.54	13226.95		
	3.0	-825.54	12401.41		
	4.0	-825.54	11575.86		
5	-----				
	1*****				
	.0	88420.43	-295551.58		
	1.0	88420.43	-207131.15		
	2.0	88420.43	-118710.71		
	3.0	88420.43	-30290.28		
	4.0	88420.43	58130.15		
7	-----				
	1*****				
	.0	98771.74	-234036.59		
	1.0	98771.74	-135264.84		
	2.0	98771.74	-36493.10		
	3.0	98771.74	62278.64		
	4.0	98771.74	161050.38		
11	-----				
	1*****				
	.0	99495.52	-207039.39		
	1.0	99495.52	-107543.88		
	2.0	99495.52	-8048.36		
	3.0	99495.52	91447.16		
	4.0	99495.52	190942.67		
15	-----				
	1*****				
	.0	97727.91	-193439.45		
	1.0	97727.91	-95711.54		
	2.0	97727.91	2016.37		

isa Statis Portal (13 lantai) Kg/m

A N E E L E M E N T F O R C E S

ELEM ID	LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ		
				SHEAR	MOMENT	SHEAR	MOMENT			
19	-----		3.0	97727.91	99744.29					
			4.0	97727.91	197472.20					
			1*****							
			0.0	94371.77	-182088.34					
23	-----		1.0	94371.77	-97716.57					
			2.0	94371.77	6655.21					
			3.0	94371.77	101026.98					
			4.0	94371.77	195398.75					
			1*****							
27	-----		0.0	89703.21	-169733.81					
			1.0	89703.21	-80030.59					
			2.0	89703.21	9672.62					
			3.0	89703.21	99375.83					
			4.0	89703.21	189079.05					
31	-----		1*****							
			0.0	83805.71	-155425.59					
			1.0	83805.71	-71619.88					
			2.0	83805.71	12185.82					
			3.0	83805.71	95991.53					
35	-----		4.0	83805.71	179797.24					
			1*****							
			0.0	76713.94	-138916.45					
			1.0	76713.94	-62202.51					
			2.0	76713.94	14511.43					
39	-----		3.0	76713.94	91225.37					
			4.0	76713.94	167939.31					
			1*****							
			0.0	68446.35	-120177.70					
			1.0	68446.35	-51731.35					
43	-----		2.0	68446.35	16715.00					
			3.0	68446.35	85161.35					
			4.0	68446.35	153607.70					
			1*****							
			0.0	59014.29	-99317.44					
43	-----		1.0	59014.29	-40303.14					
			2.0	59014.29	18711.15					
			3.0	59014.29	77725.44					
			4.0	59014.29	136739.74					
			1*****							
43	-----		0.0	48472.93	-76778.01					
			1.0	48472.93	-28305.08					
			2.0	48472.93	20167.85					
			3.0	48472.93	68640.79					
			4.0	48472.93	117113.72					

sa Statis Portal (13 lantai) Kg/m

M E M B E R E N T F O R C E S

LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ

1-75724.34							
	.0		36706.72	-53899.71			
	1.0		36706.72	-17192.99			
	2.0		36706.72	19513.73			
	3.0		36706.72	56220.45			
	4.0		36706.72	92927.17			

1-30920.87							
	.0		28355.03	-36339.54			
	1.0		28355.03	-7984.51			
	2.0		28355.03	20370.52			
	3.0		28355.03	48725.55			
	4.0		28355.03	77080.58			

4	1*****						
		.0	71820.10	-274397.87			
		1.0	71820.10	-202577.77			
		2.0	71820.10	-130757.67			
		3.0	71820.10	-58937.56			
		4.0	71820.10	12882.54			

8	1*****						
		.0	65639.99	-169661.86			
		1.0	65639.99	-104021.87			
		2.0	65639.99	-38381.88			
		3.0	65639.99	27258.11			
		4.0	65639.99	92898.10			

2	1*****						
		.0	62727.75	-134074.42			
		1.0	62727.75	-71346.67			
		2.0	62727.75	-8618.92			
		3.0	62727.75	54108.83			
		4.0	62727.75	116836.58			

.6	1*****						
		.0	60286.51	-118422.70			
		1.0	60286.51	-58136.19			
		2.0	60286.51	2150.32			
		3.0	60286.51	62436.82			
		4.0	60286.51	122723.33			

20	1*****						
		.0	57645.01	-108174.85			
		1.0	57645.01	-50529.84			
		2.0	57645.01	7115.17			
		3.0	57645.01	64760.19			
		4.0	57645.01	122405.20			

24	1*****						
		.0	54463.44	-98554.58			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ	
		SHEAR	MOMENT	SHEAR	MOMENT		
	1.0	54463.44	-44091.13				
	2.0	54463.44	10372.31				
	3.0	54463.44	64835.76				
	4.0	54463.44	119299.20				
28	-----						
	1*****						
	1.0	50647.25	-88198.15				
	2.0	50647.25	-37550.90				
	3.0	50647.25	13096.35				
	4.0	50647.25	63743.60				
32	-----						
	1*****						
	1.0	46168.16	-76716.79				
	2.0	46168.16	-30548.63				
	3.0	46168.16	15619.52				
	4.0	46168.16	61787.68				
36	-----						
	1*****						
	1.0	41012.29	-64012.00				
	2.0	41012.29	-22999.71				
	3.0	41012.29	18012.58				
	4.0	41012.29	59024.87				
40	-----						
	1*****						
	1.0	35151.26	-50121.27				
	2.0	35151.26	-14970.00				
	3.0	35151.26	20181.26				
	4.0	35151.26	55332.52				
44	-----						
	1-83118.28						
	1.0	28496.79	-35379.84				
	2.0	28496.79	-6883.05				
	3.0	28496.79	21613.74				
	4.0	28496.79	50110.54				
48	-----						
	1-46966.94						
	1.0	21729.28	-21216.06				
	2.0	21729.28	513.22				
	3.0	21729.28	22242.51				
	4.0	21729.28	43971.79				
52	-----						
	1-17257.22						
	1.0	12307.08	-7770.21				
	2.0	12307.08	4536.87				
	3.0	12307.08	16843.95				
	4.0	12307.08	29151.03				

lisa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

LT LOAD ID	COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE SHEAR	MOMENT	1-3 PLANE SHEAR	MOMENT	AXIAL TORQ
53			4.0	12307.08	41458.11			
	1	7213.92						
			1.0	20408.19	130090.46			
			2.0	-27548.19	82134.09			
			4.0	-42888.19	19897.72			
			6.0	-50028.19	-73018.65			
			8.0	-57168.19	-180215.03			
56								
	1	-2296.08						
			1.0	-29849.71	168401.99			
			2.0	-36989.71	101562.57			
			4.0	-52329.71	20443.15			
			6.0	-59469.71	-91356.28			
			8.0	-66609.71	-217435.70			
59								
	1	-7069.75						
			1.0	30641.28	171637.78			
			2.0	-37781.28	103215.21			
			4.0	-53121.28	20512.65			
			6.0	-60261.28	-92869.91			
			8.0	-67401.28	-220532.48			
62								
	1	-10317.31						
			1.0	-28495.04	163031.45			
			2.0	-35635.04	98901.38			
			4.0	-50975.04	20491.30			
			6.0	-58115.04	-88598.77			
			8.0	-65255.04	-211968.84			
65								
	1	-13379.64						
			1.0	-25095.04	149335.22			
			2.0	-32235.04	92005.14			
			4.0	-47575.04	20395.06			
			6.0	-54715.04	-81895.02			
			8.0	-61855.04	-198465.10			
68								
	1	-16355.13						
			1.0	-20944.56	132584.82			
			2.0	-28084.56	83555.70			
			4.0	-43424.56	20246.59			
			6.0	-50564.56	-73742.52			
			8.0	-57704.56	-182011.63			
71								
	1	-19295.76						
			1.0	-16189.31	113369.84			
			2.0	-23329.31	73851.22			
			4.0	-38669.31	20052.60			
			6.0	-45809.31	-64426.02			
			8.0	-52949.31	-163184.64			
74								
	1	-22216.90						

Isa Statis Portal (13 lantai) Kg/m

ELEMENT FORCES

T LOAD D COMB	AXIAL DIST FORCE ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
		SHEAR	MOMENT	SHEAR	MOMENT	
	.0	-10869.82	91855.35			
	2.0	-18009.82	62975.71			
	4.0	-33349.82	19816.06			
	6.0	-40489.82	-54023.58			
	8.0	-47629.82	-142143.22			
7	1-25130.67					
	.0	-5004.62	68117.94			
	2.0	-12144.62	50968.70			
	4.0	-27484.62	19539.45			
	6.0	-34624.62	-42569.80			
	8.0	-41764.62	-118959.04			
30	1-28022.84					
	.0	1369.71	42306.03			
	2.0	-5770.29	37905.44			
	4.0	-21110.29	19224.85			
	6.0	-28250.29	-30135.74			
	8.0	-35390.29	-93776.33			
83	1-30735.22					
	.0	8137.17	14890.48			
	2.0	997.17	24024.81			
	4.0	-14342.83	18879.14			
	6.0	-21482.83	-16946.52			
	8.0	-28622.83	-67052.19			
86	1-29621.57					
	.0	14723.22	-11964.60			
	2.0	7583.22	10341.83			
	4.0	-7756.78	18368.26			
	6.0	-14896.78	-4285.30			
	8.0	-22036.78	-41218.87			
54	1 4171.19					
	.0	11997.35	42198.85			
	3.0	1287.35	62125.91			
	6.0	-17622.65	33522.96			
	9.0	-36532.65	-43609.98			
	12.0	-47242.65	-169272.92			
57	1 -2188.46					
	.0	6626.69	74341.19			
	3.0	-4083.31	78156.26			
	6.0	-22993.31	33441.34			
	9.0	-41903.31	-59803.58			
	12.0	-52613.31	-201578.50			
60	1 -4208.85					
	.0	5241.99	82707.12			
	3.0	-5468.01	82368.09			
	6.0	-24378.01	33499.07			

isa Statis Portal (13 lantai) Kg/m

A M E E L E M E N T F O R C E S

T LOAD COMB	AXIAL FORCE	DIST ENDI	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	SHEAR	MOMENT	
3	1	9.0	-43288.01	-63899.95			
		12.0	-53998.01	-209828.97			
		.0	5318.85	82302.83			
		3.0	-5391.15	82194.37			
		6.0	-24301.15	33555.91			
6	1	9.0	-43211.15	-63612.55			
		12.0	-53921.15	-209311.01			
		.0	6091.47	77718.39			
		3.0	-4618.53	79927.79			
		6.0	-23528.53	33607.19			
9	1	9.0	-42438.53	-61243.41			
		12.0	-53148.53	-204624.02			
		.0	7328.42	70340.80			
		3.0	-3381.58	76261.05			
		6.0	-22291.58	33651.29			
12	1	9.0	-41201.58	-57488.46			
		12.0	-51911.58	-197158.21			
		.0	8954.11	60624.53			
		3.0	-1755.89	71421.86			
		6.0	-20665.89	33689.18			
15	1	9.0	-39575.89	-52573.50			
		12.0	-50285.89	-187366.18			
		.0	10937.53	48755.89			
		3.0	227.53	65503.49			
		6.0	-18682.47	33721.10			
18	1	9.0	-37592.47	-46591.30			
		12.0	-48302.47	-175433.69			
		.0	13256.33	34869.73			
		3.0	2546.33	58573.72			
		6.0	-16363.67	33747.71			
21	1	9.0	-35273.67	-39608.30			
		12.0	-45983.67	-161494.32			
		.0	15877.08	19164.29			
		3.0	5167.08	50730.53			
		6.0	-13742.92	33766.77			
24	1	9.0	-32652.92	-31726.99			
		12.0	-43362.92	-145750.75			

isa Statis Portal (15 lantai) Kg/m

A M E L L E M E N T F O R C E S

T LOAD	AXIAL	DIST	1-2 PLANE		1-3 PLANE		AXIAL
D COMB	FORCE	ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
4	1-18533.72						
		1.0	18724.95	2074.47			
		3.0	8014.95	42184.33			
		6.0	10895.05	33764.19			
		9.0	29805.05	23185.94			
		12.0	40515.05	128666.08			
37	1-17773.89						
		1.0	21486.80	-14501.92			
		3.0	10776.80	33893.49			
		6.0	8133.20	33758.90			
		9.0	-27043.20	-14905.63			
		12.0	37753.20	-112100.27			
55	1-6130.11						
		1.0	-19799.78	122893.82			
		3.0	-26939.78	76154.26			
		4.0	-42279.78	15134.71			
		6.0	-49419.78	-76564.85			
		8.0	-56559.78	-182544.40			
58	1-2912.28						
		1.0	-30805.47	166511.27			
		2.0	-37945.47	97760.32			
		4.0	53285.47	14729.37			
		6.0	-60425.47	-98981.57			
		8.0	-67565.47	-226972.52			
61	1-2441.24						
		1.0	-32846.55	174553.14			
		2.0	-39986.55	101720.04			
		4.0	-55326.55	14606.93			
		6.0	-62466.55	-103186.18			
		8.0	-69606.55	-235259.28			
64	1-2641.49						
		1.0	-31763.46	170249.53			
		2.0	-38903.46	99582.60			
		4.0	-54243.46	14635.67			
		6.0	-61383.46	-100991.25			
		8.0	-68523.46	-230898.18			
67	1-3181.57						
		1.0	-29303.54	160508.54			
		2.0	-36443.54	94761.46			
		4.0	-51783.54	14734.38			
		6.0	-58923.54	-95972.70			
		8.0	-66063.54	-220959.78			
70	1-3816.19						
		1.0	-25975.47	147346.43			

isa Statis Portal (10 Lantai) Kg/m

N A M E E L E M E N T F O R C E S

I D	LOAD COMB	AXIAL FORCE	1-2 PLANE		1-3 PLANE		AXIAL TORQ
			DISP ENDE	SHEAR	MOMENT	SHEAR	
75	1	2.0	-33115.47	88255.48			
		4.0	-48455.47	14884.54			
		6.0	-55595.47	-89166.41			
		8.0	-62735.47	-207497.35			
76	1	2.0	21926.90	131347.52			
		4.0	29066.90	90353.73			
		6.0	-44406.90	15079.24			
		8.0	-51546.90	-80873.85			
79	1	2.0	17201.39	112683.31			
		4.0	24341.39	71140.53			
		6.0	-29681.39	15317.74			
		8.0	-46821.39	71185.05			
82	1	2.0	-11818.66	91430.82			
		4.0	-18958.66	60653.51			
		6.0	34298.66	15596.20			
		8.0	-41438.66	-60141.11			
85	1	2.0	-5823.83	67767.01			
		4.0	-12963.83	48979.35			
		6.0	-28303.83	15911.69			
		8.0	-35443.83	-47835.97			
88	1	2.0	608.66	42347.35			
		4.0	-6531.34	36424.66			
		6.0	-21871.34	16221.98			
		8.0	-29011.34	-34660.71			
89	1	2.0	7050.28	17166.44			
		4.0	-89.72	24127.01			
		6.0	-15429.72	16807.58			
		8.0	-22569.72	-21191.86			
89	1	2.0	11568.11	-15504.64			
		4.0	7188.11	3251.58			
		6.0	-3071.89	13247.80			
		8.0	-7451.89	2724.02			

isa Statis Portal (13 lantai) kg/m

A M E E L E M E N T F O R C E S

T LOAD	AXIAL DIST	1 2 PLANE		1-3 PLANE		AXIAL
D COMB	FORCE ENDI	SHEAR	MOMENT	SHEAR	MOMENT	TORQ
	8.0	-11831.89	-16559.77			
21	-----					
	1-12307.08					
	1.0	6142.78	2999.65			
	3.0	1763.78	10905.21			
	4.0	-8497.22	10050.77			
	6.0	-12877.22	-11323.67			
	8.0	-17257.22	41458.11			
20	-----					
	1-40662.11					
	1.0	13261.91	-4983.90			
	3.0	6691.91	24946.84			
	6.0	-5758.09	23407.58			
	9.0	-18208.09	-9601.67			
	12.0	-24778.09	-74080.93			

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.003359	.000235	-.001423
6	.004166	-.002006	-.001528
7	.005635	-.001799	-.001339
8	.006896	-.002869	-.001803
9	.010001	.000411	-.001837
10	.011044	-.003757	-.001756
11	.013144	-.003503	-.001415
12	.014670	-.005411	-.001565
13	.017700	.000480	-.001989
14	.018667	-.005282	-.001874
15	.020847	-.005106	-.001446
16	.022282	-.007649	-.001610
17	.025690	.000447	-.002031
18	.026503	-.006597	-.001906
19	.028625	-.006588	-.001456
20	.029988	-.009590	-.001619
21	.033682	.000327	-.002025
22	.034328	-.007722	-.001902
23	.036348	-.007943	-.001434
24	.037627	-.011245	-.001591
25	.041513	.000137	-.001983
26	.041989	-.008671	-.001864
27	.043865	-.009163	-.001379
28	.045044	-.012627	-.001527
29	.049041	-.000104	-.001907
30	.049340	-.009459	-.001794
31	.051034	-.010245	-.001295
32	.052098	-.013751	-.001430
33	.056128	-.000376	-.001799
34	.056242	-.010100	-.001694
35	.057718	-.011181	-.001182
36	.058650	-.014633	-.001303
37	.062644	-.000660	-.001660
38	.062563	-.010604	-.001565
39	.063791	-.011969	-.001043
40	.064573	-.015295	-.001147
41	.068470	-.000932	-.001494
42	.068182	-.010983	-.001410
43	.069133	-.012600	-.000879
44	.069750	-.015759	-.000966
45	.073491	-.001169	-.001302
46	.072981	-.011248	-.001230
47	.073640	-.013069	-.000699
48	.074070	-.016051	-.000755
49	.077586	-.001343	-.001077
50	.076871	-.011411	-.001047
51	.077199	-.013367	-.000464

isa Statis Portal (13 lantai) Kg/m

I N T E R N A L D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
52	.077343	-.016200	-.000479
53	.080937	-.001427	-.001093
54	.080101	-.011479	-.001044
55	.079455	-.013475	-.000174
56	.079367	-.016245	-.000260

tisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.011700	.000173	-.002940
6	.012804	-.002148	-.002898
7	.014343	-.001979	-.002701
8	.014999	-.002951	-.003331
9	.019714	.000367	-.001753
10	.020754	-.003880	-.001667
11	.022857	-.003639	-.001307
12	.024534	-.005490	-.001528
13	.027361	.000421	-.001999
14	.028326	-.005411	-.001887
15	.030526	-.005252	-.001455
16	.031971	-.007731	-.001581
17	.035353	.000376	-.002027
18	.036159	-.006731	-.001905
19	.038276	-.006738	-.001442
20	.039636	-.009676	-.001615
21	.043320	.000245	-.002019
22	.043957	-.007859	-.001900
23	.045967	-.008095	-.001424
24	.047248	-.011334	-.001583
25	.051123	.000045	-.001977
26	.051590	-.008812	-.001862
27	.053458	-.009319	-.001370
28	.054639	-.012719	-.001520
29	.058624	-.000203	-.001902
30	.058915	-.009604	-.001793
31	.060602	-.010402	-.001286
32	.061667	-.013845	-.001424
33	.065685	-.000482	-.001794
34	.065792	-.010247	-.001693
35	.067263	-.011341	-.001174
36	.068197	-.014730	-.001297
37	.072180	-.000771	-.001656
38	.072092	-.010754	-.001565
39	.073315	-.012129	-.001035
40	.074100	-.015394	-.001142
41	.077986	-.001047	-.001491
42	.077693	-.011135	-.001411
43	.078640	-.012762	-.000872
44	.079261	-.015860	-.000962
45	.082991	-.001286	-.001300
46	.082478	-.011401	-.001231
47	.083134	-.013231	-.000692
48	.083568	-.016153	-.000752
49	.087076	-.001462	-.001076
50	.086358	-.011565	-.001050
51	.086683	-.013529	-.000457

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
52	.086831	-.016303	-.000475
53	.090423	-.001546	-.001093
54	.089584	-.011633	-.001043
55	.088929	-.013636	-.000173
56	.088841	-.016348	-.000255

sa Statis Portal (13 lantai) Kg/m

INT DISPLACEMENTS

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.020238	.000211	-.006745
6	.020424	-.002170	-.006350
7	.020792	-.002037	-.006160
8	.020865	-.003188	-.006931
9	.047821	.000352	-.004415
10	.048994	-.004103	-.004256
11	.050764	-.003985	-.004057
12	.051553	-.006179	-.004683
13	.057619	.000489	-.001823
14	.058554	-.005585	-.001726
15	.060807	-.005597	-.001328
16	.062554	-.008673	-.001517
17	.065648	.000468	-.002098
18	.066480	-.006912	-.001976
19	.068677	-.007183	-.001513
20	.070019	-.010854	-.001618
21	.073851	.000362	-.002071
22	.074502	-.008045	-.001947
23	.076537	-.008624	-.001470
24	.077774	-.012722	-.001641
25	.081867	.000187	-.002031
26	.082342	-.009002	-.001915
27	.084232	-.009923	-.001432
28	.085372	-.014288	-.001582
29	.089601	-.000040	-.001962
30	.089905	-.009799	-.001852
31	.091618	-.011075	-.001355
32	.092639	-.015569	-.001495
33	.096928	-.000299	-.001861
34	.097051	-.010446	-.001759
35	.098550	-.012072	-.001250
36	.099437	-.016582	-.001375
37	.103713	-.000570	-.001729
38	.103646	-.010956	-.001636
39	.104899	-.012909	-.001117
40	.105635	-.017348	-.001226
41	.109833	-.000831	-.001569
42	.109562	-.011339	-.001487
43	.110543	-.013580	-.000959
44	.111113	-.017890	-.001051
45	.115170	-.001059	-.001382
46	.114681	-.011607	-.001311
47	.115373	-.014078	-.000783
48	.115755	-.018234	-.000845
49	.119599	-.001228	-.001159
50	.118909	-.011771	-.001132
51	.119274	-.014394	-.000552

isa Statis Portal (13 lantai) Kg/m

I N T E R D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
.2	.119377	-.018409	-.000580
.3	.123289	-.001308	-.001172
.4	.122478	-.011837	-.001117
.5	.121902	-.014509	-.000275
.6	.121817	-.018454	-.000377

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.022832	.000211	-.008215
6	.022889	-.002190	-.007572
7	.022907	-.001884	-.007153
8	.022800	-.002857	-.007880
9	.062860	.000324	-.008936
10	.062901	-.004126	-.008424
11	.063142	-.003689	-.008294
12	.063271	-.005496	-.009167
13	.095246	.000329	-.004847
14	.096357	-.005806	-.004667
15	.098185	-.005429	-.004457
16	.098984	-.007894	-.005102
17	.105553	.000325	-.001835
18	.106330	-.007042	-.001723
19	.108584	-.006828	-.001288
20	.110328	-.009816	-.001462
21	.113486	.000172	-.002066
22	.114139	-.008151	-.001943
23	.116246	-.008208	-.001446
24	.117527	-.011471	-.001539
25	.121368	-.000044	-.001984
26	.121825	-.009085	-.001863
27	.123708	-.009438	-.001358
28	.124876	-.012853	-.001522
29	.128862	-.000308	-.001900
30	.129135	-.009862	-.001790
31	.130827	-.010527	-.001282
32	.131889	-.013977	-.001420
33	.135902	-.000599	-.001791
34	.135993	-.010493	-.001690
35	.137470	-.011470	-.001169
36	.138400	-.014861	-.001294
37	.142376	-.000897	-.001653
38	.142276	-.010990	-.001562
39	.143505	-.012261	-.001031
40	.144288	-.015524	-.001139
41	.148166	-.001180	-.001489
42	.147862	-.011365	-.001409
43	.148816	-.012895	-.000869
44	.149436	-.015990	-.000960
45	.153159	-.001424	-.001299
46	.152637	-.011626	-.001230
47	.153302	-.013366	-.000690
48	.153736	-.016283	-.000750
49	.157236	-.001603	-.001076
50	.156513	-.011788	-.001050
51	.156846	-.013665	-.000456

lisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
52	.156993	-.016432	-.000474
53	.160586	-.001688	-.001097
54	.159742	-.011855	-.001045
55	.159087	-.013772	-.000171
56	.158999	-.016477	-.000254



lisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.023392	.000260	-.008543
6	.023488	-.002233	-.007898
7	.023545	-.001818	-.007469
8	.023442	-.002838	-.008208
9	.066975	.000416	-.010556
10	.066884	-.004208	-.009774
11	.066766	-.003560	-.009398
12	.066712	-.005451	-.010236
13	.112322	.000432	-.009470
14	.112296	-.005909	-.008941
15	.112498	-.005248	-.008804
16	.112633	-.007799	-.009699
17	.145611	.000334	-.004932
18	.146627	-.007352	-.004732
19	.148446	-.006874	-.004508
20	.149207	-.009897	-.005142
21	.156053	.000229	-.001852
22	.156655	-.008372	-.001728
23	.158848	-.008158	-.001268
24	.160508	-.011530	-.001434
25	.163867	-.000004	-.002031
26	.164334	-.009285	-.001912
27	.166321	-.009414	-.001395
28	.167500	-.012909	-.001481
29	.171464	-.000281	-.001914
30	.171732	-.010042	-.001798
31	.173453	-.010513	-.001277
32	.174503	-.014031	-.001430
33	.178535	-.000583	-.001797
34	.178611	-.010657	-.001695
35	.180105	-.011464	-.001172
36	.181034	-.014913	-.001297
37	.185022	-.000890	-.001658
38	.184908	-.011142	-.001566
39	.186154	-.012261	-.001033
40	.186934	-.015575	-.001142
41	.190823	-.001180	-.001493
42	.190508	-.011506	-.001413
43	.191477	-.012900	-.000871
44	.192096	-.016040	-.000963
45	.195827	-.001429	-.001303
46	.195295	-.011762	-.001234
47	.195976	-.013374	-.000693
48	.196408	-.016332	-.000754
49	.199917	-.001611	-.001081
50	.199185	-.011920	-.001055
51	.199533	-.013675	-.000457

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
2	.199677	-.016481	-.000477
3	.203286	-.001697	-.001104
4	.202433	-.011986	-.001050
5	.201781	-.013783	-.000171
6	.201693	-.016526	-.000257

lisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.023471	.000313	-.008603
6	.023575	-.002278	-.007958
7	.023645	-.001751	-.007537
8	.023544	-.002820	-.008283
9	.067777	.000521	-.010901
10	.067728	-.004299	-.010112
11	.067655	-.003427	-.009723
12	.067607	-.005414	-.010573
13	.116646	.000583	-.011055
14	.116488	-.006041	-.010263
15	.116327	-.005053	-.009873
16	.116283	-.007737	-.010737
17	.162530	.000500	-.009403
18	.162457	-.007508	-.008876
19	.162617	-.006626	-.008726
20	.162744	-.009787	-.009603
21	.195165	.000308	-.004853
22	.196062	-.008723	-.004640
23	.197832	-.008136	-.004398
24	.198538	-.011590	-.005009
25	.205481	.000116	-.001841
26	.205896	-.009540	-.001708
27	.207983	-.009304	-.001226
28	.209529	-.012949	-.001383
29	.213040	-.000181	-.001964
30	.213310	-.010269	-.001851
31	.215144	-.010433	-.001317
32	.216205	-.014067	-.001395
33	.220227	-.000499	-.001816
34	.220293	-.010859	-.001706
35	.221826	-.011396	-.001171
36	.222743	-.014947	-.001311
37	.226760	-.000819	-.001669
38	.226627	-.011324	-.001575
39	.227900	-.012203	-.001040
40	.228679	-.015607	-.001150
41	.232589	-.001118	-.001502
42	.232256	-.011674	-.001420
43	.233253	-.012849	-.000876
44	.233869	-.016070	-.000971
45	.237619	-.001374	-.001312
46	.237072	-.011919	-.001241
47	.237777	-.013328	-.000698
48	.238208	-.016361	-.000760
49	.241735	-.001561	-.001090
50	.240990	-.012071	-.001062
51	.241360	-.013632	-.000461

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
62	.241500	-.016509	-.000483
63	.245136	-.001650	-.001117
64	.244269	-.012135	-.001058
65	.243623	-.013741	-.000172
66	.243536	-.016555	-.000263

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.023414	.000368	-.008593
6	.023518	-.002324	-.007950
7	.023588	-.001686	-.007528
8	.023486	-.002802	-.008273
9	.067770	.000630	-.010957
10	.067729	-.004390	-.010167
11	.067668	-.003298	-.009785
12	.067623	-.005377	-.010643
13	.117319	.000746	-.011385
14	.117200	-.006178	-.010583
15	.117084	-.004858	-.010180
16	.117045	-.007681	-.011056
17	.166547	.000712	-.010912
18	.166343	-.007688	-.010135
19	.166146	-.006370	-.009729
20	.166099	-.009706	-.010578
21	.211107	.000540	-.009067
22	.210989	-.008930	-.008557
23	.211102	-.007827	-.008390
24	.211213	-.011463	-.009232
25	.242314	.000270	-.004678
26	.243076	-.009927	-.004457
27	.244768	-.009214	-.004195
28	.245408	-.012981	-.004774
29	.252317	.000006	-.001803
30	.252533	-.010556	-.001663
31	.254480	-.010264	-.001160
32	.255889	-.014080	-.001308
33	.259497	-.000335	-.001868
34	.259557	-.011113	-.001763
35	.261210	-.011258	-.001212
36	.262137	-.014957	-.001283
37	.266158	-.000672	-.001693
38	.266008	-.011549	-.001590
39	.267331	-.012080	-.001042
40	.268099	-.015614	-.001168
41	.272048	-.000986	-.001518
42	.271690	-.011878	-.001432
43	.272724	-.012736	-.000886
44	.273339	-.016076	-.000982
45	.277121	-.001253	-.001325
46	.276550	-.012108	-.001252
47	.277293	-.013223	-.000706
48	.277720	-.016365	-.000771
49	.281277	-.001446	-.001103
50	.280511	-.012251	-.001074
51	.280914	-.013531	-.000468

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
52	.281049	-.016512	-.000491
53	.284726	-.001537	-.001135
54	.283838	-.012312	-.001070
55	.283200	-.013641	-.000174
56	.283114	-.016558	-.000273

lisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.023312	.000422	-.008562
6	.023416	-.002369	-.007921
7	.023486	-.001624	-.007498
8	.023384	-.002781	-.008241
9	.067531	.000740	-.010939
10	.067491	-.004480	-.010151
11	.067430	-.003172	-.009767
12	.067384	-.005337	-.010624
13	.117100	.000912	-.011434
14	.116988	-.006315	-.010632
15	.116885	-.004669	-.010235
16	.116849	-.007622	-.011118
17	.166947	.000933	-.011218
18	.166780	-.007872	-.010430
19	.166626	-.006118	-.010012
20	.166584	-.009626	-.010872
21	.214614	.000812	-.010473
22	.214369	-.009158	-.009730
23	.214134	-.007514	-.009310
24	.214079	-.011358	-.010126
25	.256877	.000566	-.008565
26	.256714	-.010186	-.008082
27	.256777	-.008849	-.007897
28	.256868	-.012832	-.008690
29	.286124	.000236	-.004430
30	.286736	-.010978	-.004204
31	.288327	-.010105	-.003919
32	.288893	-.014080	-.004459
33	.295655	-.000080	-.001740
34	.295660	-.011435	-.001594
35	.297438	-.011030	-.001070
36	.298689	-.014938	-.001207
37	.302341	-.000442	-.001746
38	.302175	-.011835	-.001648
39	.303624	-.011883	-.001085
40	.304401	-.015592	-.001147
41	.308367	-.000774	-.001545
42	.307984	-.012133	-.001451
43	.309080	-.012554	-.000891
44	.309685	-.016051	-.001003
45	.313514	-.001054	-.001345
46	.312911	-.012343	-.001268
47	.313704	-.013050	-.000719
48	.314130	-.016339	-.000785
49	.317729	-.001256	-.001121
50	.316932	-.012474	-.001090
51	.317382	-.013364	-.000476

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
52	.317508	-.016485	-.000503
53	.321245	-.001352	-.001161
54	.320327	-.012530	-.001087
55	.319700	-.013476	-.000176
56	.319614	-.016530	-.000285

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.023198	.000476	-.008524
6	.023302	-.002413	-.007885
7	.023373	-.001564	-.007462
8	.023271	-.002758	-.008202
9	.067224	.000849	-.010900
10	.067184	-.004571	-.010113
11	.067123	-.003052	-.009727
12	.067076	-.005292	-.010582
13	.116632	.001077	-.011415
14	.116521	-.006452	-.010613
15	.116417	-.004488	-.010213
16	.116380	-.007555	-.011097
17	.166479	.001156	-.011261
18	.166320	-.008057	-.010472
19	.166177	-.005874	-.010058
20	.166138	-.009540	-.010926
21	.214694	.001093	-.010750
22	.214482	-.009392	-.009996
23	.214287	-.007208	-.009563
24	.214238	-.011250	-.010389
25	.259775	.000901	-.009846
26	.259491	-.010467	-.009151
27	.259218	-.008482	-.008717
28	.259153	-.012699	-.009488
29	.299039	.000601	-.007929
30	.298830	-.011293	-.007480
31	.298839	-.009685	-.007275
32	.298910	-.013904	-.008009
33	.325867	.000232	-.004116
34	.326313	-.011896	-.003889
35	.327785	-.010803	-.003578
36	.328269	-.014897	-.004071
37	.334780	-.000113	-.001653
38	.334559	-.012198	-.001502
39	.336146	-.011595	-.000959
40	.337219	-.015535	-.001082
41	.340866	-.000470	-.001600
42	.340455	-.012460	-.001511
43	.341685	-.012295	-.000935
44	.342298	-.015991	-.000990
45	.346157	-.000768	-.001377
46	.345520	-.012642	-.001290
47	.346388	-.012804	-.000728
48	.346804	-.016277	-.000809
49	.350461	-.000982	-.001147
50	.349625	-.012757	-.001112
51	.350135	-.013126	-.000488

lisa Statis Portal (13 lantai) Kg/m

I N T E R D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
52	.350252	-.016421	-.000518
53	.354068	-.001084	-.001195
54	.353109	-.012807	-.001110
55	.352494	-.013240	-.000179
56	.352410	-.016466	-.000301

sa Statis Portal (13 lantai) Kg/m

INT DISPLACEMENTS

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
1	.000000	-.000000	-.000000
2	.000000	-.000000	-.000000
3	.000000	-.000000	-.000000
4	.000000	-.000000	-.000000
5	.023080	-.000530	-.008484
6	.023184	-.002459	-.007848
7	.023256	-.001506	-.007424
8	.023155	-.002732	-.008161
9	.066896	-.000958	-.010853
10	.066856	-.004663	-.010068
11	.066796	-.002935	-.009681
12	.066750	-.005241	-.010534
13	-.116089	.001244	-.011374
14	-.115979	-.006592	-.010573
15	-.115875	-.004312	-.010171
16	-.115837	-.007481	-.011053
17	-.165770	.001381	-.011241
18	-.165610	-.008247	-.010452
19	-.165467	-.005638	-.010034
20	-.165427	-.009443	-.010903
21	-.213964	.001377	-.010785
22	-.213758	-.009633	-.010030
23	-.213575	-.006910	-.009599
24	-.213527	-.011131	-.010434
25	-.259507	.001246	-.010089
26	-.259252	-.010760	-.009382
27	-.259017	-.008122	-.008935
28	-.258957	-.012558	-.009715
29	-.301250	.001003	-.009065
30	-.300926	-.011638	-.008429
31	-.300616	-.009265	-.007980
32	-.300542	-.013737	-.008696
33	-.336890	.000672	-.007170
34	-.336636	-.012281	-.006762
35	-.336589	-.010327	-.006535
36	-.336636	-.014690	-.007200
37	-.360866	.000291	-.003743
38	-.361131	-.012711	-.003513
39	-.362470	-.011293	-.003176
40	-.362863	-.015447	-.003616
41	-.369031	-.000057	-.001548
42	-.368563	-.012882	-.001390
43	-.369945	-.011942	-.000827
44	-.370820	-.015888	-.000939
45	-.374419	-.000379	-.001433
16	-.373739	-.013030	-.001351
7	-.374750	-.012477	-.000772
8	-.375172	-.016171	-.000803
9	-.378877	-.000608	-.001185
0	-.377995	-.013123	-.001141
1	-.378590	-.012808	-.000497

atis Portal (13 lantai) Kg/m

D I S P L A C E M E N T S

NATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

U(X)	U(Y)	R(Z)
.378689	-.016313	-.000541
.382612	-.000717	-.001242
.381597	-.013166	-.001140
.380995	-.012924	-.000182
.380914	-.016358	-.000319

isa Statis Portal (13 lantai) Kg/m

E N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

T	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.022961	.000586	-.008443
6	.023065	-.002509	-.007810
7	.023137	-.001450	-.007385
8	.023037	-.002703	-.008120
9	.066560	.001071	-.010804
0	.066521	-.004763	-.010022
1	.066462	-.002821	-.009633
.2	.066416	-.005184	-.010483
.3	.115522	.001414	-.011327
.4	.115412	-.006743	-.010528
.5	.115309	-.004139	-.010123
.6	.115271	-.007396	-.011003
.7	.164986	.001611	-.011202
.8	.164827	-.008451	-.010413
.9	.164684	-.005406	-.009992
20	.164643	-.009332	-.010860
21	.213009	.001670	-.010765
22	.212804	-.009892	-.010008
23	.212620	-.006618	-.009573
24	.212572	-.010997	-.010408
25	.258506	.001602	-.010116
26	.258257	-.011076	-.009406
27	.258032	-.007768	-.008959
28	.257975	-.012399	-.009747
29	.300610	.001425	-.009269
30	.300311	-.012013	-.008619
31	.300036	-.008848	-.008158
32	.299966	-.013555	-.008880
33	.338343	.001157	-.008141
34	.337981	-.012715	-.007573
35	.337633	-.009849	-.007109
36	.337548	-.014480	-.007759
37	.369763	.000822	-.006291
38	.369463	-.013196	-.005931
39	.369359	-.010757	-.005681
40	.369381	-.015199	-.006267
41	.390461	.000458	-.003312
42	.390528	-.013478	-.003081
43	.391733	-.011558	-.002716
44	.392028	-.015742	-.003099
15	.397763	.000140	-.001427
6	.397019	-.013546	-.001259
7	.398204	-.012050	-.000685
8	.398851	-.016012	-.000766
9	.402363	-.000110	-.001244
0	.401419	-.013613	-.001208
1	.402162	-.012401	-.000536

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
52	.402255	-.016152	-.000537
53	.406294	-.000229	-.001309
54	.405213	-.013646	-.001181
55	.404629	-.012520	-.000176
56	.404545	-.016196	-.000348

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NO	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.022842	.000645	-.008403
6	.022946	-.002565	-.007773
7	.023019	-.001393	-.007347
8	.022919	-.002670	-.008079
9	.066225	.001190	-.010756
10	.066186	-.004877	-.009976
11	.066129	-.002707	-.009584
12	.066082	-.005119	-.010432
13	.114952	.001595	-.011280
14	.114843	-.006917	-.010482
15	.114741	-.003966	-.010073
16	.114703	-.007300	-.010952
17	.164187	.001856	-.011159
18	.164029	-.008685	-.010371
19	.163887	-.005173	-.009944
20	.163846	-.009207	-.010812
21	.212000	.001981	-.010729
22	.211795	-.010189	-.009972
23	.211611	-.006324	-.009530
24	.211563	-.010843	-.010366
25	.257325	.001981	-.010095
26	.257075	-.011439	-.009384
27	.256850	-.007412	-.008929
28	.256792	-.012219	-.009720
29	.299355	.001875	-.009286
30	.299061	-.012444	-.008634
31	.298794	-.008429	-.008169
32	.298727	-.013348	-.008899
33	.337336	.001680	-.008302
34	.336993	-.013217	-.007719
35	.336678	-.009365	-.007242
36	.336597	-.014248	-.007896
37	.370417	.001417	-.007078
38	.370017	-.013770	-.006589
39	.369631	-.010209	-.006108
40	.369536	-.014939	-.006684
41	.397031	.001113	-.005294
42	.396686	-.014118	-.004988
43	.396526	-.010947	-.004715
44	.396524	-.015445	-.005216
45	.414028	.000804	-.002830
46	.413877	-.014281	-.002592
47	.414971	-.011565	-.002209
48	.415147	-.015796	-.002506
49	.420370	.000553	-.001297
50	.419354	-.014286	-.001155
51	.420293	-.011881	-.000464

lisa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

) COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

NT	U(X)	U(Y)	R(Z)
52	.420555	-.015926	-.000513
53	.424546	.000421	-.001399
54	.423371	-.014306	-.001271
55	.422839	-.012010	-.000207
56	.422759	-.015969	-.000339

isa Statis Portal (13 lantai) Kg/m

I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.022725	.000712	-.008363
6	.022828	-.002638	-.007736
7	.022902	-.001329	-.007309
8	.022803	-.002635	-.008038
9	.065896	.001325	-.010709
10	.065857	-.005024	-.009931
11	.065800	-.002580	-.009536
12	.065754	-.005049	-.010382
13	.114394	.001800	-.011235
14	.114285	-.007139	-.010438
15	.114184	-.003774	-.010025
16	.114146	-.007198	-.010902
17	.163406	.002133	-.011118
18	.163248	-.008985	-.010329
19	.163106	-.004915	-.009897
20	.163065	-.009073	-.010765
21	.211008	.002331	-.010694
22	.210803	-.010569	-.009935
23	.210620	-.005999	-.009485
24	.210571	-.010678	-.010323
25	.256140	.002409	-.010067
26	.255890	-.011901	-.009353
27	.255665	-.007018	-.008889
28	.255607	-.012025	-.009682
29	.298008	.002382	-.009270
30	.297714	-.012992	-.008613
31	.297447	-.007964	-.008138
32	.297379	-.013126	-.008873
33	.335897	.002270	-.008313
34	.335559	-.013855	-.007726
35	.335251	-.008828	-.007240
36	.335173	-.014000	-.007905
37	.369118	.002094	-.007197
38	.368731	-.014502	-.006689
39	.368375	-.009598	-.006193
40	.368283	-.014665	-.006772
41	.396930	.001877	-.005885
42	.396492	-.014947	-.005482
43	.396070	-.010261	-.004979
44	.395968	-.015145	-.005478
45	.418159	.001648	-.004187
46	.417768	-.015205	-.003934
47	.417567	-.010803	-.003649
48	.417535	-.015464	-.004030
49	.431032	.001440	-.002298
50	.430698	-.015297	-.002105
51	.431632	-.011210	-.001581

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I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
52	.431516	-.015653	-.001785
53	.436844	.001305	-.001589
54	.435549	-.015283	-.001210
55	.435120	-.011304	-.000088
56	.435210	-.015694	-.000344

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I N T E R D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

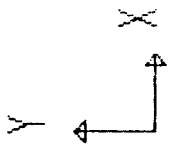
T	U(X)	U(Y)	R(Z)
1	.000000	.000000	.000000
2	.000000	.000000	.000000
3	.000000	.000000	.000000
4	.000000	.000000	.000000
5	.022593	.000624	-.008354
6	.022703	-.003133	-.007736
7	.022798	-.001606	-.007237
8	.022704	-.002790	-.007972
9	.065578	.001165	-.010699
10	.065544	-.005981	-.009926
11	.065494	-.003100	-.009462
12	.065450	-.005347	-.010312
13	.113892	.001582	-.011234
14	.113785	-.008528	-.010441
15	.113689	-.004504	-.009953
16	.113652	-.007627	-.010837
17	.162747	.001873	-.011130
18	.162590	-.010776	-.010342
19	.162454	-.005820	-.009830
20	.162414	-.009619	-.010706
21	.210226	.002047	-.010718
22	.210024	-.012734	-.009959
23	.209845	-.007046	-.009424
24	.209797	-.011330	-.010272
25	.255275	.002118	-.010105
26	.255027	-.014412	-.009388
27	.254806	-.008173	-.008835
28	.254748	-.012769	-.009640
29	.297102	.002103	-.009324
30	.296810	-.015822	-.008661
31	.296547	-.009194	-.008092
32	.296479	-.013950	-.008842
33	.335006	.002021	-.008385
34	.334670	-.016978	-.007790
35	.334365	-.010098	-.007207
36	.334287	-.014889	-.007889
37	.368325	.001894	-.007299
38	.367944	-.017893	-.006782
39	.367597	-.010874	-.006188
40	.367508	-.015606	-.006791
41	.396437	.001747	-.006077
42	.396012	-.018581	-.005649
43	.395622	-.011509	-.005047
44	.395521	-.016124	-.005561
45	.418800	.001605	-.004748
46	.418335	-.019058	-.004419
47	.417913	-.011990	-.003818
48	.417811	-.016466	-.004243
49	.435112	.001497	-.003411
50	.434663	-.019341	-.003227
51	.434259	-.012301	-.002628

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I N T D I S P L A C E M E N T S

COMBINATION 1 - DISPLACEMENTS "U" AND ROTATIONS "R"

IT	U(X)	U(Y)	R(Z)
32	.434116	-.016659	-.002887
33	.446483	.001449	-.002854
34	.445880	-.019444	-.002420
55	.444956	-.012429	-.001386
56	.444769	-.016730	-.001860

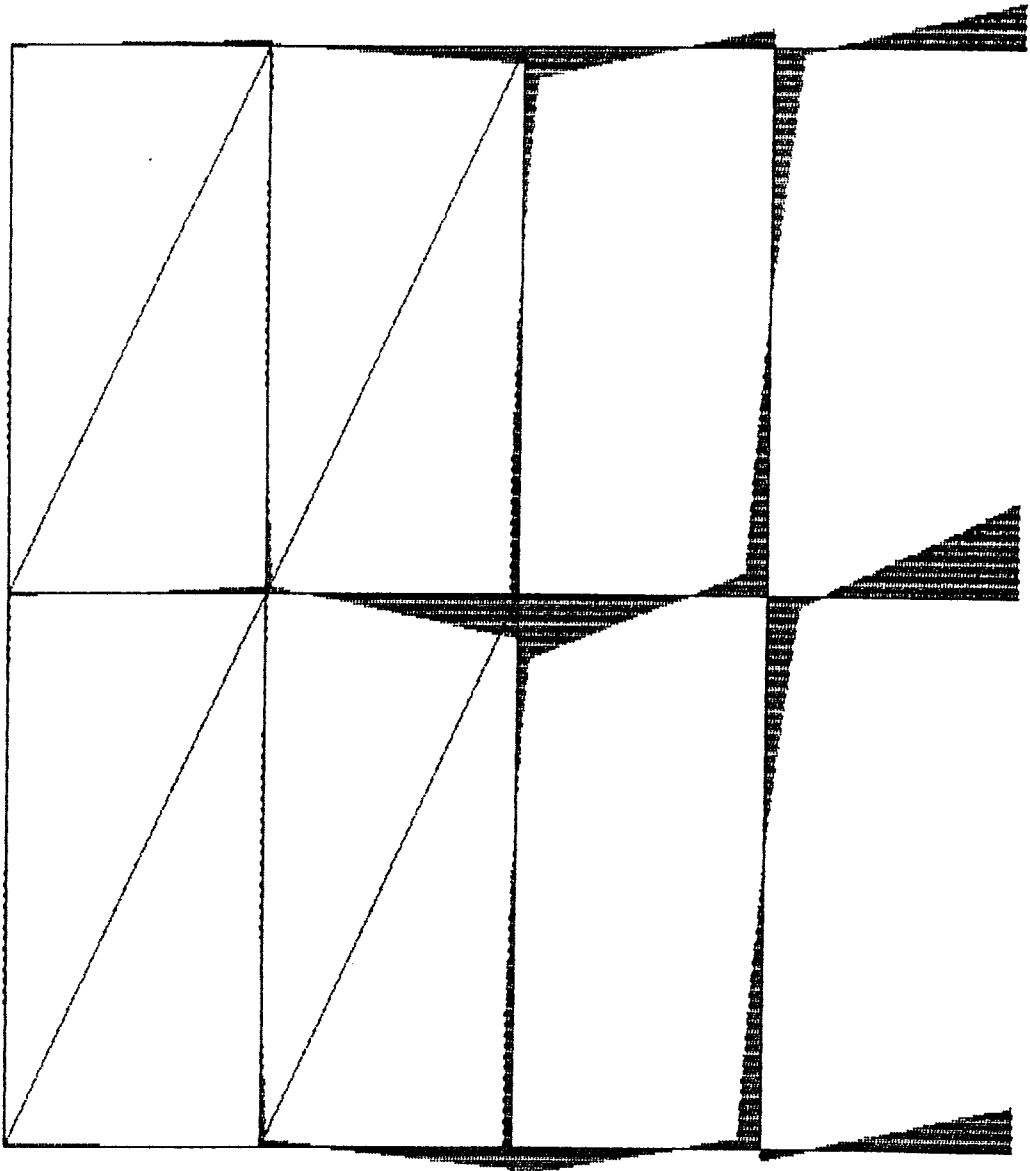


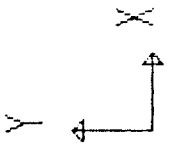
I-2
FRAME
OUTPUT M33
LOAD I

ENVELOPES

MIN < 2>
- .7789E+06
AT .00
MAX < 5>
.5682E+06
AT 4.80

SAP90





I-2

FRAME

OUTPUT V22

LOAD 1

ENVELOPES

MIN < 8 >

AT -.8842E+05

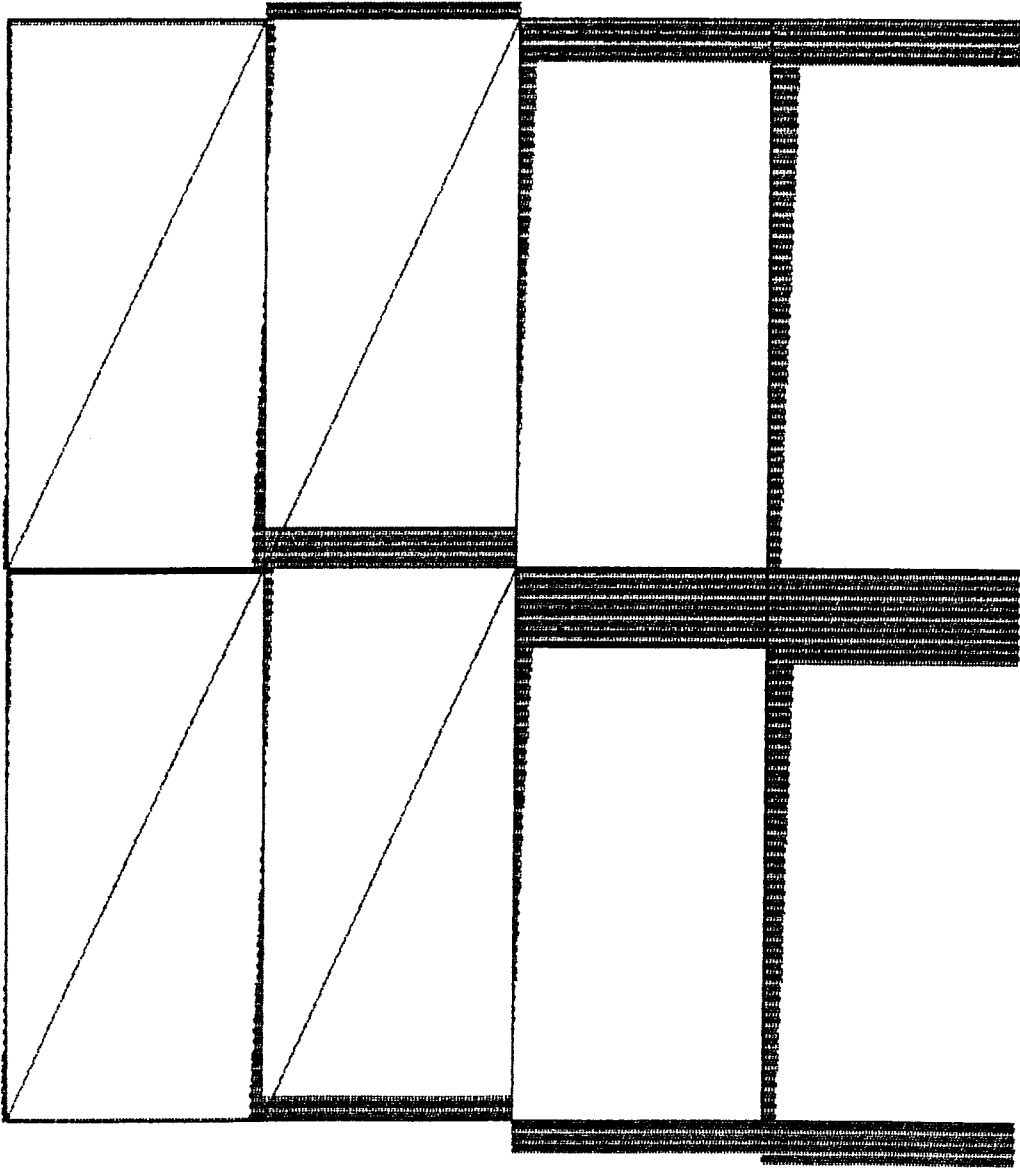
MAX < 2 >

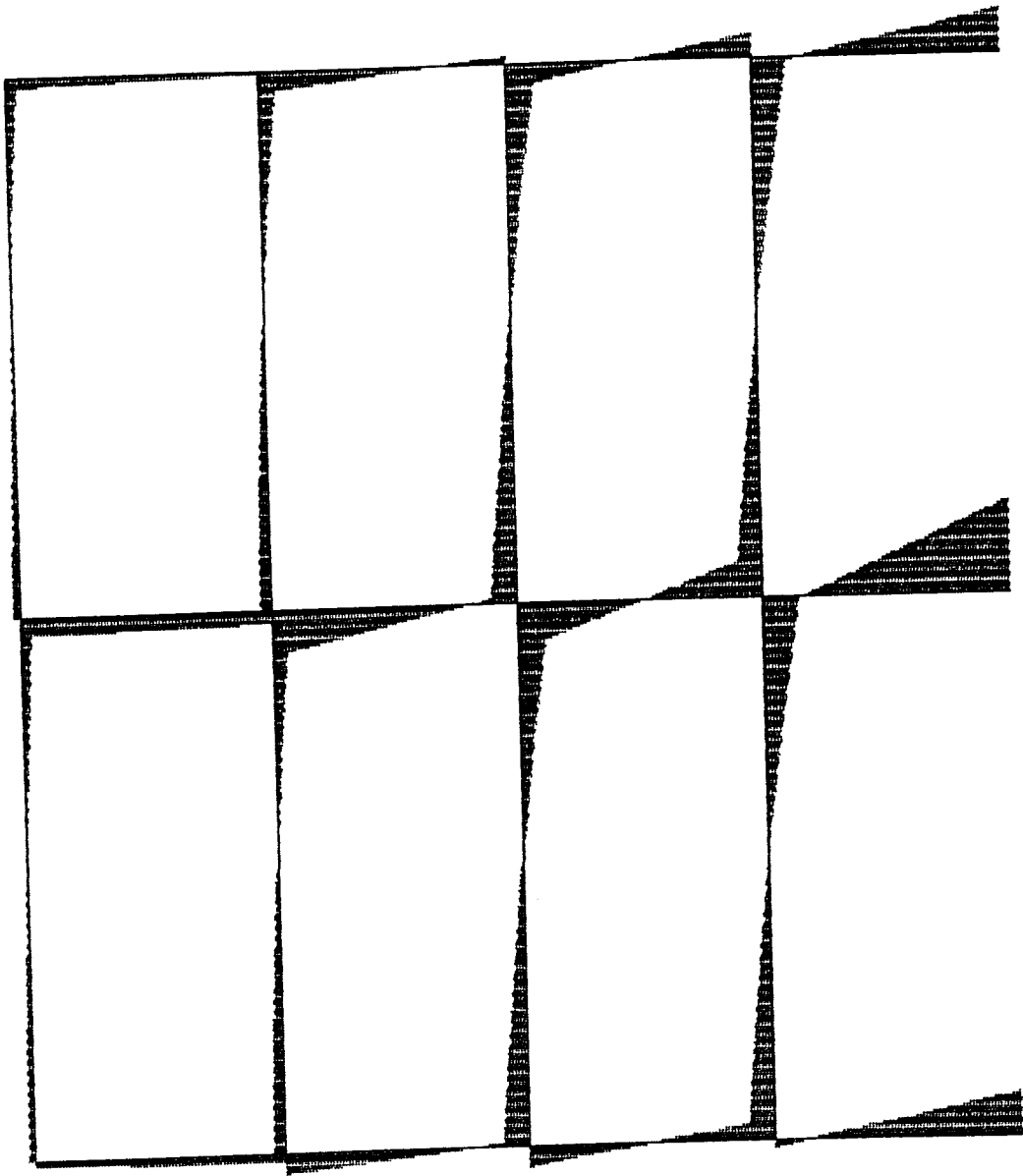
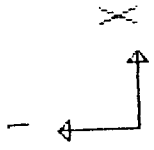
AT .2046E+06

AT .00

AT .00

SAP90





I-K

FRAME

OUTPUT M33

LOAD 1

ENVELOPES

MIN < 2 >

- .8209E+06

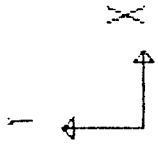
AT .00

MAX < 5 >

.4176E+06

AT 4.80

SAP90



I-K

FRAME

OUTPUT V22

LOAD I

ENVELOPES

MIN < 13 >

-.7018E+05

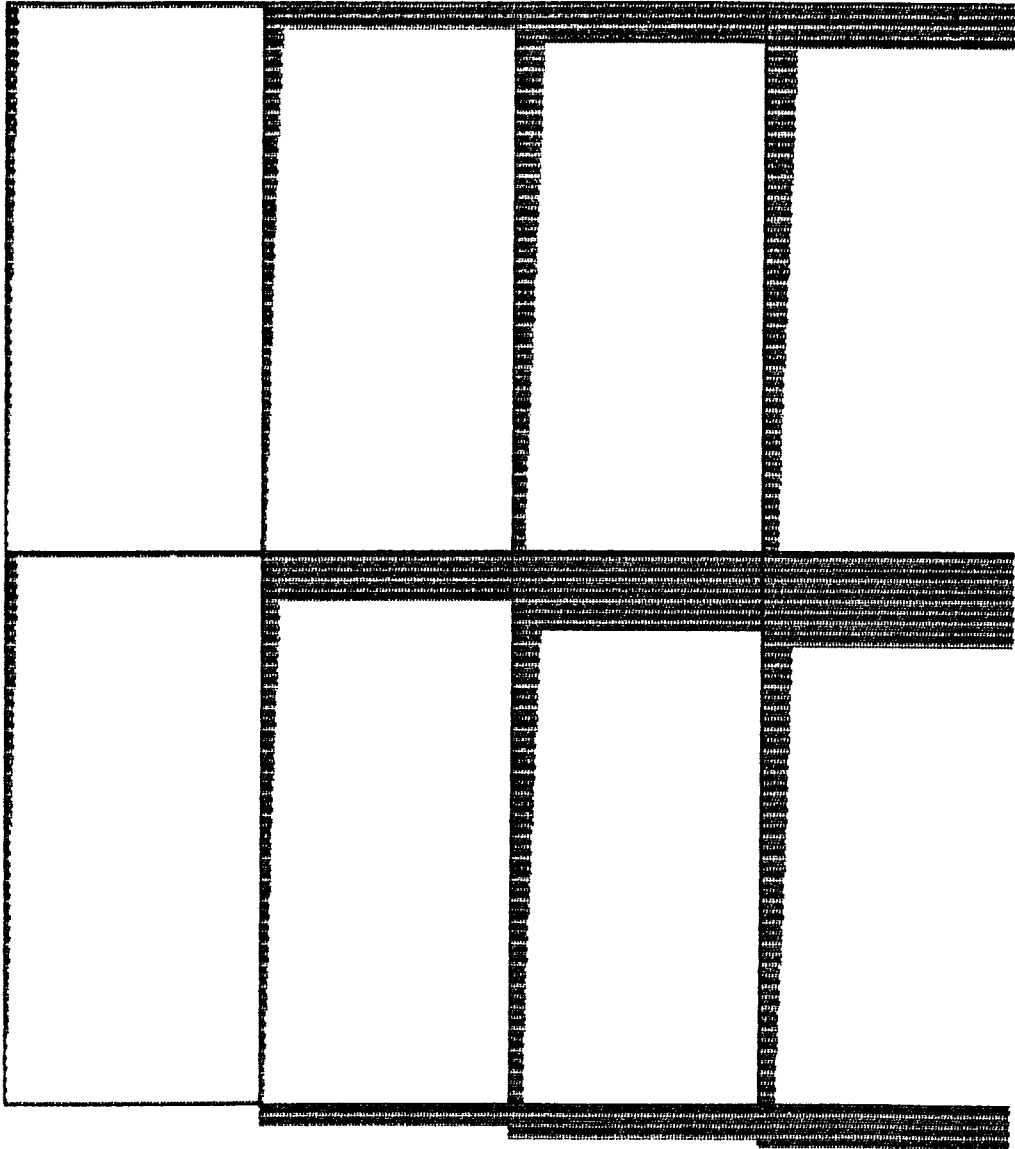
AT 10.60

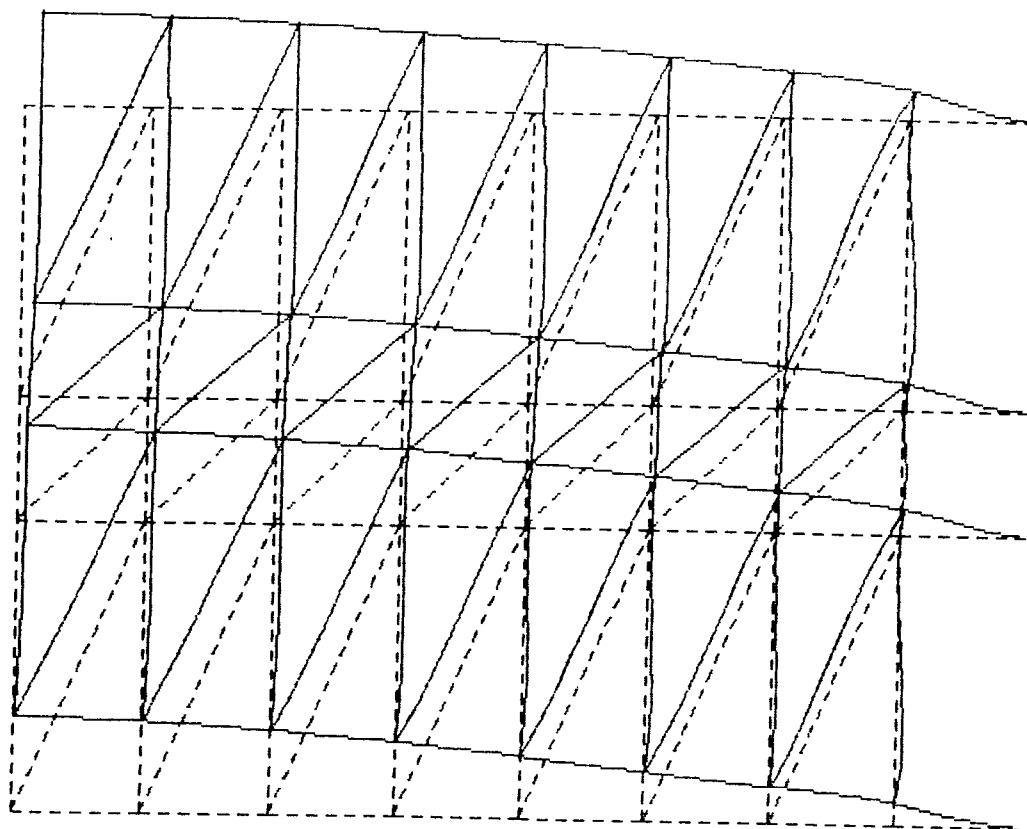
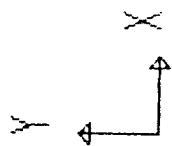
MAX < 2 >

.2019E+06

AT .00

SAP90

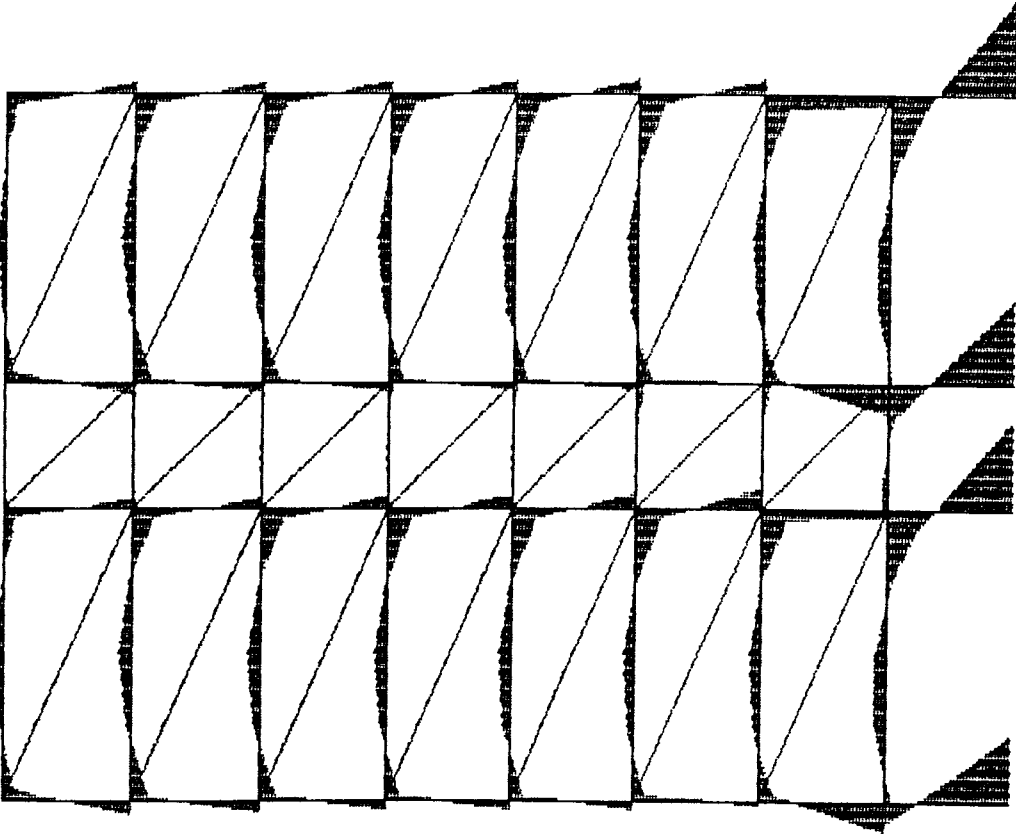
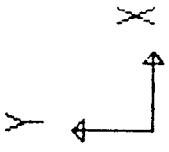




A-1
DEFORMED
SHAPE
LOAD 1

MINIMA
X .00000E+00
Y -.3292E-02
Z .00000E+00
MAXIMA
X .2010E-01
Y .00000E+00
Z .00000E+00

SAP90

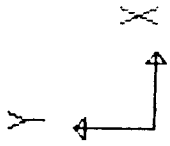


A-1
FRAME
OUTPUT M33
LOAD 1

ENVELOPES

MIN < 4>
-.5536E+05
AT .00
MAX < 4>
.3041E+05
AT 3.00

SAP90



A-1

FRAME

OUTPUT V22

LOAD 1

ENVELOPES

MIN < 35 >

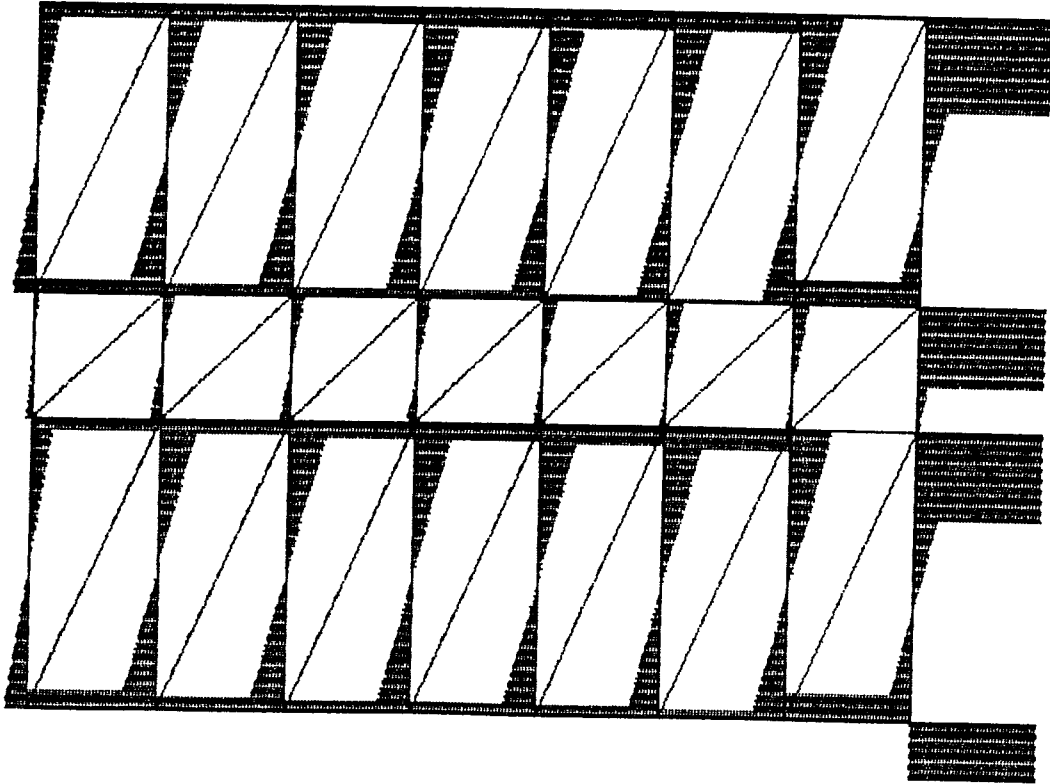
- .1428E+05

AT 7.00

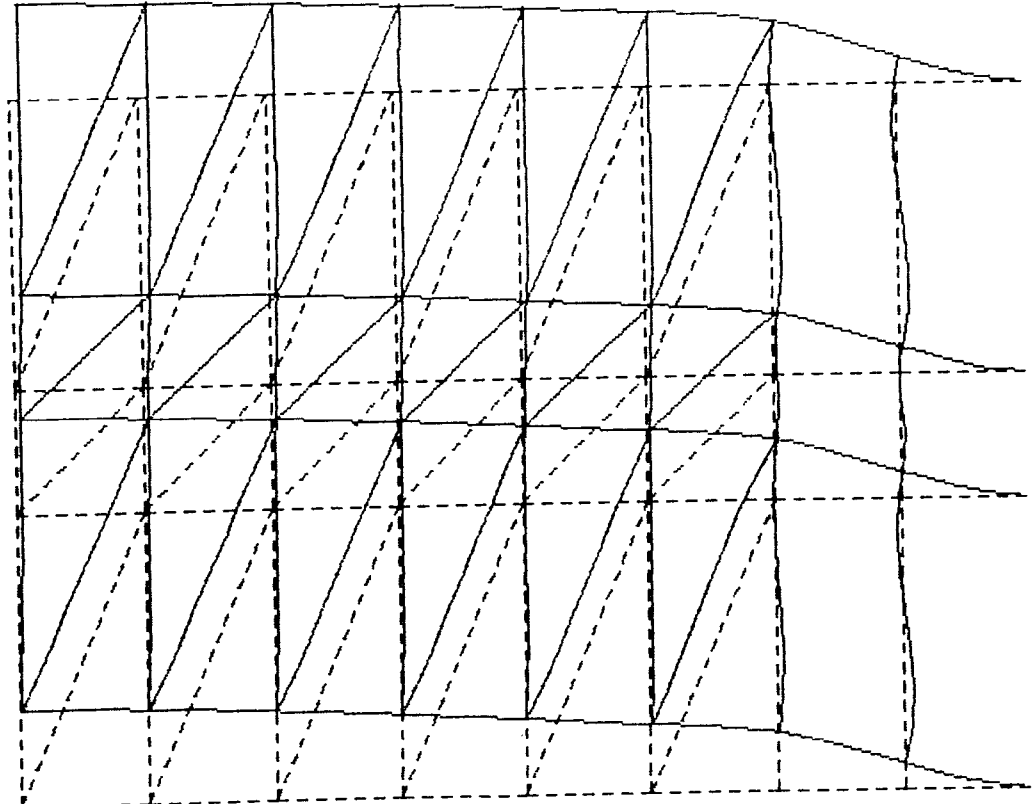
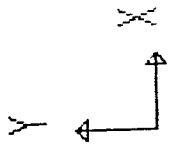
MAX < 4 >

.2859E+05

AT .00



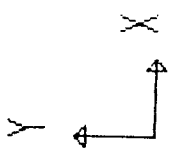
SAP90



A-2
DEFORMED
SHAPE
LOAD 1

MINIMA
X .00000E+00
Y -.3324E-02
Z .00000E+00
MAXIMA
X .3371E-01
Y .00000E+00
Z .00000E+00

SAP90

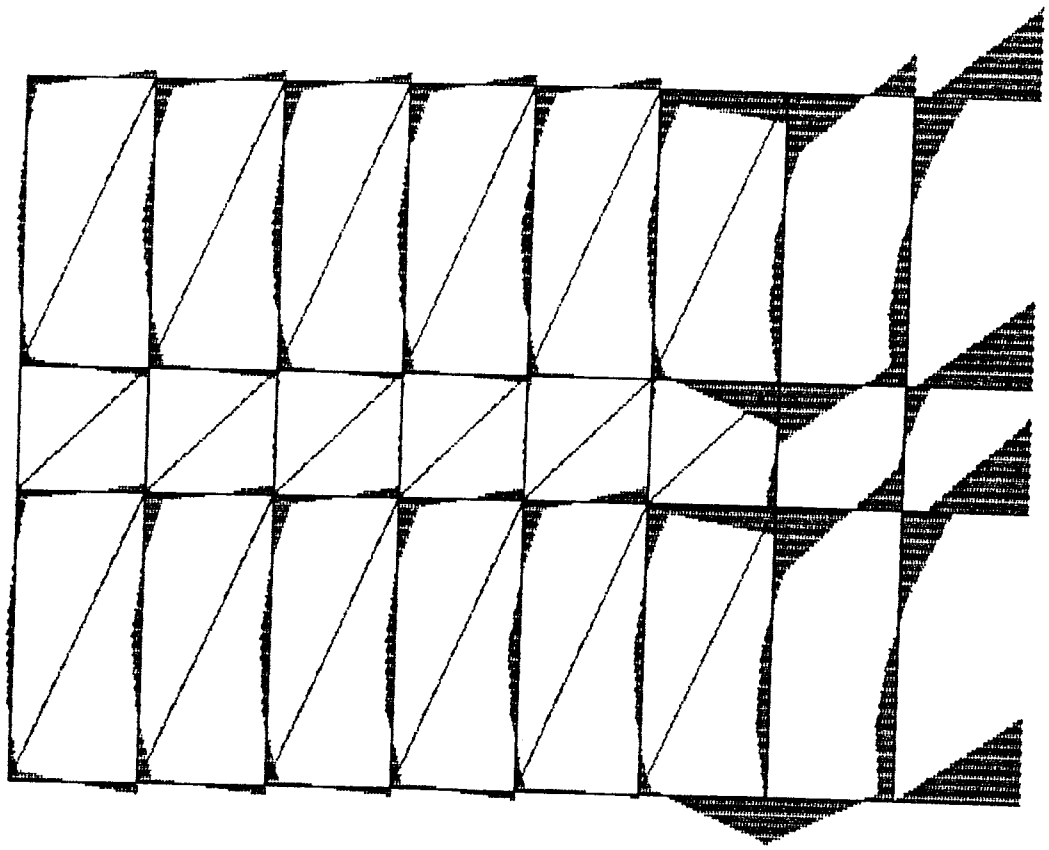


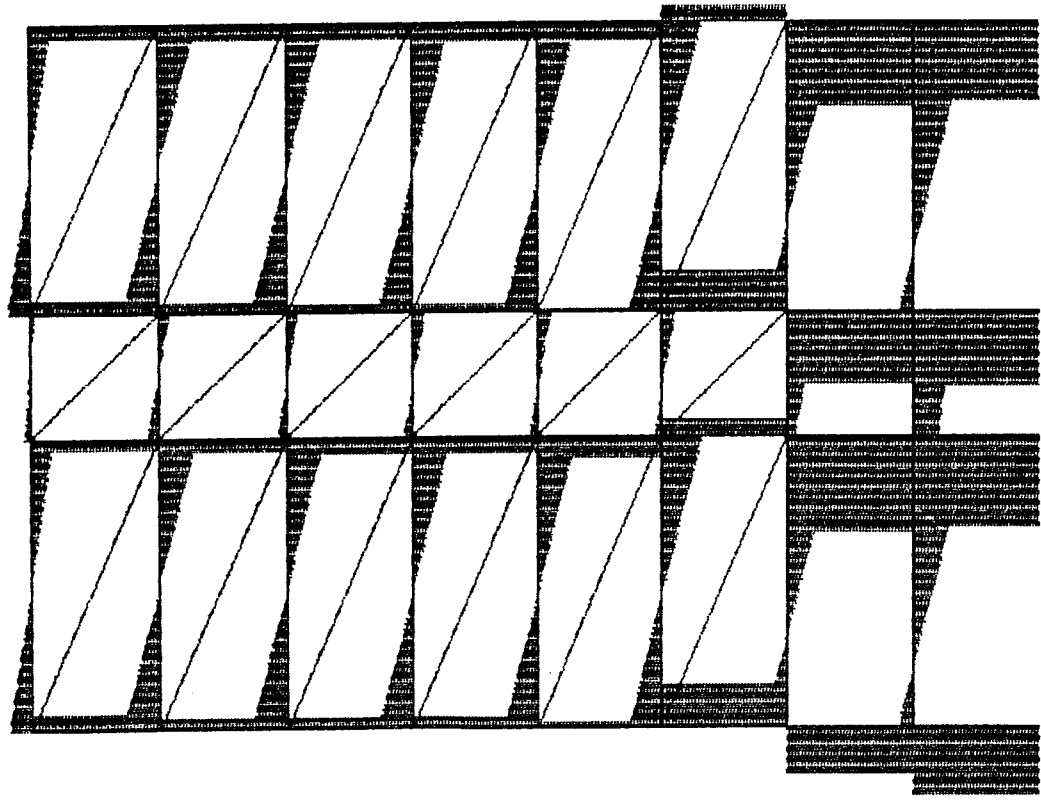
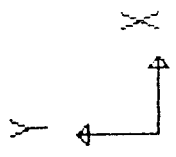
A-2
FRAME
OUTPUT M33
LOAD 1

ENVELOPES

MIN < 2>
-.6833E+05
AT .00
MAX < 6>
.5406E+05
AT 3.00

SAP90

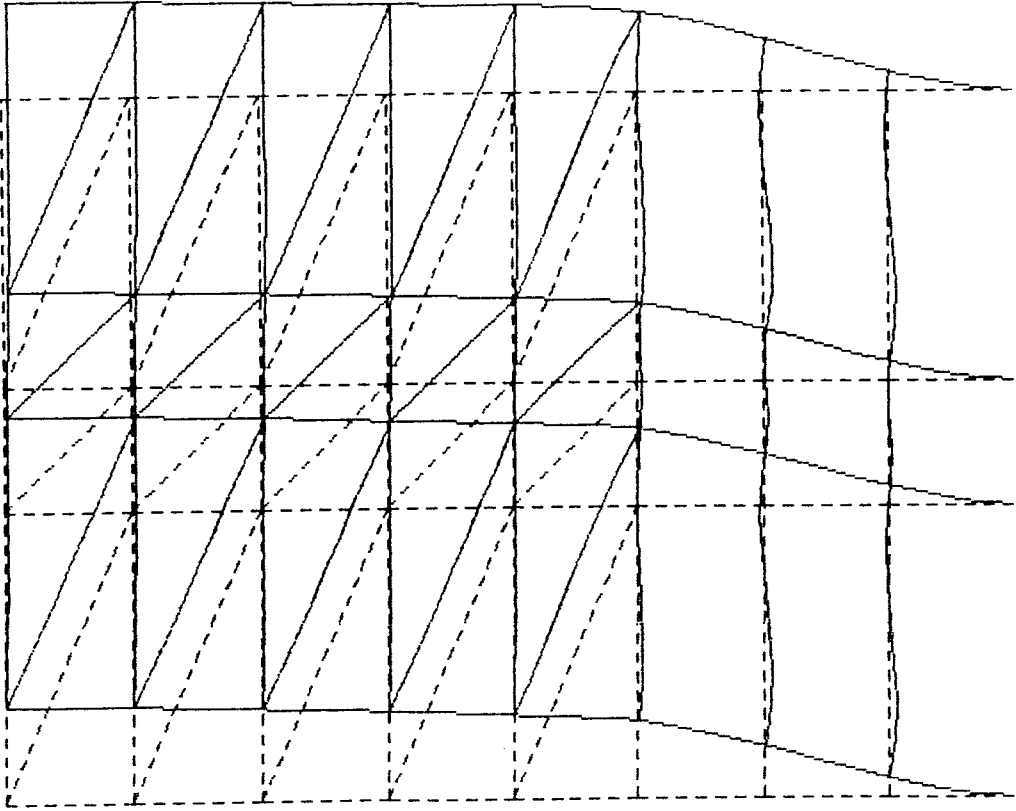
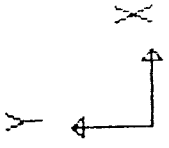




A-2
 FRAME
 OUTPUT V22
 LOAD I

ENVELOPES
 MIN < 35 >
 - .1819E+05
 AT 7.00
 MAX < 6 >
 .2938E+05
 AT .00

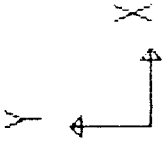
SAP90



A-3
DEFORMED
SHAPE
LOAD 1

MINIMA
X .0000E+00
Y -.3359E-02
Z .0000E+00
MAXIMA
X .5308E-01
Y .0000E+00
Z .0000E+00

SAP90



$\alpha=3$

FRAME

OUTPUT M33

LOAD 1

ENVELOPES

MIN < 2 >

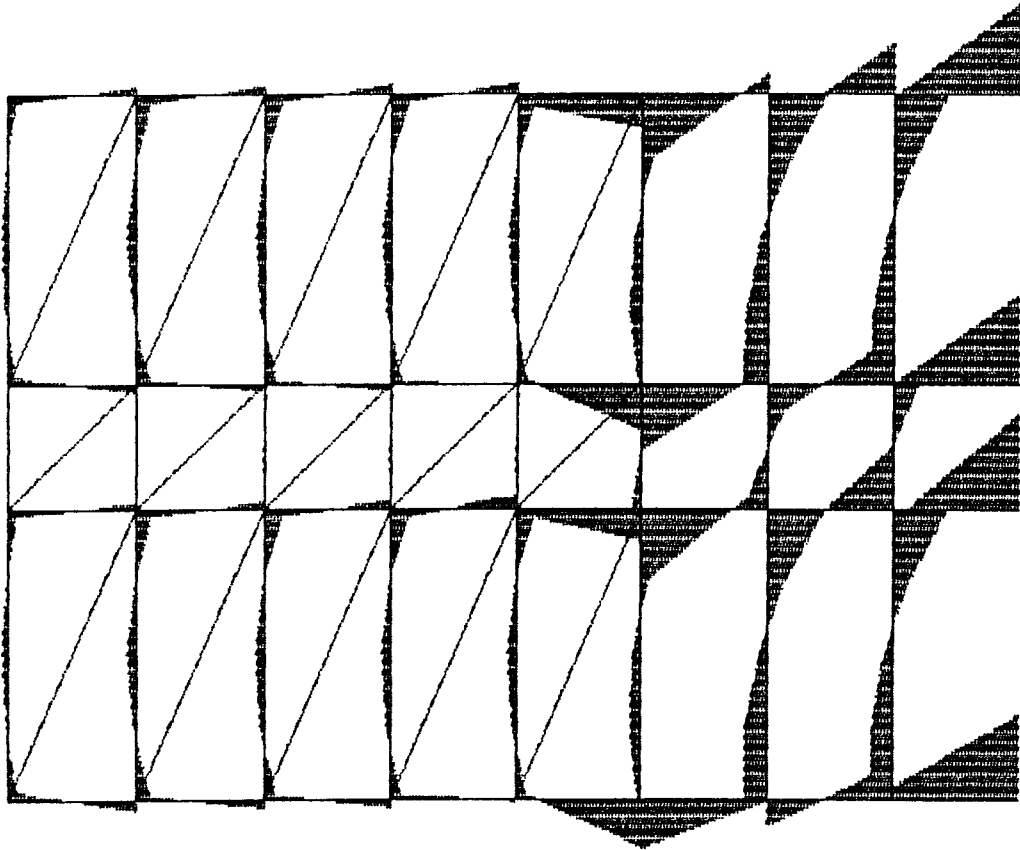
- .7527E+05

AT .00

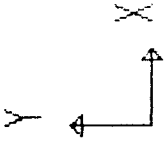
MAX < 10 >

.6103E+05

AT 3.00



SAP90



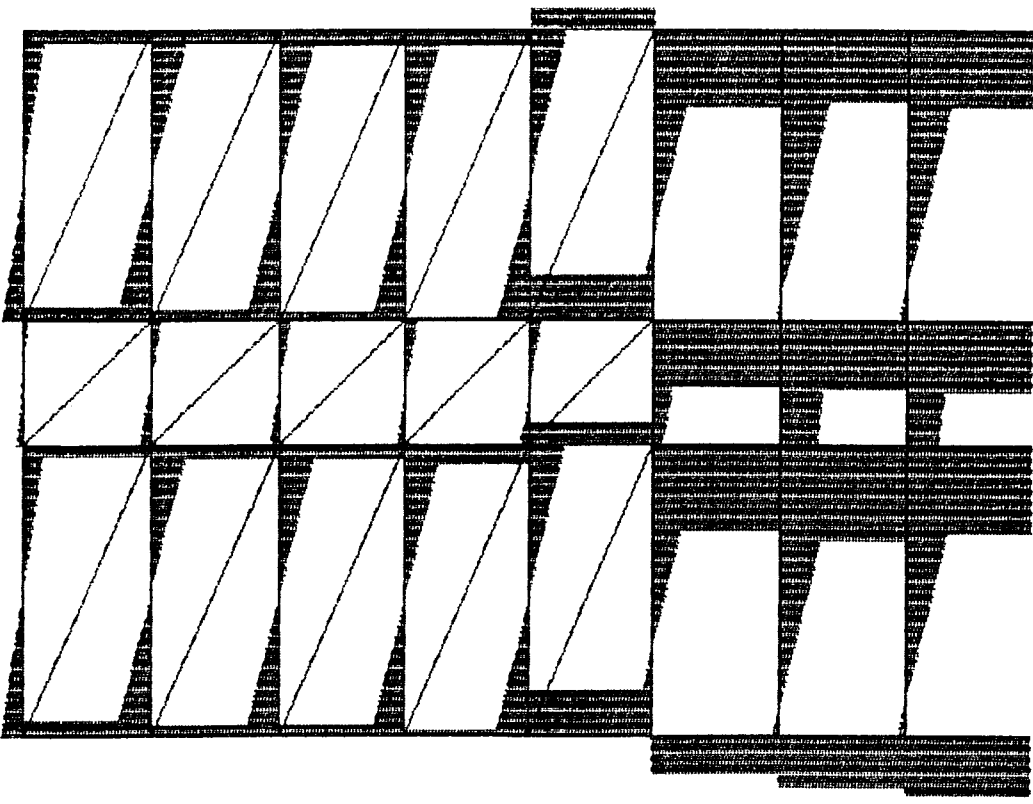
$\alpha=3$

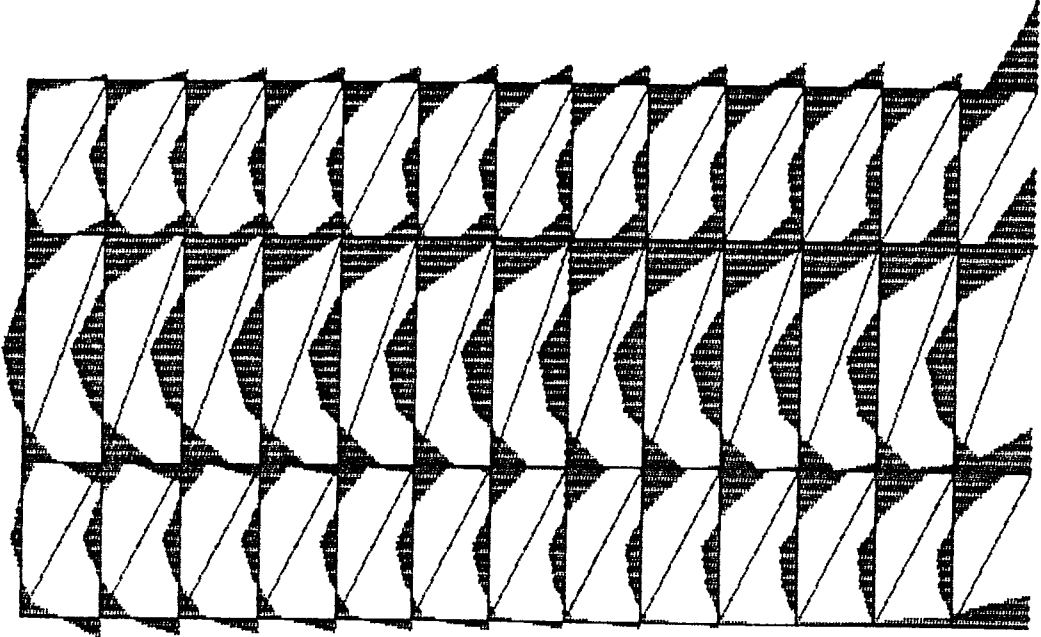
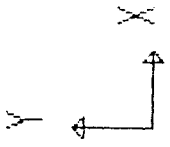
FRAME
OUTPUT V22
LOAD 1

ENVELOPES

MIN < 38>
- .2075E+05
AT 7.00
MAX < 6>
.3050E+05
AT .00

SAP90

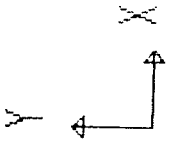




B-P
FRAME
OUTPUT M33
LOAD 1

ENVELOPES
MIN < 4>
-.9948E+05
AT .00
MAX < 15>
.4179E+05
AT 4.00

SAP90

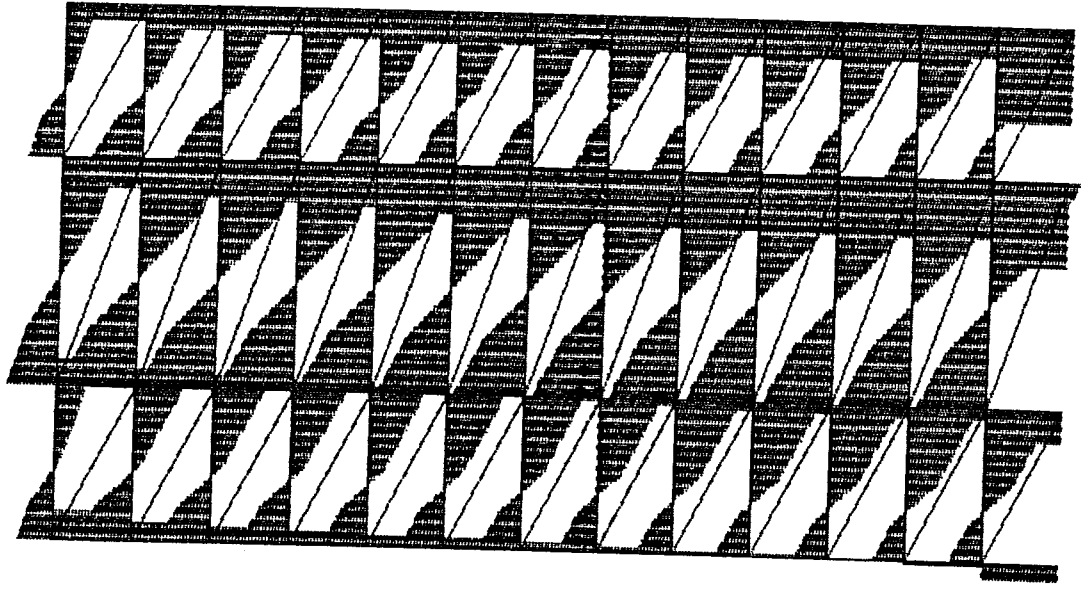


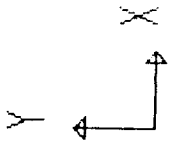
B-P
FRAME
OUTPUT V22
LOAD I

ENVELOPES

MIN < 63>
- .3321E+05
AT 12.00
MAX < 4>
.3495E+05
AT .00

SAP90





B-K

FRAME

OUTPUT M33

LOAD 1

ENVELOPES

MIN < 3>

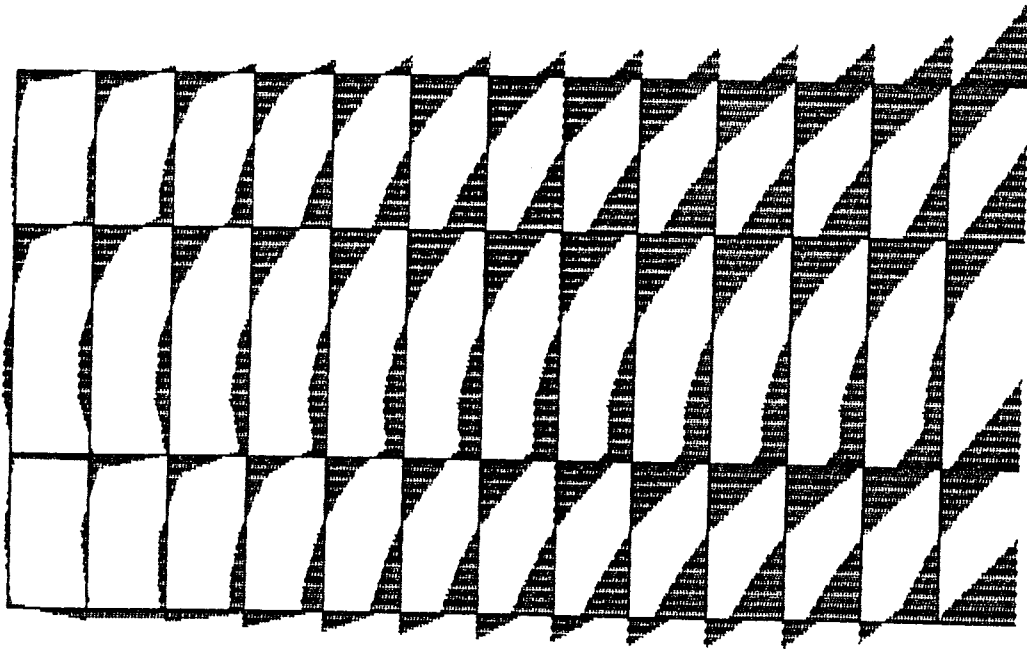
-.2956E+06

AT .00

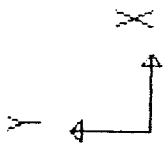
MAX < 15>

.1975E+06

AT 4.00



SAP90



B-K

FRAME

OUTPUT V22

LOAD I

ENVELOPES

MIN < 61 >

- .6961E+05

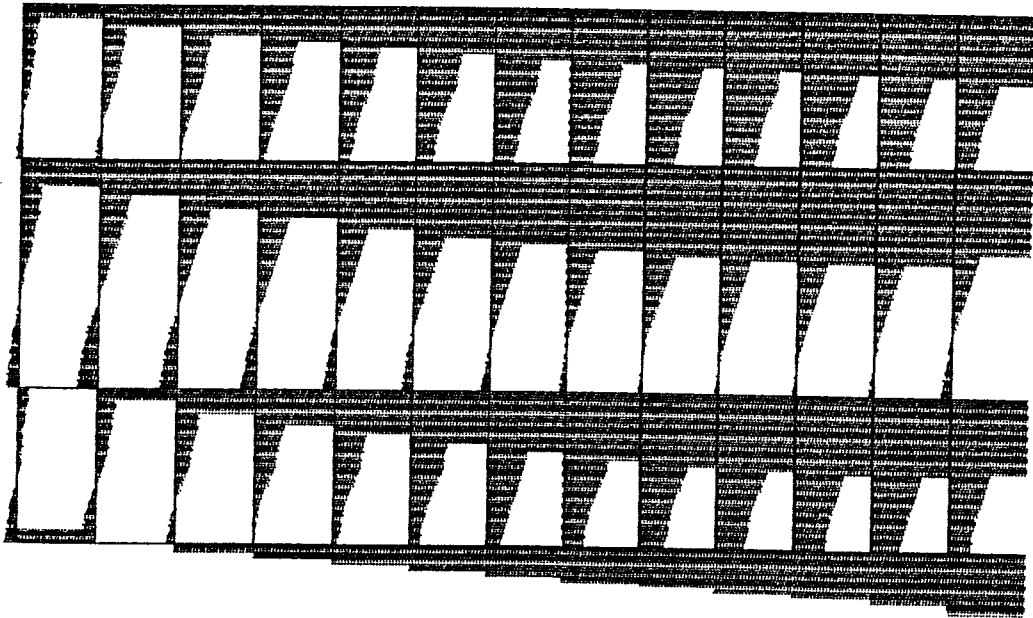
AT 8.00

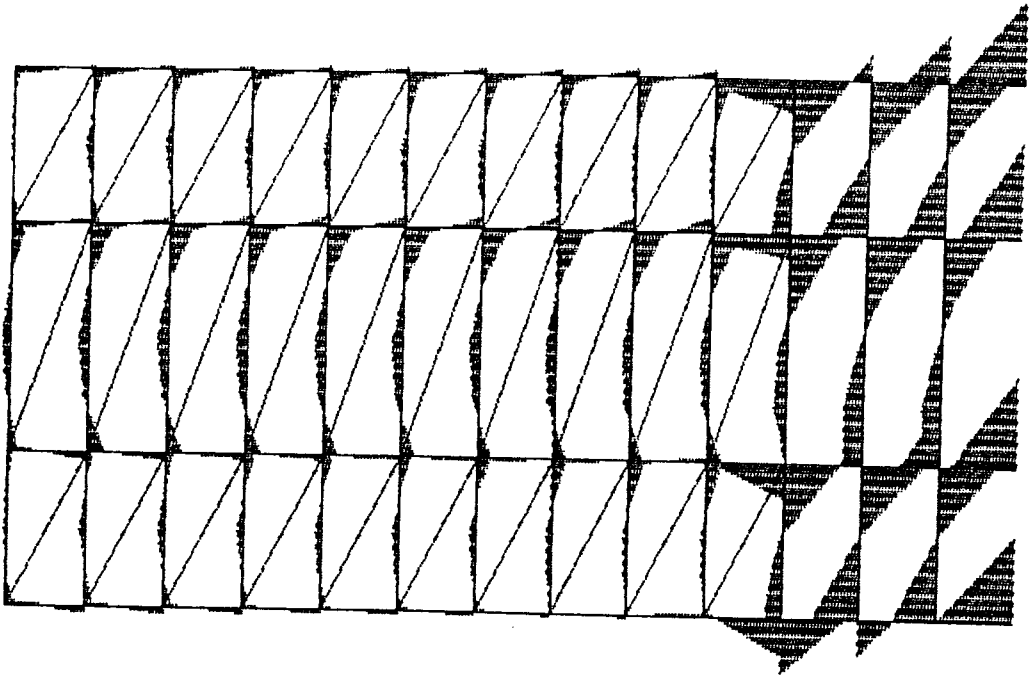
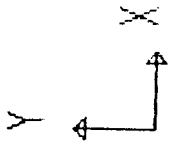
MAX < 11 >

.9950E+05

AT .00

SAP90





B-3

FRAME

OUTPUT M33

LOAD 1

ENVELOPES

MIN < 3 >

-.3001E+06

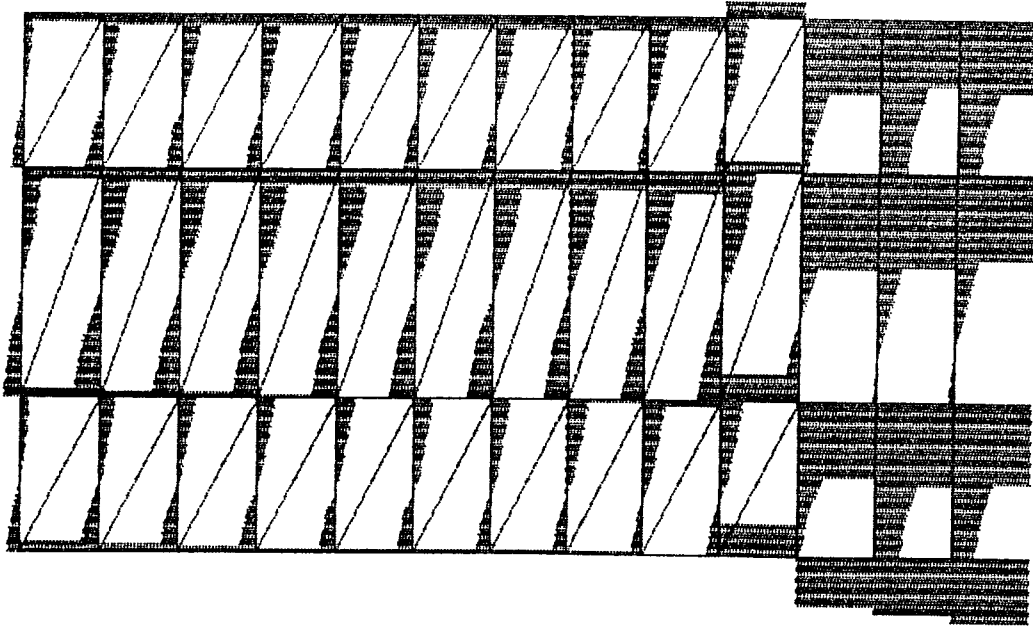
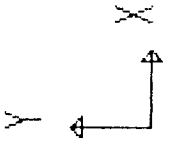
AT .00

MAX < 11 >

.2657E+06

AT 4.00

SAP90



B-3

FRAME

OUTPUT V22

LOAD 1

ENVELOPES

MIN < 58 >

- .5873E+05

AT 8.00

MAX < 11 >

.1014E+06

AT .00

SAP90

