

Lampiran 4 Verifikasi

1. Menentukan Kelas Situs Tanah

Kedalaman (m)	Tebal (m)	NSPT	$\check{N} = \text{tebal}/\text{NSPT}$
0	0	0	0.00
2	2	27	0.07
4	2	25	0.08
6	2	27	0.07
8	2	43	0.05
10	2	32	0.06
12	2	40	0.05
14	2	12	0.17
16	2	22	0.09
18	2	60	0.03
20	2	60	0.03
22	2	60	0.03
24	2	60	0.03
26	2	57	0.03

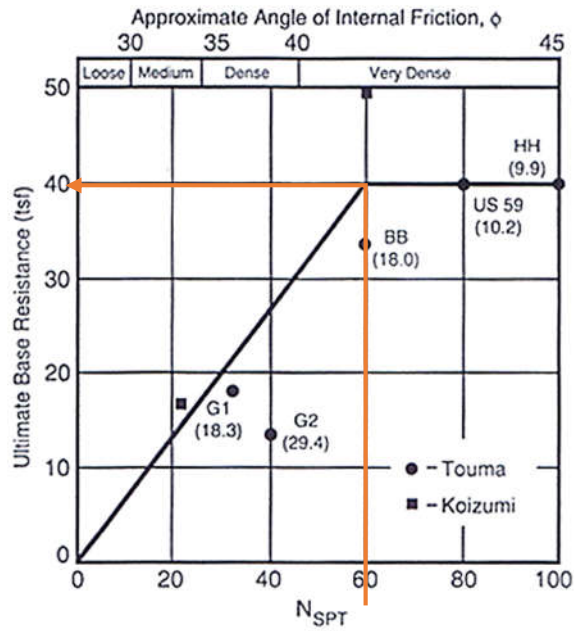
$$\begin{aligned}\Sigma \check{N} &= 0,00 + 0,07 + 0,08 + 0,07 + 0,05 + 0,06 + 0,05 + 0,17 + 0,09 + 0,03 + \\ & 0,03 + 0,03 + 0,03 + 0,04 \\ &= 0,81\end{aligned}$$

$$\bar{N} = \frac{30}{\Sigma \check{N}} = 30/0,81 = 36,89$$

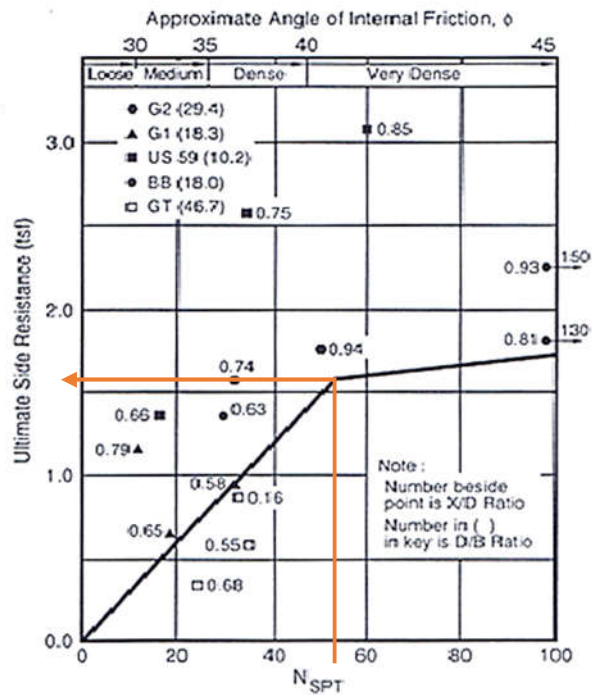
Karena $15 < \bar{N} = 36,89 < 50$ maka termasuk tanah sedang

2. Mencari Daya Dukung Fondasi

Pondasi di pasang sampai kedalaman 20m



$q_p = (N_{SPT} \text{ rata-rata} / 60) \times 40$ (dari grafik)



$f_s = (N_{SPT} \text{ rata-rata} / 53) \times 1,6$ (dari grafik)

Kedalaman	Tebal	NSPT	NSPT rata-rata	qp		fs	
				t/sf	t/m ²	t/sf	t/m ²
0	0	0	0	0	0	0	0
2	2	27	13.5	9	96.88	0.41	4.39
4	2	25	26	17.33	186.57	0.78	8.45
6	2	27	26	17.33	186.57	0.78	8.45
8	2	43	35	23.33	251.16	1.06	11.37
10	2	32	37.5	25	269.10	1.13	12.19
12	2	40	36	24	258.33	1.09	11.70
14	2	12	26	17.33	186.57	0.78	8.45
16	2	22	17	11.33	121.99	0.51	5.52
18	2	18	20	13.33	143.52	0.60	6.50
20	2	60	39	26	279.86	1.18	12.67

Diameter dipakai antara 0,4 m, 0,8 m, 1 m dan 1,2 m

$$\text{Luas penampang tiang (A)} = \frac{1}{4} \cdot \pi \cdot D^2$$

$$\text{Keliling tiang (K)} = \pi \cdot D$$

Notasi	Diameter Tiang (m)			
	0.4	0.8	1	1.2
A (m ²)	0.13	0.50	0.79	1.13
K (m)	1.26	2.51	3.14	3.77

Luas selimut tiang per segmen = Keliling tiang x Tebal

Kedalaman (m)	Tebal (m)	Keliling Tiang			
		1,26	2,51	3,14	3,77
0	0	2,51	5,03	6,28	7,54
2	2	2,51	5,03	6,28	7,54
4	2	2,51	5,03	6,28	7,54
6	2	2,51	5,03	6,28	7,54
8	2	2,51	5,03	6,28	7,54
10	2	2,51	5,03	6,28	7,54
12	2	2,51	5,03	6,28	7,54
14	2	2,51	5,03	6,28	7,54
16	2	2,51	5,03	6,28	7,54
18	2	2,51	5,03	6,28	7,54
20	2	2,51	5,03	6,28	7,54

f_s tiang per segmen = $f_s \times$ Luas selimut tiang per segmen

Kedalaman	Tebal	f_s	Diameter Tiang				Diameter Tiang			
			0,4	0,8	1	1,2	0,4	0,8	1	1,2
m	m	t/m ²	Luas Selimut Tiang per Segmen				f _s Tiang per Segmen			
0	0	0	0	0	0	0	0	0	0	0
2	2	4.39	2,51	2,51	5,03	6,28	11,03	22,05	27,56	33,08
4	2	8.45	2,51	2,51	5,03	6,28	21,23	42,47	53,08	63,70
6	2	8.45	2,51	2,51	5,03	6,28	21,23	42,47	53,08	63,70
8	2	11.37	2,51	2,51	5,03	6,28	28,58	57,17	71,46	85,75
10	2	12.19	2,51	2,51	5,03	6,28	30,63	61,25	76,56	91,88
12	2	11.70	2,51	2,51	5,03	6,28	29,40	58,80	73,50	88,20
14	2	8.45	2,51	2,51	5,03	6,28	21,23	42,47	53,08	63,70
16	2	5.52	2,51	2,51	5,03	6,28	13,88	27,77	34,71	41,65
18	2	6.50	2,51	2,51	5,03	6,28	16,33	32,67	40,83	49,00
20	2	12.67	2,51	2,51	5,03	6,28	31,85	63,70	79,63	95,55

$$Q_{st} = \sum f_s$$

Q_p = qp di ujung pondasi. Luas penampang tiang

$$Q_u = Q_{st} + Q_p$$

$$Q_{all} = \frac{Q_u}{SF}$$

Notasi	Diameter Tiang			
	0,4	0,8	1	0,4
Q _{st} (ton)	225,40	450,81	563,51	676,21
Q _p (ton)	35,17	140,67	219,80	316,52
Q _u (ton)	260,57	591,48	783,31	992,73
SF	3	3	3	3
Q _{all} (ton)	86,86	197,16	261,10	330,91

3. Mencari Jumlah Tiang

Dari data ETABS di dapat nilai :

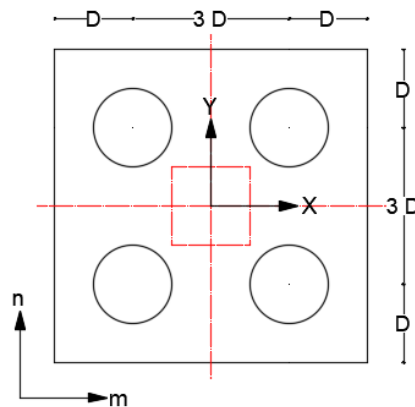
$$P_d = 255.71 \text{ ton}$$

$$P_l = 0 \text{ ton}$$

Digunakan tiang diameter 0,4 m

$$\text{Jumlah tiang} = \frac{P_d + P_l}{P_{all}} = \frac{189,56}{86,86} = 2,18 \text{ buah} \approx 4 \text{ buah untuk OF}$$

$$\text{Jumlah tiang} = \frac{P_d + P_l}{P_{all}} = \frac{192,571}{86,86} = 2,22 \text{ buah} \approx 4 \text{ buah untuk BrX}$$

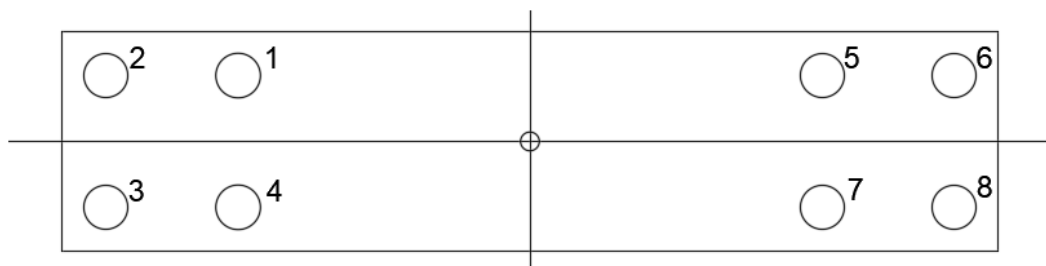


m = 2 buah

n = 2 buah

Jarak antar tiang (s) = 3d = 3 x 0,4 = 1,2 m

4. Mencari Dimensi *Pile Cap*



Panjang *pile cap* = (2x1d) + (2x3d) + 5,3 = (2x0,4) + (6x0,4) + 5,3 = 8,5 m

Lebar *pile cap* = 1d + 3d + 1d = (1 x 0,4) + (3 x 0,4) + (1 x 0,4) = 2 m

Tinggi *pile cap* = 2d = 2 x 0,4 = 0,8 m

5. Mencari Massa *Pile Cap*

Volume *pile cap* = Panjang *pile cap* x Lebar *pile cap* x Tinggi *pile cap*

$$= 8,5 \times 2 \times 0,8 = 13,6 \text{ m}^3$$

Berat jenis = 2400 kg/m³ (beton bertulang)

Berat *pile cap* (W) = Volume *pile cap* x Berat jenis

$$= 13,6 \times 2400 = 32640 \text{ kg}$$

$$m_0 = \frac{W}{g} = \frac{32640}{9,81} = 3327,22 \frac{\text{kg} \cdot \text{dt}^2}{\text{m}}$$

6. Mencari Nilai G (modulus geser tanah)

Data tanah yang digunakan

Lapisan Tanah	Kedalaman m	Deskripsi Tanah	γ_{sat}	γ_w	e	PI	OCR	Kohesi	θ
			kg/m ³	kg/m ³		%		kg/m ²	derajat
1	6	Pasir	1828	1800	0,4	5	1	300	33
2	12	Lanau	1957	1800	0,6	20	1	360	26
3	17	Lanau	1820	1800	0,6	21	1	220	26
4	20	Lempung	1960	1800	0,6	25	1	370	20
5	24	Pasir	1895	1800	0,4	5	1	200	40
6	26	Pasir	1895	1800	0,4	3	1	200	34

Plasticity Index (PI)	k
0	0
20	0,18
40	0,3
60	0,41
80	0,48
100	0,5

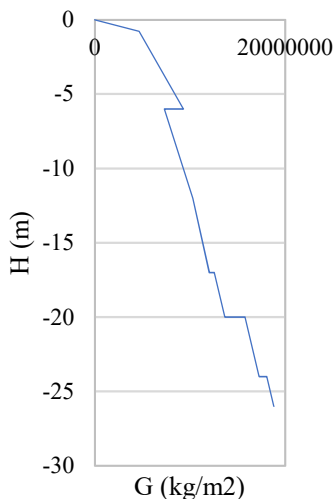
Muka air tanah berada di kedalaman -0,8 m.

$$\begin{aligned} \bar{\sigma}_v &= h \cdot \gamma \\ &= \frac{0,8 \cdot 1800}{10^4} = 0,144 \text{ kg/cm}^2 \end{aligned}$$

$$\bar{\sigma}_h = k_0 \cdot \bar{\sigma}_v$$

$$\begin{aligned}
 &= (1 - \sin \phi) \bar{\sigma}_v \\
 &= (1 - \sin(33)) * 0,144 = 0,066 \text{ kg/cm}^2 \\
 \bar{\sigma}_0 &= \frac{\bar{\sigma}_v + 2\bar{\sigma}_h}{3} \\
 &= \frac{0,144 + 2 * 0,066}{3} = 0,092 \text{ kg/cm}^2 \\
 G &= 326 \cdot OCR^k \frac{(2,973 - e)^2}{(1 + e)} \bar{\sigma}_0^{0,5} \\
 &= 326 \cdot 1^{0,086} \frac{(2,973 - 0,4)^2}{(1 + 0,4)} 0,092^{0,5} = 465,774 \text{ kg/cm}^2 = 4657739,27 \text{ kg/m}^2
 \end{aligned}$$

El.	e	PI	OCR	φ	K ₀	γ _{sat} (kg/m ³)	γ _w (kg/m ³)	σ _v	σ _h	σ̄ ₀	k	G (kg/cm ²)	G (kg/m ²)	G x h	
0	0.4	5	1	33	0.455361	1828	1800	0	0	0	0.08602381	0	0	0	
-0.8	0.4	5	1	33	0.455361	1828	1800	0.144	0.065572	0.091715	0.08602381	465.7739268	4657739.268	3726191.41	
-0.8	0.4	5	1	33	0.455361	1828	1800	0.144	0.065572	0.091715	0.08602381	465.7739268	4657739.268	0	
-6	0.4	5	1	33	0.455361	1828	1800	0.57456	0.261632	0.365941	0.08602381	930.3826901	9303826.901	48379899.9	
-6	0.6	20	1	26	0.561629	1957	1800	0.57456	0.322689	0.406646	0.1612381	729.7981036	7297981.036	0	
-12	0.6	20	1	26	0.561629	1957	1800	1.14876	0.645177	0.813038	0.1612381	1031.928695	10319286.95	61915721.7	
-12	0.6	21	1	26	0.561629	1820	1800	1.14876	0.645177	0.813038	0.16625238	1031.928695	10319286.95	0	
-17	0.6	21	1	26	0.561629	1820	1800	1.55876	0.875445	1.103216	0.16625238	1202.055896	12020558.96	60102794.8	
-17	0.6	25	1	20	0.65798	1960	1800	1.55876	1.025633	1.203342	0.18630952	1255.419322	12554193.22	0	
-20	0.6	25	1	20	0.65798	1960	1800	1.84676	1.215131	1.425674	0.18630952	1366.483563	13664835.63	40994506.9	
-20	0.4	5	1	40	0.357212	1895	1800	1.84676	0.659686	1.055377	0.08602381	1580.009256	15800092.56	0	
-24	0.4	5	1	40	0.357212	1895	1800	2.20476	0.787568	1.259965	0.08602381	1726.374697	17263746.97	69054987.9	
-24	0.4	3	1	34	0.440807	1895	1800	2.20476	0.971874	1.382836	0.07599524	1808.594203	18085942.03	0	
-26	0.4	3	1	34	0.440807	1895	1800	2.38376	1.050778	1.495106	0.07599524	1880.579674	18805796.74	37611593.5	
													sum	321785696	
													ekuivalen	12376372.9	



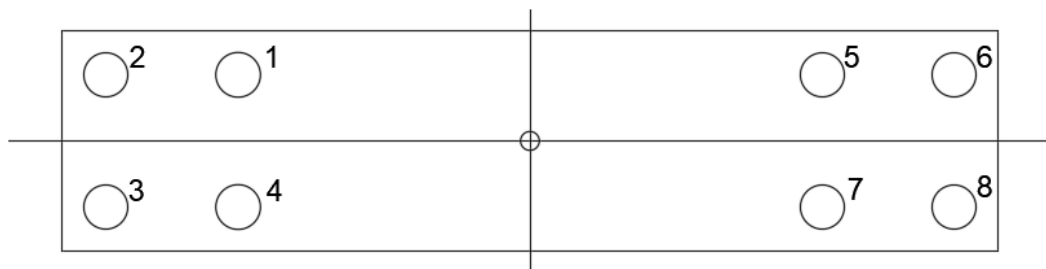
Hasil ekuivalen didapatkan dari

$$G = (1/26) * 321785696 = 12376372.92 \text{ kg/m}^2$$

7. Mencari Kekakuan dan Redaman Interaksi antara Fondasi dengan Tanah

Data tiang dan *pile cap*

f_c	= 30 Mpa
Jumlah tiang	= 8 buah
Panjang tiang	= 20 m
Dia. tiang	= 0,4 m
m	= 4 buah
n	= 2 buah



Mod. elas. tiang (E_p)	= $4700 \cdot \sqrt{f'_c} = 4700 \cdot \sqrt{30}$
	= 25742,9602 MPa
	= 2625781941 kg/m ²
Inersia tiang (I_p)	= $(1/64) \times \pi \times D^2 = (1/64) \times \pi \times 0,4^2$
	= 0,001256637 m ⁴
Panjang <i>pile cap</i>	= 8,5 m
Lebar <i>pile cap</i>	= 2 m
Tinggi <i>pile cap</i>	= 0,8 m
h <i>pile cap</i> dlm tanah	= 0,7 m (asumsi 10 cm di bawah muka tanah)
G tanah	= 12376372,92 kg/m ²
γ sat	= 1828 kg/m ³
v	= 0,4
ro tiang	= 0,2 m
ro <i>pile cap</i>	= 2,326213246 m
Sh1	= 4,1
Sh2	= 10,6
Sr1	= 2,5
Sr2	= 1,8

a. Kekakuan dan Redaman Vertikal

$$V_s = \sqrt{\frac{G_s g}{\gamma_{sat}}} = \sqrt{\frac{12376372,92 * 9,81}{1828}} = 257,7170203 \text{ m/dt}$$

$$\frac{E_p}{G_s} = \frac{2625781941}{12376372,92} = 212,1608614$$

$$\frac{L}{r_0} = \frac{20}{0,2} = 100$$

Didapat nilai konstanta f_{v1} dan f_{v2} dari grafik.

$$f_{v1} = 0,0635$$

$$f_{v2} = 0,0975$$

$$k_v^1 = \frac{E_p A}{r_0} f_{v1} = \frac{2625781941 * \left(\frac{1}{4} * \pi * 0,4^2\right)}{0,2} * 0,0635 = 104777789,2 \text{ kg/m}$$

$$c_v^1 = \frac{E_p A}{V_s} f_{v2} = \frac{2625781941 * \left(\frac{1}{4} * \pi * 0,4^2\right)}{257,7170203} * 0,0975 = 124774,3739 \text{ kgdt/m}$$

b. Kekakuan dan Redaman Horizontal

$$V_s = \sqrt{\frac{G_s g}{\gamma_{sat}}} = \sqrt{\frac{12376372,92 * 9,81}{1828}} = 257,7170203 \text{ m/dt}$$

$$\frac{E_p}{G_s} = \frac{2625781941}{12376372,92} = 212,1608614$$

Didapat nilai konstanta f_{h1} dan f_{h2} dari tabel.

$$f_{h1} = 0,0726$$

$$f_{h2} = 0,1717$$

1. Tiang tunggal

$$k_h^1 = \frac{E_p I_p}{r_0^3} f_{h1} = \frac{2625781941 * 0,001256637}{0,2^3} * 0,0726 = 29944368,23 \text{ kg/m}$$

$$c_h^1 = \frac{E_p I_p}{r_0^2 V_s} f_{h2} = \frac{2625781941 * 0,001256637}{0,2^2 * 257,7170203} * 0,1717 = 54958,60789 \text{ kgdt/m}$$

2. Kelompok tiang

$$K_R = \frac{E_p I_p}{2G(1+\nu)_{soil} L^4} = \frac{2625781941 * 0,001256637}{2 * 12376372,92 * (1+0,4) * 20^4} = 5.9511E-07 \text{ kg/m}$$

αL didapat dari grafik.

No Pile	S/(2r0)	αL
1	0	1
2,4	3	0,5
3	4,25	0,375
5	13,25	0
6	16,25	0
7	13,575	0
8	16,525	0

$$\sum \alpha L = 1*1 + 2*0,5 + 1*0,375 + 1*0 + 1*0 + 1*0 = 2,375$$

$$k_h^g = \frac{\sum k_h^1}{\sum \alpha L} = \frac{8 * 29944368,23}{2,375} = 100865240,4 \text{ kg/m}$$

$$c_h^g = \frac{\sum c_h^1}{\sum \alpha L} = \frac{8 * 58154,0098}{2,375} = 185123,7318 \text{ kgdt/m}$$

3. Pile cap

$$k_h^f = G_s h S_{h1} = 12376372,92 * 0,7 * 4,1 = 35520190,29 \text{ kg/m}$$

$$c_h^f = h r_0 S_{h2} \sqrt{G_s \rho_s} = 0,7 * 2,33 * 10,6 \sqrt{\frac{12376372,92 * 9,81}{1828}} = 828903 \text{ kgdt/m}$$

4. Total

$$k_h = 100865240,4 + 35520190,29 = 136385430,7 \text{ kg/m}$$

$$c_h = 185123,7318 + 828903 = 1014026,739 \text{ kgdt/m}$$

c. Kekakuan dan Redaman Rotasi

$$V_s = \sqrt{\frac{G_s g}{\gamma_{sat}}} = \sqrt{\frac{12376372,92 * 9,81}{1828}} = 257,7170203 \text{ m/dt}$$

$$\frac{E_p}{G_s} = \frac{2625781941}{12376372,92} = 212,1608614$$

Didapat nilai konstanta f_{hr1} , f_{hr2} , f_{r1} dan f_{r2} dari tabel.

$$f_{hr1} = -0,1365$$

$$f_{hr2} = -0,1896$$

$$f_{r1} = 0,5336$$

$$f_{r2} = 0,3377$$

1. Tiang tunggal

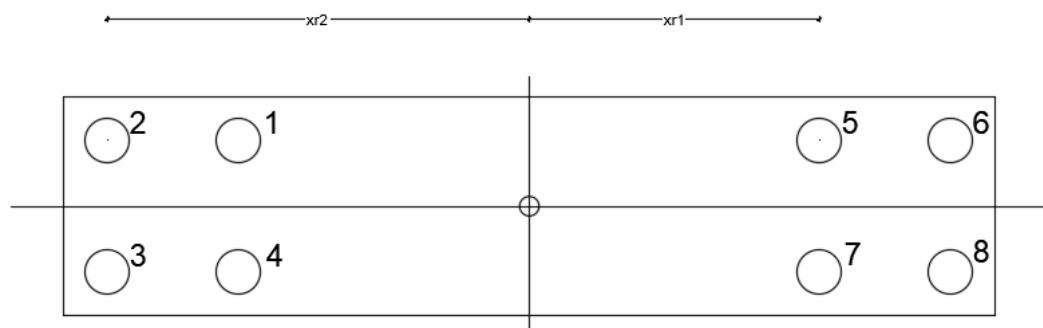
$$k_{hr}^1 = \frac{E_p I_p}{r_0^2} f_{hr1} = \frac{2625781941 * 0,001256637}{0,2^2} * (-0,1365) = -11260072,4 \text{ kg}$$

$$c_{hr}^1 = \frac{E_p I_p}{r_0 V_s} f_{hr2} = \frac{2625781941 * 0,001256637}{0,2 * 257,7170203} (-0,1896) = -12137,63 \text{ kgdt}$$

$$k_r^1 = \frac{E_p I_p}{r_0} f_{r1} = \frac{2625781941 * 0,001256637}{0,2} * 0,5336 = 8803479,278 \text{ kgm}$$

$$c_r^1 = \frac{E_p I_p}{V_s} f_{r2} = \frac{2625781941 * 0,001256637}{257,7170203} * 0,3377 = 4323,709 \text{ kgmdt}$$

2. Kelompok tiang



$$x_{r1} = 2,65 \text{ m}$$

$$x_{r1} = 3,85 \text{ m}$$

$$\begin{aligned}
k_r^g &= \sum_1^n [k_r^1 + k_v^1 x_r^2 + k_h^1 z_c^2 - 2z_c k_{hr}^1] \\
&= 8*8803479 + 4*104777789,2*(2,65^2 + 3,85^2) \\
&= +8*29944368,23*\left(\frac{0,8}{2}\right)^2 - 8*2*0,8*(-11260072,4) \\
&= 9336304311 \text{ kgm} \\
c_r^g &= \sum_1^n [c_r^1 + c_v^1 x_r^2 + c_h^1 z_c^2 - 2z_c c_{hr}^1] \\
&= 8*4323,709 + 4*124774,3739*(2,65^2 + 3,85^2) \\
&= +8*54958.60789*\left(\frac{0,8}{2}\right)^2 - 8*2*0,8*(-12137,63) \\
&= 11085402.29 \text{ kgmdt}
\end{aligned}$$

3. Pile cap

$$\delta = \frac{h}{r_0} = \frac{0,7}{2,32} = 0,3$$

$$\begin{aligned}
k_r^f &= G_s \cdot r_0^2 \cdot h \cdot \bar{S}_{r1} + G_s \cdot r_0^2 \cdot h \cdot \bar{S}_{h1} \left[\left(\frac{\delta^2}{3} \right) + \left(\frac{z_c}{r_0} \right)^2 - \delta \left(\frac{z_c}{r_0} \right) \right] \\
&= 12376372,92 * 2,32^2 * 0,7 * 2,5 + 12376372,92 * 2,32^2 * 0,7 * 4,1 * \\
&= \left[\left(\frac{0,3^2}{3} \right) + \left(\frac{0,8}{2,32} \right)^2 - 0,3 \left(\frac{0,8}{2,32} \right) \right] \\
&= 118739983.5 \text{ kgm}
\end{aligned}$$

$$\begin{aligned}
c_r^f &= \delta \cdot r_0^4 \sqrt{\frac{G_s \gamma_s}{g}} \cdot \left\{ \bar{S}_{r2} + \bar{S}_{h2} \left[\left(\frac{\delta^2}{3} \right) + \left(\frac{z_c}{r_0} \right)^2 - \delta \left(\frac{z_c}{r_0} \right) \right] \right\} \\
&= 0,3 * 2,32^4 \sqrt{\frac{12376372,92 * 1828}{9,81}} * \\
&= \left\{ 2,5 + 1,8 \left[\left(\frac{0,3^2}{3} \right) + \left(\frac{0,8}{2,32} \right)^2 - 0,3 \left(\frac{0,8}{1,13} \right) \right] \right\}
\end{aligned}$$

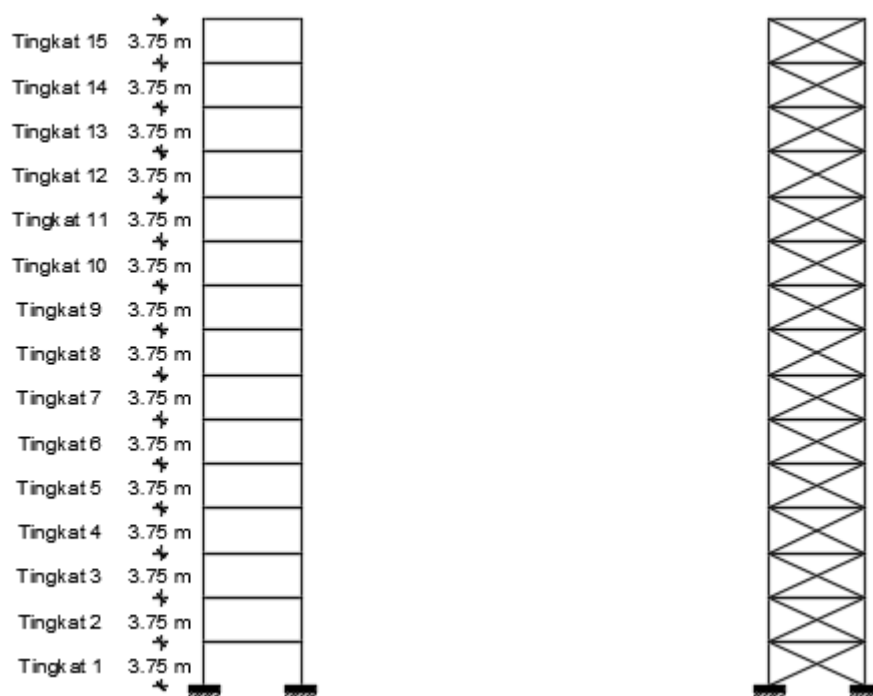
$$= 797593,6084 \text{ kgmdt}$$

4. Total

$$k_r = 9336304311 + 118739983.5 = 9455044295 \text{ kgm/rad}$$

$$c_r = 11085402.29 + 797593,6084 = 11882995.95 \text{ kgmdt/rad}$$

7. Menghitung Massa dan Kekakuan



No	Elemen Struktur	Lantai	Profil Baja	Berat (kg/m)	Luas (m ²)	Inersia (m ⁴)
1	Balok	1-5	W14X53	78.87268882	0.010064496	0.000225181
2	Balok	6-10	W14X53	78.87268882	0.010064496	0.000225181
3	Balok	11-15	W14X26	38.69226244	0.00496128	0.000101977
4	Kolom	1-5	W14X370	550.6206578	0.07032244	0.002264299
5	Kolom	6-10	W14X311	462.8189853	0.058967624	0.001802282
6	Kolom	11-15	W14X211	314.0025913	0.03999992	0.001107176
7	Bracing	1-5	W8X21	31.25144274	0.003974186	3.13422E-05
8	Bracing	6-10	W8X18	26.78695092	0.003393542	2.57647E-05
9	Bracing	11-15	W8X15	22.3224591	0.00286451	1.99791E-05

Tabel 4.3 Beban Merata Struktur 15 Tingkat

Tingkat	Beban Merata (kg/m)
	Balok 6.5 m
1-14	3427

Data :

$$\begin{aligned} \text{Modulus elastis baja (E}_{\text{baja}}) &= 200000 \text{ MPa} \\ &= 204000000000 \text{ kg/m}^2 \end{aligned}$$

$$\text{Tinggi tiap lantai} = 3,75 \text{ m}$$

$$\text{Lebar tengah} = 6,5 \text{ m}$$

$$\text{Panjang bracing tipe X} = \sqrt{6,5^2 + 3,75^2} = 7,50 \text{ m}$$

$$\begin{aligned} \alpha \text{ brX} &= 0.523278322 \text{ rad} \\ &= 29.98163937 \text{ degrees} \end{aligned}$$

$$\text{Percepatan gravitasi (g)} = 9,81 \text{ m/s}^2$$

a. Open Frame

$$m = \frac{W}{g}$$

$$\begin{aligned} M_{1-5} &= (2*550,6206578*3,75 + 1*78,87268882*6,5 + 6,5*3427)/9,81 \\ &= 2743,92 \text{ kgdt}^2/\text{m} \end{aligned}$$

$$\begin{aligned} M_{6-10} &= (2*462.8189853*3,75 + 1*78,87268882*6,5 + 6,5*3427)/9,81 \\ &= 2676,79 \text{ kgdt}^2/\text{m} \end{aligned}$$

$$\begin{aligned} M_{11-15} &= (2*314.0025913*3,75 + 1*38.69226244*6,5 + 6,5*3427)/9,81 \\ &= 2536,39 \text{ kgdt}^2/\text{m} \end{aligned}$$

$$K = \frac{12.E.I}{h^3}, K_b = \frac{H}{u} = \frac{A.E}{L} \cos^2(\alpha)$$

$$K_{1-5} = \frac{12 * 20400000000 * 0,002264299}{3,75^3} = 21022355,31 \text{ kg/m}$$

$$K_{6-10} = \frac{12 * 20400000000 * 0,001802282}{3,75^3} = 16732867,37 \text{ kg/m}$$

$$[M] = \begin{bmatrix} 2.74 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 2.74 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 2.74 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 2.74 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 2.74 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 2.68 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 2.68 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.68 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.68 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.68 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.54 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.54 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.54 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.54 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 2.54 \end{bmatrix}$$

$$[K] = \begin{bmatrix} 42.04 & -21.02 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ -21.02 & 42.04 & -21.02 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -21.02 & 42.04 & -21.02 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & -21.02 & 42.04 & -21.02 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & -21.02 & 37.76 & -16.73 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -16.73 & 33.47 & -16.73 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & -16.73 & 33.47 & -16.73 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & -16.73 & 33.47 & -16.73 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & -16.73 & 33.47 & -16.73 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -16.73 & 27.01 & -10.28 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 20.56 & -10.28 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 20.56 & -10.28 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 20.56 & -10.28 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 20.56 & -10.28 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 20.56 & -10.28 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -10.28 & 10.28 \end{bmatrix}$$

$$|[K] - \omega^2 [M]| = 0$$

Apabila di ambil notasi $\lambda = \omega^2/(k/m)$, maka didapat,

$$21,02235531 + (-379,4063527) \lambda + 1322,120434 \lambda^2 + (-1832,430664) \lambda^3 + \\ 1325,803547 \lambda^4 + (-574,8451209) \lambda^5 + 161,7881198 \lambda^6 + (-31,03923754) \lambda^7 + \\ 4,181023547 \lambda^8 + (-0,401612593) \lambda^9 + 0,027606177 \lambda^{10} + (-0,001345688) \lambda^{11} + \\ 4,53538E-05 \lambda^{12} + (-1,00353E-06) \lambda^{13} + 1,30977E-08 \lambda^{14} + (-7,6315E-11) \lambda^{15} = 0$$

$$\omega = (\lambda * (k/m))^{0.5}$$

Mode	λ	f (λ)	ω (rad/dt)	T (dt)
1	0.07158131	0.0000	8.46	0.74
2	0.5000598	0.0000	22.36	0.28
3	1.3946115	0.0000	37.34	0.17
4	2.71011	0.0000	52.06	0.12
5	4.3157727	0.0000	65.69	0.10
6	6.3276595	0.0000	79.55	0.08
7	8.1047589	0.0000	90.03	0.07
8	10.6420251	0.0000	103.16	0.06
9	12.4130224	0.0000	111.41	0.06
10	14.6913534	0.0000	121.21	0.05
11	15.7795256	0.0000	125.62	0.05
12	19.0316157	0.0000	137.96	0.05
13	22.52897226	0.0000	150.10	0.04
14	24.39934786	-0.0001	156.20	0.04
15	28.71526641	0.0021	169.46	0.04

Massa	Mode ke-														
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
	Nilai λ														
	0.0715813	0.5000598	1.3946115	2.71011	4.31577	6.32766	8.10476	10.642	12.413	14.6914	15.7795	19.0316	22.529	24.3993	28.7153
Koordinat Mode Shape	0														
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	1.990656937	1.934730307	1.817970043	1.64626621	1.436689	1.17409	0.942136	0.610963	0.379805	0.082429	-0.0596	-0.48408	-0.94057	-1.18469	-1.74802
3	2.962715042	2.743181362	2.305015077	1.71019244	1.064075	0.378487	-0.11238	-0.62672	-0.85575	-0.99321	-0.99645	-0.76567	-0.11533	0.403502	2.055591
4	3.907092314	3.372585813	2.372478315	1.16916583	0.092056	-0.72971	-1.04801	-0.99387	-0.70482	-0.1643	0.118995	0.854722	1.049046	0.706668	-1.8452
5	4.814965377	3.781862624	2.008079427	0.21456575	-0.93182	-1.23523	-0.87499	0.019508	0.588052	0.979662	0.989355	0.351916	-0.87136	-1.24069	1.169864
6	5.899053777	3.98593901	1.091031742	-1.08010303	-1.5587	-0.58863	0.50529	1.25862	1.015358	0.056732	-0.47721	-1.37807	-0.06492	1.27686	-0.55085
7	6.915592075	3.871157918	-0.06942383	-1.90650204	-1.10945	0.653816	1.230446	0.355024	-0.57357	-0.99953	-0.73916	1.087514	0.975498	-1.18945	0.258853
8	7.852939884	3.446701328	-1.21439103	-1.90635281	0.105766	1.234436	0.360286	-1.15297	-1.02354	0.293309	0.864737	0.242134	-1.49978	0.986913	-0.12052
9	8.700363629	2.746523888	-2.08842903	-1.07972003	1.247964	0.565501	-0.977	-0.69812	0.55897	0.896812	0.285797	-1.34043	1.43016	-0.68886	0.053726
10	9.448159454	1.826636693	-2.496541	0.21501646	1.528564	-0.67586	-1.04757	0.945233	1.031512	-0.60738	-1.01458	1.157979	-0.7942	0.324129	-0.01883
11	10.4893208	0.091364703	-2.25421831	2.17087076	0.267452	-1.58292	1.048472	1.000844	-1.53355	-0.73228	1.037621	-0.51394	0.244259	-0.08634	0.003858
12	11.34521453	-1.655180634	-1.23618146	2.67503671	-1.27847	-0.01851	1.04775	-1.57165	0.598473	1.797366	-0.95022	0.227596	-0.07511	0.022996	-0.00079
13	12.00072342	-3.197495828	0.207246066	1.39037225	-1.46294	1.574799	-1.04829	-0.01716	0.897449	-2.18854	0.761671	-0.09966	0.023042	-0.00612	0.000159
14	12.44426955	-4.345277025	1.579356762	-0.82405147	-0.08952	0.709324	-1.04793	1.582393	-1.55235	1.759122	-0.49205	0.04109	-0.0069	0.001599	-1.9E-05
15	12.66801878	-4.956901302	2.407984426	-2.48742169	1.379234	-1.26364	1.048111	-0.97325	0.752518	-0.67013	0.17005	-0.01112	0.00151	-0.00029	-0.00011

b. Bracing X

[M] =

m_1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	m_2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	m_3	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	m_4	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	m_5	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	m_6	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	m_7	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	m_8	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	m_9	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	m_{10}	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	m_{11}	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	m_{12}	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	m_{13}	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	m_{14}	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	m_{15}	0

[K] =

58.26	-29.13	0	0	0	0	0	0	0	0	0	0	0	0	0
-29.13	58.26	-29.13	0	0	0	0	0	0	0	0	0	0	0	0
0	-29.13	58.26	-29.13	0	0	0	0	0	0	0	0	0	0	0
0	0	-29.13	58.26	-29.13	0	0	0	0	0	0	0	0	0	0
0	0	0	-29.13	52.78	-23.65	0	0	0	0	0	0	0	0	0
0	0	0	0	-23.65	47.31	-23.65	0	0	0	0	0	0	0	0
0	0	0	0	0	-23.65	47.31	-23.65	0	0	0	0	0	0	0
0	0	0	0	0	0	-23.65	47.31	-23.65	0	0	0	0	0	0
0	0	0	0	0	0	0	-23.65	47.31	-23.65	0	0	0	0	0
0	0	0	0	0	0	0	0	-23.65	39.78	-16.12	0	0	0	0
0	0	0	0	0	0	0	0	0	-16.12	32.24	-16.12	0	0	0
0	0	0	0	0	0	0	0	0	0	-16.12	32.24	-16.12	0	0
0	0	0	0	0	0	0	0	0	0	0	-16.12	32.24	-16.12	0
0	0	0	0	0	0	0	0	0	0	0	0	-16.12	32.24	-16.12
0	0	0	0	0	0	0	0	0	0	0	0	0	-16.12	16.12

$[[K] - \omega^2 [M]] = 0$

Apabila di ambil notasi $\lambda = \omega^2/(k/m)$, maka didapat,

$$29,12819133 + (-373,0838385) \lambda + 900,9664972 \lambda^2 + (-865,160053) \lambda^3 + 433,8376735 \lambda^4 + (-130,4678827) \lambda^5 + 25,49420405 \lambda^6 + (-3,399968711) \lambda^7 + 0,318805963 \lambda^8 + (-0,021351219) \lambda^9 + 0,001025087 \lambda^{10} + (-3,49684E-05) \lambda^{11} + 8,26457E-07 \lambda^{12} + (-1,28517E-08) \lambda^{13} + 1,1815E-10 \lambda^{14} + (-4,86038E-13) \lambda^{15} = 0$$

Dengan $\omega = (\lambda*(k/m))^{0.5}$

Mode	λ	f (λ)	ω (rad/dt)	T (dt)
1	0.10003	-0.0001	10.00	0.63
2	0.72997	0.0001	27.02	0.23
3	2.0014	0.0008	44.74	0.14
4	3.95035	-0.0001	62.85	0.10
5	6.2262	-0.0004	78.91	0.08
6	9.1298	0.0005	95.55	0.07
7	11.84437	-0.0001	108.83	0.06
8	15.3937	0.0009	124.07	0.05
9	18.307	-0.0002	135.30	0.05
10	21.35242	0.0002	146.12	0.04
11	23.39022	0.0001	152.94	0.04
12	26.628985	-0.0001	163.18	0.04
13	31.1733237	-0.0002	176.56	0.04
14	33.6999406	-0.0004	183.58	0.03
15	39.159342	0.0005	197.89	0.03

Massa	Mode ke-														
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
	Nilai λ														
	0.10003	0.72997	2.0014	3.95035	6.2262	9.1298	11.8444	15.3937	18.307	21.3524	23.3902	26.629	31.1733	33.6999	39.1593
Koordinat Mode Shape															
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	1.99041284	1.930037597	1.808180126	1.62138721	1.403263	1.124974	0.864802	0.524624	0.245405	-0.04648	-0.24179	-0.5522	-0.98774	-1.2299	-1.75314
3	2.961743274	2.725045125	2.269515367	1.62889648	0.969148	0.265566	-0.25212	-0.72477	-0.93978	-0.99784	-0.94154	-0.69508	-0.02437	0.51265	2.07351
4	3.904679001	3.329401949	2.295512457	1.01968471	-0.04329	-0.82622	-1.08283	-0.90486	-0.47603	0.092853	0.469436	0.936018	1.011811	0.599391	-1.88202
5	4.810179945	3.70082581	1.881184636	0.02440727	-1.0299	-1.19504	-0.68432	0.250061	0.822956	0.993524	0.828037	0.178211	-0.97504	-1.24984	1.225933
6	5.868432128	3.83936543	0.926627413	-1.21256308	-1.48802	-0.36154	0.763018	1.217925	0.644442	-0.40111	-1.01622	-1.31504	0.165626	1.444015	-0.61274
7	6.859238768	3.655898013	-0.24100846	-1.89918109	-0.88167	0.851205	1.171995	0.031696	-0.88958	-0.81171	-0.12947	1.215118	0.713074	-1.45329	0.305445
8	7.771212583	3.165810948	-1.35322426	-1.72380786	0.355391	1.171063	-0.01395	-1.21059	-0.55248	0.769044	1.105217	0.027576	-1.29346	1.276471	-0.15063
9	8.593872312	2.410207766	-2.15426516	-0.76604095	1.338217	0.262513	-1.18091	-0.31176	0.946695	0.463112	-0.63027	-1.24434	1.332742	-0.93621	0.071003
10	9.317763176	1.4524608	-2.4599312	0.53941296	1.363738	-0.9214	-0.74082	1.13847	0.454605	-0.97897	-0.67196	1.290842	-0.81448	0.476061	-0.02682
11	10.22275312	-0.131507109	-2.07845632	2.09559609	-0.03019	-1.24037	1.384078	0.311925	-1.67038	0.428993	1.916447	-0.78411	0.315229	-0.15634	0.006717
12	10.96469754	-1.700168902	-1.03371878	2.33184008	-1.39415	0.246271	0.895112	-1.28022	1.080404	0.37643	-2.64245	0.470159	-0.12189	0.051328	-0.00168
13	11.53176294	-3.070947992	0.340891982	1.0993435	-1.37408	1.374416	-1.2843	0.26987	0.677532	-0.9577	2.65354	-0.2718	0.046833	-0.0168	0.000418
14	11.91490506	-4.084299365	1.606719642	-0.82558925	0.010081	0.501823	-1.03828	1.157582	-1.70303	0.968696	-1.94672	0.140264	-0.01723	0.005345	-9.1E-05
15	12.10801305	-4.622279029	2.359821997	-2.23051359	1.384239	-1.10127	1.168553	-0.79593	0.88748	-0.40287	0.713226	-0.04321	0.004331	-0.00123	-4E-05

8. Metode β -Newmark

Dalam verifikasi ini hanya di tunjukkan untuk *open frame* dikarenakan caranya sama.

$$\beta = 2*(5\%*8,46 - 5\%*22,36) / ((8,46)^2 - (22,36)^2) = 0,003244374$$

$$\alpha = 2*5\%*8,46 - 0,003244374*(8,46)^2 = 0,613820873$$

Rumus di bawah ini menggunakan 3 tingkat, tetapi konsep untuk 15 tingkat hampir sama.

$$\Delta y_0 = \hat{k}^{-1} * \Delta \hat{p}$$

Δy_1 (m)	Δy_2 (m)	Δy_3 (m)	Δy_4 (m)	Δy_5 (m)	Δy_6 (m)	Δy_7 (m)	Δy_8 (m)	Δy_9 (m)	Δy_{10} (m)	Δy_{11} (m)	Δy_{12} (m)	Δy_{13} (m)	Δy_{14} (m)	Δy_{15} (m)	Δy_{16} (m)	Δy_{17} (rad)
-8.41E-08	-9.78E-08	-9.735E-08	-9.408E-08	-9.035E-08	-8.622E-08	-8.219E-08	-7.818E-08	-7.421E-08	-7.044E-08	-6.617E-08	-6.213E-08	-5.812E-08	-5.417E-08	-5.066E-08	-5.31E-08	-1.073E-09

c. Menghitung pertambahan kecepatan

$$\Delta \dot{y}_0 = \frac{\gamma}{\beta * \Delta t} \Delta y_0 - \frac{\gamma}{\beta} \dot{y}_0 + \Delta t * \left(1 - \frac{\gamma}{2 * \beta} \right) \ddot{y}_0$$

$\Delta \dot{y}_1$ (m)	$\Delta \dot{y}_2$ (m)	$\Delta \dot{y}_3$ (m)	$\Delta \dot{y}_4$ (m)	$\Delta \dot{y}_5$ (m)	$\Delta \dot{y}_6$ (m)	$\Delta \dot{y}_7$ (m)	$\Delta \dot{y}_8$ (m)	$\Delta \dot{y}_9$ (m)	$\Delta \dot{y}_{10}$ (m)	$\Delta \dot{y}_{11}$ (m)	$\Delta \dot{y}_{12}$ (m)	$\Delta \dot{y}_{13}$ (m)	$\Delta \dot{y}_{14}$ (m)	$\Delta \dot{y}_{15}$ (m)	$\Delta \dot{y}_{16}$ (m)	$\Delta \dot{y}_{17}$ (m)
-1.682E-05	-1.956E-05	-1.947E-05	-1.882E-05	-1.807E-05	-1.724E-05	-1.644E-05	-1.564E-05	-1.484E-05	-1.409E-05	-1.323E-05	-1.243E-05	-1.162E-05	-1.083E-05	-1.013E-05	-1.062E-05	-2.147E-07

d. Menghitung pertambahan percepatan

$$\Delta \ddot{y}_0 = \frac{1}{\beta * \Delta t^2} \Delta y_0 - \frac{1}{\beta * \Delta t} \dot{y}_0 - \frac{1}{2 * \beta} \ddot{y}_0$$

$\Delta \ddot{y}_1$ (m)	$\Delta \ddot{y}_2$ (m)	$\Delta \ddot{y}_3$ (m)	$\Delta \ddot{y}_4$ (m)	$\Delta \ddot{y}_5$ (m)	$\Delta \ddot{y}_6$ (m)	$\Delta \ddot{y}_7$ (m)	$\Delta \ddot{y}_8$ (m)	$\Delta \ddot{y}_9$ (m)	$\Delta \ddot{y}_{10}$ (m)	$\Delta \ddot{y}_{11}$ (m)	$\Delta \ddot{y}_{12}$ (m)	$\Delta \ddot{y}_{13}$ (m)	$\Delta \ddot{y}_{14}$ (m)	$\Delta \ddot{y}_{15}$ (m)	$\Delta \ddot{y}_{16}$ (m)	$\Delta \ddot{y}_{17}$ (m)
-0.003364	-0.0039119	-0.003894	-0.0037632	-0.0036141	-0.0034489	-0.0032877	-0.0031273	-0.0029684	-0.0028176	-0.0026468	-0.002485	-0.0023246	-0.0021667	-0.0020265	-0.0021242	-4.294E-05

e. Menghitung simpangan

$$\Delta y_1 = y_0 + \Delta y_0$$

y_1 (m)	y_2 (m)	y_3 (m)	y_4 (m)	y_5 (m)	y_6 (m)	y_7 (m)	y_8 (m)	y_9 (m)	y_{10} (m)	y_{11} (m)	y_{12} (m)	y_{13} (m)	y_{14} (m)	y_{15} (m)	y_{16} (m)	y_{17} (rad)
-8.4099E-08	-9.7798E-08	-9.735E-08	-9.4081E-08	-9.0352E-08	-8.6223E-08	-8.2191E-08	-7.8182E-08	-7.4211E-08	-7.044E-08	-6.617E-08	-6.2125E-08	-5.8116E-08	-5.4167E-08	-5.0663E-08	-5.3104E-08	-1.0735E-09

f. Menghitung kecepatan

$$\Delta \dot{y}_1 = \dot{y}_0 + \Delta \dot{y}_0$$

\dot{y}_1 (m)	\dot{y}_2 (m)	\dot{y}_3 (m)	\dot{y}_4 (m)	\dot{y}_5 (m)	\dot{y}_6 (m)	\dot{y}_7 (m)	\dot{y}_8 (m)	\dot{y}_9 (m)	\dot{y}_{10} (m)	\dot{y}_{11} (m)	\dot{y}_{12} (m)	\dot{y}_{13} (m)	\dot{y}_{14} (m)	\dot{y}_{15} (m)	\dot{y}_{16} (m)	\dot{y}_{17} (m)
-1.7E-05	-2E-05	-1.9E-05	-1.9E-05	-1.8E-05	-1.7E-05	-1.6E-05	-1.6E-05	-1.5E-05	-1.4E-05	-1.3E-05	-1.2E-05	-1.2E-05	-1.1E-05	-1E-05	-1.1E-05	-2.1E-07

g. Menghitung percepatan

$$\Delta \ddot{y}_1 = \ddot{y}_0 + \Delta \ddot{y}_0$$

\ddot{y}_1 (m)	\ddot{y}_2 (m)	\ddot{y}_3 (m)	\ddot{y}_4 (m)	\ddot{y}_5 (m)	\ddot{y}_6 (m)	\ddot{y}_7 (m)	\ddot{y}_8 (m)	\ddot{y}_9 (m)	\ddot{y}_{10} (m)	\ddot{y}_{11} (m)	\ddot{y}_{12} (m)	\ddot{y}_{13} (m)	\ddot{y}_{14} (m)	\ddot{y}_{15} (m)	\ddot{y}_{16} (m)	\ddot{y}_{17} (m)
-0.00336	-0.00391	-0.00389	-0.00376	-0.00361	-0.00345	-0.00329	-0.00313	-0.00297	-0.00282	-0.00265	-0.00249	-0.00232	-0.00217	-0.00203	-0.00212	-4.3E-05

h. Menghitung simpangan relatif/neto

SN 0 (m)	SN 1 (m)	SN 2 (m)	SN 3 (m)	SN 4 (m)	SN 5 (m)	SN 6 (m)	SN 7 (m)	SN 8 (m)	SN 9 (m)	SN 10 (m)	SN 11 (m)	SN 12 (m)	SN 13 (m)	SN 14 (m)	SN 15 (m)
-5.31042E-08	-8.4099E-08	-9.7798E-08	-9.735E-08	-9.4081E-08	-9.0352E-08	-8.6223E-08	-8.2191E-08	-7.8182E-08	-7.4211E-08	-7.044E-08	-6.617E-08	-6.2125E-08	-5.8116E-08	-5.4167E-08	-5.0663E-08

i. Menghitung sudut rotasi fondasi

$$\phi = -1.07349E-09 \text{ rad}$$

j. Menghitung simpangan rotasi

$$y = h * \tan(\phi)$$

SR 1 (m)	SR 2 (m)	SR 3 (m)	SR 4 (m)	SR 5 (m)	SR 6 (m)	SR 7 (m)	SR 8 (m)	SR 9 (m)	SR 10 (m)	SR 11 (m)	SR 12 (m)	SR 13 (m)	SR 14 (m)	SR 15 (m)
-4.0256E-09	-8.0512E-09	-1.2077E-08	-1.6102E-08	-2.0128E-08	-2.4154E-08	-2.8179E-08	-3.2205E-08	-3.623E-08	-4.0256E-08	-4.4281E-08	-4.8307E-08	-5.2333E-08	-5.6358E-08	-6.0384E-08

k. Menghitung simpangan total

Simpangan total merupakan jumlah dari simpangan relatif fondasi ditambah simpangan relatif ditambah simpangan rotasi.

ST 0 (m)	ST 1 (m)	ST 2 (m)	ST 3 (m)	ST 4 (m)	ST 5 (m)	ST 6 (m)	ST 7 (m)	ST 8 (m)	ST 9 (m)	ST 10 (m)	ST 11 (m)	ST 12 (m)	ST 13 (m)	ST 14 (m)	ST 15 (m)
-5.3104E-08	-1.41229E-07	-1.58953E-07	-1.62531E-07	-1.63287E-07	-1.63584E-07	-1.63481E-07	-1.63475E-07	-1.63491E-07	-1.63545E-07	-1.638E-07	-1.63556E-07	-1.63536E-07	-1.63553E-07	-1.63629E-07	-1.64151E-07

l. Menghitung drift ratio

$$yy(t) = \frac{y_j(t) - y_{j-1}(t)}{H} * 100\%$$

Drift ratio simpangan relatif

DR 1 (%)	DR 2 (%)	DR 3 (%)	DR 4 (%)	DR 5 (%)	DR 6 (%)	DR 7 (%)	DR 8 (%)	DR 9 (%)	DR 10 (%)	DR 11 (%)	DR 12 (%)	DR 13 (%)	DR 14 (%)	DR 15 (%)
-2.2426E-06	-3.653E-07	1.19E-08	8.72E-08	9.94E-08	1.1E-07	1.08E-07	1.07E-07	1.06E-07	1.01E-07	1.14E-07	1.08E-07	1.07E-07	1.05E-07	9.34E-08

Drift ratio simpangan total

DR 1 (%)	DR 2 (%)	DR 3 (%)	DR 4 (%)	DR 5 (%)	DR 6 (%)	DR 7 (%)	DR 8 (%)	DR 9 (%)	DR 10 (%)	DR 11 (%)	DR 12 (%)	DR 13 (%)	DR 14 (%)	DR 15 (%)
-3.76611E-06	-4.72644E-07	-9.5E-08	-2E-08	-7.9E-09	2.73E-09	1.74E-10	-4.3E-10	-1.4E-09	-6.8E-09	6.53E-09	5.14E-10	-4.4E-10	-2E-09	-1.4E-08

m. Menghitung gaya horizontal

$$F_j(t) = y_j(t) * K_j$$

FH 1 (kg)	FH 2 (kg)	FH 3 (kg)	FH 4 (kg)	FH 5 (kg)	FH 6 (kg)	FH 7 (kg)	FH 8 (kg)	FH 9 (kg)	FH 10 (kg)	FH 11 (kg)	FH 12 (kg)	FH 13 (kg)	FH 14 (kg)	FH 15 (kg)
-1.47999	-0.29739	-0.05932	-0.00966	0.009317	0.001607099	0.000380589	0.000635582	0.003364687	0.019191394	0.002317723	0.000366727	0.000617635	0.004576953	0.036017246

n. Menghitung gaya geser

$$V(t) = \sum_{j=1}^n F(t)$$

V 1 (kg)	V 2 (kg)	V 3 (kg)	V 4 (kg)	V 5 (kg)	V 6 (kg)	V 7 (kg)	V 8 (kg)	V 9 (kg)	V 10 (kg)	V 11 (kg)	V 12 (kg)	V 13 (kg)	V 14 (kg)	V 15 (kg)
-1.767964	-0.287976	0.0094121	0.0687327	0.0783931	0.069075634	0.067468535	0.06708795	0.066452364	0.063087678	0.043896284	0.041578561	0.041211834	0.040594198	0.036017246

o. Menghitung momen guling

$$M_g = F_j(t) * H$$

MG 0 (kgm)	MG 1 (kgm)	MG 2 (kgm)	MG 3 (kgm)	MG 4 (kgm)	MG 5 (kgm)	MG 6 (kgm)	MG 7 (kgm)	MG 8 (kgm)	MG 9 (kgm)	MG 10 (kgm)	MG 11 (kgm)	MG 12 (kgm)	MG 13 (kgm)	MG 14 (kgm)	MG 15 (kgm)
-5.1109942	1.51887009	2.59878056	2.56348517	2.30573762	2.011763549	1.752729921	1.499722914	1.248143113	0.998946747	0.762367956	0.597756893	0.441837291	0.287292915	0.135064671	0

2. Step ke 2 (0,02 detik)

$$\Delta p = (\ddot{y}b_2 - \ddot{y}b_1) \begin{bmatrix} m_1 \\ m_2 \\ m_3 \\ m_0 + m_1 + m_2 + m_3 \\ m_1h_1 + m_2h_2 + m_3h_3 \end{bmatrix}$$

Rumus di atas untuk 3 tingkat, sedangkan untuk 15 tingkat sama konsepnya.

Δp_1 (kg)	Δp_2 (kg)	Δp_3 (kg)	Δp_4 (kg)	Δp_5 (kg)	Δp_6 (kg)	Δp_7 (kg)	Δp_8 (kg)	Δp_9 (kg)	Δp_{10} (kg)	Δp_{11} (kg)	Δp_{12} (kg)	Δp_{13} (kg)	Δp_{14} (kg)	Δp_{15} (kg)	Δp_{16} (kg)	Δp_{17} (kgm)
-17.9695	-17.9695	-17.9695	-17.9695	-17.9695	-17.5299	-17.5299	-17.5299	-17.5299	-17.5299	-16.6104	-16.6104	-16.6104	-16.6104	-16.6104	-282.338	-7689.05

a. Menghitung pertambahan pembebanan

$$\Delta \hat{p} = \Delta p_1 + a * \dot{y}_1 + b * \ddot{y}_1$$

$\Delta \hat{p}_1$ (kg)	$\Delta \hat{p}_2$ (kg)	$\Delta \hat{p}_3$ (kg)	$\Delta \hat{p}_4$ (kg)	$\Delta \hat{p}_5$ (kg)	$\Delta \hat{p}_6$ (kg)	$\Delta \hat{p}_7$ (kg)	$\Delta \hat{p}_8$ (kg)	$\Delta \hat{p}_9$ (kg)	$\Delta \hat{p}_{10}$ (kg)	$\Delta \hat{p}_{11}$ (kg)	$\Delta \hat{p}_{12}$ (kg)	$\Delta \hat{p}_{13}$ (kg)	$\Delta \hat{p}_{14}$ (kg)	$\Delta \hat{p}_{15}$ (kg)	$\Delta \hat{p}_{16}$ (kg)	$\Delta \hat{p}_{17}$ (kgm)
-81.9501	-88.206	-89.4675	-89.7328	-89.8358	-87.6012	-87.5973	-87.6013	-87.6184	-87.7047	-83.0232	-83.0154	-83.0192	-83.0427	-83.2114	-1361.18	-38399.6

b. Menghitung pertambahan simpangan

$$\Delta y_1 = \hat{k}^{-1} * \Delta \hat{p}$$

Δy_1 (m)	Δy_2 (m)	Δy_3 (m)	Δy_4 (m)	Δy_5 (m)	Δy_6 (m)	Δy_7 (m)	Δy_8 (m)	Δy_9 (m)	Δy_{10} (m)	Δy_{11} (m)	Δy_{12} (m)	Δy_{13} (m)	Δy_{14} (m)	Δy_{15} (m)	Δy_{16} (m)	Δy_{17} (rad)
-4.363E-07	-5.432E-07	-5.538E-07	-5.391E-07	-5.389E-07	-4.95E-07	-4.716E-07	-4.485E-07	-4.256E-07	-4.043E-07	-3.79E-07	-3.554E-07	-3.322E-07	-3.097E-07	-2.907E-07	-1.817E-07	-6.224E-09

c. Menghitung pertambahan kecepatan

$$\Delta \dot{y}_1 = \frac{\gamma}{\beta * \Delta t} \Delta y_1 - \frac{\gamma}{\beta} \dot{y}_1 + \Delta t * \left(1 - \frac{\gamma}{2 * \beta} \right) \ddot{y}_1$$

$\Delta \dot{y}_1$ (m)	$\Delta \dot{y}_2$ (m)	$\Delta \dot{y}_3$ (m)	$\Delta \dot{y}_4$ (m)	$\Delta \dot{y}_5$ (m)	$\Delta \dot{y}_6$ (m)	$\Delta \dot{y}_7$ (m)	$\Delta \dot{y}_8$ (m)	$\Delta \dot{y}_9$ (m)	$\Delta \dot{y}_{10}$ (m)	$\Delta \dot{y}_{11}$ (m)	$\Delta \dot{y}_{12}$ (m)	$\Delta \dot{y}_{13}$ (m)	$\Delta \dot{y}_{14}$ (m)	$\Delta \dot{y}_{15}$ (m)	$\Delta \dot{y}_{16}$ (m)	$\Delta \dot{y}_{17}$ (m)
-5.361E-05	-6.952E-05	-7.181E-05	-7.019E-05	-6.764E-05	-6.452E-05	-6.145E-05	-5.842E-05	-5.544E-05	-5.268E-05	-4.933E-05	-4.624E-05	-4.32E-05	-4.027E-05	-3.787E-05	-1.51E-05	-8.155E-07

d. Menghitung pertambahan percepatan

$$\Delta \ddot{y}_1 = \frac{1}{\beta * \Delta t^2} \Delta y_1 - \frac{1}{\beta * \Delta t} \dot{y}_1 - \frac{1}{2 * \beta} \ddot{y}_1$$

$\Delta \ddot{y}_1$ (m)	$\Delta \ddot{y}_2$ (m)	$\Delta \ddot{y}_3$ (m)	$\Delta \ddot{y}_4$ (m)	$\Delta \ddot{y}_5$ (m)	$\Delta \ddot{y}_6$ (m)	$\Delta \ddot{y}_7$ (m)	$\Delta \ddot{y}_8$ (m)	$\Delta \ddot{y}_9$ (m)	$\Delta \ddot{y}_{10}$ (m)	$\Delta \ddot{y}_{11}$ (m)	$\Delta \ddot{y}_{12}$ (m)	$\Delta \ddot{y}_{13}$ (m)	$\Delta \ddot{y}_{14}$ (m)	$\Delta \ddot{y}_{15}$ (m)	$\Delta \ddot{y}_{16}$ (m)	$\Delta \ddot{y}_{17}$ (m)
-0.003995	-0.0060799	-0.0065746	-0.0065113	-0.0062996	-0.0060054	-0.0057152	-0.0054292	-0.0051516	-0.0049008	-0.0045729	-0.0042773	-0.0039912	-0.0037207	-0.0035202	0.00122925	-7.722E-05

e. Menghitung simpangan

$$\Delta y_2 = y_1 + \Delta y_1$$

y_1 (m)	y_2 (m)	y_3 (m)	y_4 (m)	y_5 (m)	y_6 (m)	y_7 (m)	y_8 (m)	y_9 (m)	y_{10} (m)	y_{11} (m)	y_{12} (m)	y_{13} (m)	y_{14} (m)	y_{15} (m)	y_{16} (m)	y_{17} (rad)
-5.2037E-07	-6.4099E-07	-6.5111E-07	-6.3318E-07	-6.0925E-07	-5.8125E-07	-5.5384E-07	-5.2664E-07	-4.9984E-07	-4.7472E-07	-4.4517E-07	-4.1756E-07	-3.9036E-07	-3.6385E-07	-3.4132E-07	-2.3479E-07	-7.2979E-09

f. Menghitung kecepatan

$$\Delta \dot{y}_2 = \dot{y}_1 + \Delta \dot{y}_1$$

\dot{y}_1 (m)	\dot{y}_2 (m)	\dot{y}_3 (m)	\dot{y}_4 (m)	\dot{y}_5 (m)	\dot{y}_6 (m)	\dot{y}_7 (m)	\dot{y}_8 (m)	\dot{y}_9 (m)	\dot{y}_{10} (m)	\dot{y}_{11} (m)	\dot{y}_{12} (m)	\dot{y}_{13} (m)	\dot{y}_{14} (m)	\dot{y}_{15} (m)	\dot{y}_{16} (m)	\dot{y}_{17} (m)
-7E-05	-8.9E-05	-9.1E-05	-8.9E-05	-8.6E-05	-8.2E-05	-7.8E-05	-7.4E-05	-7E-05	-6.7E-05	-6.3E-05	-5.9E-05	-5.5E-05	-5.1E-05	-4.8E-05	-2.6E-05	-1E-06

g. Menghitung percepatan

$$\Delta \ddot{y}_2 = \ddot{y}_1 + \Delta \ddot{y}_1$$

\ddot{y}_1 (m)	\ddot{y}_2 (m)	\ddot{y}_3 (m)	\ddot{y}_4 (m)	\ddot{y}_5 (m)	\ddot{y}_6 (m)	\ddot{y}_7 (m)	\ddot{y}_8 (m)	\ddot{y}_9 (m)	\ddot{y}_{10} (m)	\ddot{y}_{11} (m)	\ddot{y}_{12} (m)	\ddot{y}_{13} (m)	\ddot{y}_{14} (m)	\ddot{y}_{15} (m)	\ddot{y}_{16} (m)	\ddot{y}_{17} (m)
-0.00736	-0.00999	-0.01047	-0.01027	-0.00991	-0.00945	-0.009	-0.00856	-0.00812	-0.00772	-0.00722	-0.00676	-0.00632	-0.00589	-0.00555	-0.00089	-0.00012

h. Menghitung simpangan relatif/neto

SN 0 (m)	SN 1 (m)	SN 2 (m)	SN 3 (m)	SN 4 (m)	SN 5 (m)	SN 6 (m)	SN 7 (m)	SN 8 (m)	SN 9 (m)	SN 10 (m)	SN 11 (m)	SN 12 (m)	SN 13 (m)	SN 14 (m)	SN 15 (m)
-2.34789E-07	-5.2037E-07	-6.4099E-07	-6.5111E-07	-6.3318E-07	-6.0925E-07	-5.8125E-07	-5.5384E-07	-5.2664E-07	-4.9984E-07	-4.7472E-07	-4.4517E-07	-4.1756E-07	-3.9036E-07	-3.6385E-07	-3.4132E-07

i. Menghitung sudut rotasi fondasi

$$\phi = -7.29786E-09 \text{ rad}$$

j. Menghitung simpangan rotasi

$$y = h * \tan(\phi)$$

SR 1 (m)	SR 2 (m)	SR 3 (m)	SR 4 (m)	SR 5 (m)	SR 6 (m)	SR 7 (m)	SR 8 (m)	SR 9 (m)	SR 10 (m)	SR 11 (m)	SR 12 (m)	SR 13 (m)	SR 14 (m)	SR 15 (m)
-2.7367E-08	-5.4734E-08	-8.2101E-08	-1.0947E-07	-1.3683E-07	-1.642E-07	-1.9157E-07	-2.1894E-07	-2.463E-07	-2.7367E-07	-3.0104E-07	-3.284E-07	-3.5577E-07	-3.8314E-07	-4.105E-07

k. Menghitung simpangan total

Simpangan total merupakan jumlah dari simpangan relatif fondasi ditambah simpangan relatif ditambah simpangan rotasi.

ST 0 (m)	ST 1 (m)	ST 2 (m)	ST 3 (m)	ST 4 (m)	ST 5 (m)	ST 6 (m)	ST 7 (m)	ST 8 (m)	ST 9 (m)	ST 10 (m)	ST 11 (m)	ST 12 (m)	ST 13 (m)	ST 14 (m)	ST 15 (m)
-2.3479E-07	-7.8252E-07	-9.3051E-07	-9.68005E-07	-9.77442E-07	-9.80872E-07	-9.80242E-07	-9.80196E-07	-9.80364E-07	-9.80934E-07	-9.83181E-07	-9.80998E-07	-9.80751E-07	-9.80919E-07	-9.81779E-07	-9.86612E-07

l. Menghitung drift ratio

$$yy(t) = \frac{y_j(t) - y_{j-1}(t)}{H} * 100\%$$

Drift ratio simpangan relatif

DR 1 (%)	DR 2 (%)	DR 3 (%)	DR 4 (%)	DR 5 (%)	DR 6 (%)	DR 7 (%)	DR 8 (%)	DR 9 (%)	DR 10 (%)	DR 11 (%)	DR 12 (%)	DR 13 (%)	DR 14 (%)	DR 15 (%)
-1.3877E-05	-3.2164E-06	-2.7E-07	4.78E-07	6.38E-07	7.47E-07	7.31E-07	7.25E-07	7.15E-07	6.7E-07	7.88E-07	7.36E-07	7.25E-07	7.07E-07	6.01E-07

Drift ratio simpangan total

DR 1 (%)	DR 2 (%)	DR 3 (%)	DR 4 (%)	DR 5 (%)	DR 6 (%)	DR 7 (%)	DR 8 (%)	DR 9 (%)	DR 10 (%)	DR 11 (%)	DR 12 (%)	DR 13 (%)	DR 14 (%)	DR 15 (%)
-2.08674E-05	-3.94622E-06	-1E-06	-2.5E-07	-9.1E-08	1.68E-08	1.24E-09	-4.5E-09	-1.5E-08	-6E-08	5.82E-08	6.58E-09	-4.5E-09	-2.3E-08	-1.3E-07

m. Menghitung gaya horizontal

$$F_j(t) = y_j(t) * K_j$$

FH 1 (kg)	FH 2 (kg)	FH 3 (kg)	FH 4 (kg)	FH 5 (kg)	FH 6 (kg)	FH 7 (kg)	FH 8 (kg)	FH 9 (kg)	FH 10 (kg)	FH 11 (kg)	FH 12 (kg)	FH 13 (kg)	FH 14 (kg)	FH 15 (kg)
-8.40376	-2.32271	-0.58986	-0.12626	0.034722	0.009764199	0.003603374	0.006718351	0.028051577	0.116578467	0.019908203	0.004258919	0.007119939	0.040838317	0.231631634

n. Menghitung gaya geser

$$V(t) = \sum_{j=1}^n F(t)$$

V 1 (kg)	V 2 (kg)	V 3 (kg)	V 4 (kg)	V 5 (kg)	V 6 (kg)	V 7 (kg)	V 8 (kg)	V 9 (kg)	V 10 (kg)	V 11 (kg)	V 12 (kg)	V 13 (kg)	V 14 (kg)	V 15 (kg)
-10.9394	-2.535637	-0.212924	0.3769351	0.5031946	0.468472981	0.458708782	0.45510541	0.448387057	0.42033548	0.303757013	0.283848809	0.27958989	0.272469951	0.231631634

o. Menghitung momen guling

$$M_g = F_j(t) * H$$

MG 0 (kgm)	MG 1 (kgm)	MG 2 (kgm)	MG 3 (kgm)	MG 4 (kgm)	MG 5 (kgm)	MG 6 (kgm)	MG 7 (kgm)	MG 8 (kgm)	MG 9 (kgm)	MG 10 (kgm)	MG 11 (kgm)	MG 12 (kgm)	MG 13 (kgm)	MG 14 (kgm)	MG 15 (kgm)
-34.445721	6.57703249	16.085673	16.8841377	15.470631	13.58365127	11.82687759	10.10671966	8.400074377	6.718622913	5.142364863	4.003276065	2.93884303	1.890380942	0.868618627	0

Untuk detik selanjutnya caranya sama.