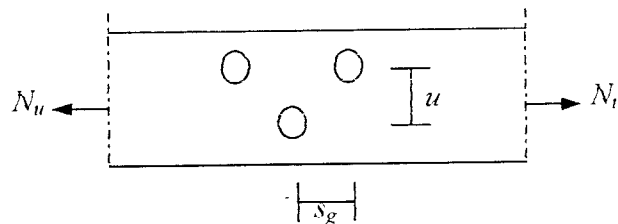


Tata letak baut harus memenuhi persyaratan:

- Jarak antar pusat baut tidak boleh kurang dari 3 kali diameter nominal baut
- Jarak antara pusat pengencang tidak boleh melebihi  $15t_p$  ( $t_p$  = tebal lapis tertapis didalam sambungan)
- Jarak tepi minimum:  $1,75.d_b$  (tepi dipotong dengan tangan),  $1,50d_b$  (tepi dipotong dengan mesin) dan  $1,25d_b$  (tepi profil bukan hasil potongan) dengan  $d_b$  = diameter nominal baut
- Jarak tepi maksimum:  $15t_p$  dan 150 mm

f. Efek lubang baut terhadap luas netto penampang profil

Efek lubang baut mempengaruhi luas bersih penampang profil, dijelaskan pada Gambar 3.2.



**Gambar 3.3** Efek lubang-lubang tak segaris terhadap luas bersih

$$A_{nl} = A_g - n.d.t + \sum s_g^2 . t / 4u \quad (3-24)$$

dengan:

$A_g$  = penampang brutto, mm<sup>2</sup>

$t$  = tebal penampang, mm

$d$  = diameter lubang, mm

$n$  = banyaknya lubang dalam garis potongan

$s_g$  = jarak antara sumbu lubang pada arah sejajar sumbu komponen struktur, mm

$u$  = jarak antara sumbu lubang pada arah tegak lurus sumbu komponen struktur

## 7. Kegagalan Robekan pada Lubang Baut

Bila material yang direkatkan oleh baut tersebut cukup tipis, keadaan batas kegagalan robekan, yang dikenal sebagai geser blok, dapat mempengaruhi kekuatan suatu batang tarik seperti sambungan pada ujung suatu batang. Persamaan berikut ini dapat digunakan untuk mewakili kekuatan nominal  $Tn$  (Salmon dan Johnson, 1992)

## 1. Pelelehan geser-retakan tarik

$$T_u = 0,6 f_y A_{vg} + f_u A_{nt} \quad (3-25)$$

## 2. Retakan geser-pelelehan tarik

$$T_u = 0,6 f_u A_{ns} + f_y A_{ig} \quad (3-26)$$

dengan:

$A_{vg}$  = luas bruto yang mengalami pelelehan geser

= (panjang b-c) x tebal (lihat gambar 3.3)

$A_{nt}$  = luas bersih yang mengalami retakan tarik

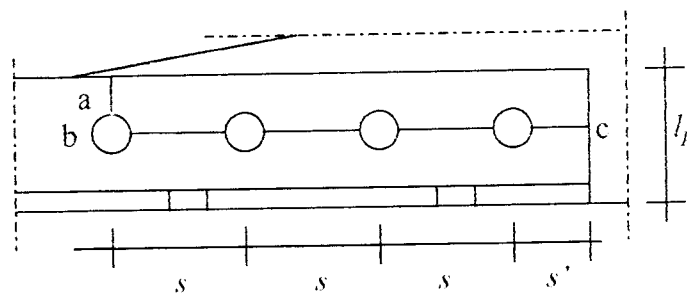
= (panjang a-b - luas lubang) x tebal (lihat Gambar 3.3)

$A_{ns}$  = luas bersih yang mengalami retakan geser

= (pancang b-c - luas lubang) x tebal (lihat Gambar 3.3)

$A_{ig}$  = luas bruto yang mengalami pelelehan tarik

= (panjang a-b) x tebal (lihat Gambar 3.3)



**Gambar 3.4** Daerah yang diarsir dapat terjadi kegagalan robekan

### 3.3 Dasar Perencanaan Struktur Beton Bertulang

Dasar perencanaan struktur beton bertulang meliputi, peraturan-peraturan, analisis struktur, perencanaan pelat, perencanaan struktur portal dengan daktilitas penuh dan terbatas, penulangan balok, dan penulangan kolom.

#### 3.3.1 Peraturan-Peraturan

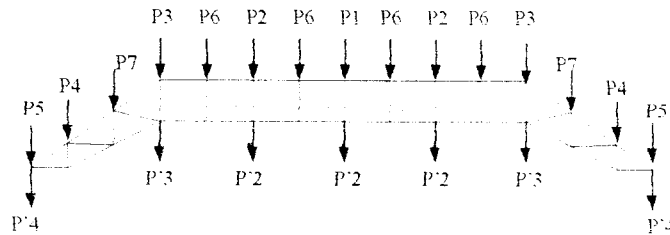
Peraturan-peraturan yang dipergunakan antara lain:

1. Tata Cara Perhitungan Struktur Beton Untuk Bangunan Gedung SK SNI T-15-1991-03.
2. Petunjuk Perencanaan Beton Bertulang dan Struktur Dinding Bertulang Untuk Rumah dan Gedung 1987.
3. Pedoman Perencanaan Pembebanan Untuk Rumah dan Gedung 1987.

## 10. Beban titik P'4

- Langit-langit ( rusuk-rusuk kayu dan asbes ) :  
 $= 0,11 \times \text{Luas pembebanan} = 0,11 \times 3,6 \times 1,8 = 0,713 \text{ kN}$
- Penggantung baja ( $d=3/4''$ ) =  $0,022 \times 0 = \underline{0,000 \text{ kN}}$  +  
 $P'4 = 0,713 \text{ kN}$

## c. Perencanaan Beban Mati Kuda-Kuda K3



Gambar. 4.9 Perencanaan beban mati kuda-kuda K3

## 1. Beban titik P1

- Berat sendiri  $PK1 + PK3 = 1,19 + 1,10 = 2,29 \text{ kN}$
- Berat penutup atap:  $0,5 \cdot 2,197 \cdot 3,6 = 3,95 \text{ kN}$
- Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
- Berat sagrod:  $0,022 \cdot 2,197 \cdot 2,1,25 = \underline{0,121 \text{ kN}}$  +  
 $P1 = 7,051 \text{ kN}$

## 2. Beban titik P2

- Berat sendiri:  $PK2 + PK3 = 1,16 + 1,1 = 2,26 \text{ kN}$
- Berat penutup atap:  $0,5 \cdot 2,197 \cdot 3,6 = 3,95 \text{ kN}$
- Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
- Berat sagrod:  $0,022 \cdot 2,197 \cdot 2,1,25 = \underline{0,121 \text{ kN}}$  +  
 $P2 = 7,021 \text{ kN}$

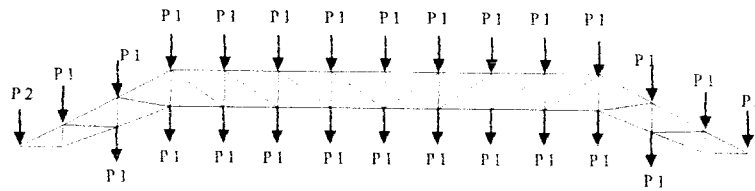
## 3. Beban titik P3

- Berat sendiri:  $2 \cdot P'K3 = 2 \cdot 2,28 = 4,56 \text{ kN}$
- Berat penutup atap:  $0,5 \cdot 2,197 \cdot 3,6 = 3,95 \text{ kN}$

- Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
  - Berat sagrod:  $0,022 \cdot 2,197 \cdot 2 \cdot 1,25 = \underline{0,121 \text{ kN}}$  +  
 $P3 = 9,321 \text{ kN}$
4. Beban titik P4
- Berat sendiri:  $PK3 + PK4 = 1,1 + 1,00 = 2,1 \text{ kN}$
  - Berat penutup atap:  $0,5 \cdot 2,197 \cdot 3,6 = 3,95 \text{ kN}$
  - Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
  - Berat sagrod:  $0,022 \cdot 1,953 \cdot 2 \cdot 1,25 = \underline{0,054 \text{ kN}}$  +  
 $P4 = 6,364 \text{ kN}$
5. Beban titik P5
- Berat sendiri:  $PK3 = 1,1 \text{ kN}$
  - Berat penutup atap:  $0,5 \cdot 1,099 \cdot 3,6 = 1,99 \text{ kN}$
  - Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
  - Berat sagrod:  $0,022 \cdot 0,855 \cdot 2 \cdot 1,25 = \underline{0,047 \text{ kN}}$  +  
 $P5 = 4,229 \text{ kN}$
6. Beban titik P6
- Berat sendiri:  $PK3 = 1,1 \text{ kN}$   
 $P6 = 1,1 \text{ kN}$
7. Beban titik P7
- Berat sendiri:  $PK3 = 1,1 \text{ kN}$
  - Berat penutup atap:  $0,5 \cdot 2,197 \cdot 3,6 = 3,95 \text{ kN}$
  - Berat gording:  $0,153 \cdot 3,6 \cdot 1,25 = 0,69 \text{ kN}$
  - Berat sagrod:  $0,022 \cdot 2,197 \cdot 2 \cdot 1,25 = \underline{0,121 \text{ kN}}$  +  
 $P5 = 5,861 \text{ kN}$
8. Beban titik P'2
- Langit-langit ( rusuk-rusuk kayu dan asbes ) :  
 $= 0,11 \times \text{Luas pembebanan} = 0,11 \times 3,6 \times 5,4 = 2,138 \text{ kN}$
  - Penggantung baja ( $d = 3/4''$ )  $= 0,022 \times 1,848 = \underline{0,041 \text{ kN}}$  +  
 $P'2 = 2,179 \text{ kN}$



### c. Perencanaan Beban Hidup Kuda-Kuda K3

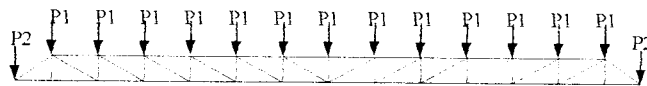


**Gambar. 4.13** Perencanaan beban hidup kuda-kuda K3

Beban hidup:  $P1 = 1 \text{ kN}$

$P2 = 2 \text{ kN}$

### d. Perencanaan beban hidup kuda-kuda K4



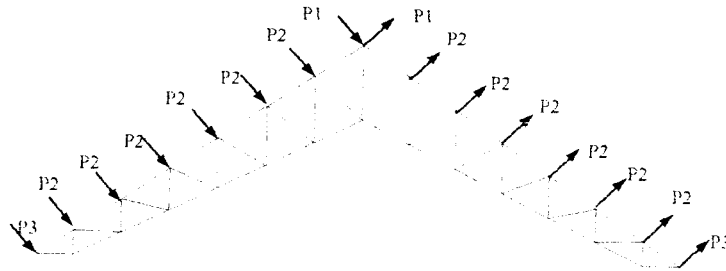
**Gambar. 4.14** Perencanaan beban hidup kuda-kuda K4

Beban hidup:  $P1 = 1 \text{ kN}$

$P2 = 2 \text{ kN}$

## 5. Perencanaan Beban Angin Kuda-Kuda

### a. Perencanaan beban angin kuda-kuda K1



**Gambar. 4.19** Perencanaan beban angin kuda-kuda K1

Beban Angin:

- P1

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6 : 2) = 0,18 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,15 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,10 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (2,197 \times 3,6 : 2) = 0,24 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,19 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,14 \text{ kN}$$

- P2

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 1,42 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,17 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,82 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (2,197 \times 3,6) = 1,9 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,55 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 1,09 \text{ kN}$$

- P3

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 0,87 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,72 \text{ kN}$$

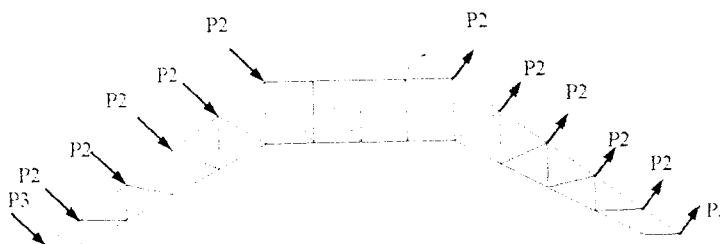
$$W_x = W \cdot \sin 35^\circ = 0,50 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (1,099 \times 3,6) = 1,16 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,95 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,67 \text{ kN}$$

### b. Perencanaan beban angin kuda-kuda K2



**Gambar. 4.20** Perencanaan beban angin kuda-kuda K2

- P2

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 1,42 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,17 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,82 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (2,197 \times 3,6) = 1,9 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,55 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 1,09 \text{ kN}$$

- P3

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 0,87 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,72 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,50 \text{ kN}$$

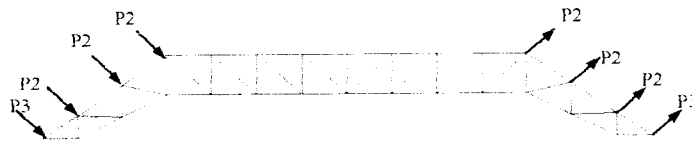
$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (1,099 \times 3,6) = 1,16 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,95 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,67 \text{ kN}$$



### c. Perencanaan beban angin kuda-kuda K3



**Gambar. 4.21** Perencanaan beban angin kuda-kuda K3

- P2

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 1,42 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,17 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,82 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (2,197 \times 3,6) = 1,9 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,55 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 1,09 \text{ kN}$$

- P3

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 0,87 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,72 \text{ kN}$$

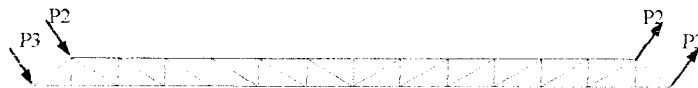
$$W_x = W \cdot \sin 35^\circ = 0,50 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (1,099 \times 3,6) = 1,16 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,95 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,67 \text{ kN}$$

#### d. Perencanaan beban angin kuda-kuda K4



**Gambar. 4.22** Perencanaan beban angin kuda-kuda K4

- P2

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 1,42 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,17 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,82 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (2,197 \times 3,6) = 1,9 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 1,55 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 1,09 \text{ kN}$$

- P3

$$\text{Tiup angin} = (0,02 \cdot 35 - 0,4) \cdot 0,6 \cdot (2,197 \times 3,6) = 0,87 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,72 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,50 \text{ kN}$$

$$\text{Hisap angin} = -0,4 \cdot 0,6 \cdot (1,099 \times 3,6) = 1,16 \text{ kN}$$

$$W_y = W \cdot \cos 35^\circ = 0,95 \text{ kN}$$

$$W_x = W \cdot \sin 35^\circ = 0,67 \text{ kN}$$

#### 4.3.2 Analisis Struktur Kuda-Kuda

Analisis struktur kuda-kuda rangka baja menggunakan program aplikasi komputer SAP 90, dengan input data-data sebagai berikut ini:

1. Nomor joint dan element, sesuai bentuk dan ukuran rangka
2. Dukungan rangka baja dianggap sendi dan roll.
3. Luas profil baja yang dipakai.

Tabel 4.8 Gaya Batang Kuda-Kuda (Satuan kN)

Kuda-kuda	No.	N.D	N.L	N.H	N.W.ki	N.W.ka	N.u.1	N.u.2	N.u.3	N.u.4	N.u.5	N.u.6	N.u.7	N.u.8	N.u.9	N.u tarik	N.u tekan	Jenis	
[1]	[2]	[3]	[4]	[5]	[6]	[7]	[8]	[9]	[10]	[11]	[12]	[13]	[14]	[15]	[16]	[17]	[18]	[19]	
K1	1	8,7000	3,4100	0,9800	1,2300	-1,6200	12,1800	12,1450	10,9300	16,8800	14,6000	13,7440	10,0390	6,2310	9,9360	16,8800	-	-	a
	2	-60,1900	-15,6600	-7,5300	8,8800	11,9100	-84,2660	-80,0560	-75,9930	-104,3880	-87,7560	-91,6020	-64,5750	-42,6270	-69,6540	-	-104,3880	a	
	3	-93,7200	-24,4800	-11,3800	-13,1900	17,6700	-131,2060	-124,7040	-118,1540	-162,1840	-137,4960	-141,8510	-101,7330	-67,2010	-107,3190	-	-162,1840	a	
	4	-105,6000	-28,0700	-12,8700	-14,4000	19,3200	-147,8400	-140,7550	-133,1550	-183,1520	-156,1760	-159,4750	-115,6390	-76,3200	-120,1560	-	-183,1520	a	
	5	-108,4200	-28,7300	-13,0300	-13,9100	18,7000	-151,7880	-144,4690	-136,6190	-187,2000	-161,1120	-162,5520	-120,1590	-79,4950	-121,8880	-	-187,2000	a	
	6	-103,3900	-27,5800	-12,3700	-12,3500	16,6600	-144,7460	-137,8580	-130,2530	-178,0760	-154,8680	-153,9130	-116,2000	-76,9960	-114,7090	-	-178,0760	a	
	7	-94,9400	-25,1700	-11,1400	-10,3600	13,6600	-132,9160	-126,5130	-119,4980	-162,2560	-143,2720	-139,6040	-108,7550	-72,3550	-103,2040	-	-162,2560	a	
	8	-103,3900	-27,5800	-12,3700	-10,3600	13,9500	-132,9160	-126,5130	-119,4980	-162,4880	-143,0400	-139,6040	-108,3780	-71,9780	-103,5810	-	-172,9160	a	
	9	-108,4200	-28,7300	-13,0300	-1,9100	6,7200	-151,7880	-144,4690	-136,6190	-177,6000	-170,8960	-146,9520	-135,7330	-95,0950	-106,3140	-	-177,6000	a	
	10	-108,4200	-28,7300	-13,0300	1,3300	3,6300	-147,8400	-140,7550	-133,1550	-170,5680	-168,7280	-139,0260	-136,0360	-96,7690	-99,7590	-	-170,5680	a	
	11	-93,7200	-24,4800	-11,3800	3,3500	1,8000	-131,2060	-124,7040	-118,1540	-148,9520	-150,6880	-120,3490	-123,1700	-88,7030	-85,8820	-	-150,6880	a	
	12	-60,1900	-15,6600	-7,5300	3,3700	0,2200	-84,2660	-80,0560	-75,9930	-94,5880	-97,4600	-75,6770	-80,3440	-58,5520	-53,8850	-	-97,4600	a	
	13	8,7000	3,4100	0,9800	0,0100	0,0000	12,1800	12,1450	10,9300	15,9040	15,8960	12,1580	12,1450	7,8170	7,8300	15,8960	-	-	a
	14	-7,1200	-2,7900	-0,8000	-1,5100	2,0000	-9,9680	-9,9390	-8,9440	-14,2160	-11,4080	-11,9020	-7,3390	-4,4450	-9,0080	-	-14,2160	b	
	15	-7,9100	-3,1200	-0,8900	11,2800	-11,0400	-11,0740	-11,0520	-9,9370	-5,4600	-23,3160	3,6120	-25,4040	-21,7830	7,2330	-	-25,4040	b	
	16	55,4500	14,4300	6,9300	19,8100	-22,4400	77,6300	73,7550	70,0050	105,4760	118,4240	99,5080	44,5830	24,1520	79,0770	105,4760	-	-	b
	17	86,3000	22,5400	10,4800	23,0400	-26,8000	120,8200	114,8300	108,8000	157,8880	176,4680	159,5530	80,3800	47,9910	112,1200	157,8880	-	-	b
	18	97,2300	25,8500	11,8500	21,6600	-25,0100	136,1220	129,6010	122,6010	176,4680	136,5960	169,5530	94,7610	57,5550	122,3470	176,4680	-	-	b
	19	99,8500	26,4600	12,0000	21,6600	-25,0100	139,7900	133,0500	125,8200	179,4840	142,1480	161,2080	100,5370	61,7070	122,3780	179,4840	-	-	b
	20	95,0600	25,3600	11,3700	19,2900	-21,8900	133,0840	126,7520	119,7570	170,0800	137,1360	151,8290	98,2950	60,4770	114,0110	170,0800	-	-	b
	21	95,0600	25,3600	11,3700	10,9200	-13,5200	133,0840	126,7520	119,7570	166,3840	143,8320	140,9480	109,1760	71,3580	103,1300	166,3840	-	-	b
	22	99,8500	26,4600	12,0000	6,0500	-9,4200	139,7900	133,0500	125,8200	163,9600	154,8200	140,9150	120,8040	82,0000	102,1110	166,9960	-	-	b
	23	97,2300	25,8500	11,8500	1,8400	-5,6400	136,1220	129,6010	122,6010	159,5080	153,5240	131,9930	122,2690	85,1150	94,8390	159,5080	-	-	b
	24	86,3000	22,5400	10,4800	-1,2400	-2,4700	120,8200	114,8300	108,8000	138,6320	137,6480	113,2180	111,6190	79,2820	80,8810	138,6320	-	-	b
	25	55,4500	14,4300	6,9300	-2,3500	-0,3600	77,6300	73,7550	70,0050	87,7480	89,3400	70,7000	73,2870	52,9600	50,3730	89,3400	-	-	b
	26	-7,9100	-3,1200	-0,8900	0,0100	0,0000	-11,0740	-11,0520	-9,9370	-14,4920	-14,4840	-11,0650	-11,0520	-7,1060	-7,1190	-	-14,4920	b	
	27	-7,1200	-2,7900	-0,8000	0,0100	0,0000	-9,9680	-9,9390	-8,9440	-13,0160	-13,0080	-9,9520	-9,9390	-6,3950	-6,4080	-	-13,0160	b	
	28	-48,0200	-12,5100	-5,9700	9,4900	9,4900	-67,2280	-63,8790	-60,6090	-83,3200	-70,0480	-73,1090	-51,5420	-33,9880	-55,5550	-	-83,3200	vp	
	29	-17,7800	-4,2500	-2,2900	-1,9300	2,6000	-24,8920	-23,4610	-22,4810	-29,6800	-26,0560	-25,9700	-20,0810	-13,4930	-19,3820	-	-29,6800	vp	
	30	-1,0400	0,3900	-0,1500	1,3500	-1,7600	-1,4560	-1,0530	-1,3230	0,4560	-2,0320	0,7020	-3,3410	-2,6910	1,3520	-	-3,3410	vp	
	31	100,7300	27,7000	12,2200	11,6800	-15,6900	141,0220	134,7260	126,9860	174,5400	152,6440	149,9100	114,3290	75,4730	111,0540	152,6440	-	-	vp
	32	-1,0400	0,3900	-0,1500	-4,1300	3,7100	-1,4560	-1,0530	-1,3230	-3,9280	2,3440	-6,4220	3,7700	4,4330	-5,7590	-	-6,4220	vp	
	33	-17,7800	-4,2500	-2,2900	-0,7200	1,3700	-24,8920	-23,4610	-22,4810	-28,7120	-27,0400	-24,3970	-21,6800	-15,0660	-17,7830	-	-28,7120	vp	
	34	-48,0200	-12,5100	-5,9700	2,9500	-0,8300	-67,2280	-63,8790	-60,6090	-75,2800	-78,3040	-60,0440	-64,9580	-47,0530	-42,1390	-	-78,3040	vp	
	35	56,2200	15,5700	6,9400	7,5600	-10,1100	78,7080	75,2490	70,9340	98,4240	84,2880	85,0770	62,1060	40,7780	63,7410	98,4240	-	-	d
	36	28,1300	7,4000	3,2400	2,7800	-3,7200	39,3820	37,4560	35,3760	47,8200	42,8200	41,0700	32,6200	21,7030	30,1530	47,8200	-	-	d
	37	10,4700	3,1700	1,3100	0,1900	-0,2900	14,6580	14,1490	13,2190	17,8880	17,4040	14,3960	13,7720	9,1760	9,8000	17,8880	-	-	d
	38	2,7000	0,6400	0,1500	-1,4200	1,8500	3,7800	3,5600	3,3150	3,1280	5,7440	1,7140	5,9650	4,2760	0,0250	5,9650	-	-	d
	39	-5,3400	-1,2300	-0,7000	-2,6600	3,5100	-7,4760	-7,0230	-6,7580	-10,5040	-5,5680	-10,4810	-2,4600	-1,3480	-9,3690	-	-10,5040	d	
	40	-9,0900	-2,6000	-1,3400	3,6800	4,8500	-12,7260	-12,2060	-11,5780	-18,0120	-11,1880	-16,9920	-5,9030	-3,3970	-14,4860	-	-18,0120	d	
	41	-0,9900	-2,6000	-1,3400	6,6600	-5,4700	-12,7260	-12,2060	-11,5780	-9,7400	-19,4480	-3,5500	-19,3190	-6,9390	-1,0700	-	-19,4480	d	
	42	-5,3400	-1,2300	-0,7000	5,4900	-4,6400	-7,4760	-7,0230	-6,7580	-3,9840	-12,0680	0,1140	-13,0550	-11,9330	1,2260	-	-13,0550	d	
	43	2,7000	0,6400	0,1500	4,3400	-3,8900	3,7800	3,5600	3,3150	7,7360	1,1520	9,2020	-1,4970	-3,2120	7,4870	9,2020	-	-	d